

This code solves the example of 2D truss structure solved in class

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%% 2D truss structure
% Nodal coordinates
% xi yi
nodes = [0 0; % node 1
         1 0; % node 2 we assume a nominal distance a=1 (**)
         0 -1]; % node 3
n = size(nodes,1); % number of nodes
% Elements
% i j EA we assume a nominal EAo=1 (**)
elem = [1 2 1; % element 1
        3 2 1]; % element 2 (the elements have same EA, not same k!)
m = size(elem,1); % number of elements
% Stiffness matrix
K = zeros(2*n,2*n);
% vector of stiffnesses, angles of inclination, and stiffness matrix;
for q = 1:m
    i = elem(q,1); xi = nodes(i,1); yi = nodes(i,2); % extracting the coordinates of the nodes of pertinence
    j = elem(q,2); xj = nodes(j,1); yj = nodes(j,2);
    L = sqrt((xj-xi)^2+(yj-yi)^2); Lv(q)=L;
    kq = elem(q,3)/L; % stiffness
    tht = atan((yj-yi)/(xj-xi)); % angle
    Ne = kq*[-cos(tht);-sin(tht);cos(tht);sin(tht)];
    %-- alternative path
    qv = ([xj;yj]-[xi;yi])/norm([xj;yj]-[xi;yi]);
    Ne = (kq*[-qv;qv]);
    %--
    Nev(q,:) = Ne;
    Ke = 1/kq*Ne*Ne';
    edofs = [2*i-1,2*i,2*j-1,2*j];
    K(edofs,edofs) = K(edofs,edofs)+Ke;
end
% Boundary conditions
fix_dofs = [1 2 5 6]; % dofs constrained to be 0
free_dofs = setdiff(1:2*n,fix_dofs); % the total number of dofs is 2*n (2 per node); this function curtails the dofs that are not
% constrained
% Loads
% To respect the condition of linear elasticity we need to apply a load that is small enough to leave a strain no bigger than 1%
% (**) If you do some calculations you will find that the nominal strain is  $eo = Po/EAo$ , where  $Po$  is the nominal load;
% Let us assume  $eo = 1\% = 0.01$ ; Hence the nominal load becomes  $Po = 0.01 EAo$ 
Po = 0.01;
f = [0 0 0 -Po 0 0];
% Solution
u = zeros(2*n,1); % the function 'zeros' gives a matrix by defaults, if one of the sizes is 1, that gives a tensor
u(free_dofs) = f(free_dofs)/K(free_dofs,free_dofs);
% Plot results
figure(1)
hold on
grid on
grid minor
figure(2)
hold on
grid on
grid minor
for q = 1:m
    i = elem(q,1); j = elem(q,2);
    xi = nodes(i,1); yi = nodes(i,2); % undeformed configuration
    xj = nodes(j,1); yj = nodes(j,2);
    xid = xi+u(2*i-1); yid = yi+u(2*i); % deformed configuration
    xjd = xj+u(2*j-1); yjd = yj+u(2*j);
    figure(1)
    % undeformed; deformed;

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plot([xi,xj],[yi,yj],'k-o',[xid,xjd],[yid,yjd],'b-o')
figure(2)
Neq = Nev(q,:);
edofs = [2*i-1,2*i,2*j-1,2*j];
Nq = Neq*u(edofs);
Nv(q) = Nq;
if Nq > 0
    plot([xi,xj],[yi,yj],'r','LineWidth',Nq*400) % If the axial force is positive, i.e. it creates tension, then the color is red;
                                                % The thickness of the line is proportional to the magnitude of the force
elseif Nq < 0
    plot([xi,xj],[yi,yj],'b','LineWidth',abs(Nq)*400) % If the axial force is negative, the color of the line is blue;
end
end
figure(1)
title('Structural displacements','fontsize',15)
legend({'Udeformed cofiguration','Deformed cofiguration'},'Location','southeast')
figure(2)
title('Diagram of axial forces','fontsize',15);

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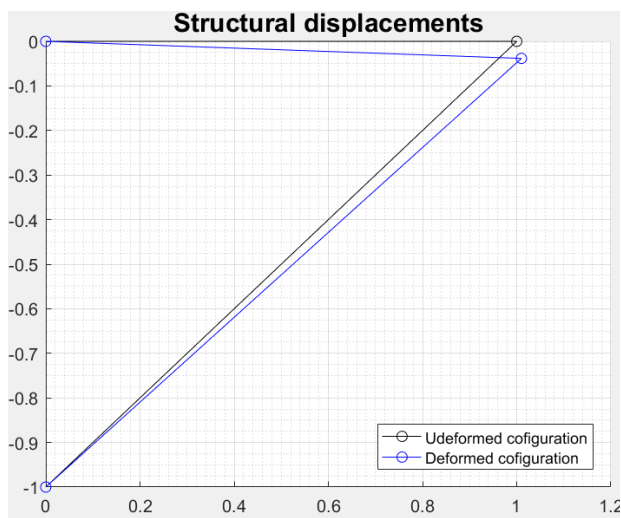


Figure 1

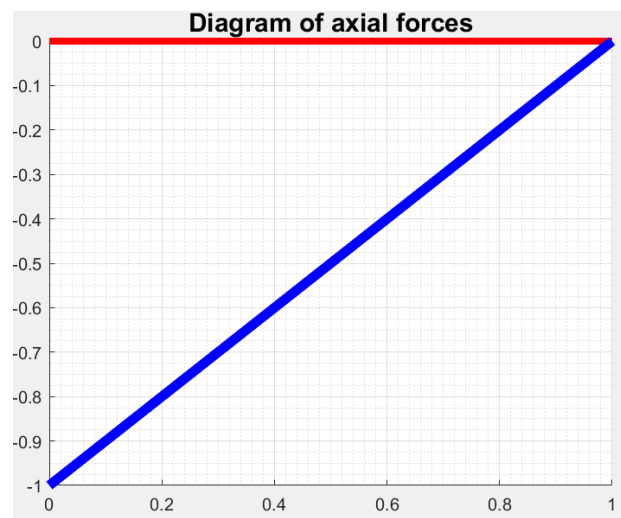


Figure 2