# Calcul numérique des solides et structures non linéaires

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### Third course

# Introduction to Non linear elasticity

### Some characteristics of non linear mechanics

- In non linear mechanics, stresses and problems can be defined on the deformed or reference configurations while for linear problems everything is defined on the reference configuration
- This leads to several stress tensors
- The stress tensors are generally obtained from the strain by a non linear relation
- Problems can only be solved by iterative process

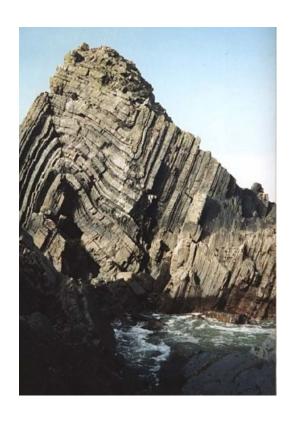
### Overview

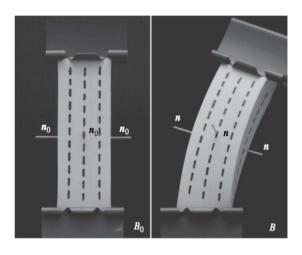
- 1. Kinematics
- 2. Stress tensors
- 3. Derivatives
- 4. Constitutive laws
- 5. Hyperelastic materials
- 6. Minimisation of the potential energy
- 7. Linearisation
- 8. FEM implementation
- 9. References

# 1. Kinematics

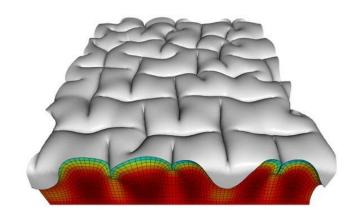
### Large deformations

Large strain and large stress

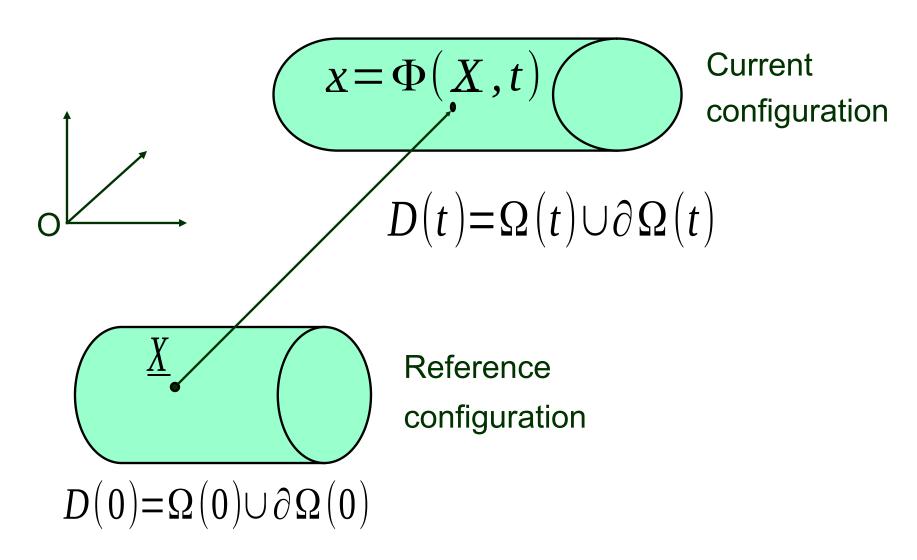




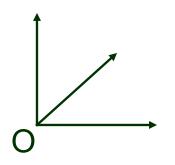
Hyperelastic blocs in large deformations

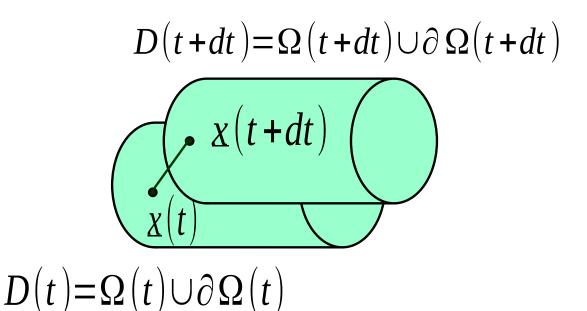


### Lagrangian description



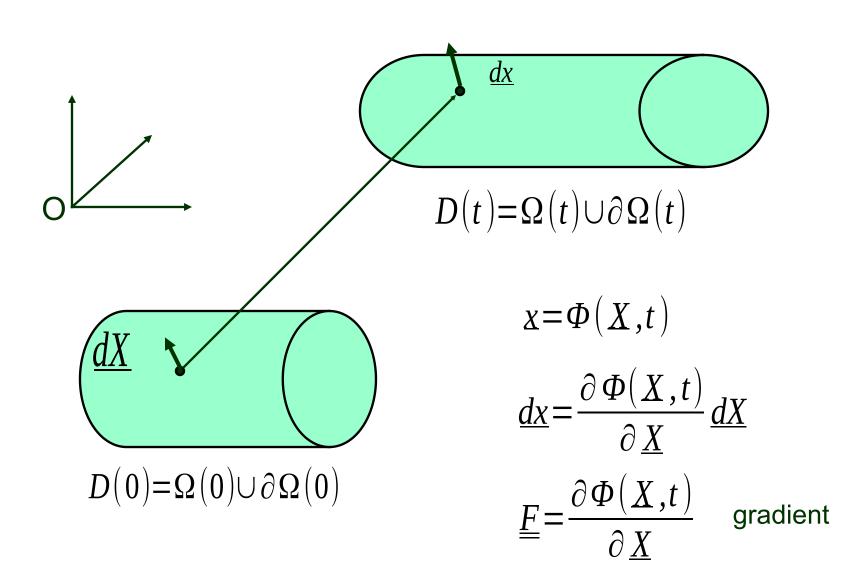
### Eulerian description





Velocity U(x,t) defined on the current configuration D(t)

### Gradient of the transformation (Lagrange)



### Strain tensors

$$\underline{\underline{X}} = \underline{\underline{X}} + \underline{\underline{\xi}} \quad (=\Phi(X,t))$$

$$\underline{\underline{F}} = \underline{\underline{1}} + \underline{\underline{\nabla}}\underline{\underline{\xi}} \quad \text{displacement} \quad \underline{\underline{\xi}}$$

$$\underline{\underline{C}} = \underline{\underline{f}}\underline{\underline{F}}.\underline{\underline{F}}$$

Large strain

$$\underline{e} = \frac{1}{2} {}^{t} \underline{E} \cdot \underline{E} - \underline{1}$$

$$= \frac{1}{2} {}^{t} (\underline{1} + \underline{\nabla} \underline{\xi}) \cdot (\underline{1} + \underline{\nabla} \underline{\xi}) - \underline{1}$$

$$= \frac{1}{2} (\underline{\nabla} \underline{\xi} + {}^{t} \underline{\nabla} \underline{\xi} + {}^{t} \underline{\nabla} \underline{\xi} \cdot \underline{\nabla} \underline{\xi} )$$

Small strain

$$\underline{\underline{\nabla \xi}} \ll 1$$

$$\underline{\epsilon} = \frac{1}{2} \left( \underline{\nabla \xi} + ^{t} \underline{\nabla \xi} \right)$$

### Transport of vectors and volumes

$$\underline{dx} = \underline{F} \cdot \underline{dX}$$
 Vector 
$$dV = J(\underline{X}, t) dV_0$$
 Volume 
$$J(\underline{X}, t) = \det(\underline{F}(\underline{X}, t))$$
 
$$\underline{n} da = J^t \underline{F}^{-1} \cdot \underline{N} dA$$
 Surface

### Time derivative

### Lagrangian

$$\underline{v}(\underline{X},t) = \frac{\partial \Phi(X,t)}{\partial t}$$

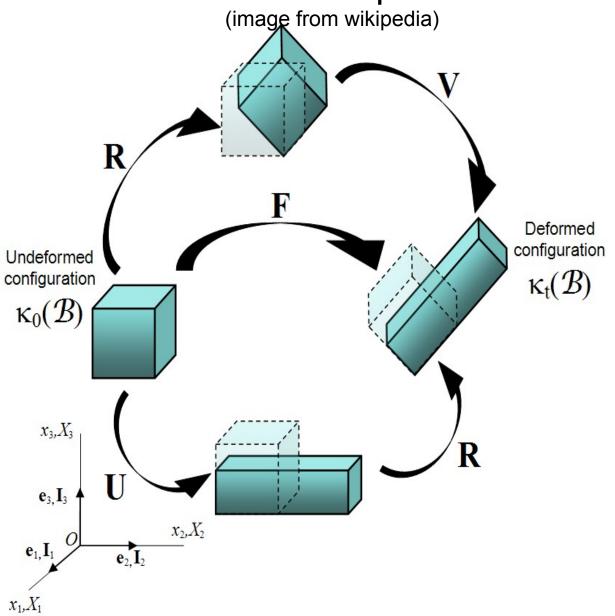
$$\underline{a}(\underline{X},t) = \frac{\partial^2 \Phi(\underline{X},t)}{\partial t^2}$$

### Eulerian

$$\frac{Df(x,t)}{Dt} = \frac{\partial f(x,t)}{\partial t} + \operatorname{grad} f \cdot \underline{v}$$

$$\frac{D v(x,t)}{Dt} = \frac{\partial v(x,t)}{\partial t} + \operatorname{grad} v.v$$

### Polar Decomposition



Separate deformation from rigid-body rotation

Unique decomposition of deformation gradient

$$\underline{\underline{F}} = \underline{\underline{R}} \cdot \underline{\underline{U}} = \underline{\underline{V}} \cdot \underline{\underline{R}}$$

<u>R</u>: orthogonal tensor (rigid-body rotation)

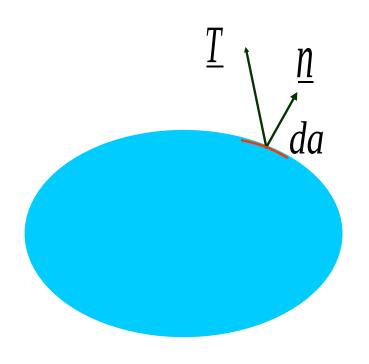
<u>U</u>, <u>V</u>: right- and left-stretch tensor (symmetric)

 $\underline{\underline{U}}$  and  $\underline{\underline{V}}$  have the same eigenvalues but different eigenvectors

## 2. Stress tensors

# Stress tensor: a tool to compute a force vector from a surface element

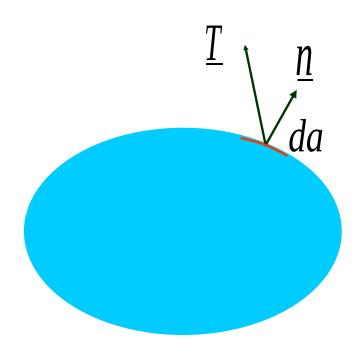
Force of the exterior domain on the surface element da



The force and surface element can be in the reference or current configurations

=> different stress tensors

# Cauchy stress tensor $\underline{\underline{0}}$



Force and normal in the current configuration

$$\underline{T}$$
  $da = \underline{\sigma} . \underline{n} da$   
 $\underline{\sigma}$  symmetric

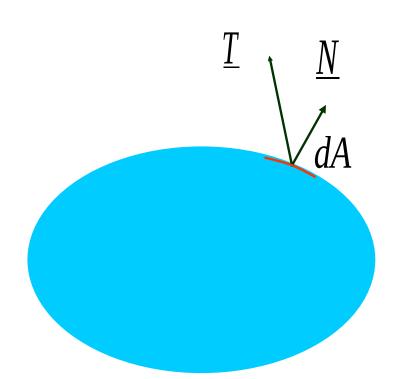
Equilibrium equations

$$\operatorname{div} \underline{\sigma} + \underline{b} = 0 \quad \text{in } \Omega$$

$$\underline{\sigma} \cdot \underline{n} = \underline{g} \quad \text{on } \partial \Omega_g$$

One must note that the problem is written on the current configuration which is unknown

### First Piola-Kirchhoff stress tensor P



Equilibrium equations

DIV 
$$\underline{\underline{P}} + \underline{b}_0 = 0$$
 in  $\Omega_0$   
 $\underline{\underline{P}} \cdot \underline{\underline{N}} = \underline{g}_0$  on  $\partial_g \Omega_0$ 

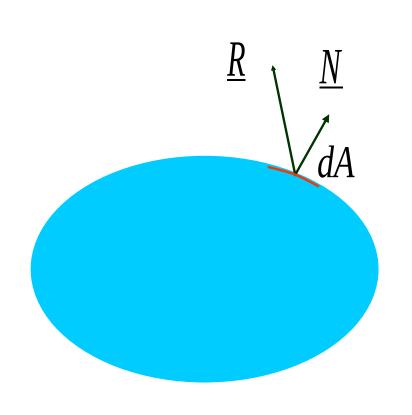
The reference configuration is known

Force in the current configuration and normal in the reference configuration

$$\underline{\underline{T}} da = \underline{\underline{P}} \cdot \underline{\underline{N}} dA$$

P Not symmetric

### Second Piola-Kirchhoff stress tensor <u>S</u>



Force and normal in the reference configuration

$$\underline{R} dA = \underline{S} \cdot \underline{N} dA$$

$$\underline{\underline{S}}$$
 symmetric

### Relations between these tensors

$$\begin{cases} \underline{dx} = \underline{\underline{F}} \cdot \underline{dX} \\ \underline{n} \, da = J^t \, \underline{\underline{F}}^{-1} \cdot \underline{N} \, dA \end{cases}$$

Transport relations

$$\begin{cases} R dA = \underline{S} \cdot N dA = \underline{F}^{-1} \cdot T da = J \underline{F}^{-1} \cdot \underline{\sigma} \cdot \underline{F}^{-1} N dA \\ \underline{S} = J \underline{F}^{-1} \cdot \underline{\sigma} \cdot \underline{F}^{-1} \end{cases}$$

$$\underbrace{P.\underline{N} dA = \underline{T} da = \underline{F}.\underline{R} dA = \underline{F}.\underline{S}.\underline{N} dA}_{P = \underline{F}.\underline{S}}$$

Tensor	Symbol	force	aera	symmetry
Cauchy	<u><u></u></u>	Current configuration	Current configuration	Yes
First Piola-Kirchhoff stress tensor (also Boussinesq tensor)	<u>P</u>	Current configuration	Reference configuration	No
Second Piola-Kirchhoff stress tensor (also Piola tensor)	<u>S</u>	Reference configuration	Reference configuration	Yes

# 3. Derivatives

For a function

$$f(x+dx)=f(x)+\frac{df}{dx}.dx+O(|dx|^2)$$

Almost the same for a tensor:

The derivative with respect to a tensor is defined such that

$$f\left(\underline{e} + d\underline{e}\right) = f\left(\underline{e}\right) + \left(\frac{\partial f}{\partial \underline{e}}\right) : d\underline{e} + O(|\underline{e}|^2)$$

 $\frac{\partial f}{\partial e}$  is the derivative of the function f with respect to the tensor  $\underline{e}$ 

### Examples

Derivative of a square (if e symmetric)

$$(\underline{e} + d\underline{e}) : (\underline{e} + d\underline{e}) = \underline{e} : \underline{e} + 2\underline{e} : d\underline{e} + d\underline{e} : d\underline{e}$$

$$\frac{\partial f}{\partial \underline{e}} = 2\underline{e}$$

Derivative of a determinant

$$\det (\underline{a} + d\underline{a}) = \det (\underline{a}. (\underline{1} + \underline{a}^{-1}. d\underline{a}))$$

$$= \det (\underline{a}). \det (\underline{1} + \underline{a}^{-1}. d\underline{a})$$

$$= \det (\underline{a}). (1 + Tr (\underline{a}^{-1}. d\underline{a}) + o(|d\underline{a}|^2))$$

$$= \det (\underline{a}) + \det (\underline{a}). (\underline{a}^{-1}: d\underline{a}) + o(|d\underline{a}|^2)$$

$$\frac{\partial \det (\underline{a})}{\partial \underline{a}} = \det (\underline{a}).^t \underline{a}^{-1}$$

### Derivatives of the invariants of a second order tensor

The three invariants of a second order tensor A are

$$\begin{split} I_{1}(\underline{A}) &= tr(\underline{A}) \\ I_{2}(\underline{A}) &= \frac{1}{2} \big[ (tr(\underline{A}))^{2} - Tr(\underline{A}^{2}) \big] \\ I_{3}(\underline{A}) &= det(\underline{A}) \end{split}$$

Their derivatives are

$$\begin{array}{rcl} \frac{\partial \, I_1}{\partial \, \underline{A}} & = & \underline{1} \\ \\ \frac{\partial \, I_2}{\partial \, \underline{A}} & = & I_1 \underline{1} - {}^t \underline{A} \\ \\ \frac{\partial \, I_3}{\partial \, \underline{A}} & = & \det(A)^t \underline{A}^{-1} \end{array}$$

# 4. Constitutive laws

\* First principle of thermodynamics

$$\dot{E}_c = \frac{d}{dt} \left( \int_{\Omega} \frac{\rho}{2} v \cdot v \, dx + \int_{\Omega} \rho \, e \, dx \right) = \dot{W} + \dot{Q} \tag{1}$$

 $\dot{E}_c$  rate of variation of total energy

W rate of work done on the system

Q rate of heat recieved by the system

e internal energy

$$\frac{d}{dt} \left( \int_{\Omega} \frac{\rho}{2} \mathbf{v} \cdot \mathbf{v} \, d\mathbf{x} \right) = P_e(\mathbf{v}) + P_i(\mathbf{v}) = \dot{W} + P_i(\mathbf{v}) \tag{2}$$

because  $P_e = \dot{W}$ 

\* One gets

$$-\frac{d}{dt}\left(\int_{\Omega} \rho \, e \, dx\right) + \dot{Q} = P_i(\underline{v}) \tag{2} - (1)$$

<sup>\*</sup> Theorem of kinetic energy

\* Second principle of thermodynamics: Clausius Duhem inequality

$$\int_{\Omega} \rho T \dot{s} dx - \dot{Q} + \int_{\Omega} T q. grad(\frac{1}{T}) \ge 0$$

\* With the last relation of the precedent slide

$$-\frac{d}{dt}\left(\int\limits_{\Omega}\rho\,e\,dx\right)+\int\limits_{\Omega}\rho\,T\,\dot{s}\,dx-P_{i}(\underline{v})+\int\limits_{\Omega}T\,q\,.\,grad\left(\frac{1}{T}\right)\geq0$$

If the temperature is uniform and

• Introducing the free energy  $\psi = e - Ts$ 

$$\text{- Noting that } \quad P_{i}(\underline{v}) \! = \! - \int\limits_{\Omega} \underline{\sigma} \! : \! \underline{d} \, dx \qquad \qquad (d \! = \! sym(\nabla \, \dot{x}))$$

One gets

$$-\rho \dot{\psi} + \underline{\sigma} : \underline{d} \ge 0$$

### One also has

$$\int_{\Omega} \left(\underline{\underline{\sigma}} : \underline{\underline{d}}\right) dx = \int_{\Omega_0} \left(\underline{\underline{S}} : \underline{\underline{\dot{e}}}\right) dx_0$$

### because

$$\underline{\underline{\sigma}} : \underline{\underline{d}} = \underline{\underline{\sigma}} : \left(\frac{\partial \underline{U}}{\partial \underline{x}}\right) \\
= \underline{\underline{\sigma}} : \left(\frac{\partial \underline{U}}{\partial \underline{X}} \cdot \frac{\partial \underline{X}}{\partial \underline{x}}\right) \\
= Tr \left(\underline{\underline{\sigma}} \cdot \underline{\underline{F}}^{-1} \cdot \underline{\underline{F}}\right) \\
= Tr \left(\underline{\underline{F}}^{-1} \cdot \underline{\underline{\sigma}} \cdot \underline{\underline{F}}^{-1} \cdot \underline{\underline{F}} \cdot \underline{\underline{F}}\right) \\
= \left(\underline{\underline{F}}^{-1} \cdot \underline{\underline{\sigma}} \cdot \underline{\underline{F}}^{-1}\right) : \left(\underline{\underline{F}} \cdot \underline{\underline{F}}\right) \\
= \left(\underline{\underline{F}}^{-1} \cdot \underline{\underline{\sigma}} \cdot \underline{\underline{F}}^{-1}\right) : \left(\underline{\underline{F}} \cdot \underline{\underline{F}}\right) \\
= \frac{1}{J} \left(J \underline{\underline{F}}^{-1} \cdot \underline{\underline{\sigma}} \cdot \underline{\underline{F}}^{-1}\right) : \underline{\underline{c}} \\
= \frac{1}{J} \underline{\underline{S}} : \underline{\underline{c}} \\$$

The Clausius Duhem inequality is then (uniform temperature)

$$-\rho_0\dot{\psi}+\underline{\underline{S}}:\underline{\dot{e}}\geq 0$$

$$-\rho_0\dot{\psi}+\underline{\underline{S}}:\underline{\dot{e}}\geq 0$$

With the free energy  $\psi$  function of the deformation tensor  $\underline{e}$ 

$$\left(-\rho_0 \frac{\partial \psi}{\partial \underline{e}} + \underline{S}\right) : \underline{\dot{e}} \ge 0$$

This is possible only if (in case of no internal kinematical condition)

$$\underline{\underline{S}} = \rho_0 \frac{\partial \psi}{\partial \underline{e}}$$

If isothermal and isentropic one also has  $(by\ denoting\ W = \rho_0\psi)$ 

$$\underline{S} = \frac{\partial W}{\partial \underline{e}}$$

One also has

$$\underline{P} = \underline{F} \cdot \underline{S} = \underline{F} \cdot \frac{\partial W}{\partial \underline{e}} = \frac{\partial W}{\partial \underline{F}}$$

The energy should satisfy the growth conditions

$$W(\underline{e}) \rightarrow \infty$$
 when  $J = \det \underline{F} \rightarrow 0^+$   
 $W(\underline{e}) \rightarrow \infty$  when  $J = \det \underline{F} \rightarrow \infty$ 

$$w(\underline{e}) = \frac{\mu}{2} (\underline{F} : \underline{F} - 3) - \mu \ln J + \frac{\lambda}{2} (\ln J)^2 \qquad \text{good}$$

$$w(\underline{e}) = \frac{\lambda}{2} tr(\underline{e})^2 + \mu \, \underline{e} : \underline{e}$$
 Not so good

### Case of isotropic materials

The deformation energy can be written according to the invariants

$$\begin{split} W(\underline{e}) &= W(I_1(\underline{C}), I_2(\underline{C}), I_3(\underline{C})) \qquad \text{(with } \underline{C} = \underline{t}\underline{F} \cdot \underline{F} = 2\left(\underline{e} + \underline{1}\right)) \\ I_1(\underline{C}) &= tr(\underline{C}) = \lambda_1^2 + \lambda_2^2 + \lambda_3^2 \\ I_2(\underline{C}) &= \frac{1}{2} \left[ (tr(\underline{C}))^2 - Tr(\underline{C}^2) \right] = \lambda_1^2 \lambda_2^2 + \lambda_1^2 \lambda_3^2 + \lambda_2^2 \lambda_3^2 \\ I_3(\underline{C}) &= det(\underline{C}) = \lambda_1^2 \lambda_2^2 \lambda_3^2 \end{split}$$

The second Piola-Kirchoff tensor is then

$$\underline{\underline{S}}(\underline{e}) = \frac{\partial W}{\partial \underline{e}} = 2\frac{\partial W}{\partial \underline{C}} = 2\frac{\partial W}{\partial I_1} \frac{\partial I_1}{\partial \underline{C}} + 2\frac{\partial W}{\partial I_2} \frac{\partial I_2}{\partial \underline{C}} + 2\frac{\partial W}{\partial I_3} \frac{\partial I_3}{\partial \underline{C}}$$

$$= 2\left(\frac{\partial W}{\partial I_1} + I_1 \frac{\partial W}{\partial I_2}\right) \underline{1} - 2\underline{C} \frac{\partial W}{\partial I_2} + 2\frac{\partial W}{\partial I_3} I_3 \underline{\underline{C}}^{-1}$$
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### Case of incompressible materials

In this case there is no change of volume

During a deformation the material should satisfy the constraint

$$J^2 = I_3 = (\det \underline{F})^2 = \det \underline{C} = 1$$

A general displacement is such that

$$\frac{\partial \det C}{\partial \underline{C}} : \underline{\dot{C}} = \det(\underline{C})\underline{C}^{-1} : \underline{\dot{C}} = \underline{C}^{-1} : \underline{\dot{C}} = 0$$

Remember the Clausius Duhem inequality

$$\left(-\frac{\partial W}{\partial \underline{e}} + \underline{S}\right) : \underline{\dot{e}} \ge 0$$

But in this case for all displacements satisfying the internal constraint

$$\underline{\underline{C}}^{-1}: \underline{\underline{\dot{C}}} = 0 \qquad (\text{with } \underline{\underline{C}} = \underline{\underline{f}} \cdot \underline{\underline{F}} = 2(\underline{\underline{e}} + \underline{\underline{1}}))$$

$$\underline{S} - \frac{\partial W}{\partial \underline{e}}$$
 can be decomposed as

$$\underline{S} - \frac{\partial W}{\partial \underline{e}} = \underline{S}_0 - p \underline{C}^{-1} \quad \text{with} \quad \underline{S}_0 : \underline{C}^{-1} = 0$$

Taking  $\underline{\dot{e}} = -\underline{\underline{S}}_0$  one sees that  $\underline{\underline{S}}_0 = 0$  and

$$\underline{S} = \frac{\partial W}{\partial \underline{e}} - p \underline{C}^{-1}$$

# 5. Hyperelastic materials

### Some properties

- Stress-strain relationship derives from a strain energy density function
- Stress is independent of history and depends only of the final state
- Different strain energy densities are possible
- Often comes with incompressibility (J = 1)
- Example: rubber, biological tissues

#### Kirchoff-Saint Venant model

Simplest generalisation of linear elastic materials

$$w(\underline{e}) = \frac{\lambda}{2} (Tr(\underline{e}))^2 + \mu Tr(\underline{e} \cdot \underline{e}) = \frac{1}{8} (\lambda + 2\mu) (I_1 - 3)^2 - \frac{\mu}{2} (I_2 - 2I_1 + 3)$$
  
$$S(\underline{e}) = \lambda tr(\underline{e}) 1 + 2\mu \underline{e}$$

Do not use for really large deformation as the energy is finite for  $\det \underline{F} \rightarrow 0^+$  (but large rotation possible)

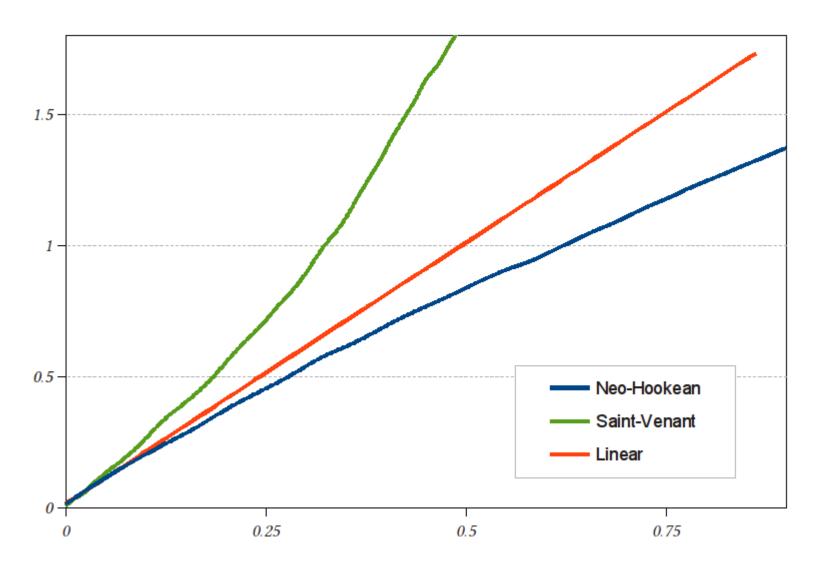
$$\underline{\sigma} = \frac{1}{J} \underline{E} \cdot \underline{S} \cdot {}^{t}\underline{E}$$

$$= \frac{1}{J} (\lambda \operatorname{Tr}(\underline{e}) \underline{F} \cdot {}^{t}\underline{F} + 2 \mu \underline{F} \cdot \underline{e} \cdot {}^{t}\underline{F})$$

$$= \frac{1}{J} (\lambda \operatorname{Tr}(\underline{e}) \underline{B} + \mu (\underline{B} \cdot \underline{B} - \underline{B}))$$

with 
$$\underline{\underline{B}} = \underline{\underline{F}} \cdot {}^{t}\underline{\underline{F}}$$

#### Example of stress strain curve



## Incompressible Neo-Hook model

Elastic energy

$$w(\underline{e}) = \frac{\mu}{2}(I_1 - 3)$$

Second Piola-Kirchhoff stress tensor

$$\underline{S} = 2 \frac{\partial W}{\partial I_1} \underline{1} - p \underline{C}^{-1} = \mu \underline{I} - p \underline{C}^{-1}$$

Cauchy stress tensor

$$\underline{\sigma} = \frac{1}{J} \underline{E} \cdot \underline{S} \cdot {}^{t} \underline{E}$$

$$\underline{\sigma} = -p \underline{I} + \mu \underline{F} \cdot {}^{t} \underline{F}$$

## Compressible Neo-Hook model

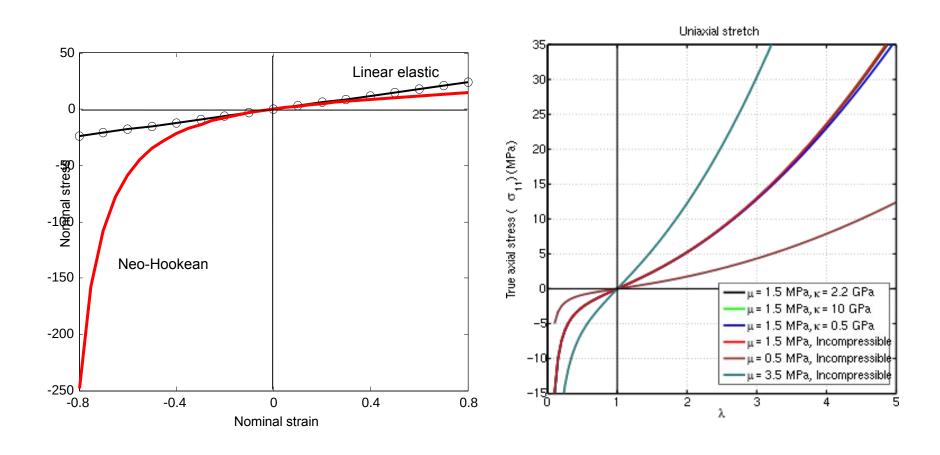
Elastic energy

$$w(\underline{e}) = \frac{\mu}{2} (I_1 - 3) - \mu \ln J + \frac{\lambda}{2} (\ln J)^2$$

Exercice: compute the second Piola-Kirchhoff stress tensor and the Cauchy stress tensor

#### Use for moderate deformation (less than 20%)

#### Use for plastic and rubber



## Mooney-Rivlin model

$$\begin{split} &w(\underline{e}) = \frac{\mu_1}{2}(\frac{I_1}{J^{2/3}} - 3) + \frac{\mu_2}{2}(\frac{I_2}{J^{4/3}} - 3) + \frac{K}{2}(J - 1)^2 \quad (Compressible) \\ &w(\underline{e}) = \frac{\mu_1}{2}(I_1 - 3) + \frac{\mu_2}{2}(I_2 - 3) \quad (Incompressible) \end{split}$$

$$\begin{split} &\underline{\underline{S}} \!=\! \! \left( \! \frac{\mu_1}{J^{2/3}} \!+\! \mu_2 \frac{I_1}{J^{4/3}} \right) \! \underline{\underline{I}} \!-\! \frac{\mu_2}{J^{4/3}} \underline{\underline{C}} \!+\! \left( \! K\!J \left( J \!-\! 1 \right) \!-\! \frac{\mu_1 I_1}{3J^{2/3}} \!-\! \frac{2\,\mu_2 I_2}{3\,J^{4/3}} \right) \! \underline{\underline{C}}^{-1} \quad \! \! \left( Compressible \right) \\ &\underline{\underline{S}} \!=\! \left( \mu_1 \!+\! \mu_2 I_1 \right) \underline{\underline{I}} \!-\! \mu_2 \underline{\underline{C}} \!-\! p\, \underline{\underline{C}}^{-1} \quad \! \! \left( Incompressible \right) \end{split}$$

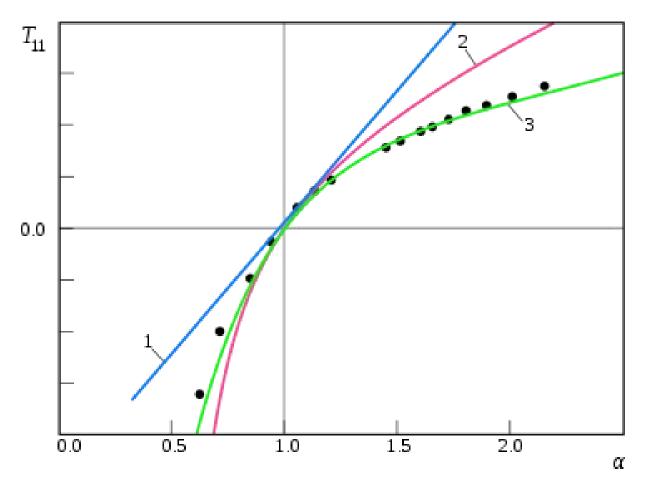
$$\underline{\underline{\sigma}} = \frac{1}{J} \underline{\underline{F}} \cdot \underline{\underline{S}} \cdot {}^{t}\underline{\underline{F}}$$

$$\underline{\underline{\sigma}} = \frac{1}{J} \left( \left( \frac{\mu_{1}}{J^{2/3}} + \mu_{2} \frac{I_{1}}{J^{4/3}} \right) \underline{\underline{B}} - \frac{\mu_{2}}{J^{4/3}} \underline{\underline{B}}^{2} + \left( KJ(J-1) - \frac{\mu_{1}I_{1}}{3J^{2/3}} - \frac{2\mu_{2}I_{2}}{3J^{4/3}} \right) \underline{\underline{1}} \right) \quad (Compressible)$$

$$\underline{\underline{\sigma}} = (\mu_{1} + \mu_{2}I_{1}) \underline{\underline{B}} - \mu_{2}\underline{\underline{B}}^{2} - p\underline{\underline{I}} \quad (Incompressible)$$

Exercice: check the formulas for  $\underline{S}$  and  $\underline{\sigma}$ 

## Example of traction compression curve



Experimental results (dots)
Linear Hooke's law(1, blue line)
Neo-Hookean solid(2, red line) and
Mooney–Rivlin solid models(3, green line)

#### Other models

#### Ogden model

$$W(\lambda_1, \lambda_2, \lambda_3) = \sum_{i=1}^{N} \frac{\mu_i}{\alpha_i} (\lambda_1^{\alpha_i} + \lambda_2^{\alpha_i} + \lambda_3^{\alpha_i} - 3)$$

#### General model

$$W(I_1, I_2, I_3) = \sum_{m+n+k=1}^{\infty} A_{mnk} (I_1 - 3)^m (I_2 - 3)^n (I_3 - 1)^k$$

#### General incompressible model

$$W(I_1,I_2) = \sum_{m+n=1}^{\infty} A_{mn} (I_1 - 3)^m (I_2 - 3)^n$$

6. Minimisation of the potential energy

Consider a solid that has a reference configuration  $\Omega_0$  and is submitted

- to body forces  $\underline{b}_0$  in  $\Omega_0$
- to surface forces  $g_0$  on  $\partial_g \Omega_0$
- and is such that  $\underline{\mathbf{u}} = \underline{\mathbf{u}}_0$  on  $\partial_{\mathbf{u}}\Omega_0$

The total potential energy is

$$\begin{split} E(u) &= E_{\text{int}}(u) + E_{\text{ext}}(u) \\ &= \int_{\Omega_0} W(\underline{u}) dx_0 - \int_{\Omega_0} \underline{b}_0 . \underline{u} dx_0 - \int_{\partial_g \Omega_0} \underline{g}_0 . \underline{u} ds_0 \end{split}$$

The solution of the mechanical problem is the displacement minimizing  $E(\underline{u})$  among the functions such that  $\underline{u} = \underline{u}_0$  on  $\partial_u \Omega_0$ 

At the minimum of the potential energy one should have

$$E(\underline{u}) \leq E(\underline{u} + h\underline{v}) \quad \forall \underline{v}, h$$

This implies that the derivative is zero for h=0

$$E'(u,y) = \frac{d}{dh} E(u+hy) = 0 \quad \text{for } h = 0$$

With the energy given by

$$E(\underline{u}+h\underline{v})=\int_{\Omega_0}W(\underline{e}(\underline{u}+h\underline{v}))dx_0-\int_{\Omega_0}\underline{b}_0.(\underline{u}+h\underline{v})dx_0-\int_{\partial_g\Omega_0}\underline{g}_0.(\underline{u}+\underline{v})ds_0$$

One gets the derivative

$$E'(u, y) = \int_{\Omega_0} \frac{\partial W}{\partial \underline{e}}(\underline{e}(u)) : \underline{e}(y) dx_0 - \int_{\Omega_0} b_0 . y dx_0 - \int_{\partial_g \Omega_0} g_0 . y ds_0$$
 with  $\underline{e}_d(y) = \frac{d}{dh} \underline{e}(\underline{u} + h y)$  for  $h = 0$ 

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$$E'(u,v) = \int_{\Omega_0} \frac{\partial W}{\partial \underline{e}}(\underline{e}(u)) : \underline{e}_d(v) dx_0 - \int_{\Omega_0} b_0 \cdot v dx_0 - \int_{\partial_a \Omega_0} g_0 \cdot v ds_0$$

Is the variational formulation defined for all  $\underline{\mathbf{v}} = 0$  on  $\partial_{\mathbf{u}}\Omega_0$ 

The non linearity is in the strain energy term and

$$\underline{e}_{d}(\underline{v}) = \frac{d\underline{e}(\underline{u} + h\underline{v})}{dh}(0)$$

$$= \frac{1}{2} \frac{d}{dh} ({}^{t}(\nabla (\underline{u} + h\underline{v})) \cdot \nabla (\underline{u} + h\underline{v}) - \underline{1})(0)$$

$$= \frac{1}{2} ({}^{t}(\nabla \underline{u}) \cdot \nabla \underline{v} + {}^{t}(\nabla \underline{v}) \cdot \nabla \underline{u})$$

$$= \frac{1}{2} ({}^{t}\underline{F} \cdot \nabla \underline{v} + {}^{t}(\nabla \underline{v}) \cdot \underline{F})$$

$$= ({}^{t}\underline{F} \cdot \nabla \underline{v})_{\text{sym}}$$

$$E'(u,v) = \int_{\Omega_0} \frac{\partial W}{\partial \underline{e}}(\underline{e}(u)) : ({}^{t}\underline{E}.\nabla v)_{\text{sym}} dx_0 - \int_{\Omega_0} b_0.v dx_0 - \int_{\partial_g \Omega_0} g_0.v ds_0$$

$$\int_{\Omega_0} \frac{\partial W}{\partial \underline{e}}(\underline{e}(\underline{u})) : ({}^{t}\underline{E} \cdot \nabla \underline{v})_{\text{sym}} dx_0 = \int_{\Omega_0} \underline{b}_0 \cdot \underline{v} dx_0 + \int_{\partial_g \Omega_0} \underline{g}_0 \cdot \underline{v} ds_0$$

$$a(\underline{u},\underline{v}) = l(\underline{v})$$

- $a(\underline{u},\underline{v})$  Non linear in  $\underline{u}$ , linear in  $\underline{v}$
- l(v) Linear in v

$$\int_{\Omega_{0}} \frac{\partial W}{\partial \underline{e}}(\underline{e}(\underline{u})) : ({}^{t}\underline{F} \cdot \nabla \underline{v})_{\text{sym}} dx_{0} = \int_{\Omega_{0}} \underline{S}(\underline{e}(\underline{u})) : ({}^{t}\underline{F} \cdot \nabla \underline{v}) dx_{0} 
= \int_{\Omega_{0}} (\underline{F} \cdot \underline{S}(\underline{e}(\underline{u}))) : \nabla \underline{v} dx_{0} 
= \int_{\Omega_{0}} \underline{P}(\underline{e}(\underline{u})) : \nabla \underline{v} dx_{0}$$

And one recovers the equation in the reference configuration

DIV 
$$\underline{\underline{P}} + \underline{b}_0 = 0$$

$$\underline{\underline{P}} \cdot \underline{\underline{N}} = \underline{g}_0 \quad on \quad \partial_q \Omega_0$$

## 7. Linearisation

One defines the residual 
$$R(u)=a(u,v)-l(v)$$

The objective is to solve 
$$R(u)=a(u,v)-l(v)=0$$

This is a non linear equation

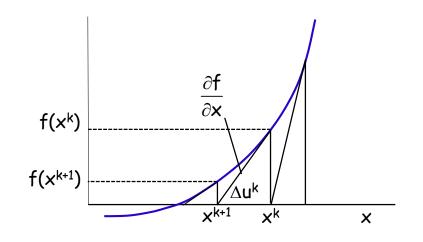
One possibility is to solve by the Newton – Raphson algorithm

For a scalar function around a point x<sub>0</sub>

$$f(x) \simeq f(x_0) + f'(x_0)(x - x_0)$$

An approximation of the point such that f(x)=0 is solution of

$$f(x_0)+f'(x_0)(x-x_0)=0$$



$$x_{k+1} = x_k - \frac{f(x_k)}{f'(x_k)}$$

$$R(u+\Delta u) \simeq R(u) + \frac{\partial R}{\partial u}(u) \cdot \Delta u + o(|\Delta u|^2)$$

One solves the approximate equation

$$\frac{\partial R}{\partial \underline{u}}(\underline{u}^k).\Delta \underline{u}^k = -R(\underline{u}^k)$$
$$\underline{u}^{k+1} = \underline{u}^k + \Delta \underline{u}^k$$

The problem is to compute  $\frac{\partial R}{\partial u}$ 

One notes that 
$$\frac{\partial R}{\partial \underline{u}} = \frac{\partial a(\underline{u},\underline{v})}{\partial \underline{u}}$$

(it is possible to have loads and thus  $l(\underline{v})$  depending on the displacement but this case will not be considered here)

The left-hand side of the variational relation is

$$a(u,v) = \int_{\Omega_0} \frac{\partial W}{\partial \underline{e}}(\underline{e}(u)) : ({}^{t}\underline{E} \cdot \nabla v)_{\text{sym}} dx_0$$

$$a(u+\Delta u,v) = \int_{\Omega_0} \underline{S}(\underline{e}(u+\Delta u)) : (({}^{t}\underline{E}+{}^{t}(\Delta \underline{E})) \cdot \nabla v)_{\text{sym}} dx_0$$

This expression should be developed at the first order in  $\Delta \underline{u}$ Consider first

$$\Delta \underline{F} = \Delta \left( \frac{\partial \underline{X}}{\partial \underline{X}} \right) \\
= \Delta \left( \frac{\partial (\underline{X} + \underline{u})}{\partial \underline{X}} \right) \\
= \frac{\partial (X + \underline{u} + \Delta \underline{u})}{\partial \underline{X}} - \frac{\partial (X + \underline{u})}{\partial \underline{X}} \\
= \frac{\partial \Delta \underline{u}}{\partial \underline{X}} \\
= \nabla (\Delta \underline{u})$$

$$\Delta \underline{F} = \nabla (\Delta u)$$

$$a(u+\Delta u, v) = \int_{\Omega_0} \underline{S}(\underline{e}(u+\Delta u)) : (({}^{t}\underline{E}+{}^{t}(\Delta \underline{E})).\nabla v)_{\text{sym}} dx_0$$

Consider now

$$\underline{\underline{S}}(\underline{e}(\underline{u} + \Delta \underline{u})) = \underline{\underline{S}}(\underline{e}(\underline{u})) + \frac{\partial \underline{\underline{S}}}{\partial \underline{e}} : \Delta \underline{e} + o(|\Delta \underline{e}|^2)$$

$$= \underline{\underline{S}}(\underline{e}(\underline{u})) + \underline{\underline{D}} : \Delta \underline{e} + o(|\Delta \underline{e}|^2)$$

$$\Delta \underline{e} = \frac{1}{2} ({}^{t}(\Delta \underline{F}).\underline{F} + {}^{t}\underline{F}.\underline{\Delta}F)$$
$$= sym({}^{t}\underline{F}.\nabla(\Delta \underline{u}))$$

 $\underline{\underline{\underline{D}}}$  are the tangent elastic coefficients

So one gets the development

$$a(\underline{u} + \Delta \underline{u}, \underline{v}) = \int_{\Omega_0} \underline{S}(\underline{e}(\underline{u} + \Delta \underline{u})) : (({}^t\underline{\underline{F}} + {}^t(\Delta \underline{\underline{F}})) . \nabla \underline{v})_{\text{sym}} dx_0$$

$$= a(\underline{u}, \underline{v}) + \int_{\Omega_0} ((\underline{\underline{\underline{D}}} : \Delta \underline{e}) : ({}^t\underline{\underline{F}} . \nabla \underline{v})_{\text{sym}} + \underline{\underline{S}} : ({}^t(\nabla (\Delta \underline{u})) . \nabla \underline{v})_{\text{sym}} dx_0 + o(|\underline{u}|^2)$$

$$\frac{\partial R}{\partial \underline{u}} \cdot \Delta \underline{u} = \frac{\partial a(\underline{u}, \underline{v})}{\partial \underline{u}} \cdot \Delta \underline{u}$$

$$= \int_{\Omega_0} \left( \left( \underline{\underline{\underline{D}}} : \Delta \underline{e} \right) : \left( \underline{\underline{F}} \cdot \nabla \underline{v} \right)_{\text{sym}} + \underline{\underline{S}} : \left( \underline{t} (\nabla (\Delta \underline{u})) \cdot \nabla \underline{v} \right)_{\text{sym}} \right) dx_0$$

The first term is the material tangent stiffness

The second term is geometric stiffness

The step k of the Newton Raphson algorithm consists in solving

$$\frac{\partial R}{\partial \underline{u}}(\underline{u}^k).\Delta \underline{u}^k = -R(\underline{u}^k)$$

$$\int_{\Omega_0} \left( \left( \underline{\underline{\underline{P}}} : \Delta \underline{\underline{e}} \right) : \left( \underline{\underline{F}} . \nabla \underline{\underline{v}} \right)_{\text{sym}} + \underline{\underline{S}} : \left( (\nabla (\Delta \underline{\underline{u}})) . \nabla \underline{\underline{v}} \right)_{\text{sym}} \right) dx_0 = l(\underline{\underline{v}}) - a(\underline{\underline{u}}^k, \underline{\underline{v}})$$

- The right-hand side is known
- The left-hand side is linear in  $\Delta \underline{u}$
- Both sides are linear in v

In discrete form this can be written as

$${}^{t}\underline{V} \cdot \underline{\underline{K}} \cdot \Delta \underline{U} = {}^{t}\underline{V} \cdot \underline{R}$$

- $\underline{V}$  is the vector of nodal values of the function  $\underline{v}$
- $\Delta \underline{U}$  is the vector of nodal values of the function  $\Delta \underline{u}$
- $\underline{K}$  is a matrix resulting from the discretisation of the left-hand side
- R is a vector resulting from the discretisation of the right-hand side

# 8. FEM implementation

## Displacement gradient and deformation

Displacement gradient in an element with N<sub>e</sub> nodes

$$[\nabla \underline{u}]_{ij} = \sum_{I=1}^{I=N_e} \frac{\partial N_I(\underline{X}(\underline{\xi}))}{\partial \underline{X}} . \underline{u}_I$$

$$= \sum_{I=1}^{I=N_e} \left( \frac{\partial N_I(\underline{\xi})}{\partial \underline{\xi}} . \frac{\partial \underline{\xi}}{\partial \underline{X}} \right) . \underline{u}_I$$

 $\frac{\partial N_I(\xi)}{\partial \xi}$  Is easy to compute from the definition of the shape functions

$$\frac{\partial \xi}{\partial X} \quad \text{Is the inverse matrix of} \quad \frac{\partial \underline{X}}{\partial \xi} = \sum_{I=1}^{I=N_c} \frac{\partial N_I(\xi)}{\partial \xi} \underline{X}_I$$

Displacement gradient

$$[F]_{ij} = \delta_{ij} + [\nabla \underline{u}]_{ij}$$
 for  $i, j = 1...3$ 

$$[F] = \begin{bmatrix} 1 + u_{1,1} \\ u_{1,2} \\ u_{1,3} \\ u_{2,1} \\ 1 + u_{2,2} \\ \vdots \end{bmatrix} \qquad [e] = \begin{bmatrix} u_{1,1} + \frac{1}{2} \left( u_{1,1} u_{1,1} + u_{2,1} u_{2,1} + u_{3,1} u_{3,1} \right) \\ \frac{1}{2} \left( u_{1,2} + u_{2,1} + u_{1,1} u_{1,2} + u_{2,1} u_{2,2} + u_{3,1} u_{3,2} \right) \\ \frac{1}{2} \left( u_{1,3} + u_{3,1} + u_{1,1} u_{1,3} + u_{2,1} u_{2,3} + u_{3,1} u_{3,3} \right) \\ \vdots$$

$$[S]=[D][e]$$

From the matrix of tangent elastic coefficients  $\,\lfloor D 
floor$ 

One also needs  $({}^{t}\underline{E}.\nabla v)_{\mathrm{sym}}$ 

From

$$[F] = \begin{bmatrix} 1 + u_{1,1} \\ u_{1,2} \\ u_{1,3} \\ u_{2,1} \\ 1 + u_{2,2} \\ \vdots \end{bmatrix} \quad \text{and} \quad \begin{bmatrix} \nabla \underline{v} \end{bmatrix}_{ij} = \underbrace{\sum_{I=1}^{I=N_e} \frac{\partial N_I(\underline{X}(\underline{\xi}))}{\partial \underline{X}}.\underline{v}_I}_{I=N_e} \underbrace{\sum_{I=1}^{I=N_e} \left( \frac{\partial N_I(\underline{\xi})}{\partial \underline{\xi}}.\frac{\partial \underline{\xi}}{\partial \underline{X}} \right).\underline{v}_I}_{I=N_e}$$

$$\left[ \left( {}^{t}\underline{\underline{F}}.\nabla\underline{v} \right)_{\text{sym}} \right] = \left[ B_{m} \right] \left[ v \right]$$

Where  $[B_m]$  is a matrix and [v] the vector of nodal values of the function v in the element

$$\int_{\Omega_{0}} \left( \left( \underline{\underline{\underline{P}}} : \Delta \underline{\underline{e}} \right) : ({}^{t}\underline{\underline{F}} . \nabla \underline{\underline{v}})_{\text{sym}} + \underline{\underline{S}} : ({}^{t}(\nabla (\Delta \underline{\underline{u}})) . \nabla \underline{\underline{v}})_{\text{sym}} \right) dx_{0} = l(\underline{\underline{v}}) - a(\underline{\underline{u}}^{k}, \underline{\underline{v}})$$

The right-hand side can be put under the form

$$R(\underline{u}^k) = a(\underline{u}^k, \underline{v}) - l(\underline{v}) = {}^t[V][dF]^k$$

As

$$[\Delta e] = [B_m][d]$$

With [d] the vector of the nodal values of  $\Delta \, \underline{u}$ 

Material stiffness

$$\int_{\Omega_0} \left( \left( \underline{\underline{\underline{P}}} : \Delta \underline{\underline{e}} \right) : \left( {}^t \underline{\underline{F}} . \nabla \underline{\underline{v}} \right)_{\text{sym}} \right) dx_0 = {}^t [V] \left[ \int_{\Omega_0} {}^t [B_m] [D] [B_m] dx_0 \right] [d]$$

Geometric stiffness

$$\int_{\Omega_0} \left( \underline{\underline{S}} : ({}^t(\nabla(\Delta \underline{u})) . \nabla \underline{v})_{sym} \right) dx_0 = {}^t[V] \left[ \int_{\Omega_0} {}^t[B_g][S][B_g] dx_0 \right] [d]$$

One gets the tangent stiffness matrix

$$[K_T] = \int_{\Omega_0} [{}^t [B_m][D][B_m] + {}^t [B_g][S][B_g]] dx_0$$

So that the incremental equation is

$${}^{t}[V][K_{T}][d_{k}] = {}^{t}[V][dF]^{k}$$

 $K_{\mathsf{T}}$  changes at each step One continues until the norm of [d] is inferior to a given small value

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## THE END