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1.	STRUCTURAL DESIGN IS AND CONSTRUCTION MUST BE IN ACCORDANCE WITH THE FOLLOWING: UFC 1-200-01 GENERAL BUILDING REQUIREMENTS, 8 OCTOBER 2019 UFC 3-301-01 STRUCTURAL ENGINEERING, 1 OCTOBER 2019 IBC 2018 2018 INTERNATIONAL BUILDING CODE (AS MODIFIED BY UFC 3-301-01) ASCE 7-16 MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES ACI 301-10 SPECIFICATIONS FOR STRUCTURAL CONCRETE ACI 315-99 DETAILS AND DETAILING OF CONCRETE REINFORCEMENT ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AISC 303-16 CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (15TH EDITION MANUAL, LRFD) AISI S100-12 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS AWS D1.1 AMERICAN WELDING SOCIETY, STRUCTURAL WELDING CODE - STEEL SJI 100-2015 STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS, LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS, 44TH EDITION SDI MANUAL STEEL DECK INSTITUTE, DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS	B.	FOUNDATIONS AND SLABS ON-GROUND (CONT.)
2.	DESIGN LOADS: ROOF LOADS SUPERIMPOSED DEAD LOAD: AS CALCULATED LIVE LOAD: 20 PSF WIND LOADS DESIGN WIND PRESSURE, $p = qG Cp$ RISK CATEGORY II, EXPOSURE C ULTIMATE DESIGN WIND SPEED (3-SEC. GUST): 107 MPH NOMINAL DESIGN WIND SPEED = 83 MPH INTERNAL PRESSURE COEFFICIENT: +/- 0.18 EARTHQUAKE LOADS ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE PER ASCE 7, 12.8 SEISMIC BASE SHEAR, $V = Sds \times W / (R/I)$ W = DEAD LOAD RESPONSE MODIFICATION FACTOR R=3 STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE SEISMIC IMPORTANCE FACTOR, I = 1.0 RISK CATEGORY: II MAPPED SPECTRAL RESPONSE ACCELERATIONS: $Ss = 0.244g$, $S1 = 0.093g$ SPECTRAL RESPONSE ACCELERATIONS: $Sds = 0.26g$, $Sd1 = 0.15g$ SITE CLASS: D SEISMIC DESIGN CATEGORY: C SNOW LOADS - GROUND SNOW LOAD = 5 PSF FUTURE LOADS - NONE	C.	CONCRETE AND REINFORCING STEEL 1. CONCRETE FOR BUILDING AND SITE STRUCTURES MUST BE NORMAL WEIGHT (W=145 PCF) WITH 3500 PSI 28-DAY COMPRESSIVE STRENGTH (f_c). 2. CONCRETE EXPOSED TO WEATHER MUST HAVE A TOTAL AIR CONTENT BETWEEN 4.5% AND 7.5%. 3. FURNISH MATERIALS, MIX, PLACE, FINISH, AND CURE ALL CONCRETE IN ACCORDANCE WITH ACI 301 AND ACI 318. 4. CONCRETE REINFORCING BAR COVER MUST BE AS INDICATED IN TABLE "CAST IN PLACE CONCRETE COVER FOR REINFORCING BARS". 5. NO PIPING OR ELECTRICAL CONDUITS ARE ALLOWED TO BE EMBEDDED HORIZONTALLY WITHIN THE THICKNESS OF CONCRETE SLABS. 6. CHAMFER ALL EXPOSED EDGES, UNLESS INDICATED OTHERWISE, WITH A 1/2" RADIUS/EDGE. 7. BARS FOR CAST IN PLACE CONCRETE MUST BE PER ASTM A 615, GRADE 60. 8. DETAILING OF REINFORCING MUST BE IN ACCORDANCE WITH ACI 318 AND ACI 315. 9. SECURE ALL REBARS IN THE CORRECT POSITION PRIOR TO PLACING CONCRETE. TOLERANCES FOR PLACEMENT OF REBARS MUST BE IN ACCORDANCE WITH ACI 301. 10. REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES, AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.
3.	THESE GENERAL NOTES APPLY TO ALL STRUCTURAL WORK EXCEPT WHERE THEY CONFLICT WITH DETAILS OR NOTES SPECIFICALLY SHOWN.	D.	STRUCTURAL STEEL 1. DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL MUST BE IN ACCORDANCE WITH AISC 360, SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (LRFD DESIGN) AND THE AISC AND THE AISC "STEEL CONSTRUCTION MANUAL", 15TH EDITION. 2. STEEL GRADES: A. ANGLES AND MISCELLANEOUS PLATES - ASTM A36 ($F_y = 36$ KSI) B. STRUCTURAL TUBING (SQ., RECT., ROUND) - ASTM A500, GRADE C; OR ASTM A1085 C. CHANNELS - ASTM A36 ($F_y = 36$ KSI), OR ASTM A572, GRADE 50 ($F_y = 50$ KSI) D. W SHAPES - ASTM A992, ($F_y = 50$ KSI) 3. ALL BOLTS MUST BE 3/4 INCH DIAMETER, HIGH STRENGTH, ASTM F3125 (GRADE F1852 (TWIST-OFF)), TYPE 1, IN FULLY TENSIONED BEARING CONNECTIONS. UNLESS INDICATED OR REQUIRED OTHERWISE. BOLT TIGHTNESS MUST BE VERIFIED BY THE USE OF APPROVED TWIST OFF BOLTS. 4. ALL WELDING MUST BE IN ACCORDANCE WITH ANSI/AWS D1.1 STRUCTURAL WELDING CODE. WELDING ELECTRODES MUST BE E70 SERIES. 5. EXCEPT AS OTHERWISE INDICATED ON DRAWINGS, ALL CONNECTIONS MUST BE BOLTED FRAMED BEAM CONNECTIONS (DOUBLE ANGLE) AS GIVEN IN TABLES 10-1 AND 10-2 OF THE AISC STEEL CONSTRUCTION MANUAL, OR SINGLE-PLATE SHEAR CONNECTIONS (AISC MANUAL, TABLES 10-10). AT THE CONTRACTOR'S OPTION, CONNECTIONS MUST BE SIZED AND DESIGNED BY THE CONTRACTOR TO ACCOMMODATE, AT A MINIMUM, 125% OF THE UNFACTORED BEAM END REACTIONS INDICATED IN KILOPOUNDS (KIPS, k) ON THE FRAMING PLANS. REGARDLESS OF PLAN DESIGNATION, NO CONNECTION MAY BE DESIGNED FOR LESS THAN 10 KIPS. 6. MINIMUM WELD SIZE MUST BE 3/16" FILLET WELDS ALL AROUND, UNLESS INDICATED OTHERWISE. 7. SHOP CONNECTIONS MUST BE WELDED AND FIELD CONNECTIONS MUST BE WELDED OR BOLTED AS INDICATED OR APPROVED ON SHOP DRAWINGS. 8. NO SHOP SPlice OR OTHER CONNECTION WILL BE PERMITTED UNLESS SHOWN ON APPROVED SHOP DRAWINGS. 9. NATURAL CAMBER IN BEAMS MUST BE INSTALLED CROWN UP. 10. THE ERECTOR MUST PROVIDE ALL TEMPORARY SHORING AND BRACING REQUIRED FOR STABILITY UNTIL STRUCTURES ARE COMPLETE.
4.	RESOLVE CONFLICTS ON DRAWINGS AND SPECIFICATIONS WITH THE CONTRACTING OFFICER BEFORE PROCEEDING WITH CONSTRUCTION.	E.	COLD-FORMED STEEL MEMBERS 1. ALL LIGHT GAUGE STEEL FRAMING AND SUBFRAMING SUPPORT SYSTEMS (SEE SPECIFICATIONS FOR EXTENT) MUST BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AND THE COLD FORMED STEEL DESIGN MANUAL. 2. CALCULATIONS AND SHOP DRAWINGS MUST BE PRODUCED, SEALED, AND SIGNED BY A LICENSED PROFESSIONAL ENGINEER. INCLUDE DESIGN OF CONNECTIONS TO PRIMARY SUPPORTING STEEL STRUCTURE. 3. INDICATED LIGHT GAUGE STEEL FRAMING MEMBERS MUST BE HOT-DIPPED GALVANIZED, G-60 COATING WEIGHT, AND MUST CONFORM TO ASTM DESIGNATION C 955 AND A 653. MINIMUM YIELD POINT, UNLESS INDICATED OTHERWISE, MUST BE 50 KSI FOR 16 GA. AND HEAVIER AND 33 KSI FOR 18 GA. AND LIGHTER. 4. FRAMING MEMBERS MUST HAVE THE MINIMUM STRUCTURAL PROPERTIES INDICATED ON THE STRUCTURAL AND/OR ARCHITECTURAL DRAWINGS AND SPECIFICATIONS SECTION 05 40 00. MAXIMUM SPACING OF MEMBERS MUST BE 16". MINIMUM REQUIREMENTS FOR PERIMETER WALLS ARE 18 GAUGE C-SHAPED STUDS, 2.5" AND 6" DEEP X 1.625" WIDE WITH 1/2" WIDE RETURN LIPS. 5. MINIMUM REQUIREMENTS FOR JAMBS ON PERIMETER WALLS ARE (2) 18 GAUGE C-SHAPED 6" DEEP X 1.625" WIDE WITH 1/2" WIDE RETURN LIPS. THE JAMBS MUST RUN THE FULL HEIGHT OF THE WALL. 6. MINIMUM REQUIREMENTS FOR HEADERS ON PERIMETER WALLS ARE BOXED HEADERS, (2) 18 GAUGE C-SHAPED
5.	ELEVATIONS SHOWN ON FOUNDATION PLANS ARE TO TOPS OF FOOTINGS.		
6.	SLABS ON GROUND MUST BE PLACED OVER A 15 MIL MINIMUM THICKNESS PUNCTURE RESISTANT VAPOR BARRIER (SEE SPECIFICATIONS), ON A 6 INCH MINIMUM THICK CAPILLARY WATER BARRIER ON STRUCTURAL FILL (SEE CIVIL FOR REQUIREMENTS). TURN THE VAPOR BARRIER UP AT INTERSECTING VERTICAL SURFACES AND UTILITY PENETRATIONS AND SEAL WITH VAPOR BARRIER MANUFACTURER'S STANDARD, COMPATIBLE TAPE. SEAL ALL JOINTS WITH TAPE TO PROVIDE A CONTINUOUS UNBROKEN VAPOR BARRIER BETWEEN THE SUBGRADE AND THE SLAB ABOVE.		
7.	SAWN JOINTS IN CONCRETE SLABS MUST BE MADE THE SAME DAY AS PLACEMENT, AS SOON AS POSSIBLE WITHOUT DAMAGING THE CONCRETE, AND IN NO CASE LATER THAN 12 HOURS AFTER CONCRETE PLACEMENT.		
8.	COORDINATE EXACT LOCATIONS OF SLAB CONTROL AND CONSTRUCTION JOINTS WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH FINISHES.		
9.	BACKFILL FOR FOUNDATION AND RETAINING WALLS MUST BE A FREE-DRAINING SATISFACTORY SOIL MATERIAL. BACKFILL MUST BE COMPACTED SUFFICIENTLY TO PREVENT SUBSIDENCE OF SURFACE ADJACENT TO WALL.		
10.	ALL UTILITIES ENTERING THE BUILDING MUST BE ROUTED BENEATH FOOTINGS. SEE DETAIL B6/CU-503.		
11.	REFER TO CIVIL DRAWINGS FOR UTILITY STRUCTURES, PAVEMENT, WALKS, AND OTHER STRUCTURES OUTSIDE THE BUILDING LINE NOT SHOWN ON STRUCTURAL DRAWINGS THAT REQUIRE REINFORCED CONCRETE WORK.		
12.	THE CONTRACTOR MUST VERIFY ALL CONDITIONS AND DIMENSIONS OF EXISTING CONSTRUCTION PRIOR TO FABRICATION OF STRUCTURAL AND REINFORCING STEEL ITEMS ATTACHED TO OR BUILT ADJACENT TO EXISTING CONSTRUCTION. REPORT SIGNIFICANT DEVIATIONS TO THE ENGINEER AND ARCHITECT OF RECORD PRIOR TO FABRICATION OR INSTALLATION OF THE NEW CONSTRUCTION.		
13.	LINEWORK SHOWN IN THESE STRUCTURAL DRAWINGS THAT IS SCREENED (SUBDUED, NOT BOLD) INDICATES EXISTING CONSTRUCTION. SOLID (BOLD) LINEWORK INDICATES NEW CONSTRUCTION.		
14.	ALL HOLES CUT THROUGH EXISTING CONCRETE SLABS, WALLS, AND WALL PANELS MUST BE CAREFULLY AND CLEANLY CUT, FULL-DEPTH, SO AS TO PREVENT SPALLING. USE DIAMOND BLADED SAWS AND CORING BITS. DO NOT OVERCUT CORNERS OF RECTANGULAR SAWN OPENINGS. HOLES MUST BE CUT ONLY WITH THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD, AND MUST BE NO LARGER THAN THE ABSOLUTE MINIMUM SIZE NECESSARY.		

1.	FOUNDATIONS: CONVENTIONAL SHALLOW STRIP FOUNDATIONS. DESIGN IS SIMILAR TO EXISTING FOUNDATIONS. DESIGN OF FOUNDATIONS IS BASED ON RECOMMENDATIONS PROVIDED IN A REPORT ENTITLED "20C19028.00 ARTC BLDG 640, ANNISTON, AL_ GEOTECH REPORT - 091820", BY SCHNABEL ENGINEERING, LLC, COLUMBIA, SC., PHONE: 803-796-6240	9.	DEFLECTION OF FRAMING (STUD WALLS) MUST NOT EXCEED L/240 UNDER LOAD FROM INDICATED WIND SPEED.
2.	NET ALLOWABLE BEARING CAPACITY = 150 PSF.	8.	SUBMIT SHOP DRAWINGS AND CALCULATIONS STAMPED BY A LICENSED PROFESSIONAL ENGINEER FOR REVIEW AND APPROVAL OF COLD-FORMED STEEL MEMBERS AND ASSEMBLIES PRIOR TO FABRICATION. INCLUDE DESIGN OF CONNECTIONS TO PRIMARY SUPPORTING STEEL STRUCTURE.
3.	EXCEPT WHERE INDICATED OTHERWISE, INSTALLATION OF COMPACTED BACKFILL MUST BE DONE SIMULTANEOUSLY ON BOTH SIDES OF WALLS TO PRECLUDE UNBALANCED SOIL LOADS. PLACE FILL USING HAND DIRECTED COMPACTION EQUIPMENT. DO NOT USE HEAVY CONSTRUCTION EQUIPMENT ADJACENT TO THE FOUNDATION.		
4.	DO NOT PERMIT FOUNDATION BEARING SOIL TO BECOME SATURATED OR DRY EXCESSIVELY. CONCRETE FOR CONVENTIONAL SHALLOW FOUNDATIONS MUST BE PLACED AS SOON AFTER THE COMPLETION OF THE EXCAVATIONS AS POSSIBLE. IF A DELAY OF MORE THAN ONE DAY IS EXPECTED FOR FOUNDATION CONCRETE PLACEMENT IN ANY EXCAVATION, PLACE A 2 INCH THICK LEAN CONCRETE (f _c = 2000 PSI MIN.) "MUD SEAL" ATOP THE EXPOSED BEARING SOILS.		

5. ELEVATIONS SHOWN ON FOUNDATION PLANS ARE TO TOPS OF FOOTINGS.
6. SLABS ON GROUND MUST BE PLACED OVER A 15 MIL MINIMUM THICKNESS PUNCTURE RESISTANT VAPOR BARRIER (SEE SPECIFICATIONS), ON A 6 INCH MINIMUM THICK CAPILLARY WATER BARRIER ON STRUCTURAL FILL (SEE CIVIL FOR REQUIREMENTS). TURN THE VAPOR BARRIER UP AT INTERSECTING VERTICAL SURFACES AND UTILITY PENETRATIONS AND SEAL WITH VAPOR BARRIER MANUFACTURER'S STANDARD, COMPATIBLE TAPE. SEAL ALL JOINTS WITH TAPE TO PROVIDE A CONTINUOUS UNBROKEN VAPOR BARRIER BETWEEN THE SUBGRADE AND THE SLAB ABOVE.
7. SAWN JOINTS IN CONCRETE SLABS MUST BE MADE THE SAME DAY AS PLACEMENT, AS SOON AS POSSIBLE WITHOUT DAMAGING THE CONCRETE, AND IN NO CASE LATER THAN 12 HOURS AFTER CONCRETE PLACEMENT.
8. COORDINATE EXACT LOCATIONS OF SLAB CONTROL AND CONSTRUCTION JOINTS WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH FINISHES.
9. BACKFILL FOR FOUNDATION AND RETAINING WALLS MUST BE A FREE-DRAINING SATISFACTORY SOIL MATERIAL. BACKFILL MUST BE COMPACTED SUFFICIENTLY TO PREVENT SUBSIDENCE OF SURFACE ADJACENT TO WALL.
10. ALL UTILITIES ENTERING THE BUILDING MUST BE ROUTED BENEATH FOOTINGS. SEE DETAIL B6/CU-503.

1. CONCRETE FOR BUILDING AND SITE STRUCTURES MUST BE NORMAL WEIGHT (W=145 PCF) WITH 3500 PSI 28-DAY COMPRESSIVE STRENGTH (f_c).
2. CONCRETE EXPOSED TO WEATHER MUST HAVE A TOTAL AIR CONTENT BETWEEN 4.5% AND 7.5%.
3. FURNISH MATERIALS, MIX, PLACE, FINISH, AND CURE ALL CONCRETE IN ACCORDANCE WITH ACI 301 AND ACI 318.
4. CONCRETE REINFORCING BAR COVER MUST BE AS INDICATED IN TABLE "CAST IN PLACE CONCRETE COVER FOR REINFORCING BARS".
5. NO PIPING OR ELECTRICAL CONDUITS ARE ALLOWED TO BE EMBEDDED HORIZONTALLY WITHIN THE THICKNESS OF CONCRETE SLABS.
6. CHAMFER ALL EXPOSED EDGES, UNLESS INDICATED OTHERWISE, WITH A 1/2" RADIUS EDGE.
7. BARS FOR CAST IN PLACE CONCRETE MUST BE PER ASTM A 615, GRADE 60.
8. DETAILING OF REINFORCING MUST BE IN ACCORDANCE WITH ACI 318 AND ACI 315.
9. SECURE ALL REBARS IN THE CORRECT POSITION PRIOR TO PLACING CONCRETE. TOLERANCES FOR PLACEMENT OF REBARS MUST BE IN ACCORDANCE WITH ACI 301.
10. REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES, AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.

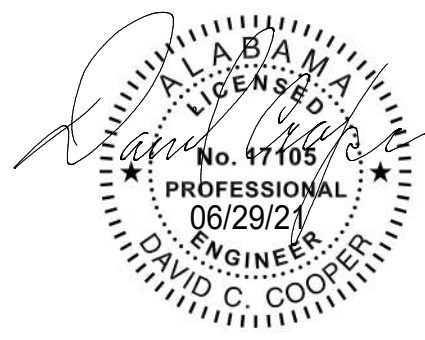
1. DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL MUST BE IN ACCORDANCE WITH AISC 360, SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (LRFD DESIGN) AND THE AISC AND THE AISC "STEEL CONSTRUCTION MANUAL", 15TH EDITION.
2. STEEL GRADES:
 - A. ANGLES AND MISCELLANEOUS PLATES - ASTM A36 ($F_y = 36$ KSI)
 - B. STRUCTURAL TUBING (SQ., RECT., ROUND) - ASTM A500, GRADE C; OR ASTM A1085
 - C. CHANNELS - ASTM A36 ($F_y = 36$ KSI), OR ASTM A572, GRADE 50 ($F_y = 50$ KSI)
 - D. W SHAPES - ASTM A992, ($F_y = 50$ KSI)
3. ALL BOLTS MUST BE 3/4 INCH DIAMETER, HIGH STRENGTH, ASTM F3125 (GRADE F1852 (TWIST-OFF)), TYPE 1, IN FULLY TENSIONED BEARING CONNECTIONS, UNLESS INDICATED OR REQUIRED OTHERWISE. BOLT TIGHTNESS MUST BE VERIFIED BY THE USE OF APPROVED TWIST OFF BOLTS.
4. ALL WELDING MUST BE IN ACCORDANCE WITH ANSI/AWS D1.1 STRUCTURAL WELDING CODE. WELDING ELECTRODES MUST BE E70 SERIES.
5. EXCEPT AS OTHERWISE INDICATED ON DRAWINGS, ALL CONNECTIONS MUST BE BOLTED FRAMED BEAM CONNECTIONS (DOUBLE ANGLE) AS GIVEN IN TABLES 10-1 AND 10-2 OF THE AISC STEEL CONSTRUCTION MANUAL, OR SINGLE-PLATE SHEAR CONNECTIONS (AISC MANUAL, TABLES 10-10). AT THE CONTRACTOR'S OPTION, CONNECTIONS MUST BE SIZED AND DESIGNED BY THE CONTRACTOR TO ACCOMMODATE, AT A MINIMUM, 125% OF THE UNFACTORED BEAM END REACTIONS INDICATED IN KILOPOUNDS (KIPS, k) ON THE FRAMING PLANS. REGARDLESS OF PLAN DESIGNATION, NO CONNECTION MAY BE DESIGNED FOR LESS THAN 10 KIPS.
6. MINIMUM WELD SIZE MUST BE 3/16" FILLET WELDS ALL AROUND, UNLESS INDICATED OTHERWISE.
7. SHOP CONNECTIONS MUST BE WELDED AND FIELD CONNECTIONS MUST BE WELDED OR BOLTED AS INDICATED OR APPROVED ON SHOP DRAWINGS.
8. NO SHOP SPlice OR OTHER CONNECTION WILL BE PERMITTED UNLESS SHOWN ON APPROVED SHOP DRAWINGS.
9. NATURAL CAMBER IN BEAMS MUST BE INSTALLED CROWN UP.
10. THE ERECTOR MUST PROVIDE ALL TEMPORARY SHORING AND BRACING REQUIRED FOR STABILITY UNTIL STRUCTURES ARE COMPLETE.

1. ALL LIGHT GAUGE STEEL FRAMING AND SUBFRAMING SUPPORT SYSTEMS (SEE SPECIFICATIONS FOR EXTENT) MUST BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AND THE COLD FORMED STEEL DESIGN MANUAL.
2. CALCULATIONS AND SHOP DRAWINGS MUST BE PRODUCED, SEALED, AND SIGNED BY A LICENSED PROFESSIONAL ENGINEER. INCLUDE DESIGN OF CONNECTIONS TO PRIMARY SUPPORTING STEEL STRUCTURE.
3. INDICATED LIGHT GAUGE STEEL FRAMING MEMBERS MUST BE HOT-DIPPED GALVANIZED, G-60 COATING WEIGHT, AND MUST CONFORM TO ASTM DESIGNATION C 955 AND A 653. MINIMUM YIELD POINT, UNLESS INDICATED OTHERWISE, MUST BE 50 KSI FOR 16 GA. AND HEAVIER AND 33 KSI FOR 18 GA. AND LIGHTER.
4. FRAMING MEMBERS MUST HAVE THE MINIMUM STRUCTURAL PROPERTIES INDICATED ON THE STRUCTURAL AND/OR ARCHITECTURAL DRAWINGS AND SPECIFICATIONS SECTION 05 40 00. MAXIMUM SPACING OF MEMBERS MUST BE 16". MINIMUM REQUIREMENTS FOR PERIMETER WALLS ARE 18 GAUGE C-SHAPED STUDS, 2.5" AND 6" DEEP X 1.625" WIDE WITH 1/2" WIDE RETURN LIPS.
5. MINIMUM REQUIREMENTS FOR JAMBS ON PERIMETER WALLS ARE (2) 18 GAUGE C-SHAPED 6" DEEP X 1.625" WIDE WITH 1/2" WIDE RETURN LIPS. THE JAMBS MUST RUN THE FULL HEIGHT OF THE WALL.
6. MINIMUM REQUIREMENTS FOR HEADERS ON PERIMETER WALLS ARE BOXED HEADERS, (2) 18 GAUGE C-SHAPED STUDS 3.625" DEEP X 1.625" WIDE WITH 1/2" WIDE RETURN LIPS AND (2) 18 GAUGE TRACKS 6" DEEP X 1.25" WIDE WITH 1/2" WIDE RETURN LIPS.
7. MINIMUM REQUIREMENTS FOR SILLS ON PERIMETER WALLS ARE 18 GAUGE TRACKS 6" DEEP X 1.25" WIDE WITH 1/2" WIDE RETURN LIPS.
8. DEFLECTION OF FRAMING (STUD WALLS) MUST NOT EXCEED L/240 UNDER LOAD FROM INDICATED WIND SPEED.
9. SUBMIT SHOP DRAWINGS AND CALCULATIONS STAMPED BY A LICENSED PROFESSIONAL ENGINEER FOR REVIEW AND APPROVAL OF COLD-FORMED STEEL MEMBERS AND ASSEMBLIES PRIOR TO FABRICATION. INCLUDE DESIGN OF CONNECTIONS TO PRIMARY SUPPORTING STEEL STRUCTURE.

1. COLD-ROLLED METAL DECK FOR ROOFS MUST BE GALVANIZED STEEL (G60 COATING), ASTM A653 OR A792, DESIGNED IN ACCORDANCE WITH THE STEEL DECK INSTITUTE (SDI) "MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS."
2. DECK SUPPORTED ON OPEN WEB STEEL JOISTS MUST BE THE NON-COMPOSITE TYPE, 1.5" DEEP, TYPE F TO MATCH EXISTING DECK PROFILE (FIELD VERIFY), WITH THE FOLLOWING MINIMUM PROPERTIES PER 12" OF WIDTH:
A. 1.5" DEEP 22 GA. I=0.113 IN4 S=0.112 IN3 Fy= 33 KSI
3. OTHER COMBINATIONS OF PROPERTIES AND YIELD STRENGTHS THAT PROVIDE EQUIVALENT PERFORMANCE MAY BE SUBMITTED FOR APPROVAL.
4. STEEL ROOF DECKS ARE REQUIRED TO ACT AS DIAPHRAGMS. REFER TO "TYPICAL ROOF DECK FASTENER SCHEDULE" FOR ATTACHMENT PATTERN.
5. DO NOT WELD ROOF DECK TO OPEN WEB STEEL JOISTS. USE INDICATED SCREW CONNECTIONS.
6. DO NOT SUPPORT ANY MATERIALS OF CONSTRUCTION FROM STEEL ROOF DECK.

1. MASONRY MUST BE AS DETAILLED ON THE ARCHITECTURAL, CIVIL, AND STRUCTURAL DRAWINGS.
2. CONCRETE MASONRY UNITS MUST BE MEDIUM WEIGHT HOLLOW NON-LOAD BEARING TYPE CONFORMING TO ASTM C90, TYPE 1, GRADE N. COMPRESSIVE STRENGTH f_m IN AVERAGE THREE UNITS MUST BE 2000 PSI MINIMUM AT 28 DAYS.
3. MORTAR MUST BE ASTM C270, TYPE S.
4. GROUT MUST BE ASTM C476, MINIMUM COMPRESSIVE STRENGTH = 2000 PSI AT 28 DAYS. INDIVIDUAL GROUT LIFTS MUST NOT EXCEED 4 FEET IN HEIGHT.
5. HORIZONTAL MASONRY JOINT REINFORCEMENT MUST BE PROVIDED AT 16" ON CENTER VERTICAL SPACING IN ALL CMU WALLS.
6. REINFORCING BAR GUIDES TO CENTER VERTICAL REINFORCING IN MASONRY WALLS MUST BE PROVIDED.
7. ALL CONCRETE MASONRY UNITS MUST BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE. PROVIDE CONTINUOUS BOND BEAMS AS SHOWN ON THE STRUCTURAL DRAWINGS, SPACED NO FURTHER THAN 4'-0" ON CENTER VERTICALLY.
8. ALL BLOCK CELLS AND CAVITIES BELOW GRADE MUST BE FILLED SOLID WITH GROUT OR FINE AGGREGATE CONCRETE.
9. BRACE ALL MASONRY WALLS DURING CONSTRUCTION TO RESIST LATERAL LOADS.
10. SHOP DRAWINGS FOR REINFORCING BARS MUST INCLUDE ELEVATION VIEWS OF EXTERIOR WALLS TO INDICATE BAR PLACEMENT IN WALLS INCLUDING AROUND ALL OPENINGS.
11. DO NOT INTERRUPT BOND BEAM REINFORCEMENT AT WALL JOINTS.

NO PROVISION FOR SUPPORT OF FUTURE HORIZONTAL OR VERTICAL CONSTRUCTION IS MADE. FUTURE ADJACENT ADDITIONS MUST BE STRUCTURALLY INDEPENDENT, AND SEPARATED BY BUILDING JOINTS.



<p>ARTC - B640 RENOVATION FEMA - DHS - CDP, ANNISTON, AL</p>	<p>GENERAL STRUCTURAL NOTES</p>
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SHEET
IDENTIFICATION
S-001
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