### ESS 411/511 Geophysical Continuum Mechanics Class #23

Highlights from Class #22 — Barrett Johnson
Today's highlights on Wednesday — Madeleine Lucas

### Kinematics of Deformation and Motion

For Wednesday, please read MSM Chapter 4.11 and 4.12

- Velocity gradient and strain rate
- Material derivatives of lines are, and volumes

## Problem Set #4

• I'm still working on it ©

## Mid-term

• I'm working on it next ...

### ESS 411/511 Geophysical Continuum Mechanics

### Broad Outline for the Quarter

- Continuum mechanics in 1-D
- 1-D models with springs, dashpots, sliding blocks
- Attenuation
- Mathematical tools vectors, tensors, coordinate changes
- Stress principal values, Mohr's circles for 3-D stress
- Coulomb failure, pore pressure, crustal strength
- Measuring stress in the Earth
- Strain Finite strain; infinitesimal strains
- Moments lithosphere bending; Earthquake moment magnitude
- Conservation laws
- Constitutive relations for elastic and viscous materials
- Elastic waves; kinematic waves

#### **Kinematics**

Description without reference to forces

### Concept of particle in a continuum

Just an infinitesimal point in the material, labeled with a vector field X

### Displacement

Vector mapping of an object from initial X to final configuration x

#### Deformation

Change of shape described by a displacement field

### Rigid-body rotation and translation

 No deformation, but displacement can differ from point to point Strain or distortion

Elongation or shear

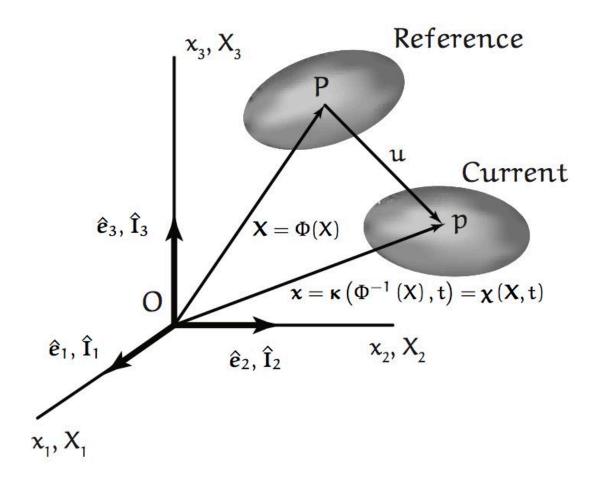
### Homogeneous deformation

Initially straight material lines stay straight

#### Finite strain

Material lines can become curved

## **Initial and Final Configurations**



## Class-prep: Strain Compatibility (Break-out rooms)

For small strains, the strain is defined (Equation 4.62) as

$$2\varepsilon_{ij} = \frac{\partial u_i}{\partial X_A} \delta_{Aj} + \frac{\partial u_j}{\partial X_B} \delta_{Bi} = \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} = u_{i,j} + u_{j,i}$$

where  $u_i = x_i - X_A d_{iA}$  is displacement, the difference between current and initial positions. (The Kroenecker delta just makes it clear how to relate the "A" subscripts in the initial coordinate system to the "i" subscripts in the current coordinate system.)

Section 4.8 describes strain compatibility, and the 81 strain-compatibility equations that relate the various second derivatives.

$$\epsilon_{ij,km} + \epsilon_{km,ij} - \epsilon_{ik,jm} - \epsilon_{jm,ik} = 0$$

#### **Assignment**

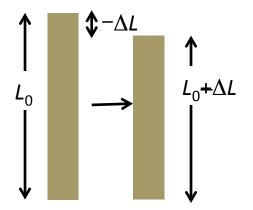
81 equations – that's a lot of equations!

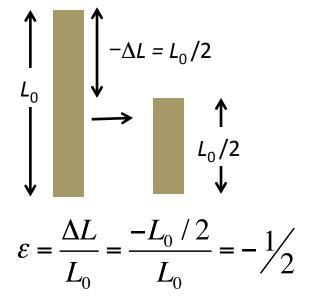
- Why are only 6 of them needed in practice?
- Compatibility with what? What would strain incompatibility look like? On page 129 "It may be shown that the compatibility equations, either Eq 4.90 or Eq 4.91, are both necessary and sufficient for a single-valued displacement field of a body occupying a simply connected domain."
- How would you paraphrase this sentence into simple concepts?

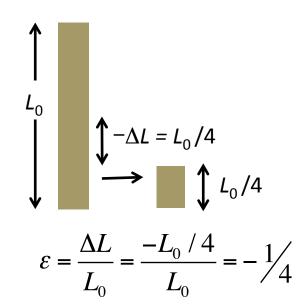
## Finite Strain in 1-D

We typically first see strain expressed as  $\varepsilon = \frac{\Delta L}{L_0}$  and when there is shortening,  $\Delta L < 0$ 

But this approach is not accurate for large strains e.g. when  $-\Delta L$  is large fraction of  $L_0$ . For example,







In both steps the height was halved, so the incremental strain should be the same ...

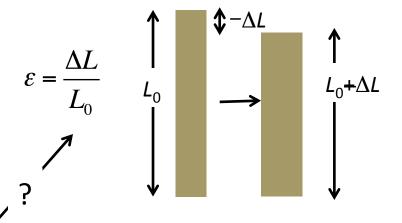
## Finite Strain in 1-D

We can address large strains by adding up a whole series of small strains e.g. when  $-\Delta L$  is large fraction of  $L_0$ .

• We have to reset  $L_0$  at each step and call it l

$$\varepsilon = \int_{L_0}^{L} \frac{dl}{l} = \ln(L) - \ln(L_o) = \ln\left(\frac{L}{L_o}\right)$$

$$\varepsilon = \ln\left(\frac{L}{L_o}\right) = \ln\left(\frac{L_0 + \Delta L}{L_o}\right) = \ln\left(1 + \frac{\Delta L}{L_o}\right)$$



(remembering that  $\Delta L < 0$ )

Now recall the Taylor series for ln(1+x)

$$\ln(1+x) = x - \frac{1}{2}x^2 + \frac{1}{3}x^3 - \dots$$

With  $x = (\Delta L / L_O)$ ,

$$\varepsilon = \ln\left(1 + \frac{\Delta L}{L_0}\right)$$

$$= \frac{\Delta L}{L_0} - \frac{1}{2} \left( \frac{\Delta L}{L_0} \right)^2 + \text{some small stuff}$$

when -1 < x < 1

- The first term looks familiar ...
- The second term corrects for the changing l as strain proceeds

## A measure for strain $(dx)^2 - (dX)^2$

$$(dx)^{2} - (dX)^{2} = (x_{i,A}dX_{A})(x_{i,B}dX_{B}) - \delta_{AB}dX_{A}dX_{B}$$
$$= (x_{i,A}x_{i,B} - \delta_{AB})dX_{A}dX_{B}$$
$$= (C_{AB} - \delta_{AB})dX_{A}dX_{B}$$

Green's deformation tensor

$$C_{AB} = x_{i,A}x_{i,B}$$
 or  $C = F^T \cdot F$ 

Lagrangian finite strain tensor

$$2E_{AB} = C_{AB} - \delta_{AB}$$
 or  $2E = C - I$ 

## A measure for strain $(dx)^2 - (dX)^2$

$$(dx)^{2} - (dX)^{2} = \delta_{ij}dx_{i}dx_{j} - (X_{A,i}dx_{i})(X_{A,j}dx_{j})$$

$$= (\delta_{ij} - X_{A,i}X_{A,j})dx_{i}dx_{j}$$

$$= (\delta_{ij} - c_{ij})dx_{i}dx_{j}$$

Cauchy deformation tensor

$$c_{ij} = X_{A,i}X_{A,j}$$
 or  $\mathbf{c} = (\mathbf{F}^{-1})^T \cdot (\mathbf{F}^{-1})$ 

Eulerian finite strain tensor

$$2e_{ij} = (\delta_{ij} - c_{ij})$$
 or  $2e = (\mathbf{I} - \mathbf{c})$ 

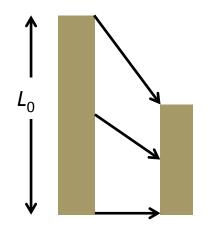
# In terms of displacements $u_i = (x_i - X_i)$

$$2E_{AB} = x_{i,A}x_{i,B} - \delta_{AB} = (u_{i,A} + \delta_{iA})(u_{i,B} + \delta_{iB}) - \delta_{AB}$$

$$2E_{AB} = u_{A,B} + u_{B,A} + u_{i,A}u_{i,B}$$

$$2e_{ij} = \delta_{ij} - X_{A,i}X_{A,j} = \delta_{ij} - (\delta_{Ai} - u_{A,i})(\delta_{Aj} - u_{A,j})$$

$$2e_{ij} = u_{i,j} + u_{j,i} - u_{A,i}u_{A,j}$$



A bar is shortened uniformly in the  $x_1$  direction

$$x_1 = kX_1$$
, e.g.  $k = 1/2$ 

$$k = \left(\frac{x_1}{X_1}\right) = \left(\frac{L_0 + \Delta L}{L_0}\right)$$
$$= \left(1 + \frac{\Delta L}{L_0}\right)$$

$$k-1 = \frac{\Delta L}{L_0}$$
 remember  $\Delta L < 0$ 

## Lagrangian Finite Strain Tensor

$$2E_{AB} = u_{A,B} + u_{B,A} + u_{i,A}u_{i,B}$$
 (\*\*)

Let's see how this fancy-pants equation applies to the 1-D case ...

The only nonzero terms occur when A=B=i=1

Displacement 
$$u_1 = x_1 - X_1 = kX_1 - X_1$$
  
field  $= (k-1)X_1$  (particles move down so  $u_1 < 0$ )

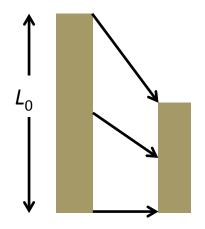
Displacement  $u_{1,1} = \frac{\partial u_1}{\partial X_1} = k - 1$  gradient

Strain 
$$2E_{AB} = 2\frac{\partial u_1}{\partial X_1} + \left(\frac{\partial u_1}{\partial X_1}\right)^2$$
 from (\*\*) field  $= 2(k-1) + (k-1)^2$  A=B=1

$$2E_{AB} = 2\left(\frac{\Delta L}{L_0}\right) + \left(\frac{\Delta L}{L_0}\right)^2 \qquad A = B = 1$$

$$\varepsilon = \ln\left(1 + \frac{\Delta L}{L_0}\right)$$

$$= \frac{\Delta L}{L_0} - \frac{1}{2}\left(\frac{\Delta L}{L_0}\right)^2 + \text{some small stuff}$$



A bar is shortened in the  $x_1$  direction

$$x_1 = kX_1$$
, e.g.  $k = \frac{1}{2}$ 

Strain is homogeneous since k is a constant

## Lagrangian Finite Strain Tensor

$$2E_{AB} = u_{A,B} + u_{B,A} + u_{i,A}u_{i,B}$$

Let's see how this fancy-pants equation applies to the 1-D case ...

$$2E_{AB} = 2\left(\frac{\Delta L}{L_0}\right) + \left(\frac{\Delta L}{L_0}\right)^2$$

$$E_{AB} = \left(\frac{\Delta L}{L_0}\right) + \frac{1}{2}\left(\frac{\Delta L}{L_0}\right)^2 \qquad A = B = 1$$

Strain tensor 
$$E_{AB} = \begin{bmatrix} \left( \left( \frac{\Delta L}{L_0} \right) + \frac{1}{2} \left( \frac{\Delta L}{L_0} \right)^2 \right) & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$