Introduction and Basic Implementation for Finite Element Methods

Chapter 4: Finite Elements for 2D second order parabolic and hyperbolic equation

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Outline

- Weak formulation
- 2 Semi-discretization
- Full discretization
- More Discussion
- 5 Second order hyperbolic equation

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- 3 Full discretization
- 4 More Discussion
- 5 Second order hyperbolic equation

Target problem

Consider the 2D second order parabolic equation

$$u_t - \nabla \cdot (c\nabla u) = f$$
, in $\Omega \times [0, T]$, $u = g$, on $\partial \Omega \times [0, T]$, $u = u_0$, at $t = 0$ and in Ω .

where Ω is a 2D domain, [0, T] is the time interval, f(x, y, t)and c(x, y, t) are given functions on $\Omega \times [0, T]$, g(x, y, t) is a given function on $\partial \Omega \times [0, T]$, $u_0(x, y)$ is given function in Ω at t = 0, and u(x, y, t) is the unknown function.

• First, multiply a function v(x, y) on both sides of the original equation,

$$u_t - \nabla \cdot (c\nabla u) = f \text{ in } \Omega$$

$$\Rightarrow u_t v - \nabla \cdot (c\nabla u)v = fv \text{ in } \Omega$$

$$\Rightarrow \int_{\Omega} u_t v \, dxdy - \int_{\Omega} \nabla \cdot (c\nabla u)v \, dxdy = \int_{\Omega} fv \, dxdy.$$

• u(x, y, t) is called a trail function and v(x, y) is called a test function.

 Second, using Green's formula (divergence theory, integration by parts in multi-dimension)

$$\int_{\Omega} \nabla \cdot (c \nabla u) v \, dxdy = \int_{\partial \Omega} (c \nabla u \cdot \vec{n}) v \, ds - \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy,$$

we obtain

$$\int_{\Omega} u_t v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy - \int_{\partial \Omega} (c \nabla u \cdot \vec{n}) v \, ds$$

$$= \int_{\Omega} fv \, dxdy.$$

- Since the solution on the domain boundary $\partial\Omega$ are given by u(x, y, t) = g(x, y, t), then we can choose the test function v(x,y) such that v=0 on $\partial\Omega$.
- Hence

$$\int_{\Omega} u_t v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy = \int_{\Omega} fv \, dxdy.$$

- What spaces should u and v belong to? Sobolev spaces! (See Chapter 3)
- Define

$$H^1(0,T;H^1(\Omega))=\{v(t,\cdot),\ \frac{\partial v}{\partial t}(t,\cdot)\in H^1(\Omega),\ \forall t\in[0,T]\}.$$

• Weak formulation: find $u \in H^1(0, T; H^1(\Omega))$ such that

$$\int_{\Omega} u_t v \ dx dy + \int_{\Omega} c \nabla u \cdot \nabla v \ dx dy = \int_{\Omega} f v \ dx dy.$$

for any $v \in H_0^1(\Omega)$.

- Let $a(u, v) = \int_{\Omega} c \nabla u \cdot \nabla v dx dy$ and $(f, v) = \int_{\Omega} f v dx dy$.
- Weak formulation: find $u \in H^1(0, T; H^1(\Omega))$ such that

$$(u_t,v)+a(u,v)=(f,v)$$

for any $v \in H_0^1(\Omega)$.

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- 2 Semi-discretization
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Galerkin formulation

- Assume there is a finite dimensional subspace $U_h \subset H^1(\Omega)$.
- Then the Galerkin formulation is to find $u_h \in H^1(0, T; U_h)$ such that

$$(u_{h_t}, v_h) + a(u_h, v_h) = (f, v_h)$$

$$\Leftrightarrow \int_{\Omega} u_{h_t} v_h \, dx dy + \int_{\Omega} c \nabla u_h \cdot \nabla v_h \, dx dy = \int_{\Omega} f v_h \, dx dy$$

for any $v_h \in U_h$.

- Basic idea of Galerkin formulation: use finite dimensional space to approximate infinite dimensional space.
- Here $U_h = span\{\phi_j\}_{j=1}^{N_b}$ is chosen to be a finite element space where $\{\phi_j\}_{j=1}^{N_b}$ are the global finite element basis functions.

Recall the following definitions from Chapter 2:

- N: number of mesh elements.
- N_m : number of mesh nodes.
- E_n ($n = 1, \dots, N$): mesh elements.
- Z_k ($k = 1, \dots, N_m$): mesh nodes.
- N_I : number of local mesh nodes in a mesh element.
- P:information matrix consisting of the coordinates of all mesh nodes.
- T: information matrix consisting of the global node indices of the mesh nodes of all the mesh elements.

- We only consider the nodal basis functions (Lagrange type) in this course.
- N_{lb} : number of local finite element nodes (=number of local finite element basis functions) in a mesh element.
- N_b : number of the finite element nodes (= the number of unknowns = the total number of the finite element basis functions).
- X_j $(j = 1, \dots, N_b)$: finite element nodes.
- P_b: information matrix consisting of the coordinates of all finite element nodes.
- T_b: information matrix consisting of the global node indices of the finite element nodes of all the mesh elements.

• Since $u_h \in H^1(0, T; U_h)$ and $U_h = span\{\phi_j\}_{j=1}^{N_b}$, then

$$u_h(x, y, t) = \sum_{j=1}^{N_b} u_j(t)\phi_j(x, y)$$

for some coefficients $u_j(t)$ $(j = 1, \dots, N_b)$.

If we can set up a linear algebraic system for

$$u_j(t) (j = 1, \cdots, N_b)$$

and solve it, then we can obtain the finite element solution u_h .

• Therefore, we choose $v_h = \phi_i$ $(i = 1, \dots, N_b)$. Then

$$\begin{split} &\int_{\Omega} \left(\sum_{j=1}^{N_b} u_j(t) \phi_j \right)_t \phi_i \ dxdy + \int_{\Omega} c \nabla \left(\sum_{j=1}^{N_b} u_j(t) \phi_j \right) \cdot \nabla \phi_i \ dxdy \\ &= \int_{\Omega} f \phi_i \ dxdy, \ i = 1, \cdots, N_b \\ &\Rightarrow \sum_{j=1}^{N_b} u_j'(t) \left[\int_{\Omega} \phi_j \phi_i \ dxdy \right] + \sum_{j=1}^{N_b} u_j(t) \left[\int_{\Omega} c \nabla \phi_j \cdot \nabla \phi_i \ dxdy \right] \\ &= \int_{\Omega} f \phi_i \ dxdy, \ i = 1, \cdots, N_b. \end{split}$$

• Here the basis functions ϕ_i $(i = 1, \dots, N_b)$ depend on (x, y)only. But the given functions c and f may depend on t and (x, y). 4 ロ ト 4 回 ト 4 重 ト 4 重 ト 9 へ 0 0

Matrix formulation

Define the stiffness matrix

$$A(t) = [a_{ij}]_{i,j=1}^{N_b} = \left[\int_{\Omega} c \nabla \phi_j \cdot \nabla \phi_i \, dx dy\right]_{i,j=1}^{N_b}.$$

Define the mass matrix

$$M = [m_{ij}]_{i,j=1}^{N_b} = \left[\int_{\Omega} \phi_j \phi_i \, dx dy\right]_{i,j=1}^{N_b}.$$

Define the load vector

$$\vec{b}(t) = [b_i]_{i=1}^{N_b} = \left[\int_{\Omega} f \phi_i \ dxdy\right]_{i=1}^{N_b}.$$

Define the unknown vector

$$\vec{X}(t) = [u_j(t)]_{j=1}^{N_b}.$$

Then we obtain the system

$$M\vec{X}'(t) + A(t)\vec{X}(t) = \vec{b}(t).$$

- At a given time t, the assembly of the stiffness matrix A(t)and the load vector $\vec{b}(t)$ is the same as that of the A and b in Chapter 3. But the given time t needs to be incorporated into the code.
- In some simulation, the functions c in the given parabolic equation may not depend on t. In this case, the stiffness matrix A(t) is actually independent of t, hence can be generated before the time marching in exactly the same way as the A in Chapter 3.
- Similarly, the functions f in the given parabolic equation may not depend on t in some simulation. In this case, the load vector $\vec{b}(t)$ is actually independent of t, hence can be generated before the time marching in exactly the same way as the \vec{b} in Chapter 3.

Assembly of the stiffness matrix

Recall Algorithm I-3 from Chapter 3:

- Initialize the matrix: $A = sparse(N_b, N_b)$;
- Compute the integrals and assemble them into A:

```
FOR n = 1, \dots, N:
         FOR \alpha = 1, \dots, N_{lb}:
                   FOR \beta = 1, \dots, N_{lb}:
                            Compute r = \int_{F_{-}} c \frac{\partial^{r+s} \psi_{n\alpha}}{\partial x^{r} \partial y^{s}} \frac{\partial^{p+q} \psi_{n\beta}}{\partial x^{p} \partial y^{q}} dxdy;
                            Add r to A(T_b(\beta, n), T_b(\alpha, n)).
                   END
          END
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```

Assembly of the stiffness matrix

Algorithm I-5:

- Specify a value for the time t based on the input time;
- Initialize the matrix: $A = sparse(N_b, N_b)$;
- Compute the integrals and assemble them into A:

```
FOR n = 1, \dots, N:
         FOR \alpha = 1, \dots, N_{lb}:
                  FOR \beta = 1, \cdots, N_{lb}:
                            Compute r = \int_{F_{a}} c(t) \frac{\partial^{r+s} \psi_{n\alpha}}{\partial y^{r} \partial y^{s}} \frac{\partial^{p+q} \psi_{n\beta}}{\partial y^{p} \partial y^{q}} dxdy;
                            Add r to A(T_b(\beta, n), T_b(\alpha, n)).
                   END
         END
END
```

Assembly of the stiffness matrix

- First, we call Algorithm I-5 with r = p = 1, s = q = 0, and c(x, y, t) to obtain A1(t).
- Second, we call Algorithm I-5 with r = p = 0, s = q = 1, and c(x, y, t) to obtain A2(t).
- Then the stiffness matrix A(t) = A1(t) + A2(t).
- If c does not depend on t, then this part is exactly the same as the assembly of the stiffness matrix with Algorithm I-3 in Chapter 3.

Assembly of the mass matrix

Any observation for the mass matrix

$$M = [m_{ij}]_{i,j=1}^{N_b} = \left[\int_{\Omega} \phi_j \phi_i \, dx dy \right]_{i,j=1}^{N_b} ?$$

• Following the same procedure for A from

$$\int_{\Omega} c \nabla \phi_j \cdot \nabla \phi_i \, \, dx dy$$

to

$$\int_{E_{-}} c \nabla \psi_{n\alpha} \cdot \nabla \psi_{n\beta} \, dx dy$$

in Chapter 3, we can also get

$$\int_{F_{-}} \psi_{\mathbf{n}\alpha} \psi_{\mathbf{n}\beta} \, \, \mathbf{d}x \mathbf{d}y \, \left(\text{from } \int_{\Omega} \phi_{j} \phi_{i} \, \, \mathbf{d}x \mathbf{d}y \right).$$

• Just use Algorithm I-3 with r = s = p = q = 0 and c = 1!

Assembly of the load vector

Recall Algorithm II-3 from Chapter 3:

- Initialize the matrix: $b = sparse(N_b, 1)$;
- Compute the integrals and assemble them into b:

```
FOR n = 1, \dots, N:
       FOR \beta = 1, \cdots, N_{lb}:
               Compute r = \int_{E_n} f \frac{\partial^{p+q} \psi_{n\beta}}{\partial x^p \partial y^q} dxdy;
               b(T_b(\beta, n), 1) = b(T_b(\beta, n), 1) + r;
       END
END
```

Assembly of the load vector

Algorithm II-5:

- Specify a value for the time t based on the input time;
- Initialize the matrix: $b = sparse(N_b, 1)$;
- Compute the integrals and assemble them into b:

```
FOR n = 1, \dots, N:
       FOR \beta = 1, \dots, N_{lb}:
               Compute r = \int_{E_n} f(t) \frac{\partial^{p+q} \psi_{n\beta}}{\partial x^p \partial v^q} dxdy;
               b(T_b(\beta, n), 1) = b(T_b(\beta, n), 1) + r:
       END
END
```

Assembly of the load vector

- We call Algorithm II-5 with p = q = 0 and f(x, y, t) to obtain b(t).
- If f does not depend on t, then this part is exactly the same as the assembly of the load vector with Algorithm II-3 in Chapter 3.

Outline

- Full discretization
- Second order hyperbolic equation

Observation

• Any observation for the system

$$M\vec{X}'(t) + A(t)\vec{X}(t) = \vec{b}(t)$$
?

- System of ordinary differential equations (ODEs)!
- How to solve it?
- Finite difference (FD) method!

Basic idea:

Consider the IVP

$$y'(t) = f(t, y(t)) \ (a \le t \le b), \ y(a) = g_a$$

More Discussion

given the initial value g_a .

- Assume that we have a uniform partition of [a, b] into Jelements with mesh size h.
- The mesh nodes are $t_i = a + jh$, $j = 0, 1, \dots, J$.
- Assume y_i is the numerical solution of $y(t_i)$.
- Then the initial condition implies: $y_0 = y(a) = g_a$.
- A straightforward discretization of f(t, y(t)) at t_i is $f(t_i, y_i)$.
- How about the discretization of y'(t) at t_i ?
- Taylor's expansion!



Theorem

Suppose that f(x) is a $(n+1)^{th}$ differentiable function on [a,b]and $x_0 \in [a, b]$. Then for any $x \in [a, b]$, we have the following Taylor's expansion of f(x) at x_0 :

$$f(x) = P_n(x) + R_n(x),$$

where

$$P_n(x) = \sum_{k=0}^n \frac{1}{k!} f^{(k)}(x_0) (x - x_0)^k$$

= $f(x_0) + f'(x_0) (x - x_0) + \frac{1}{2!} f''(x_0) (x - x_0)^2 + \cdots$
+ $\frac{1}{n!} f^{(n)}(x_0) (x - x_0)^n$,

Theorem (Continued)

$$R_n = \frac{1}{(n+1)!} f^{(n+1)}(\xi) (x-x_0)^{n+1}$$

for some $\xi \in [x_0,x]$ (Lagrange form of the remainder),

or

$$R_n = \frac{1}{n!} \int_{x_0}^x f^{(n+1)}(s)(x-s)^n ds$$

for some $\xi \in [x_0, x]$ (Integral form of the remainder).

Review of finite difference method for a first order ODE

• Pick n = 3 in the Taylor's expansion:

$$f(x) = f(x_0) + f'(x_0)(x - x_0) + \frac{1}{2}f''(x_0)(x - x_0)^2 + \frac{1}{6}f'''(x_0)(x - x_0)^3 + \frac{1}{24}f^{(4)}(\xi)(x - x_0)^4.$$

• Replace x by x + h and x_0 by x:

$$f(x+h) = f(x) + f'(x)h + \frac{1}{2}f''(x)h^2 + \frac{1}{6}f'''(x)h^3 + O(h^4).$$

 We first consider the discretization of the first derivative f'(x). Then

$$f'(x) = \frac{f(x+h) - f(x)}{h} - \frac{1}{2}f''(x)h - \frac{1}{6}f'''(x)h^2 - O(h^3)$$
$$= \frac{f(x+h) - f(x)}{h} + O(h).$$

- Assume that we have a uniform partition of [a, b] into J elements with mesh size h.
- The mesh nodes are $t_i = a + jh$, $j = 0, 1, \dots, J$.
- Then

$$f'(t_j) = \frac{f(t_j + h) - f(t_j)}{h} + O(h)$$

= $\frac{f(t_{j+1}) - f(t_j)}{h} + O(h)$
 $\approx \frac{f_{j+1} - f_j}{h}, j = 0, 1, \dots, J - 1.$

Here f_i is the approximation of $f(t_i)$. This is called forward difference.

• Recall the Taylor's expansion with n = 3:

$$f(x) = f(x_0) + f'(x_0)(x - x_0) + \frac{1}{2}f''(x_0)(x - x_0)^2 + \frac{1}{6}f'''(x_0)(x - x_0)^3 + \frac{1}{24}f^{(4)}(\xi)(x - x_0)^4.$$

• Replace x by x - h and x_0 by x:

$$f(x-h) = f(x) - f'(x)h + \frac{1}{2}f''(x)h^2 - \frac{1}{6}f'''(x)h^3 + O(h^4).$$

Then

$$f'(x) = \frac{f(x) - f(x - h)}{h} + \frac{1}{2}f''(x)h - \frac{1}{6}f'''(x)h^2 + O(h^3)$$
$$= \frac{f(x) - f(x - h)}{h} + O(h).$$

- Consider the same partition as above.
- Then

$$f'(t_{j}) = \frac{f(t_{j}) - f(t_{j} - h)}{h} + O(h)$$

$$= \frac{f(t_{j}) - f(t_{j-1})}{h} + O(h)$$

$$\approx \frac{f_{j} - f_{j-1}}{h}, j = 1, \dots, J.$$

This is called backward difference.

- Observation: Both of the forward and backward difference schemes are of first order.
- Is it possible to construct a higher order difference scheme for $f'(x_j)$? Yes!
- Recall

$$f(x+h) = f(x) + f'(x)h + \frac{1}{2}f''(x)h^2 + \frac{1}{6}f'''(x)h^3 + O(h^4),$$

$$f(x-h) = f(x) - f'(x)h + \frac{1}{2}f''(x)h^2 - \frac{1}{6}f'''(x)h^3 + O(h^4).$$

• Subtract the second equation from the first one:

$$f(x+h)-f(x-h)=2f'(x)h+\frac{1}{3}f'''(x)h^3+O(h^4)$$
.

Then

$$f'(x) = \frac{f(x+h) - f(x-h)}{2h} - \frac{1}{12}f'''(x)h^2 + O(h^3)$$
$$= \frac{f(x+h) - f(x-h)}{2h} + O(h^2).$$

This is second order!

Hence

$$f'(t_{j}) = \frac{f(t_{j} + h) - f(t_{j} - h)}{2h} + O(h^{2})$$

$$= \frac{f(t_{j+1}) - f(t_{j-1})}{2h} + O(h^{2})$$

$$\approx \frac{f_{j+1} - f_{j-1}}{2h}, j = 1, \dots, J - 1.$$

This is called centered difference.



Hence we obtain the following difference schemes:

- Forward difference for $y'(t_i) \approx \frac{y_{j+1} y_j}{h}$.
- Backward difference for $y'(t_i) \approx \frac{y_j y_{j-1}}{b}$.
- Centered difference for $y'(t_i) \approx \frac{y_{j+1} y_{j-1}}{2h}$.

Forward Euler scheme:

$$y'(t) = f(t, y(t))$$

$$\Rightarrow y'(t_j) = f(t_j, y(t_j)), \ j = 0, \dots, J - 1$$

$$\Rightarrow \frac{y(t_{j+1}) - y(t_j)}{h} + O(h) = f(t_j, y(t_j)), \ j = 0, \dots, J - 1$$

$$\Rightarrow \frac{y_{j+1} - y_j}{h} = f(t_j, y_j), \ j = 0, \dots, J - 1$$

$$\Rightarrow y_{j+1} = y_j + h \cdot f(t_j, y_j), \ j = 0, \dots, J - 1,$$

$$y_0 = y(a) = g_2.$$

Review of finite difference method for a first order ODE

Backward Euler scheme:

$$y'(t) = f(t, y(t))$$

$$\Rightarrow y'(t_{j}) = f(t_{j}, y(t_{j})), \ j = 1, \dots, J$$

$$\Rightarrow \frac{y(t_{j}) - y(t_{j-1})}{h} + O(h) = f(t_{j}, y(t_{j})), \ j = 1, \dots, J$$

$$\Rightarrow \frac{y_{j} - y_{j-1}}{h} = f(t_{j}, y_{j}), \ j = 1, \dots, J$$

$$\Rightarrow \frac{y_{j+1} - y_{j}}{h} = f(t_{j+1}, y_{j+1}), \ j = 0, \dots, J - 1$$

$$\Rightarrow y_{j+1} = y_{j} + h \cdot f(t_{j+1}, y_{j+1}), \ j = 0, \dots, J - 1,$$

$$y_{0} = y(a).$$

Review of finite difference method for a first order ODE

 Trapezoidal scheme(Crank-Nicolson scheme if it's applied to PDE):

$$\frac{y_{j+1}-y_j}{h}=\frac{f(t_{j+1},y_{j+1})+f(t_j,y_j)}{2};$$

• Two-step backward differentiation:

$$\frac{3y_{j+1}-4y_j+y_{j-1}}{2h}=f(t_{j+1},y_{j+1});$$

Three-step backward differentiation:

$$\frac{11y_{j+1}-18y_j+9y_{j-1}-2y_{j-2}}{6h}=f(t_{j+1},y_{j+1}).$$

Review of finite difference method for a first order ODE

 Actually, the forward Euler scheme, backward Euler scheme, and Crank-Nicolson scheme can be rewritten into a more general θ -scheme:

$$\frac{y_{j+1}-y_j}{h}=\theta f(t_{j+1},y_{j+1})+(1-\theta)f(t_j,y_j);$$

- $\theta = 0$: forward Euler scheme:
- $\theta = 1$: backward Euler scheme;
- $\theta = \frac{1}{2}$: Crank-Nicolson scheme.

Now let's consider the system of ODEs:

$$M\vec{X}'(t) + A(t)\vec{X}(t) = \vec{b}(t).$$

- Assume that we have a uniform partition of [0, T] into M_m elements with mesh size $\triangle t$.
- The mesh nodes are $t_m = m \triangle t, \ m = 0, 1, \cdots, M_m$.
- Assume \vec{X}^m is the numerical solution of $\vec{X}(t_m)$.
- Then the corresponding θ —scheme is

$$M\frac{\vec{X}^{m+1} - \vec{X}^m}{\triangle t} + \theta A(t_{m+1})\vec{X}^{m+1} + (1 - \theta)A(t_m)\vec{X}^m$$

= $\theta \vec{b}(t_{m+1}) + (1 - \theta)\vec{b}(t_m), \ m = 0, \dots, M_m - 1.$

Then

$$M \frac{\vec{X}^{m+1} - \vec{X}^m}{\triangle t} + \theta A(t_{m+1}) \vec{X}^{m+1} + (1 - \theta) A(t_m) \vec{X}^m$$

$$= \theta \vec{b}(t_{m+1}) + (1 - \theta) \vec{b}(t_m)$$

$$\Rightarrow \left[\frac{M}{\triangle t} + \theta A(t_{m+1}) \right] \vec{X}^{m+1}$$

$$= \theta \vec{b}(t_{m+1}) + (1 - \theta) \vec{b}(t_m) + \frac{M}{\triangle t} \vec{X}^m - (1 - \theta) A(t_m) \vec{X}^m.$$

Iteration scheme 1:

$$\tilde{A}^{m+1}\vec{X}^{m+1} = \tilde{\vec{b}}^{m+1}, \ m = 0, \cdots, M_m - 1,$$

where

$$egin{aligned} ilde{\mathcal{A}}^{m+1} &= rac{\mathcal{M}}{\triangle t} + heta \mathcal{A}(t_{m+1}), \ & \ ilde{ar{b}}^{m+1} &= heta ec{b}(t_{m+1}) + (1- heta) ec{b}(t_m) + rac{\mathcal{M}}{\triangle t} ec{X}^m - (1- heta) \mathcal{A}(t_m) ec{X}^m. \end{aligned}$$

More Discussion

Temporal discretization for the ODE system

Algorithm A:

- Generate the information matrices.
- Assemble the mass matrix M by using Algorithm I-3.
- Generate the initial vector \vec{X}^0 .
- Iterate in time:

FOR
$$m = 0, \dots, M_m - 1$$
:
 $t_{m+1} = (m+1)\triangle t$;
 $t_m = m\triangle t$;

Assemble the stiffness matrices $A(t_{m+1})$ and $A(t_m)$ by using Algorithm I-5 at t_{m+1} and t_m ;

Assemble the load vectors $\vec{b}(t_{m+1})$ and $\vec{b}(t_m)$ by using

Algorithm II-5 at t_{m+1} and t_m ;

Deal with boundary conditions Solve iteration scheme 1 for \vec{X}^{m+1} .

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Remark

The matrix A, vector \vec{b} and boundary conditions could be independent of the time. In this case, they can be handled before the loop for the time iteration starts, which can dramatically save the computational cost.

• If the function c is independent of the time t, then the stiffness matrix A is independent of time t. Then

$$Mrac{ec{X}^{m+1}-ec{X}^m}{\triangle t}+ heta Aec{X}^{m+1}+(1- heta)Aec{X}^m= heta ec{b}(t_{m+1})+(1- heta)ec{b}(t_m) \ \Rightarrow \quad \left(rac{M}{\triangle t}+ heta A
ight)ec{X}^{m+1}= heta ec{b}(t_{m+1})+(1- heta)ec{b}(t_m)+rac{M}{\triangle t}ec{X}^m-(1- heta)Aec{X}^m.$$

Iteration scheme 2:

$$\tilde{A}\vec{X}^{m+1} = \tilde{\vec{b}}^{m+1}, \ m = 0, \cdots, M_m - 1,$$

where

$$\tilde{A} = \frac{M}{\wedge t} + \theta A,$$

$$ilde{ec{b}}^{m+1} = heta ec{b}(t_{m+1}) + (1- heta) ec{b}(t_m) + \left[rac{ extbf{\textit{M}}}{\wedge t} - (1- heta) A
ight] ec{X}^m.$$

Algorithm *B*:

- Generate the information matrices.
- Assemble the mass matrix M by using Algorithm I-3.
- Assemble the stiffness matrix A by using Algorithm I-3.
- Generate the initial vector \vec{X}^0
- Iterate in time:

```
FOR m = 0, \dots, M_m - 1:
     t_{m+1} = (m+1) \triangle t;
     t_m = m \triangle t:
     Assemble the load vectors b(t_{m+1}) and b(t_m) by using
Algorithm II-5 at t_{m+1} and t_m;
     Deal with boundary conditions
     Solve iteration scheme 2 for \vec{X}^{m+1}
```

- Define $\vec{X}^{m+\theta} = \theta \vec{X}^{m+1} + (1-\theta) \vec{X}^m$.
- Then $\vec{X}^{m+1} \vec{X}^m = \frac{\vec{X}^{m+\theta} \vec{X}^m}{\theta}$ if $\theta \neq 0$.
- Hence

$$M\frac{\vec{X}^{m+1} - \vec{X}^m}{\triangle t} + \theta A \vec{X}^{m+1} + (1-\theta)A \vec{X}^m = \theta \vec{b}(t_{m+1}) + (1-\theta)\vec{b}(t_m)$$

$$\Rightarrow M\frac{\vec{X}^{m+1} - \vec{X}^m}{\triangle t} + A\left[\theta \vec{X}^{m+1} + (1-\theta)\vec{X}^m\right] = \theta \vec{b}(t_{m+1}) + (1-\theta)\vec{b}(t_m)$$

$$\Rightarrow M\frac{\vec{X}^{m+\theta} - \vec{X}^m}{\theta \triangle t} + A \vec{X}^{m+\theta} = \theta \vec{b}(t_{m+1}) + (1-\theta)\vec{b}(t_m)$$

$$\Rightarrow \left(\frac{M}{\theta \triangle t} + A\right) \vec{X}^{m+\theta} = \theta \vec{b}(t_{m+1}) + (1-\theta)\vec{b}(t_m) + \frac{M \vec{X}^m}{\theta \triangle t}.$$

• Iteration scheme 3:

$$\tilde{A}^{\theta}\vec{X}^{m+\theta} = \tilde{\vec{b}}^{m+\theta}, \ m = 0, \cdots, M_m - 1,$$

where

$$ilde{A}^{ heta} = rac{M}{ heta \triangle t} + A, \ ilde{ar{b}}^{m+ heta} = heta ar{b}(t_{m+1}) + (1- heta) ar{b}(t_m) + rac{M}{ heta \triangle t} ar{X}^m.$$

• Since $\vec{X}^{m+\theta} = \theta \vec{X}^{m+1} + (1-\theta) \vec{X}^m$, then

$$\vec{X}^{m+1} = \frac{\vec{X}^{m+\theta} - \vec{X}^m}{\theta} + \vec{X}^m.$$

Algorithm *C*:

- Generate the information matrices.
- Assemble the mass matrix M by using Algorithm I-3.
- Assemble the stiffness matrix A by using Algorithm I-3.
- Generate the initial vector \vec{X}^0
- Iterate in time:

```
FOR m = 0, \dots, M_m - 1:
     t_{m+1} = (m+1) \triangle t;
     t_m = m \triangle t:
     Assemble the load vectors b(t_{m+1}) and b(t_m) by using
Algorithm II-5 at t_{m+1} and t_m;
     Deal with boundary conditions
     Solve iteration scheme 3 for \vec{X}^{m+1}.
```

• Example 1: Use the finite element method to solve the following equation for u(x, y, t) on the domain $\Omega = [0, 2] \times [0, 1]$:

$$u_t - \nabla \cdot (2\nabla u) = -3e^{x+y+t}, \text{ on } \Omega \times [0,1],$$

$$u(x,y,0) = e^{x+y}, \text{ on } \partial \Omega,$$

$$u = e^{y+t} \text{ on } x = 0,$$

$$u = e^{2+y+t} \text{ on } x = 2,$$

$$u = e^{x+t} \text{ on } y = 0,$$

$$u = e^{x+1+t} \text{ on } y = 1.$$

• The analytic solution of this problem is $u = e^{x+y+t}$, which can be used to compute the error of the numerical solution.

Weak formulation

- Let's code for the linear and quadratic finite element method of the 2D second order parabolic equation together!
- We will use Algorithm B.
- Open your Matlab!

h	$\ u-u_h\ _{\infty}$	$ u - u_h _0$	$ u-u_h _1$
1/4	3.7039×10^{-1}	1.4423×10^{-1}	2.5748×10^{0}
1/8	9.8704×10^{-2}	3.5921×10^{-2}	1.2845×10^{0}
1/16	2.5483×10^{-2}	8.9715×10^{-3}	6.4187×10^{-1}
1/32	6.4745×10^{-3}	2.2423×10^{-3}	3.2089×10^{-1}
1/64	1.6318×10^{-3}	5.6055×10^{-4}	1.6044×10^{-1}

Table: Case 1: The numerical errors at t=1 for linear finite element and Crank-Nicolson scheme $(\theta = \frac{1}{2})$ with $\triangle t = h$.

Any Observation?

- Second order convergence $O(h^2)$ in L^2/L^{∞} norm and first order convergence O(h) in H^1 semi-norm.
- The Crank-Nicolson scheme has second order accuracy for temporal discretization.
- The linear finite element has second order accuracy in L^2/L^{∞} norm and first order accuracy in H^1 semi-norm for spatial discretization.
- Hence the accuracy order is expected to be $O(\triangle t^2 + h^2)$ in L^2/L^{∞} norm and $O(\Delta t^2 + h)$ in H^1 norm, which match the above observation since $\triangle t = h$ in case 1.

h	$\ u-u_h\ _{\infty}$	$ u - u_h _0$	$ u-u_h _1$
1/4	3.7039×10^{-1}	$1.9449 imes 10^{-1}$	2.5875×10^{0}
1/8	9.8704×10^{-2}	5.0853×10^{-2}	1.2865×10^{0}
1/16	2.5483×10^{-2}	1.2871×10^{-2}	6.4214×10^{-1}
1/32	6.4745×10^{-3}	3.2279×10^{-3}	3.2092×10^{-1}
1/64	1.6318×10^{-3}	8.0763×10^{-4}	1.6044×10^{-1}

Table: Case 2: The numerical errors at t=1 for linear finite element and backward Euler scheme ($\theta = 1$) with $\triangle t = 4h^2$.

Any Observation?

- Second order convergence $O(h^2)$ in L^2/L^{∞} norm and first order convergence O(h) in H^1 semi-norm.
- The backward Euler scheme has first order accuracy for temporal discretization.
- The linear finite element has second order accuracy in L^2/L^{∞} norm and first order accuracy in H^1 semi-norm for spatial discretization.
- Hence the accuracy order is expected to be $O(\triangle t + h^2)$ in L^2/L^∞ norm and $O(\triangle t + h)$ in H^1 norm, which match the above observation since $\triangle t = 4h^2$ in case 2.

h	$\triangle t$	$\ u-u_h\ _{\infty}$	$\ u-u_h\ _0$	$ u-u_h _1$
1/4	1/8	6.1549×10^{-3}	2.2830×10^{-3}	8.3065×10^{-2}
1/8	1/23	8.1024×10^{-4}	2.8702×10^{-4}	2.0725×10^{-2}
1/16	1/64	1.0403×10^{-4}	3.6236×10^{-5}	5.1789×10^{-3}
1/32	1/181	1.3179×10^{-5}	4.5451×10^{-6}	1.2946×10^{-3}
1/64	1/512	1.6587×10^{-6}	5.6913×10^{-7}	3.2363×10^{-4}

Table: Case 3: The numerical errors at t=1 for quadratic finite element and Crank-Nicolson scheme $(\theta = \frac{1}{2})$ with $\triangle t^2 \approx h^3$.

Any Observation?

- Third order convergence $O(h^3)$ in L^2/L^{∞} norm and second order convergence $O(h^2)$ in H^1 semi-norm.
- The Crank-Nicolson scheme has second order accuracy for temporal discretization.
- The quadratic finite element has third order accuracy in L^2/L^{∞} norm and second order accuracy in H^1 semi-norm for spatial discretization.
- Hence the accuracy order is expected to be $O(\triangle t^2 + h^3)$ in L^2/L^{∞} norm and $O(\Delta t^2 + h^2)$ in H^1 norm, which match the above observation since $\triangle t^2 \approx h^3$ in case 3.

- Case 4: The numerical errors at t = 1 for quadratic finite element and backward Euler scheme $(\theta = 1)$ with $\Delta t = 8h^3$.
- You will observe third order convergence $O(h^3)$ in L^2/L^{∞} norm and second order convergence $O(h^2)$ in H^1 semi-norm.
- The backward Euler scheme has second order accuracy for temporal discretization.
- The quadratic finite element has third order accuracy in L^2/L^{∞} norm and second order accuracy in H^1 semi-norm for spatial discretization.
- Hence the accuracy order is expected to be $O(\triangle t + h^3)$ in L^2/L^{∞} norm and $O(\Delta t + h^2)$ in H^1 norm, which match the above observation since $\triangle t = 8h^3$ in case 4.

Weak formulation

- However, you will also observe much more cost in time for this case too since $\triangle t = 8h^3$ is much smaller than that of the previous cases.
- When the mesh becomes finer and finer or the problem becomes 3D, the situation is even worse.
- This is why we need temporal discretization with higher order accuracy and efficient methods to solve linear systems.

Outline

- 2 Semi-discretization
- More Discussion
- Second order hyperbolic equation

- Forward Euler: cheap at each time iteration step, but conditionally stable, which means that $\triangle t$ must be smaller enough.
- Multi-step methods for temporal discretization: two-step backward differentiation, three-step backward differentiation, Runge-Kutta method......
- Efficient solvers for linear systems: multi-grid, PCG, GMRES.....

Boundary conditions

- The treatment of the Neumann/Robin boundary conditions is similar to that of Chapter 3.
- If the functions in the Neumann/Robin boundary conditions are independent of time, then the same subroutines from Chapter 3 can be used before the time iteration starts.
- If the functions in the Neumann/Robin boundary conditions depend on time, then the same algorithms as those in Chapter 3 can be used at each time iteration step. But the time needs to be specified in these algorithms.

Mixed boundary conditions

Consider

$$u_t - \nabla \cdot (c\nabla u) = f$$
, in $\Omega \times [0, T]$,
 $\nabla u \cdot \vec{n} = p$ on $\Gamma_N \times [0, T]$,
 $\nabla u \cdot \vec{n} + ru = q$ on $\Gamma_R \times [0, T]$,
 $u = g$ on $\partial \Omega / (\Gamma_N \cup \Gamma_R) \times [0, T]$,
 $u = u_0$, at $t = 0$ and in Ω .

Recall

$$\int_{\Omega} u_t v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy - \int_{\partial \Omega} (c \nabla u \cdot \vec{n}) \, v \, ds$$

$$= \int_{\Omega} fv \, dxdy.$$

Mixed boundary conditions

- Since the solution on the domain boundary $\partial\Omega/(\Gamma_N\cup\Gamma_R)$ are given by u(x,y,t)=g(x,y,t), then we can choose the test function v(x,y) such that v=0 on $\partial\Omega/(\Gamma_N\cup\Gamma_R)$.
- Hence, similar to the treatment of the mixed boundary condition in Chapter 3, the weak formulation is to find $u \in H^1(0, T; H^1(\Omega))$ such that

$$\int_{\Omega} u_t v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy + \int_{\Gamma_R} cruv \, ds$$

$$= \int_{\Omega} fv \, dxdy + \int_{\Gamma_N} cpv \, ds + \int_{\Gamma_R} cqv \, ds.$$

- Code?
- Combine all of the subroutines for Dirichlet/Neumann/Robin boundary conditions.

Non-isotropic second order parabolic equation with mixed boundary conditions

Consider

$$u_t - \nabla \cdot (c\nabla u) = f$$
, in $\Omega \times [0, T]$,
 $\nabla u \cdot \vec{n} = p$ on $\Gamma_N \times [0, T]$,
 $\nabla u \cdot \vec{n} + ru = q$ on $\Gamma_R \times [0, T]$,
 $u = g$ on $\partial \Omega / (\Gamma_N \cup \Gamma_R) \times [0, T]$,
 $u = u_0$, at $t = 0$ and in Ω ,

More Discussion

where

$$c=\left(\begin{array}{cc}c_{11}&c_{12}\\c_{21}&c_{22}\end{array}\right).$$

 The treatment of the non-isotropic equation is similar to that of Chapter 3. 4□ > 4回 > 4 重 > 4 重 > 重 の Q ⊙

Outline

- Weak formulation
- 2 Semi-discretization
- Full discretization
- 4 More Discussion
- 5 Second order hyperbolic equation

Weak formulation

Consider the 2D second order hyperbolic equation

$$u_{tt} - \nabla \cdot (c\nabla u) = f$$
, in $\Omega \times [0, T]$, $u = g$, on $\partial \Omega \times [0, T]$, $u = u_0$, $\frac{\partial u}{\partial t} = u_{00}$, at $t = 0$ and in Ω .

where Ω is a 2D domain, [0, T] is the time interval, f(x, y, t)and c(x, y, t) are given functions on $\Omega \times [0, T]$, g(x, y, t) is a given function on $\partial\Omega\times[0,T]$, $u_0(x,y)$ and $u_{00}(x,y)$ are given functions in Ω at t=0, and u(x,y,t) is the unknown function.

Weak formulation

• First, multiply a function v(x,y) on both sides of the original equation,

$$u_{tt} - \nabla \cdot (c\nabla u) = f \text{ in } \Omega$$

$$\Rightarrow u_{tt}v - \nabla \cdot (c\nabla u)v = fv \text{ in } \Omega$$

$$\Rightarrow \int_{\Omega} u_{tt}v \, dxdy - \int_{\Omega} \nabla \cdot (c\nabla u)v \, dxdy = \int_{\Omega} fv \, dxdy.$$

• u(x, y, t) is called a trail function and v(x, y) is called a test function.

 Second, using Green's formula (divergence theory, integration by parts in multi-dimension)

$$\int_{\Omega} \nabla \cdot (c \nabla u) v \, dxdy = \int_{\partial \Omega} (c \nabla u \cdot \vec{n}) v \, ds - \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy,$$

More Discussion

we obtain

$$\int_{\Omega} u_{tt} v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy - \int_{\partial \Omega} (c \nabla u \cdot \vec{n}) v \, ds$$

$$= \int_{\Omega} f v \, dxdy.$$

Weak formulation

- Since the solution on the domain boundary $\partial\Omega$ are given by u(x, y, t) = g(x, y, t), then we can choose the test function v(x,y) such that v=0 on $\partial\Omega$.
- Hence

$$\int_{\Omega} u_{tt} v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy = \int_{\Omega} f v \, dxdy.$$

- What spaces should u and v belong to? Sobolev spaces! (See Chapter 3)
- Define

$$H^2(0,T;H^2(\Omega)) = \{v(t,\cdot), \ \frac{\partial v}{\partial t}(t,\cdot), \ \frac{\partial^2 v}{\partial t^2}(t,\cdot) \in H^2(\Omega), \ \forall t \in [0,T]\}.$$

Weak formulation

Weak formulation

• Weak formulation: find $u \in H^2(0, T; H^2(\Omega))$ such that

$$\int_{\Omega} u_{tt} v \ dx dy + \int_{\Omega} c \nabla u \cdot \nabla v \ dx dy = \int_{\Omega} f v \ dx dy.$$

for any $v \in H_0^1(\Omega)$.

- Let $a(u, v) = \int_{\Omega} c \nabla u \cdot \nabla v dx dy$ and $(f, v) = \int_{\Omega} f v dx dy$.
- Weak formulation: find $u \in H^2(0, T; H^2(\Omega))$ such that

$$(u_{tt},v)+a(u,v)=(f,v)$$

for any $v \in H_0^1(\Omega)$.

Galerkin formulation

• Assume there is a finite dimensional subspace $U_h \subset H^2(\Omega)$. Define U_{h0} to be the space which consists of the functions of U_h with value 0 on the Dirichlet boundary.

More Discussion

• Then the Galerkin formulation is to find $u_h \in H^2(0, T; U_h)$ such that

$$(u_{htt}, v_h) + a(u_h, v_h) = (f, v_h)$$

$$\Leftrightarrow \int_{\Omega} u_{htt} v_h \, dxdy + \int_{\Omega} c \nabla u_h \cdot \nabla v_h \, dxdy = \int_{\Omega} f v_h \, dxdy$$

for any $v_h \in U_{h0}$.

- Basic idea of Galerkin formulation: use finite dimensional space to approximate infinite dimensional space.
- Here $U_h = span\{\phi_j\}_{j=1}^{N_b}$ is chosen to be a finite element space where $\{\phi_i\}_{i=1}^{N_b}$ are the global finite element basis functions.

Galerkin formulation

• For an easier implementation, we use the following Galerkin formulation (without considering the Dirichlet boundary condition, which will be handled later): find $u_h \in H^2(0, T; U_h)$ such that

$$\begin{aligned} &(u_{h_{tt}}, v_h) + a(u_h, v_h) = (f, v_h) \\ \Leftrightarrow & \int_{\Omega} u_{h_{tt}} v_h \ dxdy + \int_{\Omega} c \nabla u_h \cdot \nabla v_h \ dxdy = \int_{\Omega} f v_h \ dxdy \end{aligned}$$

for any $v_h \in U_h$.

Recall the following definitions from Chapter 2:

- N: number of mesh elements.
- N_m : number of mesh nodes.
- E_n ($n = 1, \dots, N$): mesh elements.
- Z_k ($k = 1, \dots, N_m$): mesh nodes.
- N_I : number of local mesh nodes in a mesh element.
- P:information matrix consisting of the coordinates of all mesh nodes.
- T: information matrix consisting of the global node indices of the mesh nodes of all the mesh elements.

- We only consider the nodal basis functions (Lagrange type) in this course.
- N_{lb} : number of local finite element nodes (=number of local finite element basis functions) in a mesh element.
- N_b : number of the finite element nodes (= the number of unknowns = the total number of the finite element basis functions).
- X_i $(j = 1, \dots, N_b)$: finite element nodes.
- P_b: information matrix consisting of the coordinates of all finite element nodes.
- T_b: information matrix consisting of the global node indices of the finite element nodes of all the mesh elements.

• Since $u_h \in H^2(0, T; U_h)$ and $U_h = span\{\phi_j\}_{i=1}^{N_b}$, then

$$u_h(x, y, t) = \sum_{j=1}^{N_b} u_j(t)\phi_j(x, y)$$

for some coefficients $u_i(t)$ $(j = 1, \dots, N_b)$.

If we can set up a linear algebraic system for

$$u_j(t) (j = 1, \cdots, N_b)$$

and solve it, then we can obtain the finite element solution u_h .

• Therefore, we choose $v_h = \phi_i$ $(i = 1, \dots, N_b)$. Then

$$\int_{\Omega} \left(\sum_{j=1}^{N_b} u_j(t) \phi_j \right)_{tt} \phi_i \, dx dy + \int_{\Omega} c \nabla \left(\sum_{j=1}^{N_b} u_j(t) \phi_j \right) \cdot \nabla \phi_i \, dx dy$$

$$= \int_{\Omega} f \phi_i \, dx dy, \ i = 1, \dots, N_b$$

$$\Rightarrow \sum_{j=1}^{N_b} u_j''(t) \left[\int_{\Omega} \phi_j \phi_i \, dx dy \right] + \sum_{j=1}^{N_b} u_j(t) \left[\int_{\Omega} c \nabla \phi_j \cdot \nabla \phi_i \, dx dy \right]$$

$$= \int_{\Omega} f \phi_i \, dx dy, \ i = 1, \dots, N_b.$$

• Here the basis functions ϕ_i $(i = 1, \dots, N_b)$ depend on (x, y)only. But the given functions c and f may depend on t and (x, y). 4 ロ ト 4 回 ト 4 重 ト 4 重 ト 9 へ 0 0 Weak formulation

Define the stiffness matrix

$$A(t) = [a_{ij}]_{i,j=1}^{N_b} = \left[\int_{\Omega} c \nabla \phi_j \cdot \nabla \phi_i \, dx dy \right]_{i,j=1}^{N_b}.$$

Define the mass matrix

$$M = [m_{ij}]_{i,j=1}^{N_b} = \left[\int_{\Omega} \phi_j \phi_i \, dx dy\right]_{i,j=1}^{N_b}.$$

Define the load vector

$$\vec{b}(t) = [b_i]_{i=1}^{N_b} = \left[\int_{\Omega} f \phi_i \ dxdy\right]_{i=1}^{N_b}.$$

Define the unknown vector

$$\vec{X}(t) = [u_j(t)]_{i=1}^{N_b}.$$

Then we obtain the system

$$M\vec{X}''(t) + A(t)\vec{X}(t) = \vec{b}(t).$$

Now let's consider the system of ODEs:

$$M\vec{X}''(t) + A\vec{X}(t) = \vec{b}(t).$$

- Assume that we have a uniform partition of [0, T] into M_m elements with mesh size $\triangle t$.
- The mesh nodes are $t_m = m \triangle t$, $m = 0, 1, \dots, M_m$.
- Assume \vec{X}^m is the numerical solution of $\vec{X}(t_m)$.
- Then the corresponding centered finite difference is

$$M\frac{\vec{X}^{m+1} - 2\vec{X}^m + \vec{X}^{m-1}}{\triangle t^2} + A\frac{\vec{X}^{m+1} + 2\vec{X}^m + \vec{X}^{m-1}}{4}$$

$$= \vec{b}(t_m), m = 1, \cdots, M_m - 1.$$

• Iteration scheme 2:

$$\tilde{A}\vec{X}^{m+1} = \tilde{\vec{b}}^{m+1}, \ m = 0, \cdots, M_m - 1,$$

where

$$\tilde{A} = \frac{M}{\triangle t^2} + \frac{A}{4},$$

$$\tilde{\vec{b}}^{m+1} = \vec{b}(t_m) + \left[\frac{2M}{\triangle t^2} - \frac{A}{2}\right] \vec{X}^m - \left[\frac{M}{\triangle t^2} + \frac{A}{4}\right] \vec{X}^{m-1}.$$

Algorithm *B*:

- Generate the information matrices.
- Assemble the mass matrix M by using Algorithm I-3.
- Assemble the stiffness matrix A by using Algorithm I-3.
- Generate the initial vectors \vec{X}^0 and \vec{X}^1 based on the two initial conditions.
- Iterate in time:

FOR
$$m = 1, \dots, M_m - 1$$
:
 $t_m = m \triangle t$:

Assemble the load vector $\vec{b}(t_m)$ by using Algorithm II-5

at t_m ;

Deal with boundary conditions Solve iteration scheme 2 for \vec{X}^{m+1}

END



Mixed boundary conditions

Consider

$$u_{tt} - \nabla \cdot (c\nabla u) = f$$
, in $\Omega \times [0, T]$,
 $\nabla u \cdot \vec{n} = p$ on $\Gamma_N \times [0, T]$,
 $\nabla u \cdot \vec{n} + ru = q$ on $\Gamma_R \times [0, T]$,
 $u = g$ on $\partial \Omega / (\Gamma_N \cup \Gamma_R) \times [0, T]$,
 $u = u_0$, $\frac{\partial u}{\partial t} = u_{00}$, at $t = 0$ and in Ω .

Recall

$$\int_{\Omega} u_{tt} v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy - \int_{\partial \Omega} (c \nabla u \cdot \vec{n}) v \, ds$$

$$= \int_{\Omega} fv \, dxdy.$$

Mixed boundary conditions

- Since the solution on the domain boundary $\partial \Omega/(\Gamma_N \cup \Gamma_R)$ are given by u(x, y, t) = g(x, y, t), then we can choose the test function v(x, y) such that v = 0 on $\partial \Omega / (\Gamma_N \cup \Gamma_R)$.
- Hence, similar to the treatment of the mixed boundary condition in Chapter 3, the weak formulation is to find $u \in H^2(0, T; H^1(\Omega))$ such that

$$\int_{\Omega} u_{tt} v \, dxdy + \int_{\Omega} c \nabla u \cdot \nabla v \, dxdy + \int_{\Gamma_R} cruv \, ds$$

$$= \int_{\Omega} fv \, dxdy + \int_{\Gamma_N} cpv \, ds + \int_{\Gamma_R} cqv \, ds.$$

- Code?
- Combine all of the subroutines for Dirichlet/Neumann/Robin boundary conditions.

More Discussion

Non-isotropic second order hyperbolic equation with mixed boundary conditions

Consider

$$u_{tt} - \nabla \cdot (c\nabla u) = f$$
, in $\Omega \times [0, T]$,
 $\nabla u \cdot \vec{n} = p$ on $\Gamma_N \times [0, T]$,
 $\nabla u \cdot \vec{n} + ru = q$ on $\Gamma_R \times [0, T]$,
 $u = g$ on $\partial \Omega / (\Gamma_N \cup \Gamma_R) \times [0, T]$,
 $u = u_0$, $\frac{\partial u}{\partial t} = u_{00}$, at $t = 0$ and in Ω ,

where

$$c=\left(\begin{array}{cc}c_{11}&c_{12}\\c_{21}&c_{22}\end{array}\right).$$

 The treatment of the non-isotropic equation is similar to that of Chapter 3. 4 D > 4 A > 4 B > 4 B > B = 490