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# Efficient topology optimization with linearized buckling using reduced order modelling

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Abstract Keywords Topology optimization, Buckling, Reduced order modelling, Combined approximations

#### 1 Introduction

Untortunately Topology optimization (TO), is a useful tool for designing structural components and materials. Frequently, compliance infimization subject to a volume considered. However, in practical applications also stability can be crucial inherently stiffness and stability are two competing concepts, wherefore stability considerations must be explicitly included in the optimization formulation. The most common way of analyzing stability is to perform a linearized buckling analysis (Neves et al [17], Dalklint et al [6], Gao and Ma [10], Ferrari and Sigmund [8]).

Typically, the optimization problem is solved using a nested approach by implicitly enforcing and include the equilibrium equations, which constitute the majority of the computational cost. This cost is further increased when buckling is considered, since a generalized eigenvalue problem must be solved. Eigenvalue problems are notoriously difficult to solve and scale poorly with the problem Constraint size, in comparison to linear systems of equations. This cost becomes prohibitive for large to medium scale problems, which the technique unappealing for industrial applications.

Recently, efforts have been made to extend TO with buckling constraints to large scale problengs. Dunning et al [7], presented a Block-Jacobi CG solver with a shift-and invert strategy based on information from previous design iterations. While this method was shown to be efficient it till requires a matrix factorization at each design update. Ferrari and Sigmund [9], water the wed problem by reusing the multigrid preconditioner from the initial equilibrium problem to solve the eigenvalue problem on a coarser grid. The coarse-scale eigenvectors are then projected onto the fine grid, and are refined using inverse iteration. This approach was shown to reduce the cost for solving the eigenvalue problems and filter out pseudo buckling modes, while retaining a good approximation of the global modes. Also worth noting is the work by Bian and Fang [3], where an assembly-free eigenvalue solver is used to solve the eigenvalue problem on a grid of several million elements.

A number of methods based on reduced order modeling (ROM) have been proposed to accelerate topology optimization. In ROM equilibrium equations are projected onto a reduced solution space which is much smaller than the high-fidelity problem. The efficacy of a ROM is determined by the generation of the reduced space. One approach is to built at the reduced space from previous solutions to the equilibrium equations, which are updated as new solution vectors become available [11.5]. Worth noting is the work by Kang et al. in [12] [11,5]. Worth noting is the work by Kang et al in [13] where the accuracy of the eigenvalue problem is improved sequentially such that it converges concurrently with the optimization problem.

a lower bound on the 10W464 buckling load factor

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for optimization problems Another form of ROM is reanalysis, in which the system matrix's factorization is used to build a reduced space for subsequent iterations. The use of reanalysis to solve linear systems was first discussed by Kirsch [14]. More recently, reanalysis has been applied to topology optimization. Amir [1] studied reanalysis for compliance problems and discussed the idea of a consistent sensitivity analysis, i.e. where the inaccuracies in the ROM are taken into account. Bogomolny [4] extended reanalysis to vibrational problems, wherein a reduced order model is built for each sought eigenpair. For the generalized eigenvalue problem the basis vectors must be orthogonalized to ensure that the correct eigenpair is approximated. Bogomolny also derives a consistent sensitivity analysis for the vibrational problem.

In this paper, we aim to show that ROM using Combined Approximation can effectively be implemented for buckling problems. What makes this work unique is that CA will be used for the linear analysis, generalized eigenproblem and the adjoints.

#### 2 Preliminaries

sec:prelim

Mur här Också

We consider the finite element discretized linear system

$$\mathbf{K}_L \mathbf{u}_o = \mathbf{F}_o, \tag{1}$$

in combination with appropriate boundary conditions to describe our mechanical system. In (1), we define the symmetric and positive definite linear stiffness matrix  $\mathbf{K}_L \in \mathbb{R}^{n \times n}$ , the displacement vector  $\mathbf{u}_o \in \mathbb{R}^n$  and the reference load vector  $\mathbf{F}_o \in \mathbb{R}^n$ , where n is the number of degrees-of-freedom. To solve (1), we utilize the Cholesky decomposition of  $\mathbf{K}_L$  in combination with forward/backward substitutions, i.e.

$$\mathbf{K}_L = \mathbf{U}^T \mathbf{U},$$
 (2) mechFac01

where  $\mathbf{U} \in \mathbb{R}^{n \times n}$  is upper triangular. After solving (1) for  $\mathbf{u}_o$  using (2), we compute the symmetric and indefinite stress stiffness matrix  $\mathbf{K}_G(\mathbf{u}_o) \in \mathbb{R}^{n \times n}$ , which enables the generalized eigenvalue

$$(\mathbf{K}_L + \lambda_j \mathbf{K}_G(\mathbf{u}_o)) \, \mathbf{\Phi}_j = \mathbf{0}, \quad j \in \mathbb{N}_n, \tag{3} \quad \boxed{\text{lineig01}}$$

to be solved for the eigenpairs  $(\lambda_i, \mathbf{\phi}_i) \in (\mathbb{R} \times \mathbb{R}^n)$ . However, since  $\mathbf{K}_G$  is indefinite, it is convenient

$$(\mathbf{K}_G(\mathbf{u}_o) - \mu_j \mathbf{K}_L) \, \mathbf{\Phi}_j = \mathbf{0}, \quad j \in \mathbb{N}_n, \tag{4}$$

where  $\mu_j = -\frac{1}{\lambda_i}$  and we assume that  $\mathbf{\Phi}_i^T \mathbf{K}_L \mathbf{\Phi}_j = \delta_{ij}$ . Solving the generalized eigenvalue problem in (3) or (4), is referred to as linearized buckling analysis, wherefore the eigenvalues  $\lambda_j$  are denoted the critical load factors, so that the critical load is  $\mathbf{F}_i = \lambda_i \mathbf{F}_o$ . The eigenmode associated with the critical load  $\mathbf{F}_i$  is  $\mathbf{\Phi}_i$ .

#### 3 Reduced order modelling

sec:CA

The basic idea of reduced order modelling (ROM) is to approximate solutions to linear systems using a reduced basis, i.e.

$$\tilde{\mathbf{u}} \approx \alpha_1 \hat{\mathbf{u}}_1 + \alpha_2 \hat{\mathbf{u}}_2 + \dots + \alpha_m \hat{\mathbf{u}}_m \tag{5}$$

where the number of bases, m, is significantly smaller that the number of dimension of the full problem, i.e.  $m \ll n$ . The success of the ROM method relies on the choice of the basis vectors,  $\hat{\mathbf{u}}_i$ and in this work we will use CA to establish the basis vectors.

The idea of CA is to efficiently find approximate solutions  $\tilde{\mathbf{u}} \approx \mathbf{u}$  to (1), and  $(\tilde{\mu}_i, \mathbf{\phi}_i) \approx (\mu_i, \mathbf{\phi}_i)$ to (4), which are sufficiently accurate. Let us start by considering the approximate solution of (1). In the combined approximations (CA) approach, we utilize the factorization of  $\mathbf{K}_L$  in a previous design iteration, to compute the displacement in the current design iteration, i.e.

$$(\mathbf{K}_L + \Delta \mathbf{K}_L) \mathbf{u} = \mathbf{F},\tag{6}$$

where  $\Delta \mathbf{K}_L$  is the change in stiffness due to the design change. Multiplying (6) by  $\mathbf{K}_L^{-1}$  yields

$$\left(\mathbf{1} + \mathbf{K}_{L}^{-1} \Delta \mathbf{K}_{L}\right) \mathbf{u} = \mathbf{K}_{L}^{-1} \mathbf{F}. \tag{7}$$

It can be shown that the inverse of  $(\mathbf{1} + \mathbf{K}_L^{-1} \Delta \mathbf{K}_L)$  exists as a power series if  $\|\mathbf{K}_L^{-1} \Delta \mathbf{K}_L\|_2 < 1$ , hence

$$(\mathbf{1} + \mathbf{B})^{-1} = \sum_{k=0}^{\infty} (-\mathbf{B})^k, \quad \text{where} \quad \mathbf{B} = \mathbf{K}_L^{-1} \Delta \mathbf{K}_L. \tag{8}$$

The solution to (7) can therefore be expressed as

$$\mathbf{u} = (\mathbf{1} - \mathbf{B} + \mathbf{B}^2 - \dots) \,\mathbf{K}_L^{-1} \mathbf{F}. \tag{9}$$

The idea of CA is to truncate the infinite power series (8) at e.g.  $s \ll n$ , and assume that the approximative solution is spanned by this basis, i.e.

$$\tilde{\mathbf{u}} = y_1 \mathbf{u}_1 + y_2 \mathbf{u}_2 + \ldots + y_s \mathbf{u}_s = \mathbf{R} \mathbf{y}, \tag{10}$$

where  $\mathbf{R} \in \mathbb{R}^{n \times s}$  contains the basis vectors  $\mathbf{u}_1$ ,  $\mathbf{u}_2$ , ...,  $\mathbf{u}_s$  and  $\mathbf{y} \in \mathbb{R}^s$  the coefficients. The basis vectors are obtained recursively from (9) as

$$\begin{aligned} \mathbf{u}_1 &= \mathbf{K}_L^{-1} \mathbf{F}, \\ \mathbf{u}_i &= -\mathbf{B} \mathbf{t}_{i-1}, \quad i = 2, ..., s, \end{aligned} \tag{11} \quad \boxed{\mathtt{basisvec}}$$
 
$$\mathbf{t}_i &= \mathbf{u}_i (\mathbf{u}_i^T \mathbf{u}_i)^{-1/2}, \quad i = 1, ..., s, \end{aligned}$$

where  $\mathbf{u}_1$  is known from the previous optimization step. Now, replacing the true solution  $\mathbf{u}$  with the approximative (10) in (1) and premultiplying both sides with  $\mathbf{R}^T$  yields

$$\mathbf{K}_L^R \mathbf{y} = \mathbf{F}^R,$$
 (12) redSys

where  $\mathbf{K}_L^R = \mathbf{R}^T \mathbf{K}_L \mathbf{R}$  and  $\mathbf{F}^R = \mathbf{R}^T \mathbf{F}$ . The linear system (12) has  $s \ll n$  unknowns  $\mathbf{y}$ .

If we employ Gram-Schmidt orthogonalization of the basis vectors with respect to  $\mathbf{K}_L$  before solving (12), we are able to obtain a fully decoupled system of equations. Let the orthonormalized vectors be denoted  $\mathbf{v}_1$ ,  $\mathbf{v}_2$ , ...,  $\mathbf{v}_s$ , then

$$\begin{aligned} \mathbf{v}_1 &= \mathbf{u}_1, \\ \mathbf{v}_i &= \mathbf{u}_i - \sum_{j=1}^{i-1} \left( \mathbf{u}_i^T \mathbf{K}_L \mathbf{v}_j \right) \mathbf{v}_j. \end{aligned} \tag{13}$$

Now, let  $\tilde{\mathbf{u}} = \mathbf{Vz}$ , wherefore (1) can be approximated as

$$\mathbf{z} = \mathbf{V}^T \mathbf{F},$$
 (14) redSysOrtho

and we find the approximative solution  $\tilde{\mathbf{u}} = \mathbf{V}(\mathbf{V}^T \mathbf{F})$ . This results in that any potential issues with ill-conditioning of  $\mathbf{K}_L^R$  vanishes when solving for the approximate solution.

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#### 3.1 Reanalysis of the buckling problem

Let us now turn to the approximative solution of (4). Again, we utilize the factorization of  $\mathbf{K}_L$  in a previous design iteration, to compute the eigenpairs in the current design iteration, i.e.

$$\mathbf{K}_{G}\mathbf{\Phi}_{j} = \mu_{j}\left(\mathbf{K}_{L} + \Delta\mathbf{K}_{L}\right)\mathbf{\Phi}_{j}, \quad j \in \mathbb{N}_{n}. \tag{15}$$

Premultiplication of (15) by  $\mathbf{K}_L^{-1}$  yields

$$\mathbf{K}_{L}^{-1}\mathbf{K}_{G}\mathbf{\Phi}_{j} = \mu_{j}\left(\mathbf{1} + \mathbf{B}\right)\mathbf{\Phi}_{j}, \quad j \in \mathbb{N}_{n}. \tag{16}$$

As before, the inverse of (1 + B) exists as a power series if ||B|| < 1, i.e.

$$\mathbf{\phi}_{j} = \frac{1}{\mu_{i}} \left( \mathbf{1} - \mathbf{B} + \mathbf{B}^{2} - \ldots \right) \mathbf{K}_{L}^{-1} \mathbf{K}_{G} \mathbf{\phi}_{j}, \quad j \in \mathbb{N}_{n}, \tag{17}$$

An approximation to  $\phi_j$ , i.e.  $\tilde{\phi}_j$ , is obtained by truncating the infinite sum at term  $s \ll n$  and substituting  $\phi_i$  for  $\bar{\phi}_i$  the j:th eigenvector corresponding to the matrix  $\mathbf{K}_L$ , such that

$$\tilde{\mathbf{\Phi}}_i = y_1 \mathbf{u}_1 + y_2 \mathbf{u}_2 + \ldots + y_s \mathbf{u}_s, \tag{18}$$

where the basis vectors  $\mathbf{u}_1, \mathbf{u}_2, ..., \mathbf{u}_n$  are obtained from (17) as

$$\begin{aligned} \mathbf{u}_1 &= \mathbf{K}_L^{-1} \mathbf{K}_G \bar{\mathbf{\Phi}}_j, \\ \mathbf{u}_i &= -\mathbf{B} \mathbf{t}_{i-1}, \quad i = 2, ..., s, \\ \mathbf{t}_i &= \mathbf{u}_i \left( \mathbf{u}_i^T \mathbf{K}_L \mathbf{u}_i \right)^{-1/2}, \quad i = 1, ..., s. \end{aligned} \tag{19}$$

To obtain an improved set of approximated eigenvectors, we employ deflation, i.e. Gram-Schmidt orthogonalization. In this way, we rather use

$$\begin{aligned} \mathbf{u}_1 &= \mathbf{K}_L^{-1} \mathbf{K}_G \bar{\mathbf{\Phi}}_j, \\ \mathbf{u}_i &= -\mathbf{B} \mathbf{t}_{i-1}, \quad i = 2, ..., s, \\ \mathbf{t}_i &= \mathbf{u}_i \left( \mathbf{u}_i^T \mathbf{K}_L \mathbf{u}_i \right)^{-1/2}, \quad i = 1, ..., s, \end{aligned} \tag{20}$$

$$\mathbf{v}_i &= \mathbf{t}_i - \sum_{i=1}^{k-1} \left( \tilde{\mathbf{\Phi}}_j^T \mathbf{K}_L \mathbf{t}_i \right) \tilde{\mathbf{\Phi}}_j, \quad i = 1, ..., s.$$

Here we show the equations that lead to CA for the buckling problem - how are linear systems approximated, how are EV systems approximated, e.g. [14,15].

## 4 Design representation, filtration and thresholding

The goal of our topology optimization is to optimally distribute a linear elastic material in the design domain,  $\Omega \in \mathbb{R}^3$ , which is quantified by the continuous non-dimensional volume fraction field  $z:\Omega \to [0,1]$ . A well-posed optimization problem is obtain using restriction, by which fine-scale oscillations of z are penalized via the Helmholtz PDE-filter, cf. [16]. In this way, z is replaced by the smooth field  $\nu:\Omega \to [0,1]$ . To limit regions wherein  $\nu(\boldsymbol{X}) \in (0,1)$ , we use thresholding ([12], [20]) and penalization ([2]), such that

$$\mathbf{D} = \chi_L(\bar{\nu})\mathbf{D}_o \quad \text{and} \quad \mathbf{G} = \chi_G(\bar{\nu})\mathbf{G}_o,$$
where  $\chi_L(\bar{\nu}) = \delta_o + \bar{\nu}^p(1 - \delta_o), \quad \chi_G(\bar{\nu}) = \bar{\nu}^p,$ 
and  $\bar{\nu} = H_{\beta,\eta}(\nu) = \frac{\tanh(\beta\eta) + \tanh(\beta(\nu - \eta))}{\tanh(\beta\eta) + \tanh(\beta(1 - \eta))}.$ 

$$(21) \quad \text{matInter}$$

In (21),  $\beta$  and  $\eta$  are numerical parameters defined such that  $\lim_{\beta \to \infty} H_{\beta,\eta}(\nu) = u_s(\nu - \eta)$ , where  $u_s$ is the unit step function. Increasing values of  $p \in \mathbb{R}^+$  enforce increasing levels of penalization, and  $\delta_o \in \mathbb{R}^+$  is the ersatz material stiffness scaling. Henceforth, we consider

$$\begin{aligned} \mathbf{K}_L(\rho) &= \sum \int_{\Omega} \chi_L(\rho) \mathbf{B}^T \mathbf{D} \mathbf{B} \, dV \\ \mathbf{K}_G(\rho, \mathbf{u}(\rho)) &= \sum \int_{\Omega} \chi_G(\rho) \mathbf{Y}^T \mathbf{G}(\mathbf{u}) \mathbf{Y} \, dV, \end{aligned} \tag{22}$$

where **B** and **G** contains the gradient of the element shape functions, **Y** is a symmetric block matrix of the stresses and **D** is the Voigt representation of the material tangent.

Don't forget to show the aggregation of eigenvalues using a p-norm, following Torii [19,8]

#### 5 Optimization formulation

We use the topology optimization to solve either of the two optimization problems presented below. The first optimization problem is

$$(\mathbb{TO}_1) \begin{cases} \min_{\mathbf{z}} & g_o = \gamma = \frac{1}{\lambda}, \\ \text{s.t.} & \begin{cases} g_1 = \int_{\Omega} \bar{\nu} \, dV - V_{max} \le 0, \\ 0 \le \mathbf{z}_e \le 1, \quad e = 1, ..., N_e, \end{cases} \end{cases}$$

$$(23) \quad \boxed{\text{TO1}}$$

in which we seek to maximize the smallest buckling load factor  $\lambda$  by minimizing the reciprocal  $\gamma$ , subject to a standard upper bound constraint on the structural volume. The second optimization problem is

$$(\mathbb{TO}_2) \begin{cases} \min_{\mathbf{z}} & g_o = \mathbf{F}^T \mathbf{u}, \\ g_1 = \int_{\Omega} \bar{\nu} \, dV - V_{max} \le 0, \\ g_2 = \gamma \le \gamma^*, \\ 0 \le \mathbf{z}_e \le 1, \quad e = 1, ..., N_e, \end{cases}$$

$$(24) \quad \boxed{\text{TO2}}$$

which is a standard compliance minimization problem subject to an lower bound constraint on the smallest buckling load factor and a maximum volume constraint.

- Minimize  $\gamma = \frac{1}{\lambda}$  s.t. volume Minimize compliance s.t.  $\gamma \leq \gamma^*$  and volume

#### 5.1 Sensitivity analysis

When computing the sensitivity of a non self-adjoint displacement dependent function, the implicit sensitivity of the displacements with respect to the design variables appears. In this work, we annihilate this implicit sensitivity through the adjoint method. To exemplify this process, we introduce the augmented version of a general function  $g = g(\mathbf{v}(\mathbf{z}), \mathbf{u}(\mathbf{v}(\mathbf{z})))$  as

$$q := q - \mathbf{\Lambda}^T \mathbf{r} - \mathbf{\Lambda}_{\nu}^T \mathbf{r}_{\nu}, \tag{25}$$

where  $\mathbf{r} = \mathbf{K}_L \mathbf{u} - \mathbf{F}$  and  $\mathbf{r}_{\mathbf{v}} = \mathbf{K}_{\mathbf{v}} \mathbf{v} - \mathbf{F}_{\mathbf{v}} \mathbf{z}$ . The sensitivity of (25) with respect to  $\mathbf{z}$  is

$$\frac{\partial g}{\partial \mathbf{z}} = \frac{\partial g}{\partial \mathbf{v}} \frac{\partial \mathbf{v}}{\partial \mathbf{z}} + \frac{\partial g}{\partial \mathbf{u}} \frac{\partial \mathbf{u}}{\partial \mathbf{v}} \frac{\partial \mathbf{v}}{\partial \mathbf{z}} - \mathbf{\Lambda}^T \left( \frac{\partial \mathbf{r}}{\partial \mathbf{v}} \frac{\partial \mathbf{v}}{\partial \mathbf{z}} + \frac{\partial \mathbf{r}}{\partial \mathbf{u}} \frac{\partial \mathbf{u}}{\partial \mathbf{v}} \frac{\partial \mathbf{v}}{\partial \mathbf{z}} \right) - \mathbf{\Lambda}^T_{\mathbf{v}} \left( \frac{\partial \mathbf{r}_{\mathbf{v}}}{\partial \mathbf{z}} + \frac{\partial \mathbf{r}_{\mathbf{v}}}{\partial \mathbf{v}} \frac{\partial \mathbf{v}}{\partial \mathbf{z}} \right), \tag{26}$$

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which can be rearranged to

$$\frac{\partial g}{\partial \mathbf{z}} = -\mathbf{\Lambda}_{\mathbf{v}}^{T} \frac{\partial \mathbf{r}_{\mathbf{v}}}{\partial \mathbf{z}} + \left[ \frac{\partial g}{\partial \mathbf{v}} - \mathbf{\Lambda}^{T} \frac{\partial \mathbf{r}}{\partial \mathbf{v}} - \mathbf{\Lambda}_{\mathbf{v}}^{T} \frac{\partial \mathbf{r}_{\mathbf{v}}}{\partial \mathbf{v}} + \left( \frac{\partial g}{\partial \mathbf{u}} - \mathbf{\Lambda}^{T} \frac{\partial \mathbf{r}}{\partial \mathbf{u}} \right) \frac{\partial \mathbf{u}}{\partial \mathbf{v}} \right] \frac{\partial \mathbf{v}}{\partial \mathbf{z}}.$$
 (27) [gensens03]

Here, we find that the implicit  $\frac{\partial u}{\partial \nu}$  and  $\frac{\partial \nu}{\partial z}$  sensitivities are annihilated by sequentially solving the coupled linear systems

$$\mathbf{K}_{L} \boldsymbol{\Lambda} = \frac{\partial g}{\partial \mathbf{u}},$$

$$\mathbf{K}_{\mathbf{v}} \boldsymbol{\Lambda}_{\mathbf{v}} = \frac{\partial g}{\partial \mathbf{v}} - \boldsymbol{\Lambda}^{T} \frac{\partial \mathbf{r}}{\partial \mathbf{v}},$$

$$(28) \quad \boxed{\text{gensen04}}$$

which yields the final expression for the sensitivity of g as

$$\frac{\partial g}{\partial \mathbf{z}} = -\mathbf{\Lambda}_{\mathbf{v}}^{T} \frac{\partial \mathbf{r}_{\mathbf{v}}}{\partial \mathbf{z}} \tag{29}$$

When seeking the sensitivity of the smallest buckling load factor, we let  $g = \mu_j$  and compute  $\frac{\partial g}{\partial \mathbf{u}}$  and  $\frac{\partial g}{\partial \mathbf{v}}$  from the differentiations of (4), i.e.

$$\frac{\partial \mu_{j}}{\partial \mathbf{u}} = \mathbf{\phi}_{j}^{T} \frac{\partial \mathbf{K}_{G}}{\partial \mathbf{v}} \mathbf{\phi}_{j}, \quad j \in \mathbb{N}_{n}, 
\frac{\partial \mu_{j}}{\partial \mathbf{v}} = \mathbf{\phi}_{j}^{T} \left( \frac{\partial \mathbf{K}_{G}}{\partial \mathbf{v}} - \mu_{j} \frac{\partial \mathbf{K}_{L}}{\partial \mathbf{v}} \right) \mathbf{\phi}_{j}, \quad j \in \mathbb{N}_{n}.$$
(30) bucklingsens01

- Consistent sensitivity is favorable, but inconsistent approaches can be efficient.
- Show the challenge of orthogonalization with evolving modes.

#### 6 Numerical examples

sec:examples

We demonstrate the reduced order model on three different geometries, see figures 1, 2 and 3. The optimization problem (24) is solved using Svanbergs MMA solver, with default parameters see [18] for details. A PDE-filter of Helmholtz type is used to control the length scale, following [16], with a filter radius of  $0.1 \times \min(L_x, L_y)$ . Young's modulus is set to  $2 \times 10^5$ , Poisson's ratio is set to  $\nu = 0.3$ . A continuation scheme is used for the material penalization in (21), where p = 2 initially and is incremented by 0.5 every 20 iterations until p = 6, and  $\delta_0 = 1 \times 10^{-6}$ . A constraint on the critical buckling load factor is enforced by aggregating the 6 lowest load factors with the p-norm. The penalty exponent is set to p = 8 initially and is amplified by 4 once p = 6 for the material penalization. Finally, the filtered densities are penalized using a smooth Heaviside approximation according to (21), where  $\beta = 6$  and  $\eta = 0.5$ .

### 6.1 Axially loaded rod



Fig. 1 Geometry of column

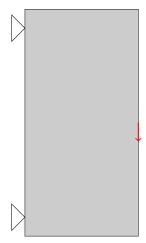
# 6.2 Spire structure



 $\bf Fig.~2~{\rm Geometry~of~spire}$ 

fig:geometry\_spi

# $6.3~\mathrm{V}~\mathrm{structure}$



 ${\bf Fig.~3~~Geometry~of~twobar}$ 

fig:geometry\_two

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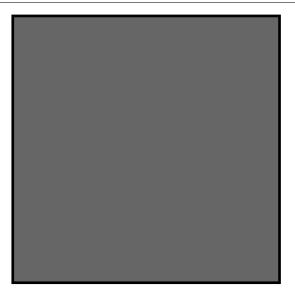
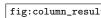
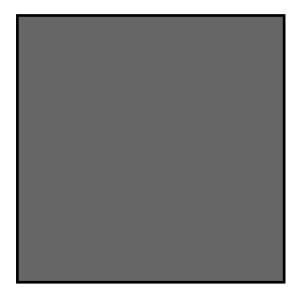


Fig. 4 Caption





 ${\bf Fig.~5}~{\rm Please~write~your~figure~caption~here}$ 

fig:1

 ${\bf Table} \ {\bf 1} \ \ {\bf Please} \ {\bf write} \ {\bf your} \ {\bf table} \ {\bf caption} \ {\bf here}$ 

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number	number	number
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tab:1

#### sec:conclusions

#### Acknowledgements

7 Conclusions

#### Replication of results

All MATLAB code is provided as supplementary material.

#### Conflict of interest

On behalf of all authors, the corresponding author states that there is no conflict of interest.

#### References

- mir2009approximate
- bendsoe1989optimal
  - bian2017large
- omolny2010topology
- oi2019accelerating
- lint2021structural

  - dunning2016level
- rari2019revisiting
- ferrari2020towards
  - gao2015topology
- gogu2015improving
- guest2004achieving
  - kang2020method
- rsch2008reanalysis
- rsch2010reanalysis
- lazarov2011filters
- ves1995generalized
- svanberg1987method
- orii2017structural
- g2014interpolation

- 1. Amir, O., Bendsøe, M.P., Sigmund, O.: Approximate reanalysis in topology optimization. International Journal for Numerical Methods in Engineering **78**(12), 1474–1491 (2009)
- 2. Bendsøe, M.P.: Optimal shape design as a material distribution problem. Structural optimization 1(4), 193–202 (1989)
- 3. Bian, X., Fang, Z.: Large-scale buckling-constrained topology optimization based on assembly-free finite element analysis. Advances in Mechanical Engineering 9(9), 1687814017715422 (2017). DOI 10.1177/1687814017715422
- 4. Bogomolny, M.: Topology optimization for free vibrations using combined approximations. International Journal for Numerical Methods in Engineering 82(5), 617–636 (2010)
- Choi, Y., Oxberry, G., White, D., Kirchdoerfer, T.: Accelerating design optimization using reduced order models. arXiv preprint arXiv:1909.11320 (2019)
- Dalklint, A., Wallin, M., Tortorelli, D.A.: Structural stability and artificial buckling modes in topology optimization. Structural and Multidisciplinary Optimization 64(4), 1751–1763 (2021)
- Dunning, P.D., Ovtchinnikov, E., Scott, J., Kim, H.A.: Level-set topology optimization with many linear buckling constraints using an efficient and robust eigensolver. International Journal for Numerical Methods in Engineering 107(12), 1029–1053 (2016)
- 8. Ferrari, F., Sigmund, O.: Revisiting topology optimization with buckling constraints. Structural and Multidisciplinary Optimization **59**(5), 1401–1415 (2019)
- 9. Ferrari, F., Sigmund, O.: Towards solving large-scale topology optimization problems with buckling constraints at the cost of linear analyses. Computer Methods in Applied Mechanics and Engineering 363, 112911 (2020)
- Gao, X., Ma, H.: Topology optimization of continuum structures under buckling constraints. Computers & Structures 157, 142–152 (2015)
- Gogu, C.: Improving the efficiency of large scale topology optimization through on-the-fly reduced order model construction. International Journal for Numerical Methods in Engineering 101(4), 281–304 (2015)
- Guest, J.K., Prévost, J.H., Belytschko, T.: Achieving minimum length scale in topology optimization using nodal design variables and projection functions. International journal for numerical methods in engineering 61(2), 238–254 (2004)
- 13. Kang, Z., He, J., Shi, L., Miao, Z.: A method using successive iteration of analysis and design for large-scale topology optimization considering eigenfrequencies. Computer Methods in Applied Mechanics and Engineering 362, 112847 (2020)
- 14. Kirsch, U.: Reanalysis of structures. Springer (2008)
- 15. Kirsch, U.: Reanalysis and sensitivity reanalysis by combined approximations. Structural and Multidisciplinary Optimization  ${\bf 40}(1)$ , 1–15 (2010)
- 16. Lazarov, B.S., Sigmund, O.: Filters in topology optimization based on helmholtz-type differential equations. International Journal for Numerical Methods in Engineering 86(6), 765–781 (2011)
- Neves, M., Rodrigues, H., Guedes, J.: Generalized topology design of structures with a buckling load criterion. Structural optimization 10(2), 71–78 (1995)
- 18. Svanberg, K.: The method of moving asymptotes—a new method for structural optimization. International journal for numerical methods in engineering 24(2), 359–373 (1987)
- Torii, A.J., De Faria, J.R.: Structural optimization considering smallest magnitude eigenvalues: a smooth approximation. Journal of the Brazilian Society of Mechanical Sciences and Engineering 39(5), 1745–1754 (2017)
- Wang, F., Lazarov, B.S., Sigmund, O., Jensen, J.S.: Interpolation scheme for fictitious domain techniques and topology optimization of finite strain elastic problems. Computer Methods in Applied Mechanics and Engineering 276, 453–472 (2014)