弹性极危理论

(II)

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参考书:

铁摩辛柯, 《板壳理论》, 中译, 科学出版社, 1977。 黄克智等, 《板壳理论》, 清华大学出版社, 1987. 刘洪文等。《板壳理论》。浙江大学出版社。1987。 王俊奎,张志民《钣壳的弯曲与稳定》,北航 陈铁云,陈伯真,《弹性薄壳力学》, 华中工学院出版 社. 1983.

• 板内所有内力的表达式为:

$$M_{x} = -D\left(\frac{\partial^{2} w}{\partial x^{2}} + \mu \frac{\partial^{2} w}{\partial y^{2}}\right)$$
$$M_{y} = -D\left(\frac{\partial^{2} w}{\partial y^{2}} + \mu \frac{\partial^{2} w}{\partial x^{2}}\right)$$

$$M_{yx} = M_{xy} = -D(1-\mu)\frac{\partial^2 w}{\partial x \partial y}$$

$$Q_{x} = -D\left(\frac{\partial^{3} w}{\partial x^{3}} + \frac{\partial^{3} w}{\partial x \partial y^{2}}\right) \quad Q_{y} = -D\left(\frac{\partial^{3} w}{\partial y^{3}} + \frac{\partial^{3} w}{\partial y \partial x^{2}}\right)$$

• 利用曲率和扭率,弯矩和扭矩的表达式为:

$$M_{x} = D(\kappa_{x} + \mu \kappa_{y})$$

$$M_{y} = D(\kappa_{y} + \mu \kappa_{x})$$

$$M_{yx} = M_{xy} = D(1 - \mu) \kappa_{xy}$$

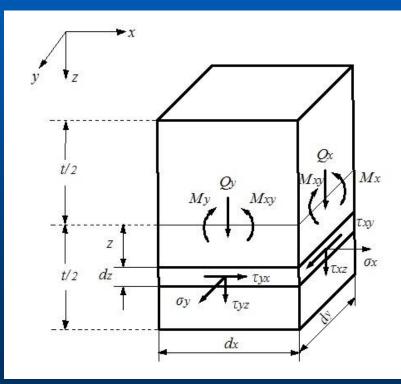
- 所有内力都是定义在单位长度上的, 所以量纲为:
 - 弯矩和扭矩: N.m/m
 - 剪力: N/m

• 各应力分量与薄板内力及横向载荷的关系:

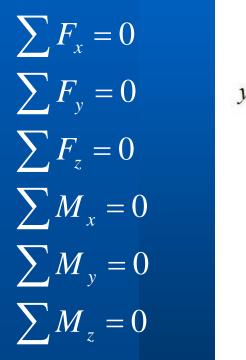
$$\sigma_{x} = \frac{12M_{x}}{t^{3}}z$$
 $\sigma_{y} = \frac{12M_{y}}{t^{3}}z$ $\tau_{xy} = \tau_{yx} = \frac{12M_{xy}}{t^{3}}z$

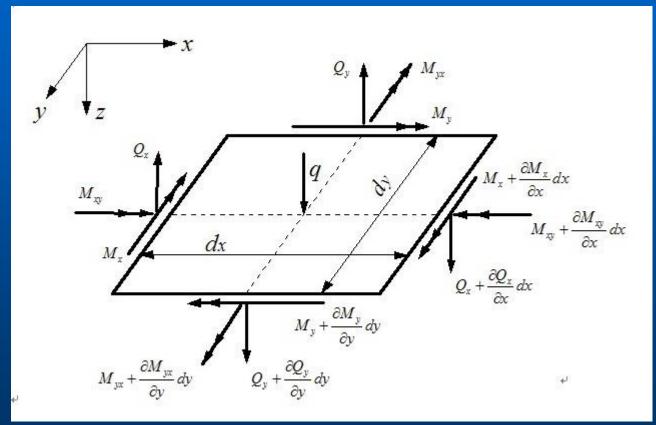
$$\tau_{xz} = \frac{6Q_x}{t^3} \left(\frac{t^2}{4} - z^2 \right) \quad \tau_{yz} = \frac{6Q_y}{t^3} \left(\frac{t^2}{4} - z^2 \right)$$

$$\sigma_z = -2q \left(\frac{1}{2} - \frac{z}{t}\right)^2 \left(1 + \frac{z}{t}\right)$$



• 由平衡条件得薄板弯曲基本微分方程:



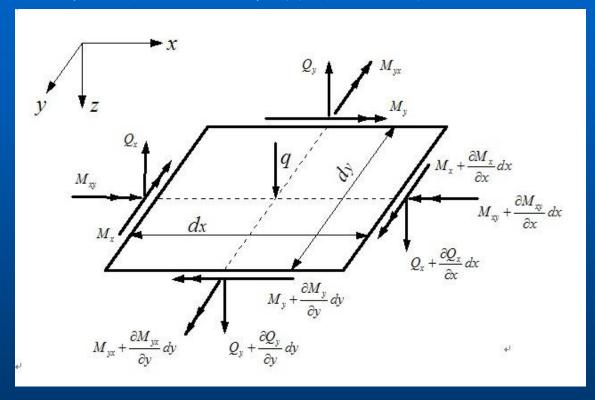


• 由平衡条件得薄板弯曲基本微分方程:

$$Q_{x} = \frac{\partial M_{x}}{\partial x} + \frac{\partial M_{yx}}{\partial y}$$

$$Q_{y} = \frac{\partial M_{y}}{\partial y} + \frac{\partial M_{xy}}{\partial x}$$

$$\frac{\partial Q_{x}}{\partial x} + \frac{\partial Q_{y}}{\partial y} + q = 0$$



$$\frac{\partial^2 M_x}{\partial x^2} + 2 \frac{\partial^2 M_{xy}}{\partial x \partial y} + \frac{\partial^2 M_y}{\partial y^2} + q = 0 \qquad \qquad D\Delta \Delta w = q$$

薄板弯曲基本微分方程: $D\Delta\Delta w = q$ 即:

$$\frac{\partial^4 w}{\partial x^4} + 2 \frac{\partial^4 w}{\partial x^2 \partial y^2} + \frac{\partial^4 w}{\partial y^4} = \frac{q}{D}$$

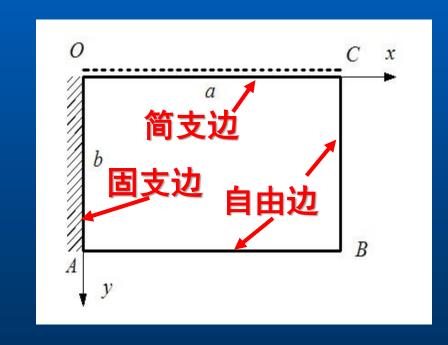
再有8个边界条件,即可完全求解。对矩形薄板四个边,每个边有两个条件,下面导出这些条件:

• 固支边、简支边、自由边:

A、固支边的边界条件:

$$x = 0$$
: (固支边)

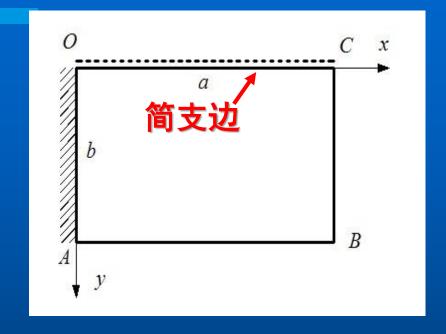
$$(w)_{x=0} = 0, \left(\frac{\partial w}{\partial x}\right)_{x=0} = 0$$



B、简支边的边界条件:

$$y=0$$
: (简支边)

$$(w)_{y=0} = 0, (M_y)_{y=0} = 0$$



$$(w)_{y=0} = 0; \left(\frac{\partial^2 w}{\partial y^2} + \mu \frac{\partial^2 w}{\partial x^2}\right)_{y=0} = 0$$

$$(w)_{y=0} = 0, \ \left(\frac{\partial^2 w}{\partial y^2}\right)_{y=0} = 0$$

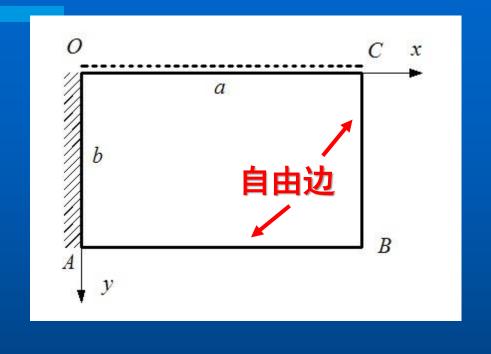
C、自由边边界条件:

自由边BC: x = a

$$M_{r} = 0; 另一个呢?$$

自由边AB: y = b

$$M_{v} = 0; 另一个呢?$$



●自由边AB: 除了 M , = 0, 其它条件要满足吗, 如:

$$M_{yx} = 0, Q_y = 0$$
?

答案: 否! 应该是两者的综合贡献,否则多了一个!

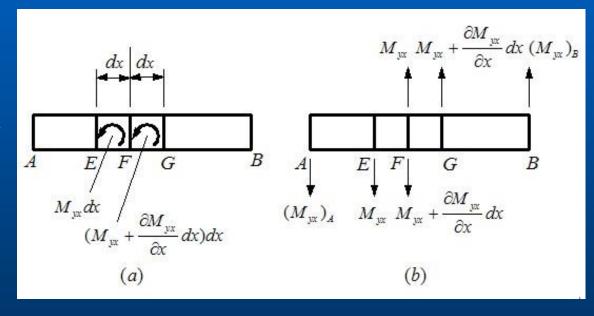
- 从AB边取出微段 EF=dx,受到扭矩 $M_{yx}dx$ 作用,将这个扭矩等效为一个力偶。两个力为 M_{yx} ,分别作用到E点和 F点。
- 取相邻微段 FG=dx, 扭矩

$$\left(M_{yx} + \frac{\partial M_{yx}}{\partial x} dx\right) dx$$

• 变换为一个力偶,力为

$$M_{yx} + \frac{\partial M_{yx}}{\partial x} dx$$

• F点合成为向下的合力

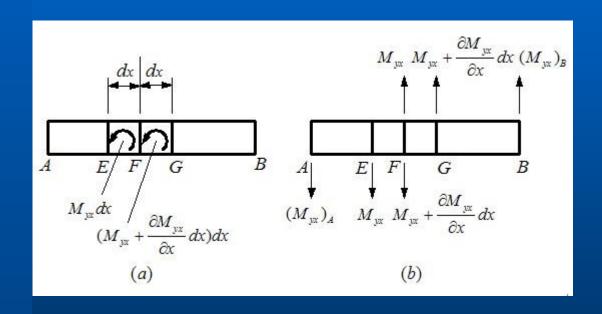


$$\frac{\partial M_{yx}}{\partial x} dx$$
,在整个边界AB上总的剪力 $V_y = 0$

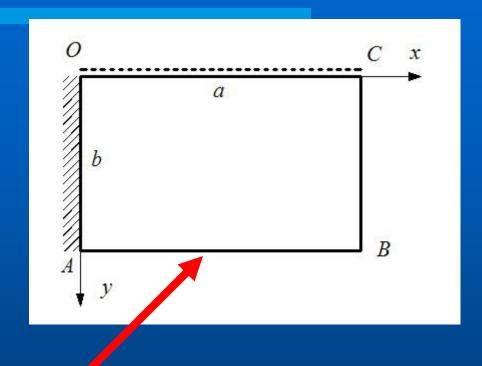
$$V_{y} = Q_{y} + \frac{\partial M_{yx}}{\partial x}$$

• 在A,B两点有未抵消的集中力

$$R_{AB} = \left(M_{yx}\right)_A, R_{BA} = \left(M_{yx}\right)_B$$



AB边的边界条件变为: $M_y = 0$, $V_y = 0$



$$M_{y} = -D\left(\frac{\partial^{2} w}{\partial y^{2}} + \mu \frac{\partial^{2} w}{\partial x^{2}}\right) = 0$$

$$V_{y} = Q_{y} + \frac{\partial M_{yx}}{\partial x} = -D \left(\frac{\partial^{3} w}{\partial y^{3}} + (2 - \mu) \frac{\partial^{3} w}{\partial y \partial x^{2}} \right) = 0$$

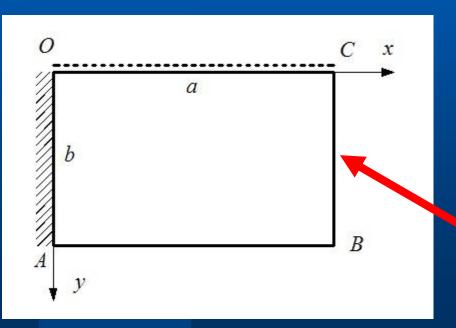
• 自由边 BC 的边界条件:

同样BC边的边界条件可以写为: $M_x = 0$, $V_x = 0$

$$M_x = 0, V_x = 0$$

其中等效剪力

$$V_{x} = Q_{x} + \frac{\partial M_{xy}}{\partial y}$$



$$M_{x} = -D\left(\frac{\partial^{2} w}{\partial x^{2}} + \mu \frac{\partial^{2} w}{\partial y^{2}}\right) = 0$$

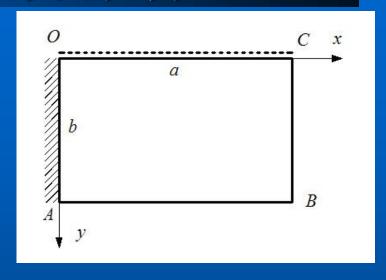
$$V_{x} = Q_{x} + \frac{\partial M_{xy}}{\partial y} = -D \left(\frac{\partial^{3} w}{\partial x^{3}} + (2 - \mu) \frac{\partial^{3} w}{\partial x \partial y^{2}} \right)$$

D、两自由边交点受集中力:

• 在B点作用有集中力:

$$R_{AB} = \left(M_{yx}\right)_{B}, R_{BC} = \left(M_{xy}\right)_{B}$$

• B点总的集中力条件为:



$$R_{B} = R_{BA} + R_{BC} = \left(M_{yx}\right)_{B} + \left(M_{xy}\right)_{B} = 2\left(M_{xy}\right)_{B} = -2D\left(1 - \mu\right)\left(\frac{\partial^{2} w}{\partial x \partial y}\right)_{B}$$

两个自由边的交点受集中力 P(向下)满足的条件:

(无集中力) 或
$$\left(\frac{\partial^2 w}{\partial x \partial y} \right)_B = \frac{P}{2D(1-\mu)}$$

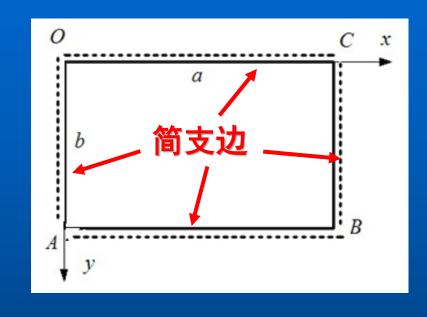
• 薄板弯曲的基本微分方程:

$$D\Delta\Delta w = q$$

● 四边简支边界条件

$$x = 0 \stackrel{\text{Right}}{=} a: \quad w = 0, \quad \frac{\partial^2 w}{\partial x^2} = 0$$

$$y = 0$$
 $\overrightarrow{y}b$: $w = 0$, $\frac{\partial^2 w}{\partial y^2} = 0$



假设挠度w为如下双三角级数:

$$w = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

• 满足边界条件,求w

求A



• 将挠度的表达式代入薄板弯曲的基本微分方程,得:

$$\pi^4 D \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \left(\frac{m^2}{a^2} + \frac{n^2}{b^2} \right)^2 A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} = q$$

- 微分方程 □□□> 代数方程
- 右边分布载荷 q 展为与左边同样的三角级数:

$$q = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} C_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

• 如何求 C_{mn} ,利用三角级数的正交性,得

$$C_{mn} = \frac{4}{ab} \int_0^b \int_0^a q \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dxdy$$

$$A_{mn} = \frac{4\int_0^a \int_0^b q \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dxdy}{\pi^4 abD(\frac{m^2}{a^2} + \frac{n^2}{b^2})^2}$$

• 如果q为均布载荷 q_0 ,上式变为

$$A_{mn} = \frac{4q_0(1 - \cos m\pi)(1 - \cos n\pi)}{\pi^6 Dmn(\frac{m^2}{a^2} + \frac{n^2}{b^2})^2}$$

• m, n只能取奇数,得

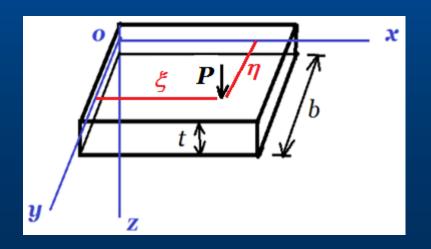
$$A_{mn} = \frac{16q_0}{\pi^6 Dmn(\frac{m^2}{a^2} + \frac{n^2}{b^2})^2}$$
 (m = 1,3,5...; n = 1,3,5...)

• 代入挠度表达式得:

$$w = \frac{16q_0}{\pi^6 D} \sum_{m=1,3,5...}^{\infty} \sum_{n=1,3,5...}^{\infty} \frac{\sin\frac{m\pi x}{a} \sin\frac{n\pi y}{b}}{mn(\frac{m^2}{a^2} + \frac{n^2}{b^2})^2}$$

• 在任意位置(ξ,η)受集中载荷P时,等效为分布载荷

$$q = P \delta (x - \xi) \delta (y - \eta)$$



• 所以A成为

$$A_{mn} = \frac{4}{\pi^4 abD (\frac{m^2}{a^2} + \frac{n^2}{b^2})^2} \iint_{S} q \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dxdy$$

$$= \frac{4P}{\pi^4 abD (\frac{m^2}{a^2} + \frac{n^2}{b^2})^2} \sin \frac{m\pi \xi}{a} \sin \frac{n\pi \eta}{b}$$

$$\pi^4 abD (\frac{m^2}{a^2} + \frac{n^2}{b^2})^2$$

• 代入挠度表达式:

$$w = \frac{4P}{\pi^4 abD} \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \frac{\sin \frac{m\pi\xi}{a} \sin \frac{n\pi\eta}{b}}{(\frac{m^2}{a^2} + \frac{n^2}{b^2})^2} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

- 双级数解法的优点:
 - 无论载荷情况如何,计算简单
- 双级数解法的缺点
 - 只适用于四边简支的矩形薄板
 - 简支边不能受力矩载荷,也不能有已知的位移
 - 双级数解答收敛速度慢,计算内力时需要很多项, 才能达到要求精度。

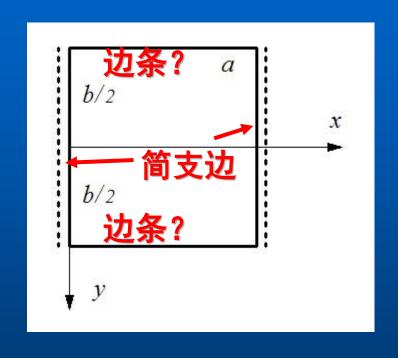
• 两个对边简支的矩形薄板,采用单级数解法

$$x = 0$$
或 a : $w = 0$, $\frac{\partial^2 w}{\partial x^2} = 0$
是任意边界条件

● 把挠度w表示为单三角级数形式:

$$w(x,y) = \sum_{m=1}^{\infty} Y_m(y) \sin \frac{m\pi x}{a}$$
• 满足 $x=0$, a 边的简支边界条件,

• 满足x=0,a边的简支边界条件, 代入薄板弯曲微分方程



$$\sum_{m=1}^{\infty} \left[\frac{d^4 Y_m}{dy^4} - 2(\frac{m\pi}{a})^2 \frac{d^2 Y_m}{dy^2} + (\frac{m\pi}{a})^4 Y_m \right] \sin \frac{m\pi x}{a} = \frac{q}{D}$$

● 偏微分方程 ==> 常微分方程

$$\sum_{m=1}^{\infty} \left[\frac{d^4 Y_m}{dy^4} - 2(\frac{m\pi}{a})^2 \frac{d^2 Y_m}{dy^2} + (\frac{m\pi}{a})^4 Y_m \right] \sin \frac{m\pi x}{a} = \frac{q}{D}$$

- 把右边展为单级数:
- 则有:

$$\frac{\partial^4 Y_m}{\partial y^4} - 2(\frac{m\pi}{a})^2 \frac{\partial^2 Y_m}{\partial y^2} + (\frac{m\pi}{a})^4 Y_m = \frac{2}{aD} \int_0^a q \sin \frac{m\pi x}{a} dx$$

• 这个常微分方程的解可以为:

$$w = \sum_{m=1}^{\infty} \left[A_m ch \frac{m\pi y}{a} + B_m \frac{m\pi y}{a} sh \frac{m\pi y}{a} + C_m sh \frac{m\pi y}{a} \right]$$

$$+D_m \frac{m\pi y}{a} ch \frac{m\pi y}{a} + f_m(y) \sin \frac{m\pi x}{a}$$
 $f_m(y) \not \in \not \in \not \in$

• 针对受均布载荷 q_0 作用的矩形板,特解为:

$$f_m(y) = \left(\frac{a}{m\pi}\right)^4 \frac{2q_0}{\pi Dm} (1 - \cos m\pi) = \frac{2q_0 a^4}{\pi^5 Dm^5} (1 - \cos m\pi)$$

• 挠度w关于x轴对称,所以 $C_m = 0$ $D_m = 0$,挠度表达式变为:

$$w = \sum_{m=1}^{\infty} \left[A_m ch \frac{m\pi y}{a} + B_m \frac{m\pi y}{a} sh \frac{m\pi y}{a} + \frac{2q_0 a^4}{\pi^5 Dm^5} (1 - \cos m\pi) \right] \sin \frac{m\pi x}{a}$$

- 假设另外两对边也是简支,则 $y=\pm \frac{b}{2}$: w=0 $\frac{\partial^2 w}{\partial y^2}=0$
- 得到关于 A_m 和 B_m 的联立代数方程:

$$\begin{cases} A_{m} \cosh a_{m} + B_{m} a_{m} \sinh a_{m} + \frac{4q_{0}a^{4}}{\pi^{5}Dm^{5}} = 0 \\ (A_{m} + 2B_{m}) \cosh a_{m} + B_{m} a_{m} \sinh a_{m} = 0 \end{cases}$$
 $(m = 1, 3, 5...)$

• 或者:

$$\begin{cases} A_m \cosh a_m + B_m a_m \sinh a_m = 0\\ (A_m + 2B_m) \cosh a_m + B_m a_m \sinh a_m = 0 \end{cases} \qquad (m = 2, 4, 6...)$$

• 其中 $a_m = \frac{m\pi b}{2a}$, 求得 $A_m B_m$ 为:

$$A_{m} = -\frac{2(2 + a_{m} \tan h)q_{0}a^{4}}{\pi^{5}Dm^{5} \cosh a_{m}}, \quad B_{m} = \frac{2q_{0}a^{4}}{\pi^{5}Dm^{5} \cosh a_{m}} \quad (m = 1, 3, 5...)$$

- 或者 $A_m = 0$, $B_m = 0$ (m = 2, 4, 6...)
- 最后得挠度表达式为:

$$w = \frac{4q_0 a^4}{\pi^5 D} \sum_{m=1,3,5...}^{\infty} (\frac{1}{m^5}) (1 - \frac{2 + a_m tha_m}{2cha_m} ch \frac{2a_m y}{b} + \frac{a_m}{2cha_m} \frac{2y}{b} sh \frac{2a_m y}{b}) sin \frac{m\pi x}{a}$$

- 单级数解法的优点:
 - 级数收敛速度快,计算量小
 - 可以求解某一边上受弯矩载荷的情况
 - 也可以求某一边已知位移的情况。

• 薄板弯矩和扭矩与挠度w的关系:

$$M_{x} = -\frac{Et^{3}}{12(1-\mu^{2})} \left(\frac{\partial^{2}w}{\partial x^{2}} + \mu \frac{\partial^{2}w}{\partial y^{2}} \right) = -D \left(\frac{\partial^{2}w}{\partial x^{2}} + \mu \frac{\partial^{2}w}{\partial y^{2}} \right)$$

$$M_{y} = -\frac{Et^{3}}{12(1-\mu^{2})} \left(\frac{\partial^{2}w}{\partial y^{2}} + \mu \frac{\partial^{2}w}{\partial x^{2}} \right) = -D \left(\frac{\partial^{2}w}{\partial y^{2}} + \mu \frac{\partial^{2}w}{\partial x^{2}} \right)$$

$$M_{xy} = -\frac{Et^{3}}{12(1+\mu)} \frac{\partial^{2}w}{\partial x \partial y} = -D(1-\mu) \frac{\partial^{2}w}{\partial x \partial y}$$

假设:薄板厚度变化比较平缓,中面仍然是平面,上式仍然成立,但弯曲刚度D是x和y的函数

• 由薄板平衡方程:

$$\frac{\partial^2 M_x}{\partial x^2} + 2\frac{\partial^2 M_{xy}}{\partial x \partial y} + \frac{\partial^2 M_y}{\partial y^2} + q = 0$$

• 得到:

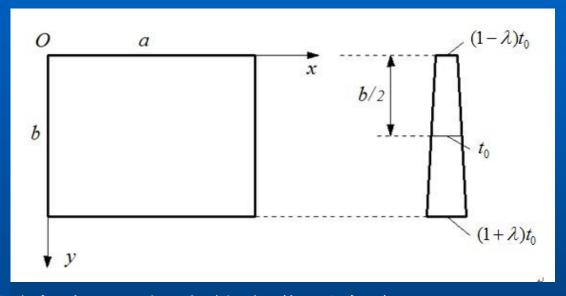
$$-\frac{\partial^{2}}{\partial x^{2}} \left[D \left(\frac{\partial^{2} w}{\partial x^{2}} + \mu \frac{\partial^{2} w}{\partial y^{2}} \right) \right] - 2 (1 - \mu) \frac{\partial^{2}}{\partial x \partial y} \left[D \frac{\partial^{2} w}{\partial x \partial y} \right]$$
$$-\frac{\partial^{2}}{\partial y^{2}} \left[D \left(\frac{\partial^{2} w}{\partial y^{2}} + \mu \frac{\partial^{2} w}{\partial x^{2}} \right) \right] + q = 0$$

• 进一步改写为:

$$D\Delta\Delta w + 2\frac{\partial D}{\partial x}\frac{\partial}{\partial x}\Delta w + 2\frac{\partial D}{\partial y}\frac{\partial}{\partial y}\Delta w + \Delta D\Delta w$$
$$-(1-\mu)(\frac{\partial^2 D}{\partial x^2}\frac{\partial^2 w}{\partial y^2} - 2\frac{\partial^2 D}{\partial x\partial y}\frac{\partial^2 w}{\partial x\partial y} + \frac{\partial^2 D}{\partial y^2}\frac{\partial^2 w}{\partial x^2}) = q$$

- 薄板厚度的不同变化规律,上面微分方程的系数取不同的函数形式,要求我们采用不同的方法求解。
- 考察厚度沿某一方向线性变化的情况

• 考察厚度沿y方向线性变化的情况



• y=b/2处厚度为 t_0 ,相应的弯曲刚度为

$$D_0 = \frac{Et_0^3}{12(1-\mu^2)}$$

任意点厚度表示为

$$t = [1 + \lambda(\frac{2y}{h} - 1)]t_0$$

• 弯曲刚度为:

$$D(y) = \frac{Et^3}{12(1-\mu^2)} = \frac{E}{12(1-\mu^2)} [1 + \lambda(\frac{2y}{b} - 1)]^3 t_0^3 = D_0 [1 + \lambda(\frac{2y}{b} - 1)]^3 \quad (d)$$

• 挠度w可以写成

$$w(x, y, \lambda) = \sum_{n=0,1,2...} w_n(x, y) \lambda^n$$
 (e)

把公式(d,e)代入薄板微分方程,得到关于λ的方程,在-1到1之间,λ取任意数值方程都成立,所以λ的所有各次幂的系数都应当等于零。得到如下常微分方程。

$$\Delta \Delta w_1(x, y) = -3 \left[\frac{4}{b} \frac{\partial}{\partial y} \Delta w_0 + (\frac{2y}{b} - 1) \Delta \Delta w_0 \right]$$

$$\Delta \Delta w_2 = -3 \left[\frac{4}{b} \frac{\partial}{\partial y} \Delta w_1 + (\frac{2y}{b} - 1) \Delta \Delta w_1 \right] - 3 \left\{ \frac{8}{b^2} \left[\Delta w_0 - (1 - \mu) \frac{\partial^2}{\partial x^2} w_0 \right] \right.$$
$$\left. + \frac{8}{b} (\frac{2y}{b} - 1) \frac{\partial}{\partial y} \Delta w_0 + (\frac{2y}{b} - 1)^2 \Delta \Delta w_0 \right\}$$

• 最后求出各个wn,代入

$$w(x, y, \lambda) = \sum_{n=0,1,2...} w_n(x, y) \lambda^n$$
 (e)

• 即得到变厚度矩形薄板的解。

§ 1.8 弹性地基上的弹性变形板

整个薄板放在弹性地基上,薄板承受横向载荷而发生挠度时,弹性地基对薄板作用一定的分布反力。弹性地基对薄板的分布反力可以表示为:

$$p(x,y) = -kw(x,y)$$

• 薄板弯曲的基本微分方程变为。

$$D\Delta\Delta w = p + q = q - kw$$

- 对于四边简支的矩形薄板,仍然可以用双三角级数法求解
- 对于具有两个简支边的矩形薄板,仍然可以用单三角级数 法求解
- 求解过程变得复杂,得到结果与k有关。

§ 1.9 薄板的温度应力

• 当薄板的温度改变时,薄板会产生变形,考虑温度的物理方程为。 1

程为:
$$\varepsilon_{x} = \frac{1}{E} \left(\sigma_{x} - \mu \sigma_{y} \right) + \alpha T \quad \varepsilon_{y} = \frac{1}{E} \left(\sigma_{y} - \mu \sigma_{x} \right) + \alpha T$$

$$\gamma_{xy} = \frac{2(1+\mu)}{E} \tau_{xy}$$

• 其中 α 是线热胀系数,

是薄板中任意一点的温度

• 应力分量为:

$$\sigma_{x} = \frac{E}{1 - \mu^{2}} \left(\varepsilon_{x} + \mu \varepsilon_{y} \right) - \frac{E \alpha T}{1 - \mu}; \quad \sigma_{y} = \frac{E}{1 - \mu^{2}} \left(\varepsilon_{y} + \mu \varepsilon_{x} \right) - \frac{E \alpha T}{1 - \mu}$$

$$\tau_{xy} = \frac{E}{2(1 + \mu)} \gamma_{xy}$$

§ 1.9 薄板的温度应力

• 应力分量关于挠度的表达式:

$$\sigma_{x} = -\frac{Ez}{1 - \mu^{2}} \left(\frac{\partial^{2} w}{\partial x^{2}} + \mu \frac{\partial^{2} w}{\partial y^{2}} \right) - \frac{E\alpha T}{1 - \mu}$$

$$\sigma_{y} = -\frac{Ez}{1 - \mu^{2}} \left(\frac{\partial^{2} w}{\partial y^{2}} + \mu \frac{\partial^{2} w}{\partial x^{2}} \right) - \frac{E\alpha T}{1 - \mu}$$

$$\tau_{xy} = -\frac{Ez}{(1 + \mu)} \frac{\partial^{2} w}{\partial x \partial y}$$

§ 1.9 薄板的温度应力

• 弯矩和扭矩的表达式为:

$$M_{x} = -\frac{Et^{3}}{12(1-\mu^{2})} \left(\frac{\partial^{2}w}{\partial x^{2}} + \mu \frac{\partial^{2}w}{\partial y^{2}}\right) - \frac{E\alpha}{1-\mu} \int_{-t/2}^{t/2} Tz dz$$

$$M_{y} = -\frac{Et^{3}}{12(1-\mu^{2})} \left(\frac{\partial^{2}w}{\partial y^{2}} + \mu \frac{\partial^{2}w}{\partial x^{2}}\right) - \frac{E\alpha}{1-\mu} \int_{-t/2}^{t/2} Tz dz$$

• 横向剪力表达式: $Q_x = -D\frac{\partial}{\partial x}\Delta w - \frac{\partial M_T}{\partial x}$, $Q_y = -D\frac{\partial}{\partial y}\Delta w - \frac{\partial M_T}{\partial y}$

• 温度沿 z 方向不能是常数

$$M_T = \frac{E\alpha}{1-\mu} \int_{-t/2}^{t/2} Tz dz$$

$$Q_{x} = \frac{\partial M_{x}}{\partial x} + \frac{\partial M_{yx}}{\partial y}, \quad Q_{y} = \frac{\partial M_{y}}{\partial y} + \frac{\partial M_{xy}}{\partial x}, \quad \frac{\partial Q_{x}}{\partial x} + \frac{\partial Q_{y}}{\partial y} = 0$$

$$\frac{\partial^{2} M_{x}}{\partial x^{2}} + 2 \frac{\partial^{2} M_{xy}}{\partial x \partial y} + \frac{\partial^{2} M_{y}}{\partial y^{2}} = 0$$

$$D \Delta \Delta w = q_T, \quad q_T = -\Delta M_T \qquad M_T = \frac{E\alpha}{1-\mu} \int_{-t/2}^{t/2} Tz dz$$

作业(2):

- 1、为什么薄板的每一个边界需要两个独立的边界条件? 试复述简支边、固支边和自由边的边界条件。
- 2、为什么说双三角函数的无穷级数展开解没有单三角函数的展开解收敛快?

3、为什么双三角函数形式的无穷级数展开解不适合在变厚度薄板弯曲问题求解时采用?