

Crash Patterns at Signalized Intersections

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Traffic signals are often implemented to provide for efficient movement and to improve traffic safety. Nevertheless, severe crashes still occur at signalized intersections. This study aims to improve understanding of signalized intersection safety by identifying crash types, locations, and factors associated with signalized intersections. For this purpose, 1,295 police-reported crashes at 87 signalized intersections were analyzed on the basis of detailed crash descriptions, that is, crash data and collision diagrams. The information from the collision diagrams was used to distinguish six crash types and to create a crash location typology to divide the signalized intersection into 13 detailed typical segments. Logistic regression modeling techniques were used to identify relations between crash types, their crash location on certain signalized intersection segments, the crash severity, and the different features that affected crash occurrence. Four dominant crash types were identified: rear-end, side (i.e., left-turn plus right-angle), head-on, and vulnerable road user crashes. The results of the logistic regression models showed that the location of these crash types was related to specific signalized intersection segments. The results also revealed important signalized intersection features that affected the crash occurrence. As a result, connections between certain signalized intersection crash types, their crash location, and signalized intersection design characteristics were found. The combination of intersection features with detailed signalized intersection segments provided valuable insights into the nature of signalized intersection crashes and the safety impact of signalized intersection design.

Intersections are crash-prone locations since they are characterized by many conflicting movements, which result in complexity and large variations in interactions between road users. To minimize the number of conflicts at intersections and to increase traffic safety, intersections are often equipped with traffic signals (1). Despite the fact that traffic signals separate movements in space and time, crashes at these intersections still occur. In Flanders, Belgium, approximately 8% of all injury crashes occur at signalized intersections and represent 4% of all road deaths (2). However, equipping intersections with traffic lights can also induce side effects. Traffic signals can change the crash pattern at intersections by decreasing head-on and angle crashes while increasing rear-end crashes (3, 4). Subsequently, traffic lights also give rise to red-light-running crashes, which tend to be more severe since they typically occur at high speeds (3).

Previous studies identified four dominant crash types at signalized intersections: rear end, angle, sideswipe, and vulnerable road user crashes (5–8). Crashes with vulnerable road users and angle crashes are of a more severe nature and result more often in dead or severely

injured road users, whereas sideswipe and rear-end crashes have a less serious outcome and result in crashes with material damage or slight injuries (5, 9, 10).

Several studies have also researched the relation between signalized intersection design and crash occurrence. The presence or absence of several signalized intersection design characteristics appears to have a beneficial or adverse effect on the traffic safety of these locations. The total number of lanes is positively related to the number of crashes (6). However, exclusive right-turn and left-turn lanes have a positive effect on traffic safety since they reduce the total number of crashes, whereas exclusive right-turn lanes (in countries with right-hand traffic) also lead to a decrease in rear-end crashes (5, 11). Medians lead to lower crash severity levels since they prevent more severe head-on crashes (9). Signalized intersection speed limits play an important role in the total number of crashes, angle crashes, left-turn crashes, head-on crashes, rear-end collisions, and crashes with vulnerable road users (9, 12). In general, red-light cameras tend to increase the number of rear-end crashes and decrease the occurrence of side crashes (i.e., left-turn + right-angle crashes) (13–15). Protected-only and protected/permitted left-turn signal phasing leads to substantial decreases in the number of injury and severe injury crashes at signalized intersections (16). These types of signal phasing also have a favorable effect on left-turn crashes (16, 17). Vulnerable road user facilities also influence traffic safety at signalized intersections. At signalized intersections with low vehicle speeds and volumes, mixing cyclists with motorized traffic at the intersection has been reported to be the safest solution (18). Pedestrian safety at signalized intersections has been found to depend on the number of lanes. The more lanes pedestrians must cross, the higher the number of pedestrian crashes (19).

STUDY OBJECTIVE

Many studies have already focused on the road safety performance of signalized intersections. However, little is known about the exact location of the crashes. Therefore, this study focuses on identifying and analyzing the crash patterns at signalized intersections by using detailed information about the location of the crash. Gstalter and Fastenmeier analyzed driver error by dividing intersections into segments according to the tasks that drivers should perform in each segment (20). The current study elaborates on this approach and tries to delineate the crash location on the signalized intersection itself in more detail to gain better insight into the crash patterns and their exact location. This method identifies the dominant crash type inside each segment and endeavors to link the crash occurrence with design characteristics of the signalized intersection. As a result, the findings of this study result in a detailed description of the crash patterns at signalized intersections, which provide insights into the safety impact and possible safety issues of this intersection design. Other studies have also applied the same or similar methods

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to other locations including stop-sign-controlled intersections (21), roundabouts (22), freeway ramps (23), and work zone crashes (24).

METHODOLOGY

Data

Crash Data

In this study, the crashes were sampled from police-reported crashes at 87 signalized intersections in the region of Flanders, Belgium. The national crash database could not be used since it does not contain detailed information about the crash location at the signalized intersection. Therefore, several police districts were selected that systematically register more detailed crash location information. Ultimately, 12 police districts were able to provide the requested data. This approach resulted in a convenience sample of signalized intersection locations.

The crashes occurred in the period of 2007 to 2011. Crash data were available for each year and for every sampled signalized intersection in this entire period. In total, 1,344 crash reports containing injury and property-damage-only crashes were obtained. These police reports provided basic (such as time, place of occurrence, weather and light conditions) and detailed (such as crash type and location) information about the registered crashes. The detailed crash information, in the form of collision diagrams, was used to develop crash types. A collision diagram is a schematic representation of all crashes that occurred at a given signalized intersection or other location over a specific period (3). This diagram indicates the dominant crash types at a signalized intersection and the maneuvers that led to these crashes while providing detailed information about the crash location at the intersection.

Intersection Design and Usage Data

Crash data alone are not sufficient to provide insights into the crash patterns at signalized intersections. It is also important to know the crash location in terms of roadway and traffic data in order to gain a full understanding of the traffic safety situation. These factors may affect the crash occurrence. Roadway data aid in detecting the physical and use characteristics of the location that may have contributed

to the crash occurrence or severity, whereas traffic volume data are used to control for use intensity of the location (25).

Based on a literature review, the most relevant signalized intersection characteristics were selected as they appear from previous crash prediction model studies (6, 26, 27). They include number of arms, presence of exclusive turn lanes, number of lanes, location in a built-up area, type of bicycle infrastructure, presence of a median, speed limit, signal phasing, crossings for vulnerable road users, and presence of a bypass and red-light camera. Traffic volume data were available for 54 of 87 sampled signalized intersections. The traffic volumes in the data are expressed as annual average daily traffic (AADT). No data were available for exposure by type of road user and the actual driving speeds at the signalized intersection. A detailed description of intersection characteristics is provided in Tables 1 and 2.

Signalized Intersection Segments

The detailed crash location was determined by dividing the signalized intersection into different typical segments according to previously established knowledge on the crash occurrence and road user behavior at signalized intersections (5–7, 13, 20). Figure 1 shows the selected 13 segments, which are described as follows:

Segment 1. 20 to 100 m from the signalized intersection; oncoming traffic and queues associated with congestion;

Segment 2. 20 m before the intersection plane until the stop line;

Segment 3. Exclusive left-turn lane, if present;

Segment 4. First half of the intersection plane; pedestrian and cyclist crossings;

Segment 5. Second half of the intersection plane for traffic going straight ahead;

Segment 6. Second half of the intersection plane for traffic turning left;

Segment 7. 20 m after the junction plane for right-turning traffic leaving the intersection; pedestrian and cyclist crossings;

Segment 8. Identical to Segment 7 but for traffic going straight ahead;

Segment 9. Identical to Segment 7 but for left-turning, leaving traffic;

Segment 10. 20 to 100 m after the intersection plane; leaving traffic;

TABLE 1 Descriptive Statistics: Crash Variables

Variable	Description	Signalized Intersection ($N_{\text{locations}} = 87$, $N_{\text{crashes}} = 1,295$)
Injury crash	Crash type with regard to crash outcome.	Property damage only = 596, injury crash = 699
Crash severity	Determined by most severe casualty.	No injuries = 596, dead = 7, severely injured = 64, slightly injured = 628
Road user	Type of road user involved; frequencies expressed at subject level.	Car = 2,098, truck = 105, bus = 27, motorcycle = 48, moped = 100, cyclist = 162, pedestrian = 42, other = 70
Crash	Crash type according to number of involved road users.	Single = 130, multiple = 1,165
Crash type	Crash type according to collision angle (0°, 90°, 180°).	Single vehicle = 130, head-on (180°) = 181, rear-end (0°) = 471, pedestrian = 41, sideswipe (45°) = 121, side crash (90°) = 351
Segment	Location of crash expressed as one of the segments (seg.) of Figure 1.	Seg. 1 = 103, Seg. 2 = 301, Seg. 3 = 97, Seg. 4 = 214, Seg. 5 = 71, Seg. 6 = 187, Seg. 7 = 79, Seg. 8 = 62, Seg. 9 = 66, Seg. 10 = 36, Seg. 11 = 8, Seg. 12 = 33, Seg. 13 = 38
Vulnerable road user (VRU)	Crash in which at least one VRU is involved.	Yes = 268, no = 1,027

NOTE: VRUs are pedestrians, cyclists, mopeds, and motorcyclists.

TABLE 2 Descriptive Statistics: Intersection Design Variables

Variable	Description	Number of Signalized Intersections
Arms	Number of intersection arms	3 = 201 (22), 4 = 1,094 (65)
Lanes	Total number of lanes at intersection ^a	1 = 90 (12), 2 = 434 (39), 3 = 379 (26), 4 = 392 (10)
Exclusive right turn	Presence of exclusive right-turn lane at intersection (at least on one intersection arm)	Yes = 455 (63), no = 840 (24)
Exclusive left turn	Presence of exclusive left-turn lane at intersection (at least on one intersection arm)	Yes = 1,186 (72), no = 109 (15)
Built-up area turn	Location of intersection in terms of inside or outside built-up area	Yes = 581 (50), no = 714 (37)
Median	Presence of a median at intersection ^b	Yes = 930 (50), no = 365 (37)
Speed limit	Speed limit at intersection	50 km/h = 442 (42), 70 km/h = 414 (31), 90 km/h = 439 (14)
Cycle facility	Type of cycle facility at intersection ^c	Mixed = 30 (4), cycle lanes = 507 (39), separated = 554 (40), grade separated = 204 (4)
Pedestrian crossing	Presence of pedestrian crossing at intersection ^b	Yes = 1,092 (81), no = 203 (6)
Cyclist crossing	Presence of cyclist crossing at intersection ^b	Yes = 815 (52), no = 480 (35)
Signal phasing	Type of signal phasing at intersection (for left turns)	Protected-only = 301 (12), protected/permitted = 236 (13), permitted = 758 (62)
Bypass	Presence of bypass at intersection ^b	Yes = 712 (29), no = 582 (58)
Red light camera (RLC)	Presence of red light camera at intersection (at least in one direction)	Yes = 657 (31), no = 638 (56)
Traffic volume	Traffic volume at intersection (AADT)	Mean = 30,959.66 SD = 11,960.80 Minimum = 14,561.73 Maximum = 67,497.13

NOTE: Values in parentheses = number of crashes that occurred in entire sample with a certain characteristic.

^aIn case of different situations at intersection arms, highest number in lane is applied.

^bIn case of different situations at intersection arms = yes.

^cIn case of different situations at intersection arms, highest cycle facility type is applied.

Segment 11. Beginning of the bypass, if present;

Segment 12. Middle section of the bypass, including pedestrian and cyclist crossings, if present; and

Segment 13. End section of the bypass until the yield markings.

Segments 11 to 13 are optional and are only relevant when the signalized intersection is characterized by a bypass.

Figure 1 is a representation of a typical signalized intersection. The segments were defined in such a way that the variety of real-world designs is represented and meaningful analyses based on the defined standard segments are possible. To capture all possible designs, a “maximal design” was used; this design represents a typical signalized intersection layout with some extra features that are not necessarily always present. For example, a bypass lane was added in order to include crashes that happen on bypass lanes at certain intersections. This layout means that only crashes at Segments 11 to 13 must be registered in case of a signalized intersection with such a bypass lane. The same applies for the cycle facilities (cycle paths and cycle crossings): pedestrian or bicyclist crossings at real-world intersections occur in different varieties. Therefore, although Figure 1 represents an adjacent cycle path, the real distance between the cycle facility and the roadway may vary between 0 and 10 m and be grade-separated. This principle also applies to the number of lanes and the number of intersection arms.

Crash Location Typology

A crash typology was created to assign the crashes to the segments shown in Figure 1. This typology is based on the crash typology

of Massie et al., who identified different crash scenarios between motorized vehicles based on crash data and collision diagrams (28).

The first step involved revising the crash data and collision diagrams to select the variables that seemed most useful to the development of a crash location typology. The main focus of this review was on the precrash movements of the involved road users. The selected variables of the initial review were used to build a preliminary crash location typology, which was modified by adding and deleting variables until the final crash location typology scheme, as shown in Figure 2, was produced. This typology is applicable for crashes between motorized road users, between motorized and vulnerable road users, and between vulnerable road users.

The southern intersection approach in Figure 1 was used as the analysis unit. Each crash was located by starting from this intersection approach. The road user who makes the precrash maneuver or movement always approaches the intersection from this side. The maneuvering road user is based on the schematic representation of the crash in the collision diagrams.

The final crash location typology includes the number of road users involved in the crash, the location of the impact point, the relative precrash orientation of the road users, and the movement of the road user who makes the maneuver. Figure 2 provides an overview of the typology. The crashes were first split according to whether the road user was involved in a crash with only one or with multiple road users (Step 1). These two groups were then divided on the basis of whether the crash took place before, after, at the intersection plane, or at the bypass (Step 2). Multiple road user crashes were split into three categories: road users approaching each other from the same direction before the crash, road users approaching from opposite directions,

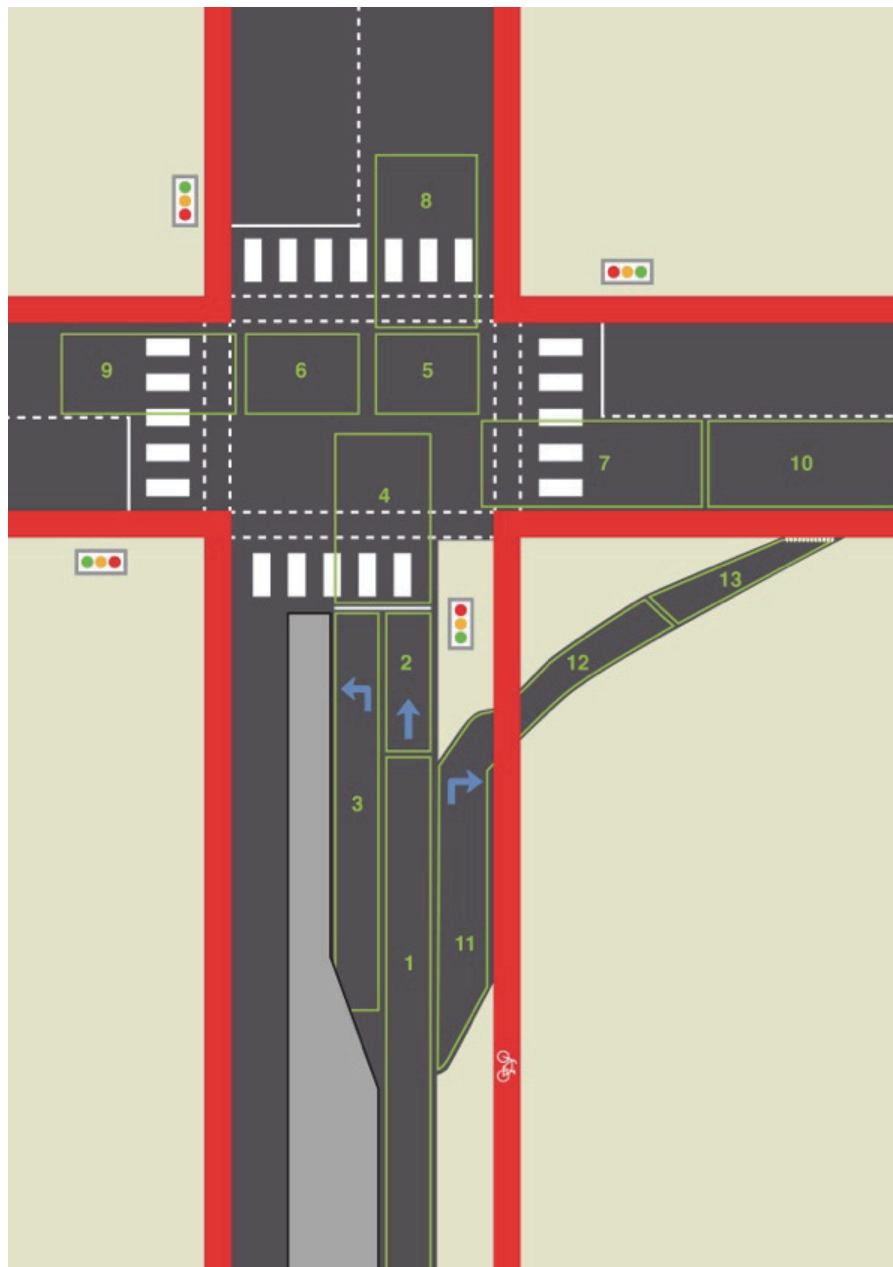


FIGURE 1 Signalized intersection segments.

and road users approaching on crossing paths (Step 3). Subsequently, the single and multiple road user crashes were further split according to whether the maneuvering road user was moving straight ahead or attempting to make a left, right, or U-turn (Step 4). Finally, the resulting subgroups were assigned to the crash location expressed as Segments 1 to 13 in Figure 1 (Step 5). Steps 4 and 5 are combined in Figure 2 for visualization purposes.

Crash Data Analysis

Several studies previously applied logistic regression analysis to test the influence of traffic crash risk factors (29–33). In this study the occurrence of certain dominant crash types at signalized intersections can be considered as a binary response variable. Therefore,

logistic regression analysis was used to predict the probability of a certain event. This analysis also allows the testing of the relation between the dominant crash types and their crash location on the signalized intersection. The structure of the fitted logistic regression models was the following (34):

$$\text{logit}(P) = \ln\left(\frac{P}{1-P}\right) = \beta_0 + \beta_1 x_1 + \beta_2 x_2 + \dots + \beta_n x_n \quad (1)$$

where

P = probability of dominant crash types,
 x_n = independent variable, and
 β_n = partial logistic regression coefficient.

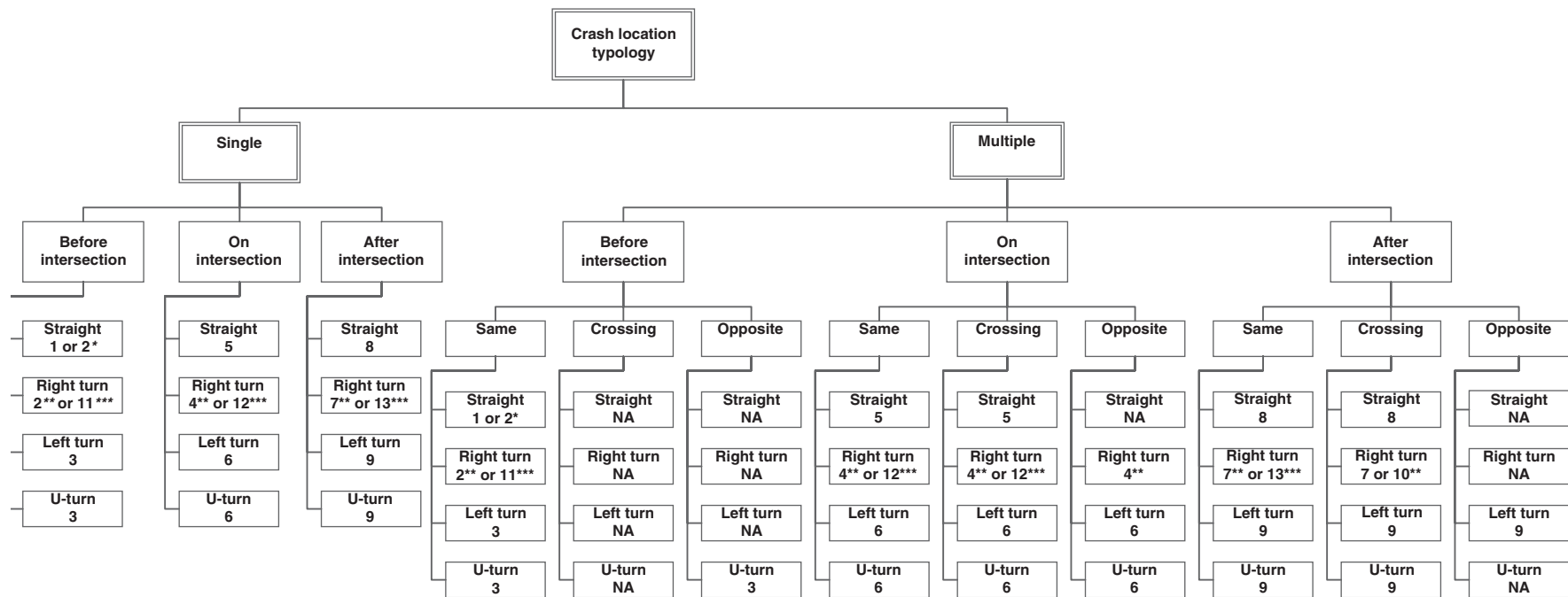


FIGURE 2 Crash location typology [numbers indicate intersection segment; NA = no segment available (not every maneuver can occur on each segment); *depends on distance to intersection; **intersection without bypass; ***intersection with bypass].

The odds of each dominant crash type were defined as the probability of the occurrence of this specific dominant crash type divided by the probability of the occurrence of all other signalized intersection crash types. Odds ratios [$OR = \exp(\beta_n)$] were calculated to determine the rate of decrease ($0 \leq OR < 1$) or increase ($OR > 1$) of the probability of the outcome when the value of the independent variables increases by one unit (35). Firth's penalized maximum likelihood was applied to overcome the most common convergence failure in logistic regression, namely, the problem of quasi-complete separation (34, 36). The logistic regression models were developed with the LOGISTIC procedure in SAS 9.3, and the variables identified in the literature as having a significant impact on signalized intersection crashes were added first.

Crash reports with missing data were omitted from the models, resulting in 1,295 complete crash records. The model fit was assessed with the Hosmer–Lemeshow test, which indicates if the final model provides a better fit than the null model. If the chi-square goodness of fit is not significant at a confidence interval of 95%, the model has an adequate fit. Since this statistic gives no indication of the error reduction of the final model, Nagelkerke's R^2 was also used. The variance inflation factor was used to identify multicollinearity between the predictor variables. According to O'Brien, variance inflation factors higher than 4 indicate a high correlation between variables (37). Since all variables in the end models had variance inflation factors below this threshold, there are no multicollinearity issues in the presented models.

RESULTS

Descriptive Statistics

All crashes within 100 m from the center of the intersection were included in the analysis to ensure that all crashes related to the signalized intersection were incorporated into the data set. Descriptive statistics of the crash data are presented in Tables 1 and 2. The registered crashes at the study locations were mostly injury crashes (53%, 699 out of 1,295). The variable "segment" indicates that most crashes occur in Segments 1, 2, and 4 before the intersection plane and on Segment 6 of the intersection plane, where left-turning traffic conflicts with oncoming vehicle streams. Segments 11 to 13 on the bypass seem to be less prone to crashes. This finding may be due to the small share of signalized intersections with a bypass ($N = 29$) in the police data.

The crashes were categorized into six different crash types: rear-end, head-on, sideswipe, single-vehicle, pedestrian, and side crashes. Three main crash types can be considered as the dominant crash types—rear-end, side (left-turn plus right-angle crashes), and head-on crashes—since they accounted for 77% of the signalized intersection crashes. In general, these three crash types typically take place between motorized road users. This characteristic is also the case in this study since 96%, 74%, and 85% of the involved road users in rear-end, side, and head-on crashes, respectively, were motorized road users. No separate crash type was developed for cyclists, as was done for pedestrians, because the action radius of cyclists is larger than that for pedestrians. Therefore, the 150 registered cyclist crashes were divided among the six defined crash types. The majority of cyclist crashes were side (71%) and head-on collisions (14%). The other crash types—single-cyclist (5%), rear-end (8%), pedestrian (1%), and sideswipe crashes (1%)—occurred less frequently.

Logistic Regression Results

Table 3 presents the factors that influence the dominant signalized intersection crash types and the factors that affect the probability that one of these dominant crash types will occur. The dependent variable was the probability that a specific dominant crash type occurred over the entire 5-year period from 2007 to 2011.

The results show that the probability of an injury increases in the case of side crashes, head-on crashes, and crashes with vulnerable road users, whereas single-vehicle crashes result significantly less in injuries. The injuries are also more severe in crashes involving vulnerable road users.

The crash types seem to be related to certain signalized intersection segments. Injury crashes are more likely on Segments 4, 5, and 6, which are the segments on the intersection plane, than on Segments 3, 10, and 13. Crashes before the intersection plane (Segments 1 to 3) and on the bypass (Segments 11 to 13) are more likely rear-end crashes than crashes on and after the intersection plane (respectively, Segments 5 and 6 and Segments 7 and 10). Side crashes are more likely on the intersection plane (Segments 4 to 6) than before (Segments 1 to 3) and after the intersection plane (Segment 10). Crashes on the intersection plane (Segments 4 to 6) are also more likely head-on crashes than crashes before the intersection plane (Segments 1 and 2). The probability of crashes with vulnerable road users is higher on the crossing facilities after the intersection plane (Segments 7 and 8) and on the bypass (Segment 12) than before (Segments 1 to 3) and on the intersection plane (Segments 5 and 6).

The type of left-turn signal phasing also influences the probability of certain dominant crash types. Injury crashes are less likely at intersections with protected-only and protected/permitted signal phasing (compared with the standard permitted signal phasing). Rear-end, head-on, and vulnerable road user crashes are less likely at signalized intersections with protected-only signal phasing. Vulnerable road user crashes are also less likely at signalized intersections with protected/permitted signal phasing whereas the probability of rear-end crashes increases. The odds of head-on crashes seem to decrease nonsignificantly at signalized intersections with protected/permitted signal phasing.

Moreover, the signalized intersection layout affects the odds of certain dominant crash types. The probability of an injury crash decreases at signalized intersections with an exclusive lane for right-turning traffic, and rear-end crashes appear to be more likely at signalized intersections with three arms. Furthermore, rear-end and vulnerable road user crashes appear to be less likely at signalized intersections with two lanes, whereas vulnerable road user crashes also are significantly more likely at signalized intersections with three lanes. Rear-end and head-on crashes are less likely at signalized intersections with medians.

Side crashes are more likely at signalized intersections located inside built-up areas, whereas the probability of head-on crashes decreases. Furthermore, injury crashes are less likely at 50-km/h intersections (compared with 70- and 90-km/h intersections), and vulnerable road user crashes are more likely at 50-km/h intersections and less likely at 70-km/h intersections (compared with 90-km/h intersections). Crashes with vulnerable road users also appear to be more likely at signalized intersections where cycle traffic is mixed with motorized traffic.

Enforcement cameras at signalized intersections also appear to affect certain crash types since the presence of a red-light camera decreases the probability of side, head-on, and vulnerable road user crashes.

TABLE 3 Factors Influencing Probability of Signalized Intersection Crash Types

Logistic Regression Results at Crash Level (N = 1,295)						
Variable	Injury Crashes ^a According to Crash Type (Y = 699)	Injury Crashes ^a According to Crash Location (Y = 699)	Rear-End Crashes (Y = 471)	Side Crashes ^b (Y = 351)	Head-On Crashes (Y = 181)	VRU Crashes ^c (Y = 268)
Intercept	0.6719****	0.8753****	-1.2603****	-1.5545****	-3.2815****	-0.3506*
Crash type (ref = sideswipe)						
Single vehicle	-0.9745 (0.38)***	na	na	na	na	na
Head-on	0.8965 (2.45)***	na	na	na	na	na
Rear-end	-0.0396 (0.96)*	na	na	na	na	na
Side	0.6679 (1.95)***	na	na	na	na	na
Pedestrian	0.6527 (1.92)*	na	na	na	na	na
Segment (ref = Segment 9)						
Segment 1	na	0.00209 (1.00)*	1.7636 (5.83)****	-0.6957 (0.50)***	-1.3791 (0.25)**	-0.7382 (0.48)***
Segment 2	na	-0.091 (0.91)*	2.7385 (15.46)****	-3.7433 (0.02)****	-2.4178 (0.09)****	-2.1688 (0.11)****
Segment 3	na	-0.7963 (0.45)****	1.4328 (4.19)****	-2.5073 (0.08)****	-0.7805 (0.46)*	-1.3573 (0.26)****
Segment 4	na	0.5328 (1.70)****	-0.1245 (0.88)*	1.545 (4.68)****	1.4759 (4.37)****	0.1931 (1.21)*
Segment 5	na	0.9915 (2.70)****	-2.1494 (0.12)****	2.799 (16.42)****	1.0755 (2.93)****	-1.0304 (0.36)****
Segment 6	na	0.7509 (2.12)****	-3.6358 (0.03)****	0.9333 (2.54)****	3.3968 (29.87)****	-1.1661 (0.31)****
Segment 7	na	-0.092 (0.91)*	-1.6861 (0.19)****	1.9607 (7.10)****	0.1590 (1.17)*	2.3335 (10.31)****
Segment 8	na	-0.0188 (0.98)*	-0.3096 (0.73)*	1.1573 (3.18)****	0.1479 (1.16)*	1.3144 (3.72)****
Segment 10	na	-0.8600 (0.42)**	-1.6962 (0.18)****	-2.6337 (0.07)***	-1.3188 (0.27)*	0.00919 (1.01)*
Segment 11	na	0.9540 (2.60)*	1.5628 (4.77)***	0.7828 (2.19)*	-0.1309 (0.88)*	0.2505 (1.28)*
Segment 12	na	-0.5459 (0.58)*	1.1263 (3.08)****	0.2856 (1.33)*	-1.3473 (0.26)*	1.1215 (3.07)***
Segment 13	na	-0.6863 (0.50)***	2.7873 (16.24)****	-0.9771 (0.38)*	0.8286 (2.29)*	-0.2283 (0.80)*
VRU (ref = no)						
Yes	1.0217 (2.78)****	1.2739 (3.57)****	-0.6937 (0.50)****	na	na	na
Exclusive right (ref = no)						
Yes	-0.1518 (0.86)***	na	na	na	na	na
Speed limit (ref = 90)						
50	-0.6209 (0.54)****	-0.6153 (0.54)****	na	na	na	1.1511 (3.16)****
70	0.1664 (1.18)*	0.1513 (1.16)*	na	na	na	-0.5889 (0.55)****
Cycle facility (ref = grade separated)						
Mixed traffic	na	na	na	na	na	1.4599 (4.31)****
Adjacent	na	na	na	na	na	-0.3425 (0.71)*
Separated	na	na	na	na	na	-0.1700 (0.84)*

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TABLE 3 (continued) Factors Influencing Probability of Signalized Intersection Crash Types

Variable	Logistic Regression Results at Crash Level ($N = 1,295$)					
	Injury Crashes ^a According to Crash Type ($Y = 699$)	Injury Crashes ^a According to Crash Location ($Y = 699$)	Rear-End Crashes ($Y = 471$)	Side Crashes ^b ($Y = 351$)	Head-On Crashes ($Y = 181$)	VRU Crashes ^c ($Y = 268$)
Signal phasing (ref = permitted)						
Protected only and protected/permitted	-0.2325 (0.79)***	-0.2232 (0.80)***	na	na	na	na
Protected only	na	na	-0.2677 (0.77)**	na	-0.7673 (0.46)****	-0.5139 (0.60)***
Protected/permitted	na	na	0.514 (1.67)****	na	-0.0103 (1.00)*	-0.3845 (0.68)***
Arms (ref = 4)						
3	na	na	0.3497 (1.42)****	na	na	na
Lanes (ref = 4)						
1	na	na	0.015 (1.02)	na	na	0.1538 (1.17)*
2	na	na	-0.7966 (0.45)****	na	na	-0.3603 (0.70)***
3	na	na	0.0113 (1.01)*	na	na	0.6654 (1.95)****
Median (ref = no)						
Yes	na	na	-0.4030 (0.67)****	na	-0.1582 (0.85)***	na
Built-up area (ref = no)						
Yes	na	na	na	0.2423 (1.27)****	-0.3889 (0.68)****	na
RLC (ref = no)						
Yes	na	na	na	-0.1814 (0.83)***	-0.4832 (0.62)****	-0.4089 (0.66)****
Crash severity (ref = slightly injured)						
Unharmful	na	na	na	na	na	-2.8083 (0.07)****
Dead	na	na	na	na	na	1.6745 (5.34)***
Severely injured	na	na	na	na	na	1.0825 (2.95)****
Hosmer and Lemeshow test	$\chi^2 = 8.9597$ (df = 8, $p = .3457$)	$\chi^2 = 6.8137$ (df = 8, $p = .5569$)	$\chi^2 = 3.5617$ (df = 8, $p = .8943$)	$\chi^2 = 7.7375$ (df = 8, $p = .4595$)	$\chi^2 = 10.4146$ (df = 8, $p = .2371$)	$\chi^2 = 14.9971$ (df = 8, $p = .0592$)
Nagelkerke R^2	.3087	.2747	.6332	.4602	.5005	.5950

NOTE: Values present parameter estimates of logistic regression model. For categorical variables with more than two categories, the category is indicated. Hosmer–Lemeshow goodness-of-fit test indicates good fit for all models. Nagelkerke statistic indicates error reduction of model in percentage (0.3087 is equal to error reduction of 30.87%). Odds ratios are shown in parentheses; odds ratios that are significant at $p \leq .05$ are in bold type. VRU = vulnerable road user; RLC = red light camera; Y = number of “Yes” cases in logistic model; df = degrees of freedom; na = not applicable.

^aBecause of convergence problems, the variables “crash type” and “segment” could not be inserted in one model.

^bSide crashes consist of left-turn and right-angle crashes.

^cVRU crashes = crashes in which at least one cyclist, motorcyclist, moped rider, or pedestrian is involved.

* $p > .10$ [not significant at 90% confidence interval (CI)]; ** $p \leq .10$ (significant at 90% CI); *** $p \leq .05$ (significant at 95% CI); **** $p \leq .01$ (significant at 99% CI).

The results of the logistic regression models were not able to reveal all the characteristics of the dominant crash types. No meaningful models could be fit for sideswipe ($N = 121$) and single-vehicle ($N = 130$) crashes. However, occurrence of sideswipe crashes is significantly more likely on the left-turn lane in Segment 3 [$\chi^2(1, N = 1,295) = 62.734, p < .0001$] and on Segment 10 where the vehicles from the bypass merge with oncoming traffic [$\chi^2(1, N = 1,295) = 18.729, p < .0001$], whereas Segment 1 before the intersection [$\chi^2(1, N = 1,295) = 8.846, p = .0003$], Segment 8 after the intersection [$\chi^2(1, N = 1,295) = 30.747, p < .0001$], Segment 9 after the intersection [$\chi^2(1, N = 1,295) = 31.801, p < .0001$], and Segment 12 on the bypass [$\chi^2(1, N = 1,295) = 7.088, p = .016$] are characterized by significantly more single-vehicle crashes. The results of the descriptive statistics also revealed that occurrence of rear-end and sideswipe crashes is significantly more likely at red-light-camera signalized intersections, whereas single-vehicle, pedestrian, head-on, and side crashes dominate signalized intersections without a red-light camera [$\chi^2(5, N = 1,295) = 66.986, p < .0001$]. Significantly more crashes occur before the intersection (Segments 2 and 3) and on or near the bypass (Segments 10 to 13) for signalized intersections with a red-light camera, whereas signalized intersections without a red-light camera are characterized by significantly more crashes at Segment 1 before the intersection and Segments 4 to 9 on and after the intersection [$\chi^2(12, N = 1,295) = 57.940, p < .0001$].

DISCUSSION OF RESULTS

The current study used an in-depth crash location approach based on crash data and collision diagrams to analyze crash patterns at signalized intersections. The collision diagram information has proved to be essential and valuable for this purpose since these diagrams not only allow the definition of dominant crash types but also show the precrash maneuvers and provide detailed information about the crash location on the signalized intersection. This crash location information was used to define 13 detailed signalized intersection segments that enabled categorization of the crash locations. This crash location approach in combination with the identification of dominant crash types and causal crash factors provide valuable insights into the nature of signalized intersection crashes and the safety impact of signalized intersection design.

Six crash types are identified of which four can be regarded as dominant signalized intersection crash types: rear-end, side, vulnerable road user, and head-on crashes. These results are more or less in line with the existing literature (5–8), but the earlier studies identified sideswipe instead of head-on crashes as the fourth dominant crash type. Except for rear-end crashes, these crash types are also characterized by higher-than-average crash severity levels. Single-vehicle crashes also appear to result in fewer injury crashes. Since more trucks are involved in this crash type [$\chi^2(1, N = 2,652) = 4.338, p = .037$], the lower crash severity levels can be accounted for by the higher mass of the truck, which protects the truck driver from serious injuries.

In addition, the results show that the crash location is related to certain signalized intersection segments. Rear-end collisions mostly occur on the entry lanes (Segments 1 to 3), possibly indicating differences in braking behavior between road users because of conflicting decisions in the dilemma zone. This relation between crash type and crash location on the intersection is supported by the results of another study (30), which indicated that rear-end crashes are the most common crash type at signalized intersections since the diversity

of actions taken increases because of the signal change. Inattentive driving by following drivers, differences between vehicles in braking performance, and following too closely at the time of a signal change are identified as specific causes of rear-end crashes (38–40).

Since rear-end crash occurrence is related to a signal change, the presented crash pattern on the entry lanes is plausible because drivers need to be confronted with the traffic signals in order to make a conflicting decision that can result in a rear-end crash. The bypass is also prone to more rear-end crashes, which can be caused by drivers yielding to vulnerable road users on the crossing facility (Segment 12) or stopping to find a gap to merge with the oncoming traffic (Segment 13). Since both situations result in braking movements, differences between drivers' braking performance and inattentiveness also result in more rear-end crashes at these locations.

Given this crash pattern, signalized intersections should be designed to be sufficiently conspicuous. The visibility of the intersection, traffic signals, or both should be improved for approaching drivers to increase their awareness. Improvements in signal coordination and optimization of change intervals also lead to a decrease in rear-end crashes (8). Segments 4 to 6 are dominated by side and head-on crashes. Possibly, these crashes are the result of red-light-running drivers approaching the intersection from opposite directions, loss of control, or left-turning vehicles that are not yielding to oncoming vehicles during the permissive phase. In their observational study, Gstalter and Fastenmeier (20) found that drivers make most errors when turning left at a signalized intersection. Therefore, driver errors can be related to the crashes in Segment 6. This finding emphasizes the importance of clear road design concepts that are easily understandable for road users, the so-called self-explaining roads. Since these crashes take place between crossing road users or road users approaching each other from opposite directions, it is expected that they occur on the intersection plane. It is well known that these crashes are above all the result of red-light running or unprotected left-turn phasing. As a result, possible countermeasures include the implementation of protected left-turn phasing and red-light cameras even though the latter measure gives rise to increases in rear-end crashes.

Additional measures such as improvements in sight distance, signal coordination, and change intervals also result in fewer head-on and side crashes (8). Side crashes between vehicles and crossing cyclists and mopeds also characterize Segments 7 and 8. Crossing the signalized intersection after the intersection plane and on the bypass seems to be more dangerous for vulnerable road users since they prevail in crashes at Segments 7, 8, and 12. In general, motorists are more focused on other motorists than on vulnerable road users. Most likely, this aspect played a role in these crashes. Furthermore, conflicts between vulnerable road users and motorized vehicles still occur frequently at signalized intersections when they are not fully protected by the signal phasing (i.e., vulnerable road users have the same green phase as the turning traffic). As such, potential countermeasures for vulnerable road user crashes include the implementation of protected phasing for vulnerable road users at the crossing facilities and improved visibility for drivers approaching the crossing facilities.

The type of signal phasing influences the proportion of certain crash types. Similar to findings by De Pauw et al. (16) and Srinivasan et al. (17), protected-only and protected/permissive left-turn signal phasing decrease the proportion of injury and vulnerable road user crashes. Srinivasan et al. found that protected-only phasing decreases rear-end crashes whereas protected/permitted left-turn signal phasing increases rear-end crashes; this finding is similar to the results presented here (17). Possibly, protected/permitted left-turn signal

phasing still results in braking or stopping maneuvers from left-turning vehicles waiting to select gaps in the opposing traffic. Protected-only signal phasing also decreases the occurrence of head-on crashes since this signal phasing type prevents possible conflicts between road users.

In line with previous studies (13, 14), red-light cameras at signalized intersections are associated with lower proportions of side and vulnerable road user crashes. The presence of red-light cameras also gives rise to fewer head-on crashes since these cameras prevent red-light running. However, χ^2 -tests also indicated that red-light cameras result in adverse effects since they lead to increases in the number of rear-end crashes. Probably, red-light cameras cause drivers to brake more abruptly in the dilemma zone since these cameras lead to higher stopping propensities (41). As a result, conflicting decisions in the dilemma zone have a higher chance to result in rear-end crashes.

The presence of a median results in a lower proportion of head-on crashes. Another study indicated that a median prevents vehicles from crossing into the path of oncoming traffic leading to fewer head-on crashes (12). Speed limits are significant for the proportion of injury crashes with an indication that higher speeds lead to higher crash severity. Similar to a study by Steinman and Hines, the proportion of vulnerable road user crashes is also affected by the speed limit at the signalized intersection (42).

At signalized intersections where cycle traffic is mixed with motorized traffic, the proportion of vulnerable road user crashes is higher. However, these differences in crash susceptibility may also be related to different cyclist volumes at the cycle facilities. Because of the lack of traffic volume data for cyclists, this hypothesis could not be tested. Elvik et al. support this hypothesis; they found that the reduction of bicycle crashes is smaller at signalized intersections with cycle lanes since cycle lanes attract more cyclists and may give rise to increased speeds among cyclists (4). In line with research by Torbic et al., the proportion of vulnerable road user crashes increases with the number of lanes (19).

One limitation of the current study concerns the sample. The sample of signalized intersections used ($N = 87$) could be a somewhat biased representation of a larger (i.e., countrywide) signalized intersection population in the sense that only intersections where at least one crash was registered for each year and where detailed crash data were available were included. A possible bias associated here is a slight overrepresentation of intersections with higher numbers of crashes. However, the objective of the study was not to make inferences about the performance of signalized intersections compared with each other but to identify crash types, locations, and factors that are associated with signalized intersection crashes. The collected sample of 1,295 complete crash records can be considered valid for that purpose.

The next issue is the accuracy of the crash allocation. The crash location typology used to allocate the crashes to the different segments is based on simplified rules. By following this typology, the allocation of the crashes to the different segments does not fully correspond to the actual location of the crash. Despite this inconsistency, the allocation is still quite accurate since the typology is based on the impact point, the precrash orientation of the road users, and the maneuver that the road users make (i.e., the most important characteristics to reconstruct a crash). The objective of the study was not to duplicate an exact replica of each crash location but to provide insights into the crash patterns of dominant signalized intersection crashes. The developed crash location typology is assumed to be valid for this purpose since the reported crash location in the collision diagrams may also slightly deviate from the actual crash

location. To ensure a consistency of 100% in both crash locations, advanced in-depth crash research such as crash reconstruction techniques is required. Since most police districts in Belgium are not familiar with these techniques, the results are not greatly affected by this variation.

Another point of discussion is the cross-section design of the study. According to Hauer, causality cannot be reliably inferred from cross-section designs since cross-section studies compare intersections with a certain characteristic with other intersections with another characteristic (43). Therefore, this study design lacks the continuity in which the intersection remains the same. The possibility of confounding factors between the different intersections is not eliminated since this requires information about why a certain characteristic is present at one intersection and is absent at another (43). Since this information is often not available and is difficult to account for but is required to draw cause-and-effect conclusions from cross-section data (43), the presence of a correlation between the proportion of crashes (the dependent variable) and certain intersection characteristics (the independent variables) is not sufficient to conclude that there is a causal relationship between both variables.

Finally, traffic flow count data were only available for 54 of 87 signalized intersections. Previous studies indicated that AADT (26, 44, 45) is a critical variable for crash analysis. However, this requirement only applies to studies that aim to explain the variation in road safety performance of a sample of locations by identifying the influence of design characteristics on the level of safety. The focus of the current study is to explore the crash location of dominant crash types at a typical signalized intersection. To fulfil this objective, crash data of intersections with missing AADT values can be used since AADT as such is not a crucial variable to define the crash location. Because this study does not predict crashes but explores available crash data by delineating the crash location on the signalized intersection itself, the lack of an AADT value does not present any analysis issues.

An important advantage of the crash location approach is the generalizability. The presented approach is based on a sort of "maximal design" representing a typical signalized intersection layout with some extra features that are not necessarily always present but are quite common. Since the intersection layout and characteristics may vary, the approach can easily be adjusted to different designs and locations by tailoring the segments to the specific intersection or location layout in question and by adding the inherent characteristics that play a role in the crashes to the typology. For example, if researchers want to study the safety difference between signalized intersections and signed intersections (i.e., controlled with stop or yield signs), they can simply add this feature to the typology.

This approach is also a useful context for exploring intersection safety since it combines crash data with collision diagram information. As such, this method combines basic in-depth crash analysis with the benefits of aggregated crash analysis, leading to more reliable quantitative analysis. As a result, a more detailed insight is gained in the development and occurrence of crash types by relating crash occurrence with design characteristics of the signalized intersection. This insight is needed to assess the safety impact and possible safety issues of this intersection design, which is necessary to select the appropriate countermeasures to decrease crashes.

CONCLUSIONS

The main goal of this study was to identify and analyze dominant crash types at signalized intersections by taking detailed information on the crash location into account. Some connections between

certain signalized intersection crash types, their crash location, and signalized intersection design characteristics have been found:

- Four dominant crash types occur at signalized intersections: Rear-end, side, vulnerable road user, and head-on crashes. Except for rear-end crashes, these crash types are also characterized by higher-than-expected crash severity levels.
- The crash location of these dominant crash types is related to specific signalized intersection segments: Rear-end crashes occur mostly before the intersection or on the bypass, side and head-on crashes mostly take place on and near the intersection plane, and vulnerable road user crashes occur predominantly at the crossing facilities after the intersection plane or on the bypass.
- Protected-only and protected-permissive left-turn signal phasing, exclusive turn lanes, and 50-km/h speed limits are associated with lower proportions of injury crashes.
- Characteristics associated with higher proportions of rear-end crash types are protected-permitted left-turn signal phasing and red-light cameras.
- Lower proportions of head-on crashes are associated with red-light cameras, protected-only left-turn signal phasing, and medians.
- Red-light cameras are associated with lower proportions of side crashes.
- Lower proportions of vulnerable road user crashes are associated with red-light cameras and protected-only and protected/permissive left-turn signal phasing.
- Intersection features combined with detailed signalized intersection segments as a proxy for the crash location features provide valuable insights into the nature of signalized intersection crashes and the safety impact of signalized intersection design.

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REFERENCES

1. Roess, R. P., E. S. Prassas, and W. R. McShane. *Traffic Engineering*. Pearson Education, Inc., New York, 2011.
2. Nuytens, N., A. Carpentier, K. Declercq, and E. Hermans. *Jaarrapport Verkeersveiligheid 2012: Analyse van verkeersveiligheidsindicatoren in Vlaanderen tot en met 2012 [Annual Road Safety Report 2012: Analysis of Road Safety Indicators in Flanders (Belgium) until 2012]* (in Dutch). Policy Research Centre for Traffic Safety and Belgian Road Safety Institute, Flanders, Belgium, 2014.
3. Ogden, K. W. *Safer Roads: A Guide to Road Safety Engineering*. Ashgate Publishing Ltd., Melbourne, Australia, 1996.
4. Elvik, R., A. Høye, T. Vaa, and M. Sørensen. *The Handbook of Road Safety Measures*. Emerald Group Publishing Limited, Bingley, United Kingdom, 2009.
5. Chandler, B. E., M. C. Myers, J. E. Atkinson, T. E. Bryer, R. Retting, J. Smithline, J. Trim, P. Wojkiewicz, G. B. Thomas, S. P. Venglar, S. Sunkari, B. J. Malone, and P. Izadpanah. *Signalized Intersections Informational Guide*, 2nd ed. Publication FHWA-SA-13-027. FHWA, U.S. Department of Transportation, 2013.
6. Abdel-Aty, M., C. Lee, X. Wang, J. Keller, S. Kowdla, and H. Prasad. *Identification of Intersection's Crash Profiles/Patterns*. Department of Civil and Environmental Engineering, University of Central Florida, Orlando, 2006.
7. Ogden, K. W., and S. W. Newstead. *Analysis of Crash Patterns at Victorian Signalized Intersections*. Publication No. 60. Accident Research Centre, Monash University, Melbourne, Victoria, Australia, 1994.
8. Antonucci, N. D., K. Kennedy Hardy, K. L. Slack, R. Pfefer, and T. R. Neuman. *NCHRP Report 500: Volume 12: A Guide for Reducing Collisions at Signalized Intersections*. Transportation Research Board of the National Academies, Washington, D.C., 2004.
9. Abdel-Aty, M., and J. Keller. Exploring the Overall and Specific Crash Severity Levels at Signalized Intersections. *Accident Analysis and Prevention*, Vol. 37, No. 3, 2005, pp. 417–425.
10. Ye, X., R. M. Pendyala, F. S. Al-Rukaibi, and K. Konduri. Joint Model of Accident Type and Severity for Two-Vehicle Crashes. Presented at 87th Annual Meeting of the Transportation Research Board, Washington, D.C., 2008.
11. Wang, X. *Safety Analysis at Signalized Intersections Considering Spatial, Temporal and Site Correlation*. PhD dissertation. University of Central Florida, Orlando, 2006.
12. Keller, J., M. Abdel-Aty, and P. Brady. Type of Collision and Crash Data Evaluation at Signalized Intersections. *ITE Journal*, Vol. 76, No. 2, 2006, pp. 30–39.
13. De Pauw, E., S. Daniels, T. Brijs, E. Hermans, and G. Wets. To Brake or to Accelerate? Safety Effects of Combined Speed and Red Light Cameras. *Journal of Safety Research*, Vol. 50, 2014, pp. 59–65.
14. Høye, A. Still Red Light for Red Light Cameras? An Update. *Accident Analysis and Prevention*, Vol. 55, 2013, pp. 77–89.
15. Shin, K., and S. Washington. The Impact of Red Light Cameras on Safety in Arizona. *Accident Analysis and Prevention*, Vol. 39, No. 6, 2007, pp. 1212–1221.
16. De Pauw, E., S. Daniels, T. Brijs, E. Hermans, and G. Wets. The Effect of Protected Left-Turn Signals on Traffic Safety. Presented at 26th Workshop of the International Cooperation on Theories and Concepts in Traffic Safety, Maribor, Slovenia, 2013.
17. Srinivasan, R., C. Lyon, B. Persaud, J. Baek, F. Gross, S. Smith, and C. Sundstrom. Crash Modification Factors for Changes to Left-Turn Phasing. In *Transportation Research Record: Journal of the Transportation Research Board*, No. 2279, Transportation Research Board of the National Academies, Washington, D.C., 2012, pp. 108–117.
18. Garder, P., L. Leden, and T. Thedeen. Safety Implications of Bicycle Paths at Signalized Intersections. *Accident Analysis and Prevention*, Vol. 26, No. 4, 1994, pp. 429–439.
19. Torbic, D. J., D. W. Harwood, C. D. Bokenkroger, R. Srinivasan, D. Carter, C. V. Zegeer, and C. Lyon. Pedestrian Safety Prediction Methodology for Urban Signalized Intersections. In *Transportation Research Record: Journal of the Transportation Research Board*, No. 2198, Transportation Research Board of the National Academies, Washington, D.C., 2010, pp. 65–74.
20. Gstalter, H., and W. Fastenmeier. Reliability of Drivers in Urban Intersections. *Accident Analysis and Prevention*, Vol. 42, No. 1, 2010, pp. 225–234.
21. Retting, R. A., H. B. Weinstein, and M. G. Solomon. Analysis of Motor-Vehicle Crashes at Stop Signs in Four U.S. Cities. *Journal of Safety Research*, Vol. 34, No. 5, 2003, pp. 485–489.
22. Polders, E., S. Daniels, W. Casters, and T. Brijs. Identifying Crash Patterns on Roundabouts. *Traffic Injury Prevention*, Vol. 16, No. 2, 2015, pp. 202–207.
23. McCart, A. T., V. S. Northrup, and R. A. Retting. Types and Characteristics of Ramp-Related Motor Vehicle Crashes on Urban Interstate Roadways in Northern Virginia. *Journal of Safety Research*, Vol. 35, No. 1, 2004, pp. 107–114.
24. Khattak, A. J., and F. Targa. Injury Severity and Total Harm in Truck-Involved Work Zone Crashes. In *Transportation Research Record: Journal of the Transportation Research Board*, No. 1877, Transportation Research Board of the National Academies, Washington, D.C., 2004, pp. 106–116.
25. Kweon, Y. Crash Data Sets and Analysis. In *Handbook of Traffic Psychology*, Academic Press, London, 2011, Chap. 8, pp. 97–105.
26. Reurings, M., T. Janssen, R. Eenink, R. Elvik, J. Cardosa, and C. Stefan. *Accident Prediction Models and Road Safety Impact Assessment: A State-of-the-Art*. Publication Deliverable No. 1. Ripcord-Iserest Programme (Road Infrastructure Safety Protection—Core-Research and Development for Road Safety in Europe; Increasing Safety and Reliability of Secondary Roads for a Sustainable Surface Transport), Federal Highway Research Institute, Bergisch Gladbach, Germany, 2006.
27. Nambuusi, B. B., T. Brijs, and E. Hermans. *A Review of Accident Prediction Models for Road Intersections*. Publication RA-MOW-2008-004. Policy Research Centre for Traffic Safety, Flanders, Belgium, 2008.

28. Massie, D., K. Campbell, and D. Blower. Development of a Collision Typology for Evaluation of Collision Avoidance Strategies. *Accident Analysis and Prevention*, Vol. 26, No. 3, 1993, pp. 241–257.
29. Al-Ghamdi, A. S. Using Logistic Regression to Estimate the Influence of Accident Factors on Accident Severity. *Accident Analysis and Prevention*, Vol. 34, 2002, pp. 729–741.
30. Yan, X. D., E. Radwan, and M. Abdel-Aty. Characteristics of Rear-End Accidents at Signalized Intersections Using Multiple Logistic Regression Model. *Accident Analysis and Prevention*, Vol. 37, No. 6, 2005, pp. 983–995.
31. Yau, K. K. W. Risk Factors Affecting the Severity of Single Vehicle Traffic Accidents in Hong Kong. *Accident Analysis and Prevention*, Vol. 36, No. 3, 2004, pp. 333–340.
32. Zhang, J., J. Lindsay, K. Clarke, G. Robbins, and Y. Mao. Factors Affecting the Severity of Motor Vehicle Traffic Crashes Involving Elderly Drivers in Ontario. *Accident Analysis and Prevention*, Vol. 32, No. 1, 2000, pp. 117–125.
33. Chen, H. H., L. Cao, and D. B. Logan. Analysis of Risk Factors Affecting the Severity of Intersection Crashes by Logistic Regression. *Traffic Injury Prevention*, Vol. 13, No. 3, 2012, pp. 300–307.
34. Allison, P. D. *Logistic Regression Using SAS: Theory and Application*. SAS Institute Inc., Cary, N.C., 1999.
35. Field, A. *Discovering Statistics Using SPSS*. SAGE Publications Ltd., London, 2009.
36. Heinze, G., and M. Schemper. A Solution to the Problem of Separation in Logistic Regression. *Statistics in Medicine*, Vol. 21, No. 16, 2002, pp. 2409–2419.
37. O'Brien, R. M. A Caution Regarding Rules of Thumb for Variance Inflation Factors. *Quality and Quantity*, Vol. 41, No. 5, 2007, pp. 673–690.
38. Abdel-Aty, M., and H. Abdelwahab. Modeling Rear-End Collisions Including the Role of Driver's Visibility and Light Truck Vehicles Using a Nested Logit Structure. *Accident Analysis and Prevention*, Vol. 36, No. 3, 2004, pp. 447–456.
39. Strandberg, L. Winter Braking Tests with 66 Drivers, Different Tires and Disconnectable ABS. Presented at International Workshop on Traffic Accident Reconstruction, Tokyo, 1998.
40. Sayer, J., M. Mefford, and R. Huang. *The Effect of Lead-Vehicle Size on Driver Following Behavior*. Technical Report UMTRI-2000-15. Transportation Research Institute, Michigan University, Ann Arbor, 2000.
41. Lum, K. M., and Y. D. Wong. A Before-and-After Study of Driver Stopping Propensity at Red Light Camera Intersections. *Accident Analysis and Prevention*, Vol. 35, No. 2, 2003, pp. 111–120.
42. Steinman, N., and K. Hines. Methodology to Assess Design Features for Pedestrian and Bicyclist Crossings at Signalized Intersections. In *Transportation Research Record: Journal of the Transportation Research Board*, No. 1878, Transportation Research Board of the National Academies, Washington, D.C., 2004, pp. 42–50.
43. Hauer, E. Cause, Effect and Regression in Road Safety: A Case Study. *Accident Analysis and Prevention*, Vol. 42, No. 4, 2010, pp. 1128–1135.
44. Chin, H., and M. Quddus. Applying the Random Effect Negative Binomial Model to Examine Traffic Accident Occurrence at Signalized Intersections. *Accident Analysis and Prevention*, Vol. 35, No. 2, 2003, pp. 153–159.
45. Liu, P., and H. Young. Neural Network Approach on Studying the Effect of Urban Signalized Intersection Characteristics on Occurrence of Traffic Accidents. Presented at 83rd Annual Meeting of the Transportation Research Board, Washington, D.C., 2004.

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