

Script of analytical calculation of depth of freshwater-seawater interface

December 19, 2020

1 Unconfined steady state flow with flux boundary condition

```
[1]: import numpy as np

# Depth to freshwater-seawater interface for different inland flux with 0.5, 0.
# → 2 and 0.1  $m^2/s$  and a recharge rate of 100 mm/year

H = 100 # m, aquifer thickness at coast
L = 1000 # m, length of the studied area
a = 0.1 * L
va = 1E-6
Dc = va * a
e = 40 # density ratio of freshwater density relative to the difference
# → between seawater and freshwater density
es = e * (1 - a / H)**(1 / 6)
K = 10 # m/d, hydraulic conductivity
Qf = -0.5 # sm per d, the groundwater flow from the basin towards the sea
Qf1 = -0.2 # sm per d, the groundwater flow from the basin towards the sea
Qf2 = -0.1 # sm per d, the groundwater flow from the basin towards the sea
N = 0.001 # m, per d, recharge rate

# make the linear space of x values from sea to L
x = np.arange(0,L,1)

# calculate the depth of the fresh-water seawater interface
# for Qf = -0.5
hs = np.sqrt(((Qf * x - N * L * x + N * ((x**2)/2))*(2 * (e**2)))/(-(1 + e) *
# → K))
y = hs

# calculate the depth of the fresh-water seawater interface
# for Qf1 = -0.2
hs1 = np.sqrt(((Qf1 * x - N * L * x + N * ((x**2)/2))*(2 * (e**2)))/(-(1 + e) *
# → K)) # the depth of the fresh-water seawater interface
y1 = hs1
```

```
# calculate the depth of the fresh-water seawater interface
# for Qf2 = -0.1
hs2 = np.sqrt(((Qf2 * x - N * L * x + N * ((x**2)/2))*(2 * (e**2)))/(-(1 + e) * K)) # the depth of the fresh-water seawater interface
y2 = hs2
```

```
[2]: # Plot the results

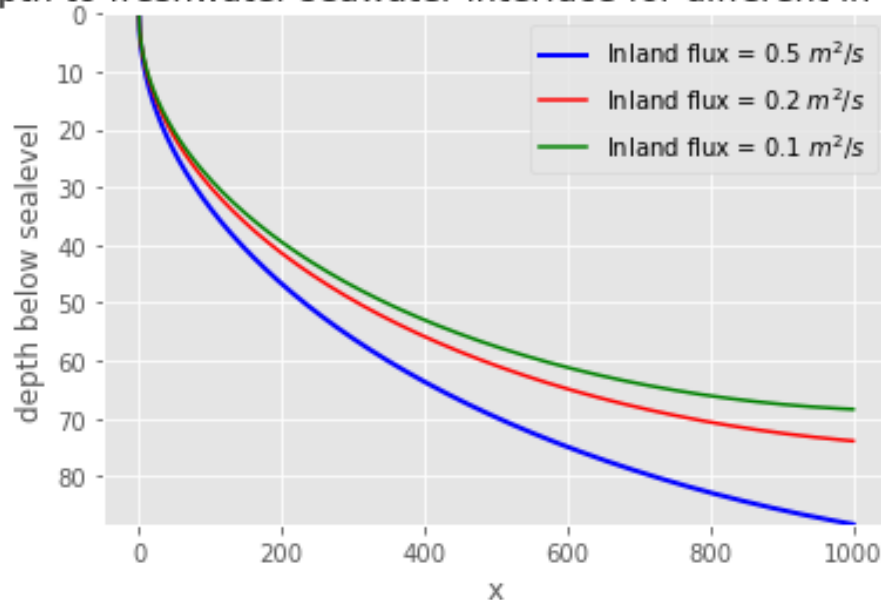
import matplotlib.pyplot as plt
plt.style.use('ggplot')

plt.plot(x,y, label = 'Inland flux = 0.5 $m^2/s$' , c = 'b' , linewidth = 2)
plt.plot(x,y1, label = 'Inland flux = 0.2 $m^2/s$' , c = 'r')
plt.plot(x,y2, label = 'Inland flux = 0.1 $m^2/s$' , c = 'g')

plt.ylim(y.max(), 0)
plt.title('Depth to freshwater-seawater interface for different in-land flux.')
plt.ylabel("depth below sealevel")
plt.xlabel('x')
plt.savefig('plot-unnamed-chunk-2-1.png')
plt.legend()
```

[2]: <matplotlib.legend.Legend at 0x7fe20d1f81f0>

Depth to freshwater-seawater interface for different in-land flux.

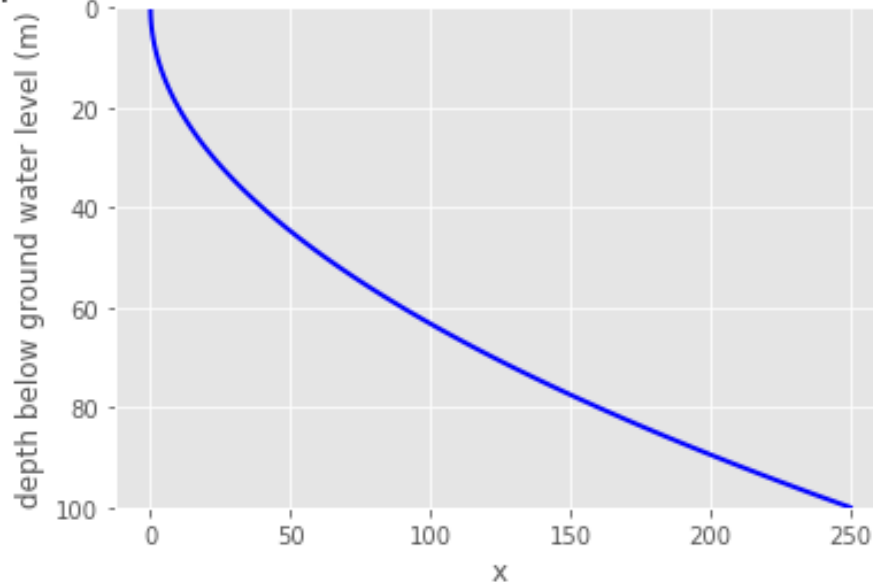


2 Confined steady state flow with flux boundary condition

```
[3]: # Depth to freshwater-seawater interface for a confined aquifer with a flux of  $Q_f = 0.5 \text{ m}^2/\text{d}$ ,  
      # aquifer thickness of 100 m and a hydraulic conductivity of 1 m/d.  
  
H = 100 # m, aquifer thickness at coast  
e = 40 #  
K = 1 # m/d, hydraulic conductivity  
L = 100 # m  
Qf = 0.5 #  $\text{m}^2/\text{d}$   
h = np.arange(0, L + 1, 1)  
x = K / (e * Qf) * ((h**2)/2)  
y = h
```

```
[4]: # plot the results  
  
import matplotlib.pyplot as plt  
plt.style.use('ggplot')  
  
plt.plot(x,y, linewidth = 2, c='b')  
  
plt.ylim(y.max(), 0)  
  
plt.ylabel("depth below ground water level (m)")  
plt.xlabel('x')  
  
plt.title('Depth to freshwater-seawater interface for a confined aquifer.  
           $Q_f = 0.5 \text{ m}^2/\text{d}$ ')  
  
plt.savefig('plot-confined_K-1.png')
```

Depth to freshwater-seawater interface for a confined aquifer.



3 Pumped wells

3.1 Unconfined aquifer

```
[5]: # Depth to transition zone for an unconfined aquifer with lateral inflow of 1
      ↳  $m^3/s$  and
      # a pumping rate of 0.1  $m^3/s$  and scenarios for high hydraulic conductivity ,
      # low hydraulic conductivity and different pumping rates based on the
      ↳ analytical solution
      # of Strack1976.

      # pumping_unconfined

      rs = 1.024
      rh = 1.000
      e = (rs - rh) / rh #
      H = 100 # aquifer thickness at coast
      K = 1 # m/s
      K1 = 5
      K2 = 0.5
      Qw = 1 #  $qm/s$ 
      Qw2 = 25
      Qx0 = 1 #  $qm/s$ 
      p = np.pi
      x = np.arange(1, 501, 1)
```

```

# calculate for Qw = 1 qm/s, Qx0= 1 qm/s and K = 1
mu = Qw / (Qx0 * x)
lamda = 2*(1 - mu / p)**(1/2) + mu / p * np.log((1 - (1 - mu / p)**(1/2)) / (1 + (1 - mu / p)**(1/2)))
y = np.sqrt((lamda * Qx0 * x)/(K * e))

# calculate for Qw = 1 qm/s, Qx0= 1 qm/s and K1 = 5
mu1 = Qw / (Qx0 * x)
lamda1 = 2*(1 - mu1 / p)**(1/2) + mu1 / p * np.log((1 - (1 - mu1 / p)**(1/2)) / (1 + (1 - mu1 / p)**(1/2)))
y1 = np.sqrt((lamda1 * Qx0 * x)/(K1 * e))

# calculate for Qw = 1 qm/s, Qx0= 1 qm/s and K2 = 0.5
mu2 = Qw / (Qx0 * x)
lamda2 = 2*(1 - mu2 / p)**(1/2) + mu2 / p * np.log((1 - (1 - mu2 / p)**(1/2)) / (1 + (1 - mu2 / p)**(1/2)))
y2 = np.sqrt((lamda2 * Qx0 * x)/(K2 * e))

# calculate for Qw2= 25 qm/s, Qx0= 1 qm/s and K = 1
mu3 = Qw2 / (Qx0 * x)
lamda3 = 2*(1 - mu3 / p)**(1/2) + mu3 / p * np.log((1 - (1 - mu3 / p)**(1/2)) / (1 + (1 - mu3 / p)**(1/2)))
y3 = np.sqrt((lamda3 * Qx0 * x)/(K * e))

```

<ipython-input-5-2512d590fe55>:38: RuntimeWarning: invalid value encountered in sqrt

```

lamda3 = 2*(1 - mu3 / p)**(1/2) + mu3 / p * np.log((1 - (1 - mu3 / p)**(1/2)) / (1 + (1 - mu3 / p)**(1/2)))

```

```

[6]: import matplotlib.pyplot as plt
plt.style.use('ggplot')

plt.plot(x,y, linewidth = 2, c='black')
plt.plot(x,y1, c = 'b', label = 'high hydraulic conductivity')
plt.plot(x,y2, c = 'g', label = 'low hydraulic conductivity')
plt.plot(x,y3, c = 'r', label = 'different pumping rates')

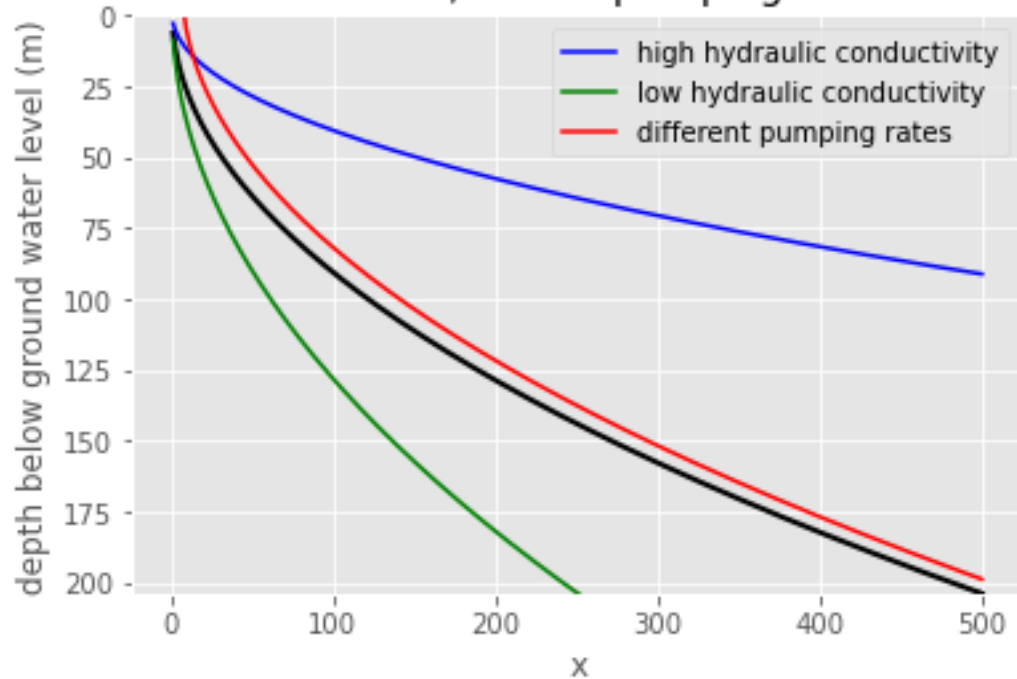
plt.ylim(y.max(), 0)
plt.title('Depth to transition zone for an unconfined aquifer with\nlateral inflow of $1 m^3/s$ and a pumping rate of $0.1 m^3/s$.')
plt.ylabel("depth below ground water level (m)")

plt.xlabel('x')
plt.legend()

plt.savefig('plot_pumping_unconfined.png')

```

Depth to transition zone for an unconfined aquifer with lateral inflow of $1\text{m}^3/\text{s}$ and a pumping rate of $0.1\text{m}^3/\text{s}$.



[7]: `## Confined aquifer`

[8]: `# For the confined aquifer dispersion and the range of the transition zone
has been implemented based on a method described in Beebe 2016.`

```
# pumping_confined

rs = 1.024
rh = 1.000
e = (rs / rh**2) * (rs - rh) #
a = 0.1 * 10
H = 100 # aquifer thickness at coast
K = 1 # m/s
K1 = 2
Qw = 1 # qm/s
Qx0 = 2 # qm/s
p = np.pi
x = np.arange(1,500,1)

# calculate for Qw = 1 qm/s, Qx0 = 2 qm/s and K = 1
mu = Qw / (Qx0 * x)
```

```

lamda = 2 * (1 - mu / p)**(1/2) + mu / p * np.log((1 - (1 - mu / p)**(1/2))/(1 +
↪ (1 - mu / p)**(1/2)))
es = e * (1 - (a/H)**(1/6))
y = np.sqrt((lamda * Qx0 * x) / (K * es))

# calculate for Qw = 1 qm/s, Qx0= 2 qm/s and K = 2
mu1 = Qw / (Qx0 * x)
lamda1 = 2 * (1 - mu1 / p)**(1/2) + mu1 / p * np.log((1 - (1 - mu1 / p)**(1/2))/(
↪ (1 + (1 - mu1 / p)**(1/2))))

y1 = np.sqrt((lamda1 * Qx0 * x) / (K1 * es))

```

```

[9]: # Plot the results

import matplotlib.pyplot as plt
plt.style.use('ggplot')

plt.plot(x,y, linewidth = 2, c='black', label = 'Hydraulic conductivity, K = 1_
↪ $m/s$')
plt.plot(x,y1, c = 'b', label = 'Hydraulic conductivity, K = 2 $m/s$')

plt.ylim(y.max(), 0)
plt.title('Depth to transition zone for a confined aquifer.')
plt.ylabel("depth below ground water level (m)")

plt.xlabel('x')
plt.legend()

plt.savefig('plot_pumping_confined.png')

```

