

# Calcul numérique des solides et structures non linéaires

D. Duhamel, C. Lestringan,  
C. Maurini, S. Neukirch

# Numerical solutions of non linear problems in structural dynamics

# Overview

1. Equations of motion
2. Variational formulation
3. Discretisation
4. Solution by implicit methods
5. Solution by explicit methods
6. Convergence of the methods
7. Conclusion

# 1. Equations of motion

# Different possibilities

- Variational principles as for non linear elasticity  
Hamilton's principle  
(also principle of least action, principle of stationary action, ...)

$$S(\underline{q}) = \int_{t_1}^{t_2} L(\underline{q}(t), \dot{\underline{q}}(t), t) dt \quad (L = W_{\text{ext}} + W_{\text{kin}} - W_{\text{int}})$$

$$\frac{\delta S}{\delta \underline{q}(t)} = 0$$

- Principle of virtual power

$$P_{\text{int}} + P_{\text{ext}} = P_a$$

**In the following, we will use this principle of virtual power**

Can be easier for non conservative forces

# conservation laws and partial differential equations

## Mass conservation (Lagrangian)

$$\begin{aligned}\int_{D(t)} \rho(\underline{x}, t) dV &= \int_{D(0)} \rho(\underline{X}, t) J(\underline{X}, t) dV_0 \\ &= \int_{D(0)} \rho_0(\underline{X}) dV_0\end{aligned}$$

$$\rho(\underline{X}, t) J(\underline{X}, t) = \rho_0(\underline{X})$$

Current value

Reference value

## Conservation of momentum (Lagrangian)

$$\underline{f}(t) = \frac{D\underline{p}}{Dt}(t) \quad (\text{Newton's law})$$

with

$$\begin{aligned} \underline{f}(t) &= \int_{D(0)} \rho_0(\underline{X}) \underline{f}(\underline{X}, t) dV_0 + \int_{\partial D(0)} \underline{P}(\underline{X}, t) \cdot \underline{N} da_0 \\ &= \int_{D(0)} \rho_0(\underline{X}) \underline{f}(\underline{X}, t) dV_0 + \int_{D(0)} \operatorname{div} \underline{P}(\underline{X}, t) dV_0 \end{aligned}$$

and

$$\begin{aligned} \frac{D\underline{p}}{Dt}(t) &= \frac{D}{Dt} \int_{D(0)} \rho_0(\underline{X}) \underline{v}(\underline{X}, t) dV_0 \\ &= \int_{D(0)} \rho_0(\underline{X}) \frac{\partial \underline{v}}{\partial t}(\underline{X}, t) dV_0 \end{aligned}$$

$$\rho_0(\underline{X}) \frac{\partial \underline{v}}{\partial t}(\underline{X}, t) = \rho_0(\underline{X}) \underline{f}(\underline{X}, t) + \operatorname{div} \underline{P}(\underline{X}, t)$$

# Static or dynamic ?

By making the equation dimensionless

$$\rho_0 \frac{\partial \underline{v}}{\partial t} = \rho_0 \underline{f} + \text{div } \underline{\underline{P}}$$

$$\frac{\rho_0 L^2}{E T^2} \frac{\partial \underline{\tilde{v}}}{\partial \underline{\tilde{t}}} = \frac{L}{E} \rho_0 \underline{f} + \text{div } \underline{\underline{\tilde{P}}}$$

The problem can be considered as static when

$$\frac{\rho_0 L^2}{E T^2} \ll 1$$

Or also

$$\left( \frac{L}{c T} \right)^2 \ll 1$$

So when the wave propagation is very fast over the distance L



## 2. Variational formulation

## Two types of boundary conditions

Displacement conditions  $u_i = u_i^d$  on  $\partial D_{u_i}$

Cinematically admissible displacement fields

Force conditions  $T_i = T_i^d$  on  $\partial D_{T_i}$

At each point of the boundary and in each direction only one condition

# Principle of virtual power

Vector space of virtual velocities

$$V = \left\{ \delta \underline{v} \mid \delta v_i = 0 \text{ on } \partial D_{u_i} \right\}$$

$$P_{\text{int}} + P_{\text{ext}} = P_a$$

And for all rigid motions

$$P_{\text{int}} = 0$$

# Lagrangian description

$$P_a = \int_{D(0)} \rho_0 \frac{\partial \underline{v}}{\partial t} \cdot \delta \underline{v} dV_0$$

Power of acceleration

$$= \int_{D(0)} \rho_0 \frac{\partial^2 \underline{u}}{\partial t^2} \cdot \delta \underline{v} dV_0$$

$$P_{\text{ext}} = \int_{D(t)} \rho \underline{f} \cdot \delta \underline{v} dV + \int_{\partial D_T(t)} \underline{T}^d \cdot \delta \underline{v} da$$

Power of external forces

$$= \int_{D(0)} \rho_0 \underline{f} \cdot \delta \underline{v} dV_0 + \int_{\partial D_T(0)} \underline{T}^d \cdot \delta \underline{v} dA_0$$

$$P_{\text{int}} = - \int_{D(t)} \underline{\underline{\sigma}} : \underline{\underline{\epsilon}}(\delta \underline{v}) dV$$

Power of internal forces

$$= - \int_{D(0)} \underline{\underline{P}} : \underline{\underline{\nabla \delta v}} dV_0$$

# Principle of virtual power

$$P_{\text{int}} + P_{\text{ext}} = P_a$$

$$\forall \delta \underline{v} \in V$$

$$\begin{aligned} \int_{D(0)} \rho_0 \frac{\partial^2 u}{\partial t^2} \cdot \delta \underline{v} dV_0 &= \int_{D(0)} \rho_0 \underline{f} \cdot \delta \underline{v} dV_0 + \int_{\partial D_T(0)} \underline{T}^d \cdot \delta \underline{v} dA_0 \\ &\quad - \int_{D(0)} \underline{P} : \underline{\nabla \delta \underline{v}} dV_0 \end{aligned}$$

Equivalent to the partial differential equation with the boundary conditions

Find  $\underline{u}$  regular enough such that

$$P_{\text{int}} + P_{\text{ext}} = P_a$$

$$\forall \delta \underline{v} \in V \quad -K(\underline{u}, \delta \underline{v}) + F(\delta \underline{v}) = M(\ddot{\underline{u}}, \delta \underline{v})$$

with

$$K(\underline{u}, \delta \underline{v}) = \int_{D(0)} \underline{\underline{P}} : \underline{\underline{\nabla}} \delta \underline{v} dV_0$$

$$F(\delta \underline{v}) = \int_{D(0)} \rho_0 \underline{f} \cdot \delta \underline{v} dV_0 + \int_{\partial D_T(0)} \underline{T}^d \cdot \delta \underline{v} dA_0$$

$$M(\ddot{\underline{u}}, \delta \underline{v}) = \int_{D(0)} \rho_0 \frac{\partial^2 \underline{u}}{\partial t^2} \cdot \delta \underline{v} dV_0$$

K bilinear for a linear problem, otherwise only linear relatively to  $\delta \underline{v}$

F and M linear in  $\delta \underline{v}$

### 3. Discretisation

# Discrete functions

Choose a basis  $\{N_i\}_{i=1}^{i=N}$  then

$$u_h(x, t) = \sum_{j=1}^{j=N} u_j(t) N_j(x), \quad u_j(t) \text{ unknowns to be found}$$

$$\delta v_h(x) = \sum_{i=1}^{i=N} \delta v_i N_i(x), \quad \delta v_i \text{ arbitrary parameters, virtual velocity}$$

Insert this into the variational formulation

$$-K(u_h, \delta v_h) + F(\delta v_h) = M(\ddot{u}_h, \delta v_h) \quad \forall \delta v_h \in V_h$$



# Form of the discrete equation

$$\begin{aligned}
 -K(\underline{u}_h, \delta \underline{v}_h) + F(\delta \underline{v}_h) &= M(\ddot{\underline{u}}_h, \delta \underline{v}_h) \\
 -K(\underline{u}_h, \sum_{i=1}^{i=N} \delta \underline{v}_i N_i(\underline{x})) + F(\sum_{i=1}^{i=N} \delta \underline{v}_i N_i(\underline{x})) &= M(\sum_{j=1}^{j=N} \ddot{u}_j N_j(\underline{x}), \sum_{i=1}^{i=N} \delta \underline{v}_i N_i(\underline{x})) \\
 -\sum_{i=1}^{i=N} \delta \underline{v}_i K(\underline{u}_h, N_i(\underline{x})) + \sum_{i=1}^{i=N} \delta \underline{v}_i F(N_i(\underline{x})) &= \sum_{i=1}^{i=N} \sum_{j=1}^{j=N} \delta \underline{v}_i \ddot{u}_j M(N_j(\underline{x}), N_i(\underline{x})) \\
 \sum_{i=1}^{i=N} \delta \underline{v}_i F_i^{\text{int}} + \sum_{i=1}^{i=N} \delta \underline{v}_i F_i^{\text{ext}} &= \sum_{i=1}^{i=N} \sum_{j=1}^{j=N} \delta \underline{v}_i \ddot{u}_j M_{ij}
 \end{aligned}$$

$$\underline{F}^{\text{int}} + \underline{F}^{\text{ext}} = \underline{\underline{M}} \ddot{\underline{u}}$$

$$\underline{F}^{\text{int}} + \underline{F}^{\text{ext}} = 0 \quad \text{for static problems}$$

## Computation of interior forces

$$\begin{aligned}
 - \int_{D(0)} \underline{\underline{P}} : \underline{\underline{\nabla \delta v}} dV_0 &= - \int_{D(0)} P_{ik} \frac{\partial \left( \sum_{j=1}^{j=n} \delta v_{kj} N_j \right)}{\partial X_i} dV_0 \\
 &= - \sum_{j=1}^{j=n} \int_{D(0)} P_{ik} \frac{\partial (\delta v_{kj} N_j)}{\partial X_i} dV_0 \\
 &= - \sum_{j=1}^{j=n} \int_{D(0)} B_{ji} P_{ik} \delta v_{kj} dV_0 \\
 &= \sum_{j=1}^{j=n} f_{kj}^{\text{int}} \delta v_{kj}
 \end{aligned}$$

with

$$\begin{aligned}
 f_{kj}^{\text{int}} &= - \int_{D(0)} P_{ik} \frac{\partial N_j}{\partial X_i} dV_0 = - \int_{D(0)} B_{ji} P_{ik} dV_0 \\
 B_{ji} &= \frac{\partial N_j}{\partial X_i}
 \end{aligned}$$

## Computation of external forces

$$\begin{aligned}
 & \int_{D(0)} \rho_0 \underline{f} \cdot \delta \underline{v} dV_0 + \int_{\partial D_T(0)} \underline{T}^d \cdot \delta \underline{v} dA_0 \\
 = & \int_{D(0)} \rho_0 f_k \sum_{j=1}^{j=n} \delta v_{kj} N_j(\underline{X}) dV_0 + \int_{\partial D_T(0)} T_k^d \sum_{j=1}^{j=n} \delta v_{kj} N_j(\underline{X}) dA_0 \\
 = & \sum_{j=1}^n f_{kj}^{ext} \delta v_{kj}
 \end{aligned}$$

*with*

$$f_{kj}^{ext} = \int_{D(0)} \rho_0 f_k N_j(\underline{X}) dV_0 + \int_{\partial D_T(0)} T_k^d N_j(\underline{X}) dA_0$$

## Computation of acceleration forces

$$\begin{aligned}
 \int_{D(0)} \rho_0 \frac{\partial^2 \underline{u}}{\partial t^2} \cdot \delta \underline{v} dV_0 &= \int_{D(0)} \rho_0 \sum_{l=1}^{l=n} \ddot{u}_{kl} N_l(\underline{X}) \sum_{j=1}^{j=n} \delta v_{kj} N_j(\underline{X}) dV_0 \\
 &= \sum_{j=1}^n f_{kj}^a \delta v_{kj}
 \end{aligned}$$

with

$$\begin{aligned}
 f_{kj}^a &= \int_{D(0)} \rho_0 \sum_{l=1}^{l=n} \ddot{u}_{kl} N_l(\underline{X}) N_j(\underline{X}) dV_0 \\
 &= \sum_{l=1}^{l=n} \int_{D(0)} \rho_0 N_l(\underline{X}) N_j(\underline{X}) dV_0 \ddot{u}_{kl} = \sum_{l=1}^{l=n} M_{jl} \ddot{u}_{kl}
 \end{aligned}$$

# Damping

- Can be included into interior forces
- Many possibilities
- For linear viscous damping the force is

$$\underline{\underline{F}}^{damp} = \underline{\underline{C}} \dot{\underline{u}}$$

Several possibilities for the damping matrix  $\underline{\underline{C}}$

One of them is the Rayleigh damping

$$\underline{\underline{C}} = a \underline{\underline{M}} + b \underline{\underline{K}}$$

a and b are constants computed from the values of the damping at the two extremities of the frequency band of interest

## Final equations

$$F^{int} + F^{ext} = \underline{\underline{M}} \ddot{u}$$

$$\underline{u}(0) = \underline{u}_0$$

$$\dot{\underline{u}}(0) = \dot{\underline{u}}_0$$

The damping force has been added to the interior force

## Remark

- The mass matrix is constant

$$\begin{aligned} M_{jl} &= \int_{D(t)} \rho N_l(\underline{x}) N_j(\underline{x}) dV \\ &= \int_{D(0)} \rho_0 N_l(\underline{X}) N_j(\underline{X}) dV_0 \end{aligned}$$

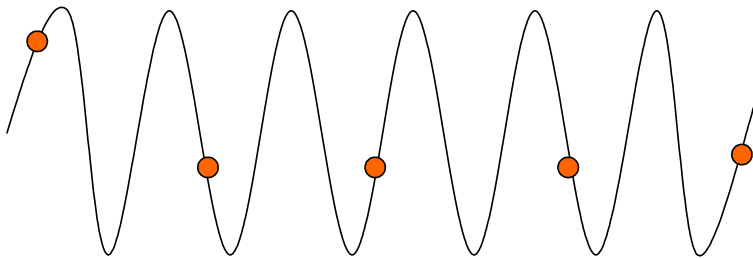
- Calculation of derivatives of shape functions

$$\frac{\partial N_j}{\partial X_i} = \frac{\partial N_j}{\partial \xi_k} \cdot \frac{\partial \xi_k}{\partial X_i} = \frac{\partial N_j}{\partial \xi_k} \cdot G_{ki}^{-1}$$

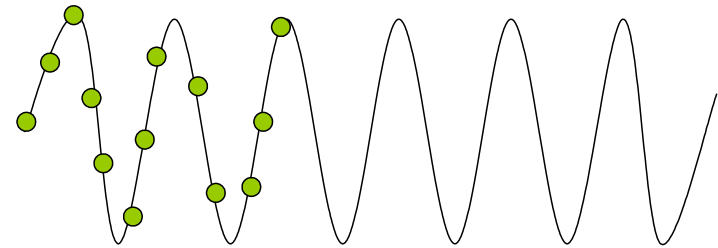
with  $G_{ki} = \frac{\partial X_k}{\partial \xi_i}$

# Mesh refinement

One needs between 5 and 10 nodes by wavelength



Not dense enough



Correct



# 4. Solution by implicit methods

Integrate equations like

$$F^{int} + F^{ext} = \underline{\underline{M}} \ddot{u}$$

$$\underline{u}(0) = \underline{u}_0$$

$$\dot{\underline{u}}(0) = \dot{\underline{u}}_0$$

Discretise in time  $\underline{u}_n = \underline{u}(n \Delta t)$

How to go from  $\{\underline{u}_n, \dot{\underline{u}}_n, \ddot{\underline{u}}_n\}$  to  $\{\underline{u}_{n+1}, \dot{\underline{u}}_{n+1}, \ddot{\underline{u}}_{n+1}\}$  ?

The system to solve can be written as

$$0 = \underline{r}(\underline{u}^{n+1}, t^{n+1}) = s \underline{\underline{M}} \underline{a}^{n+1} - \underline{f}^{int}(\underline{u}^{n+1}, t^{n+1}) - \underline{f}^{ext}(\underline{u}^{n+1}, t^{n+1})$$

with

$$s = \begin{cases} 0 & \text{for a statical problem} \\ 1 & \text{for a dynamical problem (and the damping is included} \\ & \text{in the interior force)} \end{cases}$$

$\underline{r}(\underline{u}^{n+1}, t^{n+1})$  is the residue which must equal zero for the solution

# Integration scheme

- There are many possibilities for the integration scheme of these equations
- A popular method is the Newmark scheme

$$\underline{u}^{n+1} = \tilde{\underline{u}}^{n+1} + \beta \Delta t^2 \underline{a}^{n+1}$$

$$\underline{v}^{n+1} = \tilde{\underline{v}}^{n+1} + \gamma \Delta t \underline{a}^{n+1}$$

$$\Delta t = t^{n+1} - t^n$$

$$\tilde{\underline{u}}^{n+1} = \underline{u}^n + \Delta t \underline{v}^n + \Delta t^2 \left( \frac{1}{2} - \beta \right) \underline{a}^n$$

$$0 \leq \beta \leq 1/2$$

$$0 \leq \gamma \leq 1$$

$$\tilde{\underline{v}}^{n+1} = \underline{v}^n + (1 - \gamma) \Delta t \underline{a}^n$$

if  $\beta = 0, \gamma = \frac{1}{2}$  *central difference*

if  $\beta = \frac{1}{4}, \gamma = \frac{1}{2}$  *non damped trapezoidal rule*

if  $\gamma > \frac{1}{2}$  *numerical damping proportionnal to  $\gamma - \frac{1}{2}$*

If  $\beta \geq \frac{\gamma}{2}$  and  $\gamma \geq \frac{1}{2}$  *unconditionnaly stable*

One can solve the new accelerations by

$$\underline{u}^{n+1} = \tilde{\underline{u}}^{n+1} + \beta \Delta t^2 \underline{a}^{n+1}$$

$$\underline{a}^{n+1} = \frac{1}{\beta \Delta t^2} (\underline{u}^{n+1} - \tilde{\underline{u}}^{n+1})$$

What leads to

$$0 = \underline{r}(\underline{u}^{n+1}, t^{n+1})$$

$$= \frac{s}{\beta \Delta t^2} \underline{\underline{M}}(\underline{u}^{n+1} - \tilde{\underline{u}}^{n+1}) - \underline{f}^{int}(\underline{u}^{n+1}, t^{n+1}) - \underline{f}^{ext}(\underline{u}^{n+1}, t^{n+1})$$

Non linear system of equations relative to  $\underline{u}^{n+1}$

# Solving by the Newton's method

(also Newton-Raphson)

Iterative method on the displacement between  $t^n$  and  $t^{n+1}$   
very close to the non linear elastic case

And starting at  $\underline{u}_0 = \underline{u}^n$

Development of the residue relatively to the displacement around its current value

$$\underline{r}(\underline{u}_v) + \frac{\partial \underline{r}}{\partial \underline{u}}(\underline{u}_v) \Delta \underline{u} + O(|\Delta \underline{u}|^2) = 0$$

$$\underline{A}(\underline{u}_v) = \frac{\partial \underline{r}}{\partial \underline{u}}(\underline{u}_v) \quad \begin{array}{l} \text{Jacobian matrix} \\ \text{(tangent stiffness)} \end{array}$$

Ignoring the second order terms

$$\underline{r}(\underline{u}_v) + \underline{\underline{A}}(\underline{u}_v) \Delta \underline{u} = 0$$

$$\underline{\underline{A}}(\underline{u}_v) \Delta \underline{u} = -\underline{r}(\underline{u}_v)$$

$$\Delta \underline{u} = -\underline{\underline{A}}^{-1}(\underline{u}_v) \underline{r}(\underline{u}_v)$$

$$\underline{u}_{v+1} = \underline{u}_v - \underline{\underline{A}}^{-1}(\underline{u}_v) \underline{r}(\underline{u}_v)$$

One goes on until convergence



# Computation of the jacobian matrix

$$\begin{aligned}\underline{\underline{A}}(\underline{u}_v) &= \frac{\partial \underline{r}}{\partial \underline{u}}(\underline{u}_v) \\ &= \frac{s}{\beta (\Delta t)^2} \underline{\underline{M}} - \frac{\partial \underline{f}^{int}}{\partial \underline{u}} - \frac{\partial \underline{f}^{ext}}{\partial \underline{u}}\end{aligned}$$

$$\underline{\underline{K}}^{int}(\underline{u}_v) = \frac{\partial \underline{f}^{int}}{\partial \underline{u}} \quad \text{tangent stiffness matrix}$$

$$\underline{\underline{K}}^{ext}(\underline{u}_v) = \frac{\partial \underline{f}^{ext}}{\partial \underline{u}} \quad \text{loading stiffness matrix}$$

$$\underline{\underline{A}}(\underline{u}_v) = \frac{s}{\beta (\Delta t)^2} \underline{\underline{M}} - \underline{\underline{K}}^{int}(\underline{u}_v) - \underline{\underline{K}}^{ext}(\underline{u}_v)$$

# Use implicit methods for slow dynamics

- Vibration
- Seismic
- Large deformation of elasto-plastic structures
- Response involving a small number of modes

# 5. Solution by explicit methods

# Central difference method

Simulation time  $0 \leq t \leq t_E$

Divided in time steps  $\Delta t$ ,  $t^n = n \Delta t$

The displacement at time step  $n$  is denoted  $\underline{u}^n = \underline{u}(t^n)$

For the velocity

$$\dot{\underline{u}}^{n+1/2} = \underline{v}^{n+1/2} = \frac{\underline{u}^{n+1} - \underline{u}^n}{\Delta t}$$

What is equivalent to

$$\underline{u}^{n+1} = \underline{u}^n + \Delta t \underline{v}^{n+1/2}$$

For the acceleration

$$\ddot{\underline{u}}^n = \underline{a}^n = \frac{\underline{v}^{n+1/2} - \underline{v}^{n-1/2}}{\Delta t}$$

$$\underline{v}^{n+1/2} = \underline{v}^{n-1/2} + \Delta t \underline{a}^n$$

$$\ddot{\underline{u}}^n = \underline{a}^n = \frac{\underline{u}^{n+1} - 2\underline{u}^n + \underline{u}^{n-1}}{\Delta t^2}$$

Formula of the central difference for the second derivative

## Integration of the equation of motion

$$\underline{\underline{M}} \underline{a}^n = \underline{f}^{ext}(\underline{u}^n, t^n) + \underline{f}^{int}(\underline{u}^n, t^n) = \underline{f}^n$$

With the displacement boundary conditions

$$\underline{g}_I(\underline{u}^n) = 0, \quad I = 1..n_c$$

Updating the velocity

$$\underline{v}^{n+1/2} = \underline{v}^{n-1/2} + \Delta t \underline{\underline{M}}^{-1} \underline{f}^n$$

At each time step the velocity is known, then

$$\underline{u}^{n+1} = \underline{u}^n + \Delta t \underline{v}^{n+1/2}$$

The nodal forces  $\underline{f}^n$  can be computed from the constitutive relation and the external forces and obtained from  $\underline{u}^n$

$$\underline{\underline{M}} \underline{\underline{a}}^n = \underline{\underline{f}}^{ext}(\underline{\underline{u}}^n, t^n) + \underline{\underline{f}}^{int}(\underline{\underline{u}}^n, t^n) = \underline{\underline{f}}^n$$

- If the mass matrix is diagonal one can compute the acceleration without solving any equation
- Idem for the updating of the velocity and the displacement
- The price to pay is that for the method to be stable one has to satisfy

$$\Delta t \leq \Delta t_{crit}$$

Otherwise the solution increases without limit



One has to take  $\Delta t \leq \alpha \Delta t_{crit}$  with for instance

$$\Delta t_{crit} = \frac{2}{\omega_{\max}} \leq \min_{e,I} \frac{2}{\omega_I^e} = \min_e \frac{l_e}{c_e}$$

With :

- $\omega_{\max}$  maximal pulsation of the linearized system
- $l_e$  characteristic length of element e
- $c_e$  current wave velocity in element e

$$0.8 \leq \alpha \leq 0.98$$

To take into account the  
destabilizing effect of non linearities

## summary

$$\underline{f}^n = \underline{f}^{ext}(\underline{u}^n, t^n) + \underline{f}^{int}(\underline{u}^n, t^n)$$

$$\underline{v}^{n+1/2} = \underline{v}^{n-1/2} + \Delta t \underline{\underline{M}}^{-1} \underline{f}^n$$

$$\underline{u}^{n+1} = \underline{u}^n + \Delta t \underline{v}^{n+1/2}$$

# Use explicit methods for fast dynamics

- Shock
- Wave propagation
- Response involving medium and high frequencies

Simulation  
of explosion

# Vibration of building with contact

## 6. Convergence of the methods

# Conservation of energy

$$\underline{\underline{M}} \ddot{\underline{u}} + \underline{\underline{K}} \underline{u} = 0$$

Case of a linear problem

$${}^t\dot{\underline{u}} \underline{\underline{M}} \ddot{\underline{u}} + {}^t\dot{\underline{u}} \underline{\underline{K}} \underline{u} = 0$$

This can be written as

$$\frac{d}{dt} [\mathcal{K}(t) + \mathcal{W}(t)] = 0$$

with

$$\mathcal{K}(t) = \frac{1}{2} {}^t\dot{\underline{u}} \underline{\underline{M}} \dot{\underline{u}}$$

$$\mathcal{W}(t) = \frac{1}{2} {}^t\underline{u} \underline{\underline{K}} \underline{u}$$

**Is the conservation satisfied  
for the discrete system ?**

From the relations

$$\mathcal{K}(t) = \frac{1}{2} {}^t\dot{\underline{u}} \underline{\underline{M}} \dot{\underline{u}}$$

$$\mathcal{W}(t) = \frac{1}{2} {}^t\underline{u} \underline{\underline{K}} \underline{u}$$

At the discrete level one gets

$$\begin{aligned} \mathcal{K}(t_{n+1}) - \mathcal{K}(t_n) &= 2 \dot{\underline{u}}_n^s \underline{\underline{M}} \dot{\underline{u}}_n^d \\ \mathcal{W}(t_{n+1}) - \mathcal{W}(t_n) &= 2 \underline{u}_n^s \underline{\underline{K}} \underline{u}_n^d \end{aligned}$$

with

$$\underline{u}_n^s = \frac{1}{2} (\underline{u}_{n+1} + \underline{u}_n)$$

$$\underline{u}_n^d = \frac{1}{2} (\underline{u}_{n+1} - \underline{u}_n)$$

Relations for the Newmark's scheme

$$0 = -2 \underline{u}_n^d + \Delta t \dot{\underline{u}}_n^s - \Delta t \dot{\underline{u}}_n^d + \frac{1}{2} \Delta t^2 [\ddot{\underline{u}}_n^s + (4\beta - 1) \ddot{\underline{u}}_n^d]$$

$$0 = -2 \dot{\underline{u}}_n^d + \Delta t [\ddot{\underline{u}}_n^s + (2\gamma - 1) \ddot{\underline{u}}_n^d]$$

By defining the energy like

$$\mathcal{E}(t) = \mathcal{K}(t) + \mathcal{W}(t) + \frac{1}{2} \Delta t^2 \left( \beta - \frac{\gamma}{2} \right)^t \ddot{\underline{u}} \underline{\underline{M}} \ddot{\underline{u}}$$

And after some calculations (exercise), one gets

$$\mathcal{E}(t_{n+1}) - \mathcal{E}(t_n) = 2(1 - 2\gamma)^t \underline{u}_n^d \underline{\underline{K}} \underline{u}_n^d + \Delta t^2 (\gamma - 2\beta)(2\gamma - 1)^t \ddot{\underline{u}}_n^d \underline{\underline{M}} \ddot{\underline{u}}_n^d$$



# Stability of the discrete scheme

$$\mathcal{E}(t_{n+1}) - \mathcal{E}(t_n) = 2(1 - 2\gamma)^t \underline{u}_n^d \underline{\underline{K}} \underline{u}_n^d + \Delta t^2 (\gamma - 2\beta)(2\gamma - 1)^t \underline{\underline{u}}_n^d \underline{\underline{M}} \underline{\underline{u}}_n^d$$

- If  $\gamma = 1/2$ , the energy  $\mathcal{E}(t)$  is constant
- If  $\gamma \geq \frac{1}{2}$  and  $2\beta - \gamma = 0$ , the total energy  $\mathcal{K}(t) + \mathcal{W}(t)$  is decreasing
- If  $\gamma = \frac{1}{2}$  and  $\beta = \frac{1}{4}$  the total energy is conserved
- If  $\gamma \geq \frac{1}{2}$  and  $2\beta > \gamma$  the energy  $\mathcal{E}(t)$  is decreasing
- If  $\gamma < \frac{1}{2}$  the energy is increasing if  $\gamma \leq 2\beta$   
and the scheme can diverge

## 7. Conclusion

## Essentially two possibilities:

- Implicit methods:
  - ✓ can be unconditionally stable
  - ✓ large time steps
  - ✓ complex in each iteration
  - ✓ for slow dynamics
- Explicit methods:
  - ✓ conditionally stable
  - ✓ small time steps
  - ✓ simple in each iteration
  - ✓ for fast dynamics
- Several algorithms in each family

# References

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