

NGSIM Lankershim Data Analysis (8:45 a.m. to 9:00 a.m.)

summary

report

prepared for

Federal Highway Administration

prepared by

Cambridge Systematics, Inc.

summary report

NGSIM Lankershim Data Analysis (8:45 a.m. to 9:00 a.m.)

prepared for

Federal Highway Administration

prepared by

Cambridge Systematics, Inc. 555 12th Street, Suite 1600 Oakland, California 94607

March 2006

Table of Contents

Introduction	1
Study Area Description	1
Vehicle Detection and Tracking	4
Data Analysis	5
Vehicle Type	7
Origin-Destination Distribution	7
Start-End Lane Distribution	8
Traffic Volume Analysis	9
Speed Analysis	12
Travel Time Analysis	14
Lane Change Analysis	15
Headway Analysis	18
Spacing Analysis	18
Gap Analysis	19

List of Tables

1.	Vehicle Type	7
2.	Origin-Destination Distribution	7
3.	Start-End Lane Distribution	8
4.	Traffic Volume at Intersection 1 (in Vehicles)	9
5.	Traffic Volume at Intersection 2 (in Vehicles)	10
6.	Traffic Volume at Intersection 3 (in Vehicles)	10
7.	Traffic Volume at Intersection 4 (in Vehicles)	11
8.	Average Speed in Section 2 (NB) (Feet Per Second)	12
9.	Average Speed in Section 2 (SB) (Feet Per Second)	12
10.	Average Speed in Section 3 (NB) (Feet Per Second)	13
11.	Average Speed in Section 3 (SB) (Feet Per Second)	13
12.	Average Speed in Section 4 (NB) (Feet Per Second)	13
13.	Average Speed (feet/sec.) in Section 4 (SB)	14
14.	Average Travel Time on Lankershim Blvd (NB) (in Seconds)	14
15.	Average Travel Time on Lankershim Blvd (SB) (in Seconds)	15
16.	Number of Lane Changes by O-D Pairs	17
17.	Average Lane Changes by O-D Pairs	17
18.	Average Headway on Lankershim Blvd (NB) (in Seconds)	18
19.	Average Headway on Lankershim Blvd (SB) (in Seconds)	18
20.	Average Spacing on Lankershim Blvd (NB) (in Feet)	19
21.	Average Spacing on Lankershim Blvd (SB) (in Feet)	19
22.	Average Lead and Lag Gaps on Lankershim Blvd (NB) (in Feet)	20
23.	Average Lead and Lag Gaps on Lankershim Blvd (SB) (in Feet)	20

List of Figures

1.	Study Area and Camera Coverage	2
2.	Study Area Schematic	3
3.	Vehicle Detection and Tracking Process	4
4.	Study Area Schematic with Various Identification Numbers	6
5.	Number of Lane Changes Per Vehicle	16

Introduction

This report summarizes a data collection and processing effort undertaken to provide a dataset of arterial vehicle trajectories data completed as part of the Federal Highway Administration's (FHWA) Next Generation Simulation (NGSIM) project, and provides a detailed analysis of a subset of the data. The data analyzed in this report represent vehicle trajectories on a segment of Lankershim Boulevard, located near the interchange with U.S. Highway 101 (Hollywood Freeway) in Los Angeles, California collected between 8:45 a.m. and 9:00 a.m. on June 16, 2005. Aggregate summaries of flow and speed of the vehicles, number of lane changes, headway and gap analysis, and an input-output analysis of flows are provided. The results are aggregated by time and intersection.

Study Area Description

Data presented in this report represent travel on Lankershim Boulevard, an arterial running primarily north-south in Los Angeles, California. The speed limit on the Lankershim Boulevard is 35 mph. These data were collected using video cameras mounted on a 36-story building, 10 Universal City Plaza, which is located adjacent to the U.S. Highway 101 and Lankershim Boulevard interchange in the Universal City neighborhood.

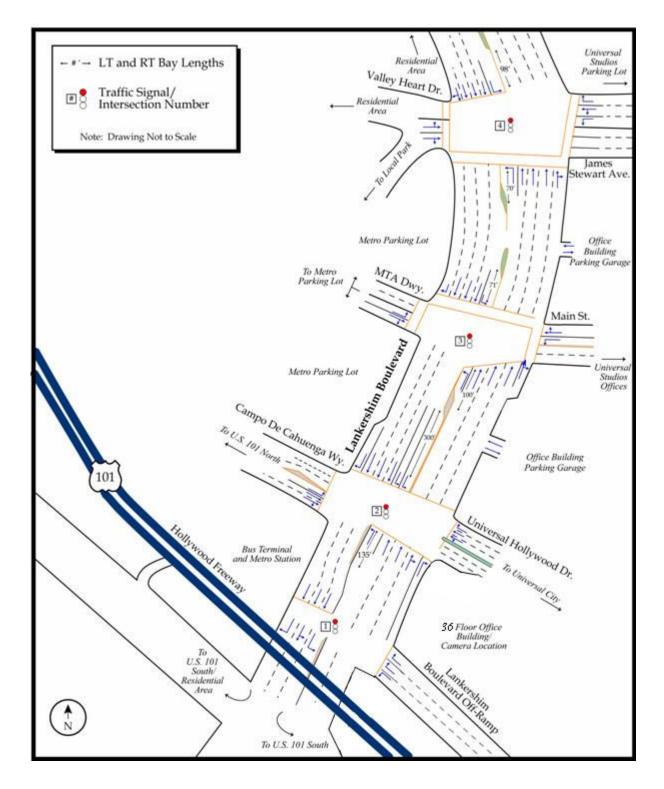
Figure 1 provides an aerial image of the location with the camera coverage. Figure 2 presents a schematic illustration of the location for the vehicle trajectory dataset. The site was approximately 1,600 feet in length, with four signalized intersections and three to four arterial through lanes in each direction through the section. Lane numbering is incremented from the left-most lane. Adjacent land use is also illustrated in Figure 2.

Video data were collected using five video cameras, cameras 1 through 5, with camera 1 recording the southernmost and camera 5 recording the northernmost section of the study area, as shown in Figure 1. Digital video images were collected over an approximate 9-hour period from 7:00 a.m. to 12:00 p.m. and from 3:00 p.m. to 7:00 p.m. on June 16, 2005. Complete vehicle trajectories were transcribed for 32 minutes from 8:28 a.m. to 9:00 a.m. at a resolution of 10 frames per second.

Figure 1. Study Area and Camera Coverage



Figure 2. Study Area Schematic

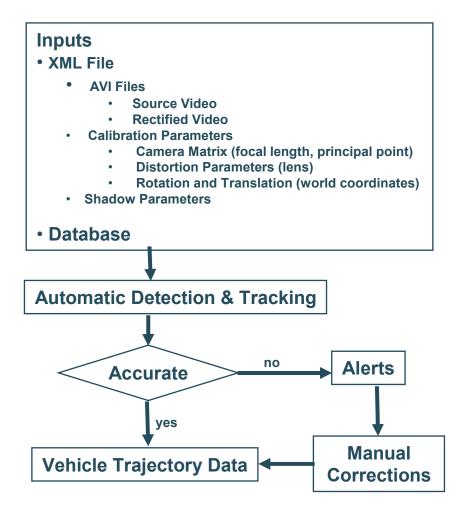


Vehicle Detection and Tracking

Vehicle trajectory data were transcribed from the video data using a customized software application, Next Generation Vehicle Interaction and Detection Environment for Operations (NG-VIDEO), developed for NGSIM. This program detects and tracks vehicles from video images and transcribes the trajectory data to a database.

The flow process for the vehicle transcription is shown in Figure 3. The software detects vehicles in a user-defined detection zone, and then tracks vehicles from the point of detection.

Figure 3. Vehicle Detection and Tracking Process



Tracking was performed for the data from 8:45 a.m. to 9:00 a.m. Immediately after 9:00 a.m., vehicle detection was stopped; however, to account for full vehicle trajectories, tracking continued to allow the vehicles which were already detected to be tracked completely to the end of the study area. Therefore, for the vehicle trajectory dataset of 8:45a.m. to 9:00 a.m., the actual tracking time is from 8:45:00 a.m. to 9:02:27 a.m.

A total of 32 minutes vehicle trajectories was processed from the video data collected on June 16, 2005, representing the period from 8:28 a.m. to 9:00 a.m. (consisting of primarily congested conditions). The data was divided into one 17-minute period and one 15-minute period for processing and analysis.

Subsequent sections of this report provide analysis of the transcribed data. This report provides data analysis for the period from 8:45 a.m. to 9:00 a.m. A separate report is available providing the same performance statistics for the preceding 8:28 a.m. to 8:45 a.m. period.

Data Analysis

Data analysis was performed for specific locations in the study area. Therefore, it is necessary to define those locations herein. Figure 4 shows the study area schematic with identification numbers for origins, destinations, intersections, sections, and lanes.

- Origin These were numbered from 101 through 111. There are 11 origins in the study area.
- **Destination** There are 10 destinations in the study area, numbered from 201 through 211. Origin 102 is a one-way off-ramp; hence, there is no associated destination number 202.
- **Intersection** These were numbered from 1 to 4, with intersection 1 at the southernmost, and intersection 4 at the northernmost section of the study area. Intersections 1, 2, 3, and 4 in this report correspond to signal numbers 87, 88, 89, and 90, respectively.
- **Section –** Lankershim Blvd was further divided into five sections between neighboring intersections.
- Lane Lane numbering was incremented from the left-most lane, except for locations where left-turn or right-turn bays exist. Left-turn bays were numbered starting from 11 and were incremented from the left-most left-turn bay. Right-turn bays were numbered starting from 31 and were incremented from the left-most right-turn bay. It was noted that there was a left-turn bay in the mid-block between intersections 3 and 4. To differentiate that left-turn bay with others, it was numbered 101.

Universal LT and RT Bay Lengths Resident Area Studios Parking Lot Valley Heart D Traffic Signal/ Intersection Number **₽**8 Residential Area Note: Drawing Not to Scale James Stewart Ave. Section 4 1 2 3 4 Metro Parking Lot To Metro Parking Lot Main St. Lankershim Boulevard Section 3 Metro Parking Lot Studios Offices Campo De Cahuenga W ⊐104 Office Building Parking Garage Section 2 al Hollywood Dr. **Bus Terminal** and Metro Station 36 Floor Office Building/ Camera Location

Figure 4. Study Area Schematic with Various Identification Numbers

To U.S. 101 South/ Residential Area

To U.S. 101 South

Vehicle Type

Vehicles are classified into three categories: 1) motorcycle, 2) automobile, and 3) truck and buses. The distribution of vehicle types is shown in Table 1.

Table 1. Vehicle Type

	Mot	orcycle	Auto	mobile	Truck a	and Buses	All		
Time Period	Vehicles	Percentage	Vehicles	Percentage	Vehicles	Percentage	Vehicles	Percentage	
8:45 a.m8:50 a.m.	0	0.0%	396	97.5%	10	2.5%	406	100.0%	
8:50 a.m8:55 a.m.	0	0.0%	433	98.0%	9	2.0%	442	100.0%	
8:55 a.m9:00 a.m.	1	0.3%	375	97.9%	7	1.8%	383	100.0%	
All	1	0.1%	1,204	97.8%	26	2.1%	1,231	100.0%	

Origin-Destination Distribution

There are 11 origins and 10 destinations in the study area, as illustrated in Figure 4. The distribution of vehicles from origins to destinations is provided in Table 2.

Table 2. Origin-Destination Distribution

					Desti	nation					
Origin	201	203	204	205	206	207	208	209	210	211	Sum
101	0	52	4	6	3	8	79	1	2	8	163
102	14	30	8	9	3	16	185	0	0	3	268
103	13	0	0	0	0	1	29	0	0	16	59
104	0	2	0	0	0	0	0	0	0	1	3
105	3	4	0	0	0	0	8	0	0	1	16
106	1	0	0	0	0	0	2	1	0	0	4
107	0	7	1	0	0	0	4	0	0	1	13
108	347	169	18	15	2	24	2	7	8	27	619
109	4	3	0	0	0	0	7	0	0	1	15
110	0	1	0	0	0	0	1	0	0	0	2
111	3	22	3	8	0	6	27	0	0	0	69
Sum	385	290	34	38	8	55	344	9	10	58	1,231

Start-End Lane Distribution

Start-end lane distribution is provided in Table 3. The start and end lane of a vehicle is the lane in which the vehicle was first and last tracked in the study area, respectively.

Table 3. Start-End Lane Distribution

1		Sum	64	66 (14	216	38	13	16	30	80	00	80	4	6	0	4) 26	228	202	153	_	_	%	1	1	44	17	80	2 1231
	1	3	0	0	0	1	0	0	4	0	1	Т	0	0	1	0	0	0	1	9	Ţ	0	0	1	0	0	0	0	0	32
	211	2	9	0	0	1	0	0	5	0	0	0	0	0	0	0	0	0	0	1	2	0	0	0	0	0	0	0	0	15
		1	2	0	0	1	0	0	^	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	11
	210	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	3	0	0	0	0	0	0	0	0	īC
	(1	1	\vdash	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	rc
	209	1	1	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	7	0	0	0	0	0	0	0	3
		4	4	13	0	46	13	0	0	9	0	0	2	1	0	0	4	1	0	0	0	0	0	0	1	0	12	0	0	103
	208	3	6	13	0	48	6	0	0	\sim	0	0	2	0	0	0	0	1	0	0	0	0	4	0	0	0	00	0	0	101
		2	13	5	0	37	1	0	0	5	0	0	2	1	0	0	0	0	0	0	0	0	2	0	0	0	3	0	0	69
		1	21	1	0	30	1	0	0	11	0	0	7	0	0	0	0	0	0	0	0	0	1	0	0	0	4	0	0	7
tion		3	2	9	0	13	3	0	0	\vdash	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0	0	31
Destination	207	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7		1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	24	0	0	0	0	0	0	0	0	0	0	0	24
	206	1	1	2	0	1	2	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	œ
	202	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	3	0	0	^
		1	1	5	0	9	1	0	0	0	0	0	0	0	0	0	0	0	12	1	0	0	0	0	0	0	Ŋ	0	0	31
	204	1	0	4	0	9	2	0	0	0	0	0	0	0	1	0	0	0	14	4	0	0	0	0	0	0	3	0	0	34
		3	3	36	0	17	4	0	0	0	0	0	0	0	1	0	0	0	3	3	0	0	0	0	0	1	0	0	0	89
	203	2	0	12	0	9	2	0	0	0	0	2	0	0	Ŋ	0	0	0	73	17	0	0	0	1	0	0	0	10	Ŋ	135
		1	0	\vdash	0	\vdash	0	0	0	0	2	2	0	0	\vdash	0	0	0	47	22	2	0	0	2	0	0	0	^	0	87
		3	0	0	0	0	0	3	0	0	0	⊣	0	0	0	0	0	0	0	28	112	0	0	3	0	0	0	0	_	
	201	2	0	0	3	0	0	5	0	0	0	1	0	1	0	0	0	0	5	98	11	0	0	0	0	0	0	0	7	114
		1	0	0	11	0	0	5	0	0	0	\vdash	0	0	0	0	0	0	29	32	9	0	0	_	0	0	0	0		123
		Lane	1	7	1	7	8		7	8			7	1	1	7	8		1	7	3			7		7	1	7	3	Sum
			101		102			103			104	105		106	107	Origin		108					109		110		111			

Cambridge Systematics, Inc.

Traffic Volume Analysis

The tables in this section provide traffic volume for each intersection. Intersections are numbered as 1, 2, 3, and 4, as shown in Figure 4. Traffic volume is grouped by moving direction [i.e., north-bound (NB), south-bound (SB), east-bound (EB), and west-bound (WB)], by movement [i.e., through (TH), left-turn (LT), and right-turn (RT)], by lane, and by time period.

Table 4. Traffic Volume at Intersection 1 (in Vehicles)

	N	IB		SB			_			
	T	Ή		TH		LT	RT			
Time Period	1	2	1	2	3	1	2	3	Sum	
8:45 a.m8:50 a.m.	19	36	29	34	38	6	57	14	233	
8:50 a.m8:55 a.m.	17	30	40	41	62	3	80	18	291	
8:55 a.m9:00 a.m.	28	25	38	32	46	4	78	6	257	
9:00 a.m9:02:27 a.m.	0	0	4	5	2	1	4	0	16	
Sum	64	91	111	112	148	14	219	38	797	

Table 5. Traffic Volume at Intersection 2 (in Vehicles)

		Sum	311	411	354	37	1,113
	RT	3	∞	13	∞	1	30
B	Ŧ	2	3	^	Ŋ	0	15
WB	TH	٧	0	0	0	0	0
	LT	1	3	4	4	2	13
	RT	V	\Box	1	1	0	8
3	Ŧ	3	П	2	1	\vdash	rv
EB	Ï	2	Ŋ	^	4	7	17
	LT	1	12	21	6	1	43
	RT	31	11	12	∞	0	31
		8	41	29	41	\vdash	142
3	TH	2	35	42	32	7	111
SB		1	30	37	32	3	102
	ī	12	22	33	43	6	107
	LT	11	18	24	31	9	26
	RT	4	18	37	21	2	78
		*	T	7	0	0	8
В	Ξ	3	41	39	34	3	117
NB	TH	2	31	39	40	3	113
		1	3 27	30	35	2	10 94 113 117
	LT	11	3	2	ſΩ	0	10
		Time Period	8:45 a.m8:50 a.m.	8:50 a.m8:55 a.m.	8:55 a.m9:00 a.m.	9:00 a.m9:02:27 a.m.	Sum

^{* &}gt; and < symbols are associated with shared lanes.

Table 6. Traffic Volume at Intersection 3 (in Vehicles)

		Sum	287	5 0 2 325	271	23	902
	RT	٧	5	2	\vdash	0	∞
WB	TH	2	0	0	0	0	0
	ΓT	1	0	ſΩ	8	0	∞
	RT	٧	0	0	1	0	1
EB	TH	2	0	0	0	0	0
	ΓT	1	0	1 0 0	0	0	1
	RT	4	rV	\vdash	7	0	∞
		3	64	29	44	\vdash	176
SB	TH	2	49	7 84 54 67	44	\vdash	148
		1	82	84	82	7	253
	ΓT	11	3	^	rV	0	15
	RT	٧	7	6	Ŋ	7	23
		31	2	35 0	0	0	7
NB	TH	3	23	35	27	9	91
Z	Ι	2	28	30	24	Ŋ	87
		1	19	30	28	Ŋ	82
	ΓT	11	0	0	2	0	7
		Time Period	8:45 a.m8:50 a.m.	8:50 a.m8:55 a.m.	8:55 a.m9:00 a.m.	9:00 a.m9:02:27 a.m.	Sum

Table 7. Traffic Volume at Intersection 4 (in Vehicles)

I	. 1	Sum	304	0 3 357	312	33	1,006
	RT	B	1	3	0	0	4
WB	TH	٧	0	0	0	0	0
S	T	2	0	0	0	0	0
	Г	П	3	7	4	0	6
	RT	٧	3	1	3	0	^
8	Н	7	0	0	0	0	0
EB	Τ	٧	0	0	0	0	0
	ΓT	1	1 0 0 3 3 0 0 1	2	4	0	^1
	RT	٧	4	0	3	0	^1
		4	6	6	_	0	25
~	I	B	22	26	38	0	152
SB	TI	7	63	63	20	0	176
		1	75	77	79	0	231
	ΓT	11	4 7 75 63 55 9 4	11 11	9	0	24
	RT	٧	4	111	13	3	31
		4	18	31	25	10	84
8	E	8	23	41	30	∞	102
NB	TH	2 3	18	25 22	23	9	69 92
		П	19 18	25	26 23	9	92
	$_{ m LT}$	11	1	0	1	0	7
		Time Period	8:45 a.m8:50 a.m.	8:50 a.m8:55 a.m.	8:55 a.m9:00 a.m.	9:00 a.m9:02:27 a.m.	Sum

Speed Analysis

Speed analysis was performed at the midpoints of each section on Lankershim Blvd, which are between two neighboring intersections as shown in Figure 4. The following tables provide average speed for both moving directions (NB and SB) and for each lane.

Table 8. Average Speed in Section 2 (NB) (Feet Per Second)

Movement		Tl	Н		TH	+RT	R	_	
Lane	1		2		3	3			
	Count	Speed	Count	Speed	Count	Speed	Count	Speed	Average
8:45 a.m8:50 a.m.	25	22.04	34	22.14	44	19.38	20	29.76	22.37
8:50 a.m8:55 a.m.	30	24.53	40	21.41	41	20.16	35	25.81	22.75
8:55 a.m9:00 a.m.	34	23.10	41	20.97	38	22.01	20	30.11	23.19
All	89	23.28	115	21.47	123	20.45	75	28.01	22.78

Table 9. Average Speed in Section 2 (SB) (Feet Per Second)

Movement		ТН									
Lane	1	L		2	3						
	Count	Speed	Count	Speed	Count	Speed	Average				
8:45 a.m8:50 a.m.	31	39.40	33	40.31	40	41.27	40.41				
8:50 a.m8:55 a.m.	41	34.30	45	32.90	59	35.27	34.26				
8:55 a.m9:00 a.m.	37	36.00	32	40.33	44	39.48	38.58				
All	109	36.33	110	37.28	143	38.24	37.37				

Table 10. Average Speed in Section 3 (NB) (Feet Per Second)

Movement			T	Н			
Lane	1	L		2	3	3	
	Count	Speed	Count	Speed	Count	Speed	Average
8:45 a.m8:50 a.m.	27	48.65	37	49.99	44	42.08	46.43
8:50 a.m8:55 a.m.	33	50.81	46	48.78	56	40.04	45.65
8:55 a.m9:00 a.m.	35	48.41	41	48.62	42	43.83	46.85
All	95	49.31	124	49.09	142	41.79	46.28

Table 11. Average Speed in Section 3 (SB) (Feet Per Second)

Movement		I	Т				Т	Ή			
Lane	1	1	1	2	1	l		2	3	3	
	Count	Speed	Average								
8:45 a.m8:50 a.m.	18	33.32	39	30.46	29	39.31	43	43.58	56	34.77	36.48
8:50 a.m8:55 a.m.	27	29.70	42	28.49	26	39.80	50	39.78	68	36.54	35.24
8:55 a.m9:00 a.m.	25	25.17	43	27.52	28	40.35	40	44.44	45	41.52	36.40
All	70	29.01	124	28.77	83	39.82	133	42.41	169	37.28	36.00

Table 12. Average Speed in Section 4 (NB) (Feet Per Second)

Movement			T	H			TH		
Lane	1	1		2	3	3	4	1	
	Count	Speed	Count	Speed	Count	Speed	Count	Speed	Average
8:45 a.m8:50 a.m.	18	39.45	22	43.61	28	40.69	18	39.39	40.90
8:50 a.m8:55 a.m.	30	48.00	21	45.08	39	40.47	41	36.28	41.62
8:55 a.m9:00 a.m.	27	42.34	23	36.78	30	39.47	36	36.72	38.75
All	75	43.91	66	41.70	97	40.22	95	37.04	40.44

Table 13. Average Speed (feet/sec.) in Section 4 (SB)

Movement			Tl	H			R		
Lane	1	l		2	3	3		4	
	Count	Speed	Count	Speed	Count	Speed	Count	Speed	Average
8:45 a.m8:50 a.m.	86	34.48	53	40.19	64	43.45	6	43.07	38.92
8:50 a.m8:55 a.m.	86	35.97	55	41.99	65	43.48	4	41.91	39.98
8:55 a.m9:00 a.m.	87	38.13	45	41.28	43	46.84	7	40.60	41.06
All	259	36.20	153	41.16	172	44.31	17	41.78	39.94

Travel Time Analysis

Average travel times for vehicles traveling NB on Lankershim Blvd from the southernmost to the northernmost section of the study area (average travel length is about 1,586 feet) are provided in Table 14. Also, average travel times for vehicles traveling SB on Lankershim Blvd from the northernmost to the southernmost section of the study area (average travel length is about 1,560 feet) are provided in Table 15.

Table 14. Average Travel Time on Lankershim Blvd (NB) (in Seconds)

From Lane		1	L			:	2		
To Lane	1	2	3	4	1	2	3	4	Average
8:45 a.m8:50 a.m.	104.7	82.2	76.6	_	89.5	106.5	114.7	100.3	96.4
8:50 a.m8:55 a.m.	114.2	84.7	79.4	93.2	_	100.2	124.4	104.6	100.1
8:55 a.m9:00 a.m.	83.7	75.2	84.2	102.6	_	113.2	122.2	122.7	100.5
Average	100.9	80.7	80.1	97.9	89.5	106.6	120.4	109.2	98.2

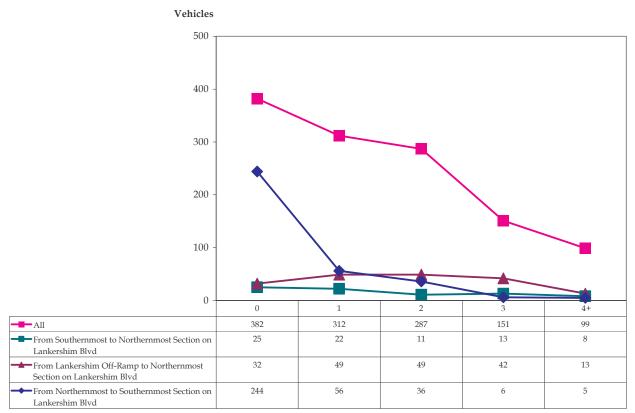
Table 15. Average Travel Time on Lankershim Blvd (SB) (in Seconds)

	1			2			3			4		
1	2	3	1	2	3	1	2	3	1	2	3	Average
67.9	94.5	_	59.1	59.3	55.8	_	36.2	68.4	_	_	_	63.0
76.3	76.1	-	69.2	65.0	59.2	59.5	90.7	67.3	-	_	-	70.4
65.5	84.7	-	53.8	50.8	48.9	70.6	31.6	47.4	_	-	-	56.7
69.9	85.1	-	60.7	58.4	54.6	65.1	52.8	61.0	-	-	-	63.4
	76.3 65.5	67.9 94.5 76.3 76.1 65.5 84.7	67.9 94.5 – 76.3 76.1 – 65.5 84.7 –	67.9 94.5 - 59.1 76.3 76.1 - 69.2 65.5 84.7 - 53.8	67.9 94.5 - 59.1 59.3 76.3 76.1 - 69.2 65.0 65.5 84.7 - 53.8 50.8	67.9 94.5 - 59.1 59.3 55.8 76.3 76.1 - 69.2 65.0 59.2 65.5 84.7 - 53.8 50.8 48.9	67.9 94.5 - 59.1 59.3 55.8 - 76.3 76.1 - 69.2 65.0 59.2 59.5 65.5 84.7 - 53.8 50.8 48.9 70.6	67.9 94.5 - 59.1 59.3 55.8 - 36.2 76.3 76.1 - 69.2 65.0 59.2 59.5 90.7 65.5 84.7 - 53.8 50.8 48.9 70.6 31.6	67.9 94.5 - 59.1 59.3 55.8 - 36.2 68.4 76.3 76.1 - 69.2 65.0 59.2 59.5 90.7 67.3 65.5 84.7 - 53.8 50.8 48.9 70.6 31.6 47.4	67.9 94.5 - 59.1 59.3 55.8 - 36.2 68.4 - 76.3 76.1 - 69.2 65.0 59.2 59.5 90.7 67.3 - 65.5 84.7 - 53.8 50.8 48.9 70.6 31.6 47.4 -	67.9 94.5 - 59.1 59.3 55.8 - 36.2 68.4 76.3 76.1 - 69.2 65.0 59.2 59.5 90.7 67.3 65.5 84.7 - 53.8 50.8 48.9 70.6 31.6 47.4	67.9 94.5 - 59.1 59.3 55.8 - 36.2 68.4 76.3 76.1 - 69.2 65.0 59.2 59.5 90.7 67.3 65.5 84.7 - 53.8 50.8 48.9 70.6 31.6 47.4

Lane Change Analysis

An analysis of lane changes occurring in the study area is provided in this section. It should be noted that vehicles making either left turns or right turns to the closest receiving lane were not counted as lane changes. The number of lane changes per vehicle for all vehicles in the study area is shown in Figure 5. The number of lane changes by each origin-destination (O-D) pair is provided in Table 16. Table 17 provides the average lane changes by each O-D pair, which was calculated by dividing the number of lane changes by the number of vehicles for that O-D pair.

Figure 5. Number of Lane Changes Per Vehicle



Number of Lane Changes Per Vehicle

Table 16. Number of Lane Changes by O-D Pairs

					Desti	nation					
Origin	201	203	204	205	206	207	208	209	210	211	Sum
101	0	111	4	20	10	31	119	1	5	15	316
102	3	56	8	31	11	53	339	0	0	4	505
103	15	0	0	0	0	2	71	0	0	14	102
104	0	0	0	0	0	0	0	0	0	5	5
105	7	7	0	0	0	0	18	0	0	3	35
106	5	0	0	0	0	0	2	4	0	0	11
107	0	17	3	0	0	0	0	0	0	3	23
108	167	301	38	18	2	3	0	1	11	51	592
109	6	14	0	0	0	0	10	0	0	2	32
110	0	4	0	0	0	0	3	0	0	0	7
111	4	11	6	33	0	28	79	0	0	0	161
Sum	207	521	59	102	23	117	641	6	16	97	1,789

Table 17. Average Lane Changes by O-D Pairs

					Desti	nation					
Origin	201	203	204	205	206	207	208	209	210	211	Avg.
101	0.0	2.1	1.0	3.3	3.3	3.9	1.5	1.0	2.5	1.9	1.9
102	0.2	1.9	1.0	3.4	3.7	3.3	1.8	0.0	0.0	1.3	1.9
103	1.2	0.0	0.0	0.0	0.0	2.0	2.4	0.0	0.0	0.9	1.7
104	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.0	1.7
105	2.3	1.8	0.0	0.0	0.0	0.0	2.3	0.0	0.0	3.0	2.2
106	5.0	0.0	0.0	0.0	0.0	0.0	1.0	4.0	0.0	0.0	2.8
107	0.0	2.4	3.0	0.0	0.0	0.0	0.0	0.0	0.0	3.0	1.8
108	0.5	1.8	2.1	1.2	1.0	0.1	0.0	0.1	1.4	1.9	1.0
109	1.5	4.7	0.0	0.0	0.0	0.0	1.4	0.0	0.0	2.0	2.1
110	0.0	4.0	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0	3.5
111	1.3	0.5	2.0	4.1	0.0	4.7	2.9	0.0	0.0	0.0	2.3
Avg.	0.5	1.8	1.7	2.7	2.9	2.1	1.9	0.7	1.6	1.7	1.5

Headway Analysis

Tables 18 and 19 provide average headway of vehicles traveling on Lankershim Blvd NB and SB, respectively. In addition to average headway for each time period, headways were also analyzed based on vehicle speed (i.e., less than 5 mph, 5 to 20 mph, and more than 20 mph).

Table 18. Average Headway on Lankershim Blvd (NB) (in Seconds)

_		_ Average		
Time Period	<5	5-20	>20	Headway
8:45 a.m8:50 a.m.	54.19	4.38	3.36	18.40
8:50 a.m8:55 a.m.	59.97	4.17	3.39	17.10
8:55 a.m9:00 a.m.	57.78	4.00	3.14	19.66
Average Headway	57.36	4.17	3.28	18.42

Table 19. Average Headway on Lankershim Blvd (SB) (in Seconds)

		Speed (mph)		_ Average	
Time Period	<5	5-20	>20	Headway	
8:45 a.m8:50 a.m.	102.03	4.55	3.31	10.10	
8:50 a.m8:55 a.m.	66.33	4.90	3.00	12.27	
8:55 a.m9:00 a.m.	51.89	5.71	3.49	8.04	
Average Headway	69.51	5.09	3.26	10.47	

Spacing Analysis

Spacing, or distance headway, was analyzed for each time period and for each speed group. Tables 20 and 21 provide average spacing of vehicles traveling on Lankershim Blvd NB and SB, respectively.

Table 20. Average Spacing on Lankershim Blvd (NB) (in Feet)

_		Average		
Time Period	<5	5-20	>20	Spacing
8:45 a.m8:50 a.m.	29.09	77.99	137.00	66.13
8:50 a.m8:55 a.m.	32.09	74.94	138.45	70.15
8:55 a.m9:00 a.m.	31.90	71.28	130.19	63.02
Average Spacing	31.05	74.70	134.53	66.36

Table 21. Average Spacing on Lankershim Blvd (SB) (in Feet)

	_ Average		
<5	5-20	>20	Spacing
26.87	84.17	138.43	111.37
28.06	95.00	121.90	89.44
27.83	88.01	146.51	110.09
27.81	90.72	134.98	101.83
	26.87 28.06 27.83	26.87 84.17 28.06 95.00 27.83 88.01	<5 5-20 >20 26.87 84.17 138.43 28.06 95.00 121.90 27.83 88.01 146.51

Gap Analysis

Tables 22 and 23 present the accepted lead and lag gaps by vehicles during lane-changing on Lankershim Blvd NB and SB, respectively. In addition to average gaps for each time period, lead and lag gaps were also analyzed based on vehicle speed (i.e., less than 5 mph, 5 to 20 mph, and more than 20 mph).

Table 22. Average Lead and Lag Gaps on Lankershim Blvd (NB) (in Feet)

		Lead Gap (Feet)				Lag Gap (Feet)			
Time Period	<5 mph	5-20 mph	>20 mph	Average	<5 mph	5-20 mph	>20 mph	Average	
8:45 a.m8:50 a.m.	15.50	44.11	57.39	51.63	39.00	60.22	77.72	70.71	
8:50 a.m8:55 a.m.	56.00	48.00	71.88	65.95	78.00	63.00	75.95	72.93	
8:55 a.m9:00 a.m.	20.00	37.47	70.30	59.81	81.00	58.59	70.33	67.07	
Average	24.60	43.16	67.51	59.33	49.60	60.28	74.13	69.47	

Table 23. Average Lead and Lag Gaps on Lankershim Blvd (SB) (in Feet)

	Lead Gap (Feet)				Lag Gap (Feet)			
Time Period	<5 mph	5-20 mph	>20 mph	Average	<5 mph	5-20 mph	>20 mph	Average
8:45 a.m. – 8:50 a.m.	-	38.56	57.90	55.00	-	62.44	59.65	60.07
8:50 a.m. – 8:55 a.m.	-	46.00	59.02	58.46	-	45.00	56.30	55.80
8:55 a.m. – 9:00 a.m.	-	68.50	52.34	54.30	-	69.25	44.62	47.61
Average	-	47.53	57.99	56.87	-	61.93	56.06	56.69