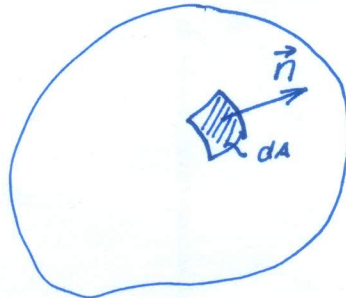
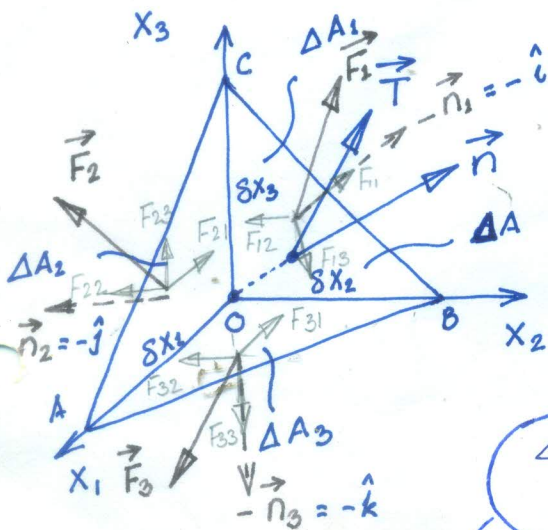


ALL ABOUT STRESS!!



$$\Delta A_1 = \frac{1}{2} \delta x_2 \cdot \delta x_3; \quad \Delta A_2 = \frac{1}{2} \delta x_1 \cdot \delta x_3;$$

$$\Delta A_3 = \frac{1}{2} \delta x_1 \cdot \delta x_2$$

$$\Delta A \vec{n} = \vec{\Delta A} = \frac{1}{2} (-\delta x_1 \hat{i} + \delta x_2 \hat{j}) \times (-\delta x_1 \hat{i} + \delta x_3 \hat{k})$$

$$= \Delta A_1 \hat{i} + \Delta A_2 \hat{j} + \Delta A_3 \hat{k}$$

Now $\Delta A_1 = \Delta A \cdot n_1$; $\Delta A_2 = \Delta A \cdot n_2$; $\Delta A_3 = \Delta A \cdot n_3$

where $\vec{n} = n_1 \hat{i} + n_2 \hat{j} + n_3 \hat{k}$

∴ force balance:

$$\vec{T} \Delta A + \vec{F}_1 \Delta A_1 + \vec{F}_2 \Delta A_2 + \vec{F}_3 \Delta A_3 + \frac{\Delta V}{\Delta A} \vec{f} = 0$$

$$\Rightarrow \vec{T} = -(\vec{F}_1 n_1 + \vec{F}_2 n_2 + \vec{F}_3 n_3)$$

$$= -F_{11} \hat{i} - F_{12} \hat{j} - F_{13} \hat{k} - F_{21} \hat{i} - F_{22} \hat{j} - F_{23} \hat{k} - F_{31} \hat{i} - F_{32} \hat{j} - F_{33} \hat{k}$$

$$= (F_{11} n_1 + F_{21} n_2 + F_{31} n_3) \hat{i} + (F_{12} n_1 + F_{22} n_2 + F_{32} n_3) \hat{j}$$

$$+ (F_{13} n_1 + F_{23} n_2 + F_{33} n_3) \hat{k}$$

$$= T_1 \hat{i} + T_2 \hat{j} + T_3 \hat{k}$$

$$\therefore \boxed{T_i = F_{ji} n_j}$$

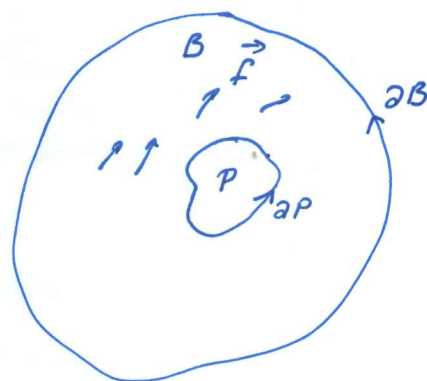
↑
stress (force per unit area)

$$= \underbrace{\sigma_{ji} n_j}_{\text{Cauchy Stress}} = [\sigma]^T \{n\}$$

Cauchy's hypothesis: The local, pointwise traction vector \vec{T} is a linear function of the normal vector \vec{n} . $[\sigma]$ is the operator and is called the stress tensor.

This relationship is true for any PORTION P of B , i.e.

$$\vec{T}|_{\partial P} = [\sigma]^T \vec{n}|_{\partial P}$$



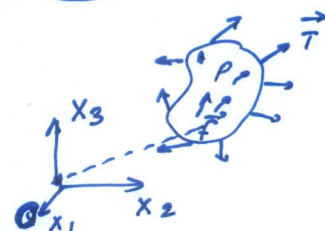
Now the portion also has to be in equilibrium or

$$\int_P \vec{f} dv + \int_{\partial P} \vec{T} dA = \vec{0}$$

$$\Rightarrow \int_P \vec{f} dv + \int_{\partial P} [\sigma]^T \vec{n} dA = \vec{0}$$

$$\Downarrow \int_P f_i dv + \int_{\partial P} \sigma_{ji} n_j dA = 0 \quad ; i=1,2,3$$

$$\Rightarrow \int_P (f_i + \sigma_{ji,j}) dv = 0 \quad \leftarrow \text{true for all arbitrary portions} \Rightarrow \boxed{f_i + \sigma_{ji,j} = 0}$$



Moment balance:

Force balance equation

Take moment about origin

$$\Rightarrow \int_P \underbrace{\vec{r} \times \vec{f}}_{r_i f_j \vec{e}_k} dv + \int_{\partial P} \underbrace{\vec{r} \times \vec{T}}_{r_i T_j \vec{e}_k} dA = \vec{0}$$

(i,j,k in cyclic symmetry)

Let us look at a specific case:

$$\int_P r_1 f_2 \vec{e}_3 dv + \int_{\partial P} \underbrace{r_1 T_2}_{(\sigma_{12} n_1 + \sigma_{22} n_2 + \sigma_{32} n_3)} \vec{e}_3 dA$$

$$\downarrow$$

$$\int_P (\underbrace{r_1}_{x_1} \sigma_{12})_{,1} + (\underbrace{r_1}_{x_1} \sigma_{22})_{,2} + (\underbrace{r_1}_{x_1} \sigma_{32})_{,3} dv$$

$$\int_P (\sigma_{12} + x_1 \sigma_{12,1} + x_1 \sigma_{22,2} + x_1 \sigma_{32,3}) dv$$

$$\boxed{\int_P (\sigma_{12} + x_1 (-f_2)) dv} + \int_P x_1 f_2 dv = \int_P \sigma_{12} dv$$

Similarly,

$$\int_P -r_2 f_1 \vec{e}_3 dv + \int_{\partial P} \underbrace{-r_2 T_1}_{(\sigma_{11} n_1 + \sigma_{21} n_2 + \sigma_{31} n_3)} \vec{e}_3 dA$$

$$= \int_P -x_2 f_1 \vec{e}_3 dv - \int_P \{ (\sigma_{11} x_2)_{,1} + (\sigma_{21} x_2)_{,2} + (\sigma_{31} x_2)_{,3} \} dv$$

$$- \int_P x_2 (\underbrace{\sigma_{11,1} + \sigma_{21,2} + \sigma_{31,3}}_{-f_1}) dv - \int_P \sigma_{21} dv = \int_P -\sigma_{21} dv$$

$$\Rightarrow \text{Corresponding to } \vec{e}_3, \text{ we have } \int_P (\sigma_{12} - \sigma_{21}) dv = 0$$

$$\Rightarrow \boxed{\sigma_{12} = \sigma_{21}} \leftarrow \text{Stress symmetry!}$$

$$\text{OR } \boxed{\sigma_{ij} = \sigma_{ji}} \Rightarrow [\sigma]^T = [\sigma]$$

This means that stress tensor is symmetric \Rightarrow

when we look for those specific directions \vec{n} such that

$$\vec{T} = [\sigma] \vec{n} = \lambda \vec{n} \sim \text{i.e. } \vec{T} \text{ directed along } \vec{n}$$

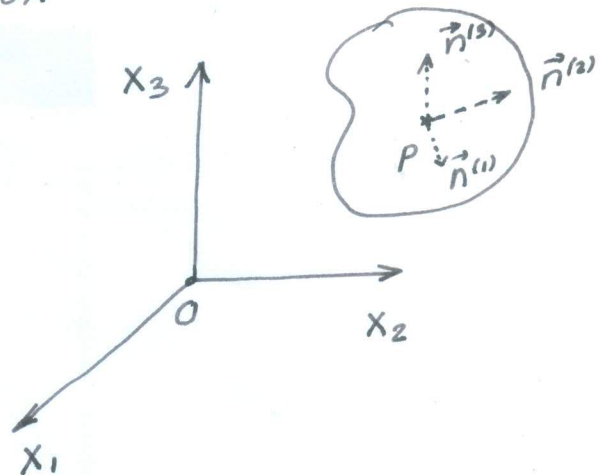
$$\Rightarrow \lambda_i \rightarrow \text{EIGENVALUE}, \vec{n}^{(i)} \rightarrow \text{EIGENVECTOR}$$

3 perpendicular directions \sim PRINCIPAL DIRECTIONS

$\lambda_i \rightarrow$ PRINCIPAL STRESSES

$\vec{n}^{(i)} \rightarrow$ PRINCIPAL DIRECTION

$\sum x_1^P, x_2^P, x_3^P \sim$ forms a rotated coordinate system.



OCTAHEDRAL PLANE:

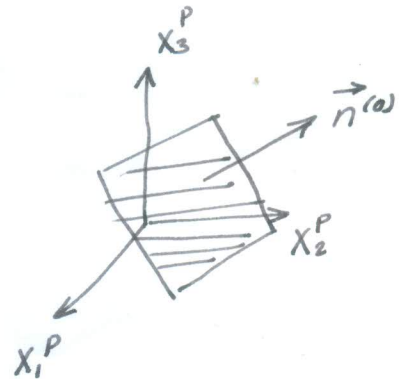
Equally inclined to the 3 principal directions,

$$\Rightarrow \vec{n}^{(0)} = \frac{1}{\sqrt{3}} (i' + j' + k')$$

as $n_1^{(0)} = n_2^{(0)} = n_3^{(0)}$ and

$$n_1^{(0)2} + n_2^{(0)2} + n_3^{(0)2} = 1$$

On this plane, the normal stress is:



$$\begin{aligned} \vec{T} \cdot \vec{n}^{(0)} &= T_1 n_1^{(0)} + T_2 n_2^{(0)} + T_3 n_3^{(0)} = T_n \\ &= (\sigma_{11} n_1^{(0)} + \cancel{\sigma_{21} n_2^{(0)}} + \cancel{\sigma_{31} n_3^{(0)}}) n_1^{(0)} + (\cancel{\sigma_{12} n_1^{(0)}} + \sigma_{22} n_2^{(0)} + \\ &\quad \cancel{\sigma_{32} n_3^{(0)}}) n_2^{(0)} + (\cancel{\sigma_{13} n_1^{(0)}} + \cancel{\sigma_{23} n_2^{(0)}} + \sigma_{33} n_3^{(0)}) n_3^{(0)} \\ &= \lambda_1 n_1^{(0)2} + \lambda_2 n_2^{(0)2} + \lambda_3 n_3^{(0)2} = \frac{(\lambda_1 + \lambda_2 + \lambda_3)}{3} \\ &= \tilde{\sigma}_H \leftarrow \text{HYDROSTATIC STRESS.} \end{aligned}$$

The shear part is $\vec{T} - \vec{T} \cdot \vec{n}^{(0)} \vec{n}$ or T_s

$$\begin{aligned} |T_s|^2 + T_n^2 &= |\vec{T}|^2 \Rightarrow |T_s|^2 = \left(\sigma_{11}^2 n_1^{(0)2} + \sigma_{22}^2 n_2^{(0)2} + \sigma_{33}^2 n_3^{(0)2} \right) - \\ &\quad \left(\frac{(\lambda_1 + \lambda_2 + \lambda_3)}{3} \right)^2 = \frac{\lambda_1^2 + \lambda_2^2 + \lambda_3^2}{3} - \frac{(\lambda_1 + \lambda_2 + \lambda_3)^2}{9} \\ &= \frac{2}{9} (\lambda_1^2 + \lambda_2^2 + \lambda_3^2) - \frac{2}{9} (\lambda_1 \lambda_2 + \lambda_1 \lambda_3 + \lambda_2 \lambda_3) \end{aligned}$$

$$= \frac{1}{9} \left[(\lambda_1 - \lambda_2)^2 + (\lambda_2 - \lambda_3)^2 + (\lambda_3 - \lambda_1)^2 \right]$$

$$\Rightarrow |T_s^{(0)}| = \sqrt{\frac{1}{9} \left[(\lambda_1 - \lambda_2)^2 + (\lambda_2 - \lambda_3)^2 + (\lambda_3 - \lambda_1)^2 \right]}$$

UNI-AXIAL LOAD $\Rightarrow \lambda_1 \neq 0, \lambda_2 = \lambda_3 = 0$

$$\Rightarrow |T_s^{(0)}| = \sqrt{\frac{1}{9} (2\lambda_1^2)^2} = \sqrt{\frac{2}{9}} \lambda_1$$

\Rightarrow At critical failure in multi-dimensional stress state the critical octahedral shear is constant, i.e.

$$|T_s^{(0)}|_c = T_{s,c} = \frac{\sqrt{2}}{3} \lambda_{1,c} = \frac{\sqrt{2}}{3} \sigma_Y$$

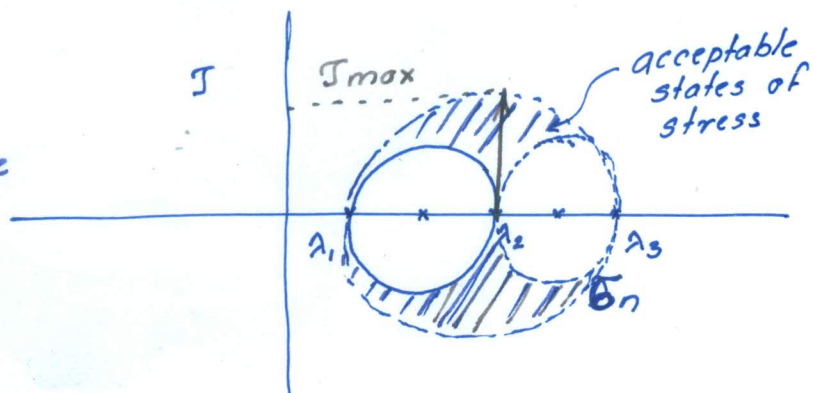
$$\Rightarrow \sqrt{\frac{1}{2} \left\{ (\lambda_1 - \lambda_2)^2 + (\lambda_2 - \lambda_3)^2 + (\lambda_3 - \lambda_1)^2 \right\}} = \sigma_Y$$

↑
MISES CRITERION

* NOTE: Since 1970 it is established that for DUCTILE metals (steel, Al-alloys) σ_H also plays a role, especially at higher temperatures, in initiation and growth of damage.

3D MOHR'S CIRCLE :

$\lambda_1, \lambda_2, \lambda_3$ define the radii of the 3 circles.



ISOTROPIC MATERIAL: Constitutive relationship for linear elasticity

RELATE STRESS TO STRAIN: (HOOKE'S LAW!)

$$\sigma_{ij} = \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij}$$

Takes care of rotation independence of relationship, as ϵ_{kk} is an invariant

$\lambda, \mu \rightarrow$ LAME'S CONSTANT

$$\begin{aligned} \therefore \sigma'_{ij} &= \lambda \epsilon'_{kk} \delta_{ij} + 2\mu \epsilon'_{ij} \\ &= \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij} \end{aligned}$$

OR $\sigma_{kk} = 3\lambda \epsilon_{kk} + 2\mu \epsilon_{kk} \Rightarrow \epsilon_{kk} = \frac{\sigma_{kk}}{(3\lambda + 2\mu)}$

$$\Rightarrow 2\mu \epsilon_{ij} = \sigma_{ij} - \frac{\lambda \sigma_{kk}}{(3\lambda + 2\mu)} \delta_{ij}$$

$$\Rightarrow \epsilon_{ij} = \frac{1}{2\mu} \sigma_{ij} - \frac{\lambda}{(3\lambda + 2\mu)(2\mu)} \sigma_{kk} \delta_{ij} \quad \text{+ } \alpha \delta_{ij} \Delta T \quad \text{for thermal}$$

$$\epsilon_{11} = \frac{\sigma_{11}}{2\mu} - \frac{\lambda}{2\mu(3\lambda + 2\mu)} (\sigma_{11} + \sigma_{22} + \sigma_{33})$$

$$\epsilon_{12} = \left(\frac{1}{2\mu} \right) \sigma_{12} \quad \text{shear modulus} \quad \frac{1}{E}$$

$$\epsilon_{11} = \left(\frac{1 \cdot \sigma_{11}}{2\mu(3\lambda + 2\mu)} \right) (2\lambda + 2\mu) - \left(\frac{\lambda}{2\mu(3\lambda + 2\mu)} \right) (\sigma_{22} + \sigma_{33})$$

$$\Rightarrow \text{Young's modulus} \quad E = \frac{\mu(3\lambda + 2\mu)}{(\lambda + \mu)}$$

$$\text{Poisson's ratio} \quad \nu = \frac{\lambda}{2(\lambda + \mu)}$$

CHECK ALGEBRA!

* SHOW THAT $-1 < \nu < \frac{1}{2}$ IS A NECESSARY MATERIAL CONSTRAINT, WITH $E > 0$

So, we get

$$\epsilon_{11}^E = \frac{1}{E} (\sigma_{11} - \nu(\sigma_{22} + \sigma_{33}))$$

$$\epsilon_{22}^E = \frac{1}{E} (\sigma_{22} - \nu(\sigma_{11} + \sigma_{33}))$$

$$\epsilon_{33}^E = \frac{1}{E} (\sigma_{33} - \nu(\sigma_{11} + \sigma_{22}))$$

$$\epsilon_{12}^E = \frac{1}{2\mu} \sigma_{12}; \quad \epsilon_{13}^E = \frac{1}{2\mu} \sigma_{13}; \quad \epsilon_{23}^E = \frac{1}{2\mu} \sigma_{23};$$

What is $2\mu = ?$

$$2\mu = \frac{E}{(1+\nu)}$$

← SHOW THIS!

* How are E, ν or λ, μ MEASURED in the laboratory?
 <FIND IT OUT> ~ ASTM standards ← testing standards.

TYPICAL E : Steel ~ 210 GPa } ν : ~ 0.3
 Al ~ 70 GPa }

THERMAL STRAIN:

Who gives temperature at every material point?



$$+ \rho \frac{\partial T}{\partial t} - k \nabla^2 T = r \quad \leftarrow \text{Heat conduction}$$

with T or q_n (heat flux) given on boundary!

ASSUMPTION:

ΔT IS UNIFORM everywhere

Thermal strain $\epsilon_{ij}^T = \alpha \Delta T \delta_{ij}$
 α coefficient of thermal expansion

Then TOTAL STRAIN $\boxed{\epsilon_{ij} = \epsilon_{ij}^E + \epsilon_{ij}^T}$

OR

$$\boxed{\epsilon_{11} = \frac{1}{E} (\sigma_{11} - \lambda (\sigma_{22} + \sigma_{33})) + \alpha \Delta T}$$

$\epsilon_{22}, \epsilon_{33}$ follow similar forms

$$\boxed{\epsilon_{12} = \frac{1}{2\mu} \sigma_{12}}; \quad \epsilon_{13} = \frac{1}{2\mu} \sigma_{13}; \quad \epsilon_{23} = \frac{1}{2\mu} \sigma_{23}$$

No thermal influence in SHEAR!

* Note that if thermal expansion is constrained, i.e. if $\epsilon_{11} = 0$, while the material is being heated, then

$$\epsilon_{11} = \epsilon_{11}^E + \epsilon_{11}^P = 0 \Rightarrow \boxed{\epsilon_{11}^E = -\epsilon_{11}^P}$$

$$\epsilon_{ij} = \frac{1}{2\mu} \sigma_{ij} - \frac{\lambda}{2\mu(3\lambda+2\mu)} (\sigma_{kk}) \delta_{ij} + \alpha \Delta T \delta_{ij}$$

$$\Rightarrow (\epsilon_{ij} - \alpha \Delta T \delta_{ij}) = \epsilon_{ij}^E = \frac{1}{2\mu} \sigma_{ij} - \frac{\lambda}{2\mu(3\lambda+2\mu)} \sigma_{kk} \delta_{ij}$$

$$\Rightarrow \sigma_{ij} = \lambda (\epsilon_{kk}^E) \delta_{ij} + 2\mu \epsilon_{ij}^E = \lambda (\epsilon_{kk} - 3\alpha \Delta T) \delta_{ij} + 2\mu (\epsilon_{ij} - \alpha \Delta T \delta_{ij})$$

EXAMPLE WITH PRINCIPAL STRESSES:

Let displacement field $\vec{U}(x_1, x_2, x_3)$ be given as:

$$u_1 = (3x_1^2 - 2x_1x_3) \times 10^{-3} \text{ m}$$

$$u_2 = (6x_1x_3) \times 10^{-3} \text{ m}$$

$$u_3 = 3x_3^2 \times 10^{-3} \text{ m}.$$

\therefore The strain components are given as:

$$E_{11} = u_{1,1} = (6x_1 - 2x_3) \times 10^{-3}$$

$$E_{22} = u_{2,2} = 0$$

$$E_{33} = u_{3,3} = 6x_3 \times 10^{-3}$$

$$2E_{12} = \gamma_{12} = u_{1,2} + u_{2,1} = (0 + 6x_3) \times 10^{-3} = 6x_3 \times 10^{-3}$$

$$2E_{13} = \gamma_{13} = u_{1,3} + u_{3,1} = (-2x_1 + 0) \times 10^{-3}$$

$$2E_{23} = \gamma_{23} = u_{2,3} + u_{3,2} = (6x_1 + 0) \times 10^{-3}$$

Assume that material is isotropic, with Young's modulus $E = 70 \text{ GPa}$; $\nu = 0.3$. Then the state of stress is given by

$$\sigma_{11} = \frac{E}{(1+\nu)(1-2\nu)} (E_{11}(1-\nu) + \nu(E_{22} + E_{33}))$$

$$\sigma_{22} = \frac{E}{(1+\nu)(1-2\nu)} (E_{22}(1-\nu) + \nu(E_{11} + E_{33}))$$

$$\sigma_{33} = \frac{E}{(1+\nu)(1-2\nu)} (E_{33}(1-\nu) + \nu(E_{11} + E_{22}))$$

$$\sigma_{13} = \frac{E}{2(1+\nu)} \gamma_{13}; \quad \sigma_{23} = \frac{E}{2(1+\nu)} \gamma_{23}; \quad \sigma_{12} = \frac{E}{2(1+\nu)} \gamma_{12}$$

* Note that $\sigma_{ij}(x_1, x_2, x_3)$ is a function of position.

Let us take 2 points to elaborate:

$$P_1 = (0, 0, 1) ; P_2 = (1, 1, 0)$$

At P_1 :

$$E_{11} = -2 \times 10^{-3} ; E_{22} = 0 ; E_{33} = 6 \times 10^{-3} ; \gamma_{12} = 6 \times 10^{-3} ; \gamma_{13} = \gamma_{23} = 0$$

At P_2 :

$$E_{11} = 6 \times 10^{-3} ; E_{22} = 0 ; E_{33} = 0 ; \gamma_{12} = 0 ; \gamma_{13} = -2 \times 10^{-3} ; \gamma_{23} = 6 \times 10^{-3}$$

At P_1 :

$$\begin{aligned} \tilde{\sigma}_{11} &= \frac{E}{1.3 \times 0.4} (E_{11} \times 0.7 + 0.3(E_{22} + E_{33})) = \frac{25E}{13} (-1.4 \times 10^{-3} + 1.8 \times 10^{-3}) \\ &= \frac{25E \times 0.4 \times 10^{-3}}{13} = \frac{10E}{13} \times 10^{-3} = \frac{10 \times 70 \times 10^9 \times 10^{-3}}{13} \approx 5.384 \times 10^7 \approx 53.8 \text{ MPa} \end{aligned}$$

$$\tilde{\sigma}_{22} = \frac{25E}{13} (0 + 0.3 \times (-2 \times 10^{-3} + 6 \times 10^{-3})) = \frac{25E}{13} \times 1.2 \times 10^{-3} = 16.154 \times 10^7 \approx 161.5 \text{ MPa}$$

$$\tilde{\sigma}_{33} = \frac{25E}{13} (0.7 \times 6 \times 10^{-3} + 0.3(-2 \times 10^{-3} + 0)) = \frac{25E}{13} \times 3.6 \times 10^{-3} = 484.6 \text{ MPa}$$

$$\tilde{\sigma}_{12} = \frac{E}{2.6} \times 6 \times 10^{-3} = \frac{30E}{13} \times 10^{-3} = 161.5 \text{ MPa}$$

$$\tilde{\sigma}_{13} = \frac{E}{2.6} \times 0 = 0 ; \tilde{\sigma}_{23} = 0$$

$$\begin{aligned} &\text{OR} \\ &\begin{bmatrix} \frac{10E}{13} \times 10^{-3} & \frac{30E}{13} \times 10^{-3} & 0 \\ \frac{30E}{13} \times 10^{-3} & \frac{30E}{13} \times 10^{-3} & 0 \\ 0 & 0 & \frac{90E}{13} \times 10^{-3} \end{bmatrix} = [\sigma] = \begin{bmatrix} 53.8 & 161.5 & 0 \\ 161.5 & 161.5 & 0 \\ 0 & 0 & 484.6 \end{bmatrix} \times 10^6 \end{aligned}$$

$[\sigma] \vec{n} = \lambda \vec{n}$ is the eigenvalue problem corresponding to this state of stress.

$$\therefore \det [[\sigma] - \lambda [I]] = 0 \Rightarrow (484.6 - \lambda) [(53.8 - \lambda)(161.5 - \lambda) - (161.5)^2] = 0$$

$$\therefore \lambda^2 - \lambda(161.5 + 53.8) + 53.8 \times 161.5 - (161.5)^2 = 0$$

$$\lambda_1 = 484.6 \text{ MPa} ; \lambda_2 = 277.9 \text{ MPa} ; \lambda_3 = -62.6 \text{ MPa}$$

Now that we have the principal stresses, we can compute the octahedral shear stress $\tau_{oct} = \sqrt{\frac{1}{9} \left[(\lambda_1 - \lambda_2)^2 + (\lambda_2 - \lambda_3)^2 + (\lambda_3 - \lambda_1)^2 \right]}$

$$\Rightarrow \tau_{oct} = \sqrt{\frac{1}{9} \left[\underbrace{(484.6 - 277.9)^2}_{42724.9} + \underbrace{(277.9 + 62.6)^2}_{115940.3} + \underbrace{(484.6 + 62.6)^2}_{299427.8} \right]}$$

$$= 225.6 \text{ MPa}$$

Hydrostatic stress $\sigma_H = \frac{1}{3} (\lambda_1 + \lambda_2 + \lambda_3) = 233.3 \text{ MPa}$

Mises criterion \Rightarrow we check for yielding, i.e. if

$$\sigma_{RMS} = \sqrt{\frac{1}{2} \left[(\lambda_1 - \lambda_2)^2 + (\lambda_2 - \lambda_3)^2 + (\lambda_3 - \lambda_1)^2 \right]} \leq \sigma_Y$$

$$\sqrt{\frac{9}{2}} \tau_{oct} = 2.12 \tau_{oct} = 478.6 \text{ MPa}$$

For Al-alloys we have $\sigma_Y \approx 324 \text{ MPa}$ (2024-Al alloy)

$\therefore \sigma_{RMS} > \sigma_Y$ — hence material has yielded.

* Note that we can also write

$$\sigma_{RMS} = \sqrt{\frac{1}{2} \left[(\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2 + 6(\sigma_{12}^2 + \sigma_{13}^2 + \sigma_{23}^2) \right]}$$

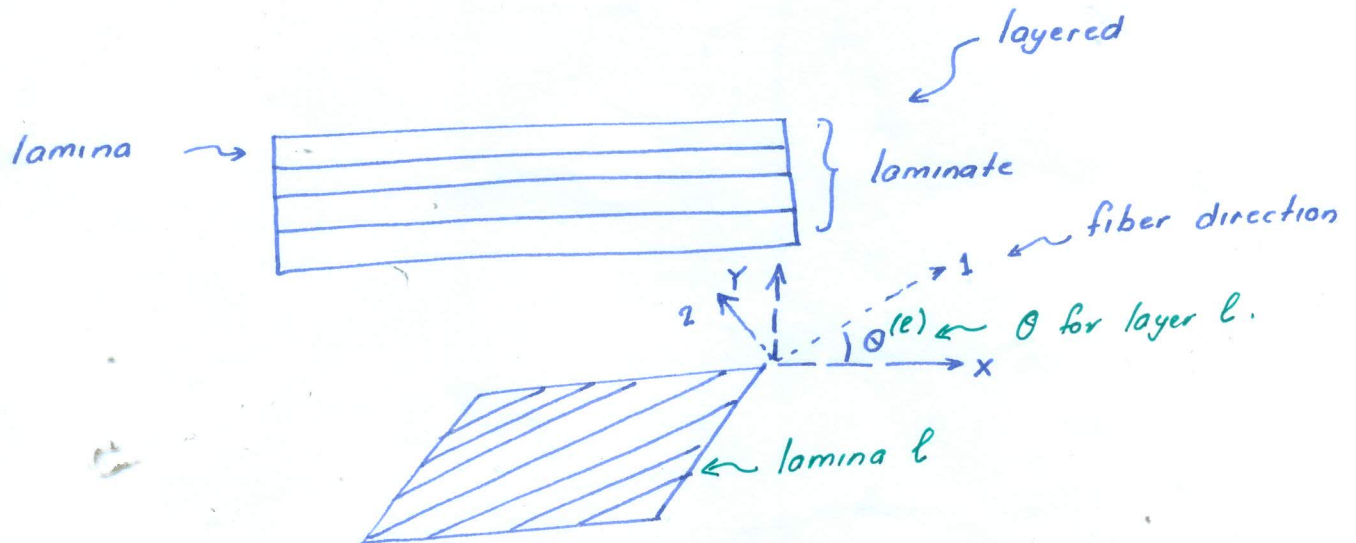
$$= \frac{1}{2} \left[\underbrace{(53.8 - 161.5)^2}_{11599.29} + \underbrace{(161.5 - 684.6)^2}_{104353.6} + \underbrace{(684.6 - 53.8)^2}_{185588.6} + 6(161.5^2 + 0) \right] = 478.6 \text{ MPa}$$

EASIER! As eigenvalues need not be computed!

* DO THE SAME EXERCISE FOR POINT P2.

Composites

- Tailorable, high specific strength and stiffness
- Good fatigue life and corrosion resistant



- Thin structures, hence plane stress assumption works
- Effective properties needed ~ tests, formulae.

3D :

$$\epsilon_{11} = \frac{\sigma_1}{E_1} - \frac{\nu_{21}}{E_2} \sigma_2 - \frac{\nu_{31}}{E_3} \sigma_3$$

$$\epsilon_2 = -\frac{\nu_{12}}{E_1} \sigma_1 + \frac{\sigma_2}{E_2} - \frac{\nu_{32}}{E_3} \sigma_3$$

$$\epsilon_3 = -\frac{\nu_{13}}{E_1} \sigma_1 - \frac{\nu_{23}}{E_2} \sigma_2 + \frac{\sigma_3}{E_3}$$

$$\gamma_{23} = \frac{\sigma_{23}}{G_{23}}; \quad \gamma_{13} = \frac{\sigma_{13}}{G_{13}}; \quad \gamma_{12} = \frac{\sigma_{12}}{G_{12}}$$

$$\nu_{ij} = -\frac{\epsilon_j}{\epsilon_i}$$

$$\nu_{ij} \neq \nu_{ji}$$

$$\nu_{ij} \frac{E_j}{E_i} = \nu_{ji}$$

The properties are given in the 1-2-3 system. To convert back to x-y-z system we use coordinate rotation.

How do we do this?

Note that we can write the above in the form

$$[\varepsilon^{(n)}] = [S^{(n)}][\sigma^{(n)}] \quad \text{or} \quad \varepsilon_{ij}^1 = S_{ijkl}^1 \sigma_{kl}^1$$

But writing the stress tensor as a vector

$$[C^{(n)}] = [S^{(n)}]^{-1}$$

$$\{\sigma^{(n)}\} = [C^{(n)}]\{\varepsilon^{(n)}\} \quad \text{with} \quad \{\sigma^{(n)}\} = [T_1]\{\sigma^{(x)}\}$$

$$[T_1] = \begin{bmatrix} m^2 & n^2 & 0 & 0 & 0 & 2mn \\ n^2 & m^2 & 0 & 0 & 0 & -2mn \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & m & -n & 0 \\ 0 & 0 & 0 & n & m & 0 \\ -mn & mn & 0 & 0 & 0 & (m^2 - n^2) \end{bmatrix}$$

$$m = \cos \theta$$

$$n = \sin \theta$$

$$\text{Also, } \{\varepsilon^{(n)}\} = [T_2]\{\varepsilon^{(x)}\}$$

Noting that normally we use γ_{ij} in the shear case ($\gamma_{ij} = 2 \varepsilon_{ij}$)

$[T_2]$ has all entries same as $[T_1]$, except:

$$\boxed{T_{26,1} = 2 T_{16,1} \quad ; \quad T_{26,2} = 2 T_{16,2}} \quad ; \quad T_{21,6} = \frac{1}{2} T_{11,6} ;$$

$$T_{22,6} = \frac{1}{2} T_{12,6} .$$

Thus we get :

$$\{\sigma^{(x)}\} = \underbrace{[T_1^{-1}][C^{(n)}][T_2]}_{[C^{(x)}]}\{\varepsilon^{(x)}\}$$

* Write a small matlab code to do this part for various values of θ .