Auteur Kostas Senetakis

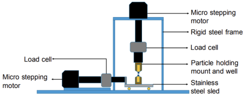
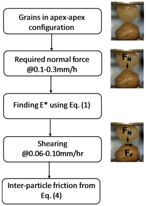


Figure 1. Schematic diagram of the inter-particle loading apparatus of the City University of Hong Kong.



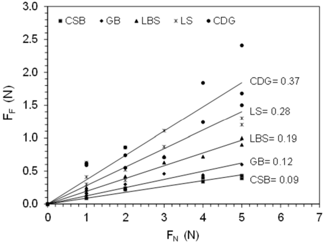


Figure 4. Variation of frictional force with normal force for the materials tested and corresponding inter-particle coefficients of friction.

[Kostas Senetakis, 2017,2018] : "Under the low normal load range applied in the study, between 1 and 5 N, we found that the frictional force is linearly correlated with the applied normal load"

Article Stress-strain behavior of sand at high strain rates (Mehdi Omidvar et al,2012)

M. Omidvar et al. / International Journal of Impact Engineering 49 (2012) 192–213

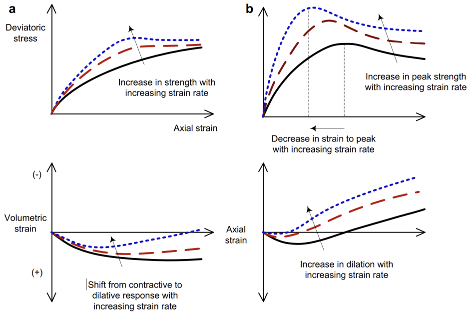


Fig. 19. Effect of increase in strain rate on stress-strain response and volumetric strains in (a) loose sand, (b) dense sand (interpreted based on data from Table 3).

"Under HSR loading, there is not enough time for strain energy accumulation, which prohibits crushing and promotes rolling-rearrangement resulting in a higher resistance to shear"

Trouver le régime de l'état critique

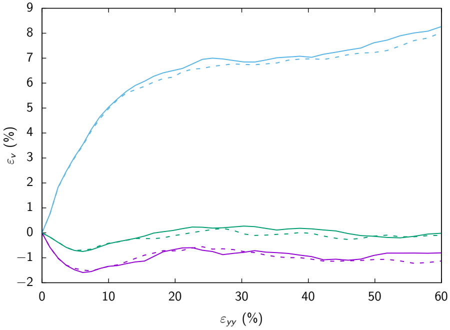


Figure 1 – petit déformation

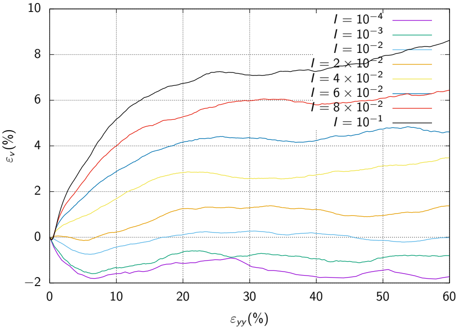


Figure 2 – grande déformation

$$\varepsilon_v = \varepsilon_x + \varepsilon_y + \varepsilon_z$$

$$\varepsilon_v = \frac{det(h)}{det(h_0)} - 1$$

Trouver le régime de l'état critique

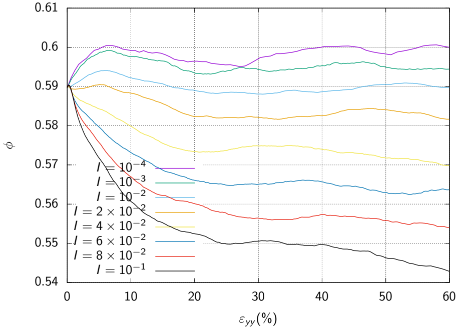


Figure 3 – Fraction solide

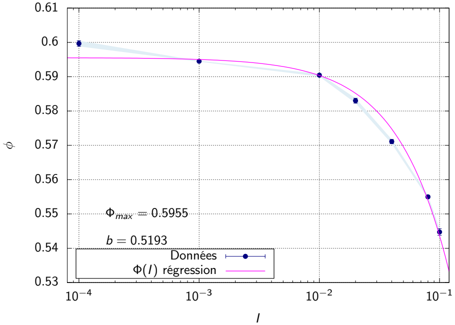


Figure 4 – grande déformation

Trouver le régime de l'état critique

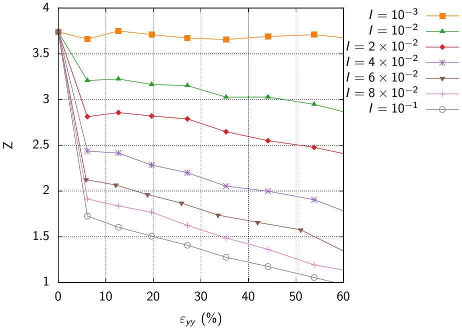


Figure 5 – Nombre de NombreCoordination

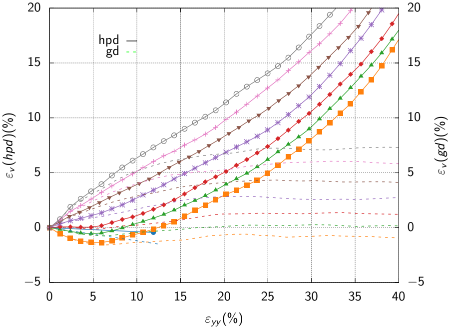


Figure 6 – Déformation Volumique

$$\text{hpd} : \varepsilon_{yy} = \frac{\Delta yy}{h_{yy}^0}; \varepsilon_v = \varepsilon_{xx} + \varepsilon_{yy} + \varepsilon_{zz} \qquad \text{gd} : \varepsilon_{yy} = \ln\left(\frac{h_{yy}}{h_{yy}^0}\right); \varepsilon_v = \frac{\Delta V}{V_0};$$

Trouver le régime de l'état critique

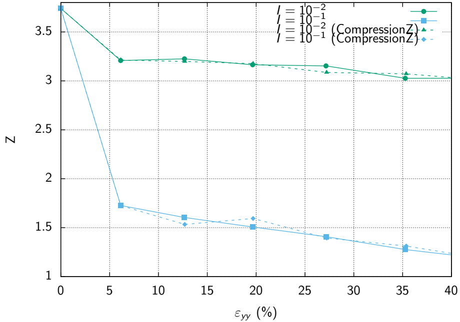


Figure 7 – Nombre de Coordination

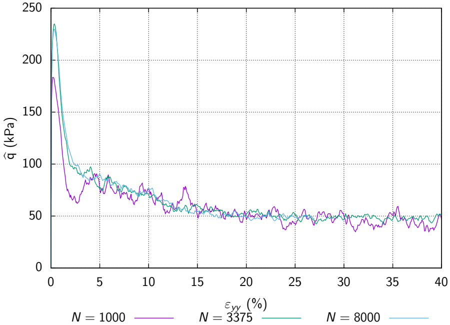


Figure 8 – Nombre de Particules

échantillon aléatoire par compression dans l'axe Z

Stabilisation d'une colonne de sol par Mohr-Coulomb

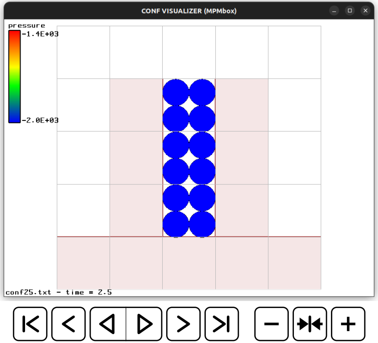


Figure 9 – MP=12

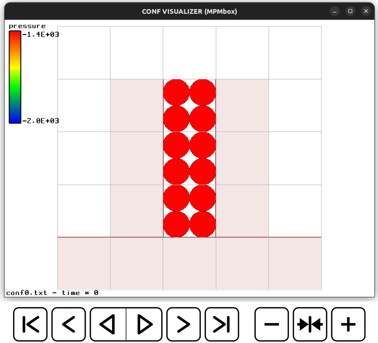


Figure 10 – augementer rigidité de condition aux limites

Effondrement d'une colonne de sol par Mohr-Coulomb

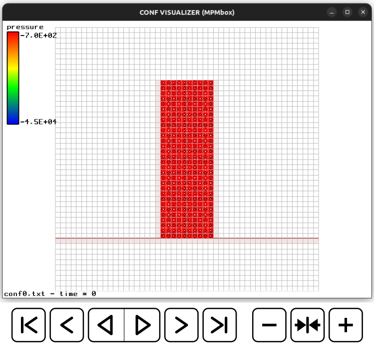


Figure 11 – MP=1200

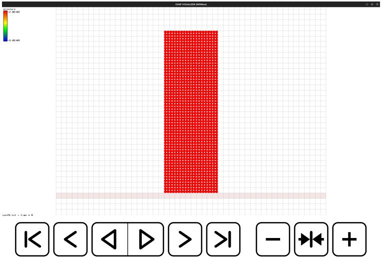


Figure 12 – MP=1200 Effondrement

Poster pour la conférence "Powder and grains"

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