

Variational Approach to Crack Propagation in a Cantilever Beam

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1 Linear Elasticity Basic Theory

We define:

$$\begin{aligned}\Omega &\subset \mathbb{R}^d \ (d = 2, 3) \\ u &: \Omega \rightarrow \mathbb{R}^d \text{ (displacement)} \\ e[v] &:= \frac{1}{2}(\nabla^T v + \nabla v^T) \text{ (strain)} \\ \nabla^T v &:= \begin{pmatrix} \partial_1 v_1 & \partial_2 v_2 \\ \partial_1 v_2 & \partial_2 v_1 \end{pmatrix} \\ \nabla v^T &:= (\nabla^T v)^T \\ \sigma[u] &:= \mathcal{C}e[u] \\ \mathcal{C} &= (C_{ijkl}) \begin{cases} C_{ijkl} = C_{klij} = C_{jikl} \\ (C_\xi) : \xi \geq C_* |\xi|^2 (\forall \xi \in \mathbb{R}_{sym}^{d \times d}) \end{cases}\end{aligned}$$

Let's consider linear elasticity problem:

$$(**) \begin{cases} -div \sigma[u] = f(x), \text{ in } \Omega \\ u = g(x) \text{ on } \Gamma_D \\ \sigma[u]\nu = q(x) \text{ on } \Gamma_N \end{cases} \quad (1)$$

$$f \in L^2(\Omega : \mathbb{R}^d), \ g \in H^1(\Omega : \mathbb{R}^d), \ q \in L^2(\Gamma_N : \mathbb{R}^d)$$

1.1 Strong Solution

$$u \in H^2(\Omega : \mathbb{R}^d) \text{ satisfies } (**) \text{ then we call } u : \text{ a strong solution} \quad (2)$$

1.2 Weak Solution

$$\begin{cases} \int_{\Omega} \sigma[u] : e[v] dx = \int_{\Omega} f \cdot v dx + \int_{\Gamma_N} q \cdot v ds (\forall v \in V := \{v \in H^1(\Omega : \mathbb{R}^d) \mid v|_{\Gamma_D} = 0\}) \\ u \in V + g \end{cases} \quad (3)$$

1.3 Proposition

$$u : \text{ strong solution} \Leftrightarrow \begin{cases} u : \text{ weak solution} \\ u \in H^2(\Omega : \mathbb{R}^d) \end{cases}$$

Proof. (\Rightarrow) Assume we choose $v \in V := \{v \in H^1(\Omega : \mathbb{R}^d) \mid v|_{\Gamma_D} = 0\}$, with v is a very smooth test function. Then we take integral over the domain for equation (1) on both side.

$$\begin{aligned} \int_{\Omega} -\operatorname{div} \sigma[u] \cdot v dx &= \int_{\Omega} f \cdot v dx \\ \int_{\Omega} \sigma[u] : \nabla v dx - \int_{\Gamma} \sigma[u] \nu \cdot v ds &= \int_{\Omega} f \cdot v dx \quad (\text{by Divergence Formula}) \\ \int_{\Omega} \sigma[u] : \nabla v dx - \left(\int_{\Gamma_D} \sigma[u] \nu \cdot v ds + \int_{\Gamma_N} \sigma[u] \nu \cdot v ds \right) &= \int_{\Omega} f \cdot v dx \end{aligned}$$

From Boundary Condition we know that:

$$\begin{cases} v = 0 & \text{on } \Gamma_D \\ \sigma[u] \nu = q & \text{on } \Gamma_N \end{cases}$$

Hence, we have:

$$\begin{aligned} \int_{\Omega} \sigma[u] : e[v] dx - \int_{\Gamma_N} q \cdot v ds &= \int_{\Omega} f \cdot v dx \\ \int_{\Omega} \sigma[u] : e[v] dx &= \int_{\Omega} f \cdot v dx + \int_{\Gamma_N} q \cdot v ds \end{aligned} \tag{4}$$

with:

$$\begin{aligned} X &:= H^1(\Omega : \mathbb{R}^d) & a(u, v) &= \int_{\Omega} (\mathcal{C}e[u]) : e[v] dx \\ a(u, v) &:= \int_{\Omega} \sigma[u] : e[v] dx & &= \int_{\Omega} e[v] : (\mathcal{C}e[u]) dx \\ l(v) &:= \int_{\Omega} f \cdot v dx + \int_{\Gamma_N} q \cdot v ds & &= a(v, u) \end{aligned}$$

Then we rewrite equation (4) in a bilinear and linear form:

$$a(u, v) = l(v)$$

(\Leftarrow) Since $u \in H^2(\Omega : \mathbb{R}^2)$, $\operatorname{div}(\sigma[u]) \in L^2(\Omega : \mathbb{R}^2)$ and $a(u, v) = 0$ for all $v \in V$, we have:

$$\begin{aligned} 0 &= \int_{\Omega} \sigma[u] : e[v] dx \\ 0 &= \int_{\Omega} \begin{pmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{21} & \sigma_{22} \end{pmatrix} : (\nabla v_1 \quad \nabla v_2) dx \\ 0 &= \int_{\Omega} \begin{pmatrix} \sigma_{11} \\ \sigma_{21} \end{pmatrix} \cdot \nabla v_1 + \begin{pmatrix} \sigma_{12} \\ \sigma_{22} \end{pmatrix} \cdot \nabla v_2 dx \\ 0 &= \int_{\Omega} \operatorname{div} \begin{pmatrix} \sigma_{11} \\ \sigma_{21} \end{pmatrix} v_1 + \operatorname{div} \begin{pmatrix} \sigma_{12} \\ \sigma_{22} \end{pmatrix} v_2 - \int_{\partial\Omega} \begin{pmatrix} \sigma_{11} \\ \sigma_{21} \end{pmatrix} \cdot \nu v_1 + \begin{pmatrix} \sigma_{12} \\ \sigma_{22} \end{pmatrix} \cdot \nu v_2 ds \\ 0 &= \int_{\Omega} (f + \operatorname{div} \sigma[u]) \cdot v dx - \int_{\partial\Omega} (\sigma[u] \nu) \cdot v ds \\ 0 &= \int_{\Omega} f \cdot v dx + \int_{\Omega} \operatorname{div} \sigma[u] \cdot v dx - \int_{\partial\Omega} (\sigma[u] \nu) \cdot v ds \end{aligned}$$

then, assume we choose $v \in C_0^\infty(\Omega) \subset V$, $v = 0$ near $\partial\Omega$
 $f \in L^1(\Omega)$, then we have:

$$\begin{aligned} \int_{\Omega} (f + \operatorname{div} \sigma[u]) \cdot v dx &= 0 \quad (\forall v \in C_0^\infty(\Omega, \mathbb{R}^2)) \\ \therefore f + \operatorname{div} \sigma[u] &= 0 \text{ in } \Omega \end{aligned}$$

then, $\forall v \in C_0^\infty(\bar{\Omega})$ s.t. $(\text{supp}(v) \cap \partial\Omega) \subset \Gamma_N$

$$\begin{aligned} \int_{\Gamma_N} (\sigma[u]\nu) \cdot v ds &= 0 \\ \sigma[u]\nu &= 0 \text{ on } \Gamma_N \end{aligned}$$

□

For $v \in V$

$$\begin{aligned} a(v, v) &= \int_{\Omega} (\mathcal{C}e[v]) : e[v] dx \\ &\geq C_* \int_{\Omega} |e[v]|^2 dx \\ &\geq C_* \|v\|_x^2 \end{aligned}$$

Properties 1. • $a(\cdot, \cdot)$ is bounded symmetric, bilinear form on $X \times X$.

- $a(\cdot, \cdot)$ is coercive on $V \times V$.
- l is bounded linear form on X .

Theorem 1. For any $g \in H^1(\Omega : \mathbb{R}^d)$,

$$\exists! u : a \text{ weak solution of } (**), \text{ and } \left\{ u = \operatorname{argmin}_{w \in V+g} E(w) \right.$$

2 Linear Elasticity with Crack Propagation Case

2.1 Strong Form

Let's consider Linear Elasticity with Crack Propagation Case:

$$\begin{cases} -\operatorname{div} \sigma[u] = f(x), & (x \in \Omega \setminus \Sigma(L)) \\ u = g & (x \in \Gamma_D) \\ \sigma[u]\nu = q & (x \in \Gamma_l) \\ \sigma[u]\nu = 0 & (x \in \Gamma_0 \cup \Sigma^+(L) \cup \Sigma^-(L)) \end{cases} \quad (5)$$

2.2 Weak Form

We define

$$V_L := \{v \in H^1(\Omega_L : \mathbb{R}^2) \mid v = 0 \text{ on } \Gamma_D\}$$

then the problem (5) becomes: Find $u \in V_L$ such that,

$$\int_{\Omega_L} \sigma[u] : e[v] dx = \int_{\Omega_L} f v dx + \int_{\Gamma_l} q v ds \quad (\forall v \in V_L) \quad (6)$$

then, we define elastic energy:

$$E(v, L) := \frac{1}{2} a_L(v, v) - l(v), \quad (v \in V_L) \quad (7)$$

3 Modelling and Simulation

In this simulation, we use a cantilever beam (Mild Steel Material) as the domain, which is a thin rectangular cross section introduced by Timoshenkol Goodier (1970), then we specify nondimensionalized parameter for simulation as shown in table ??.

Numerical Parameter	Typical Value [unit]	Nondimensionalized value
Poisson's ratio (ν)	0.3 [-]	0.3
Young's modulus (E)	4.0×10^{10} [Pa]	4.0×10^7
Gravity constant (f)	9.80655 [N/kg]	9.80655
Weight (q)	0 [N]	0
Length (L)	3.0 [m]	3.0
Depth (h)	0.3 [m]	0.3
Width (b)	0.25 [m]	0.25
Effective Fracture Toughness (γ_{eff})	0.5[-]	0.5

Table 1: Material properties and numerical parameters

With the help of FreeFem++ software, we created a 2D and 3D model as shown in figure 1,

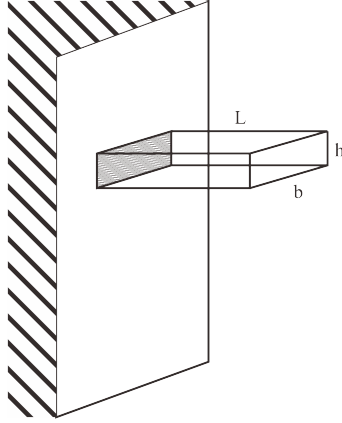


Figure 1: 3D Model of cantilever beam. We use gravity force as the body force \mathbf{f} and fixed the left part of the beam, and then we give a 1 Newton weight force act on the right part of the beam as the neumann boundary condition \mathbf{q} .

then we solve the displacement vector (u, v) . After solving the displacement, we calculate σ which stand for stress force acting on surface of the cantilever beam using equation below:

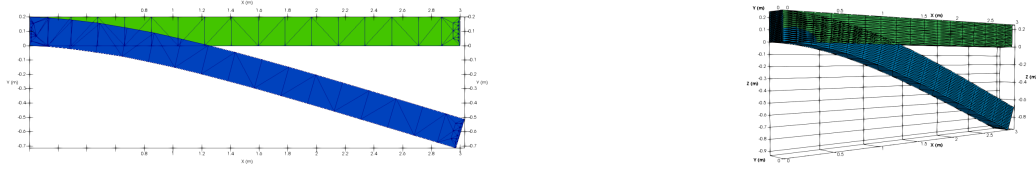
$$\sigma = (d\lambda^2 + 4\lambda\mu)div(u)^2 + (4\mu^2|e[u]|^2), \quad d = 2, 3$$

$$\lambda \text{ (Lame's first parameter)} := \frac{E\nu}{(1+\nu)(1-2\nu)}$$

$$\mu \text{ (Lame's second parameter)} := \frac{E}{2(1+\nu)}$$

4 Result and Discussion

We solved the problem in (3) by using P1 finite element method on FreeFEM++, where u and v calculated for division number of mesh = 32. In the figure 2 we can see the deformation of the cantilever beam in 2D and 3D graphics.

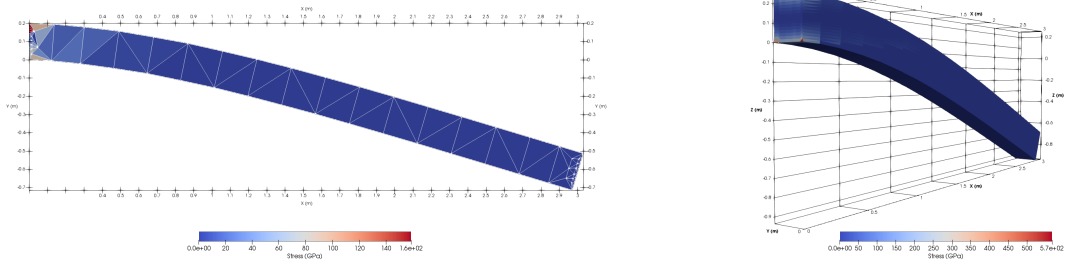


(a) Deformation in 2D. Green line show condition before gravity and weight force applied to the domain. Red line show condition after we solve linear elasticity with gravity and weight force applied to the domain. Maximal Displacement ($u = 0.03 [m]$)

(b) Deformation in 3D. Green line show condition before gravity and weight force applied to the domain. Red line show condition after we solve linear elasticity with gravity and weight force applied to the domain. Maximal Displacement ($u = 0.05 [m]$)

Figure 2: Deformation in 2D and 3D

While in the figure 3 we can see result from calculating the stress tensor on 2D and 3D case.



(a) Calculated σ on 2D case. The value of σ on the domain, mapped by the color in the picture with respect to the color palette on the lower side of the graph. Maximal stress given on the surface ($\sigma = 158.612 [GPa]$)

(b) Calculated σ on 3D case. The value of σ on the domain, mapped by the color in the picture with respect to the color palette on the lower side of the graph. Maximal stress given on the surface ($\sigma = 567.034 [GPa]$)

Figure 3: Stress Tensor in 2D and 3D

On the figure 4, we can see comparison of calculated surface stress on 2D and 3D case (sliced on the side).

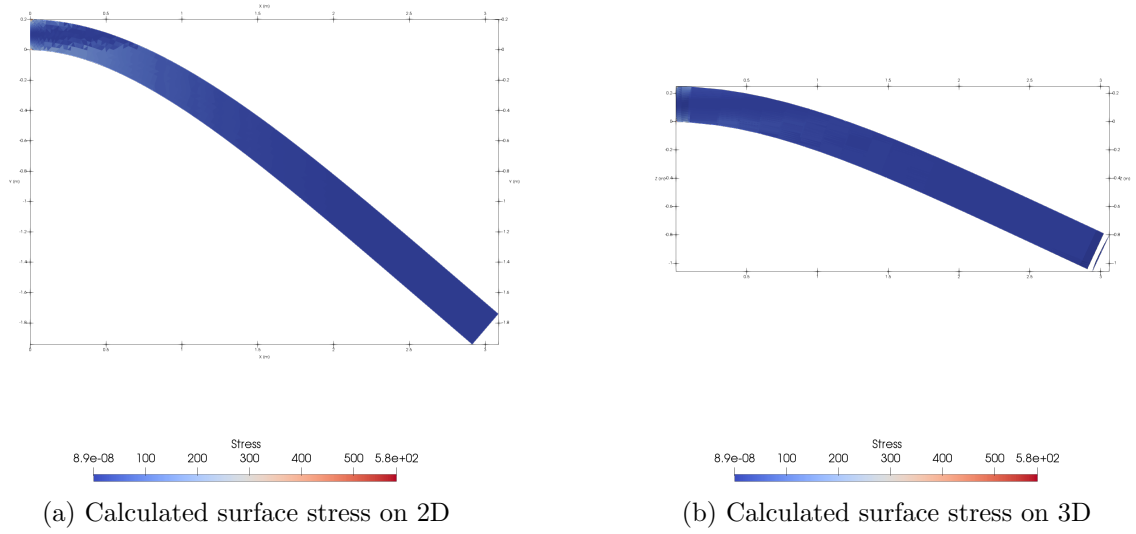


Figure 4: Comparison of Stress Tensor in 2D and 3D

On the figure 5 below, we can see the result from sliced view on 3D case, in this case, we sliced through the Y-normal plane of the 3D model.

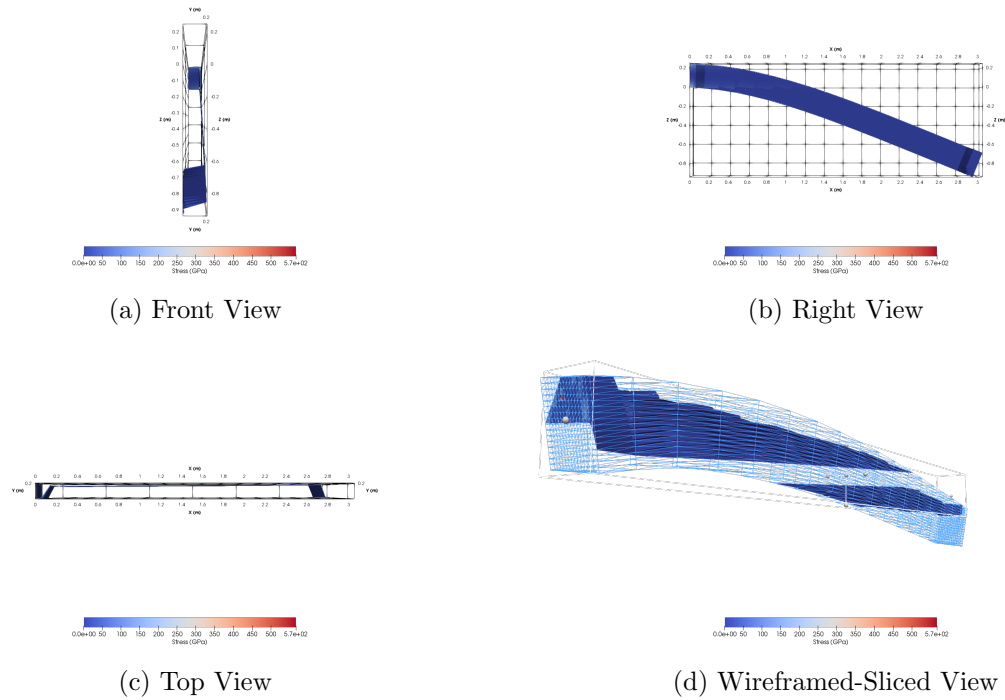


Figure 5: 3D Sliced View

4.1 Convergence Analysis

Since we don't have an exact solution for problem (1), we will define

$$u_h := u^k \iff \max |u^k - u^{k-1}| \leq \epsilon$$

k is index of the current solution and ϵ is a small number, $\epsilon > 0$.

There are three types of error that we will compute, Infinity Error, $H^1(\Omega)$ and $L^2(\Omega)$, each of them defined by:

$$\|u_h - u\|_\infty = \max |u_h - u| \quad (8)$$

$$\|u\|_{H^1(\Omega)}^2 = \int_\Omega |u|^2 dx + \int_\Omega |\nabla u|^2 dx \quad (9)$$

$$\|u\|_{L^2(\Omega)}^2 = \int_\Omega |u|^2 dx \quad (10)$$

We can see the result from 2D and 3D case in the figure 6 below:

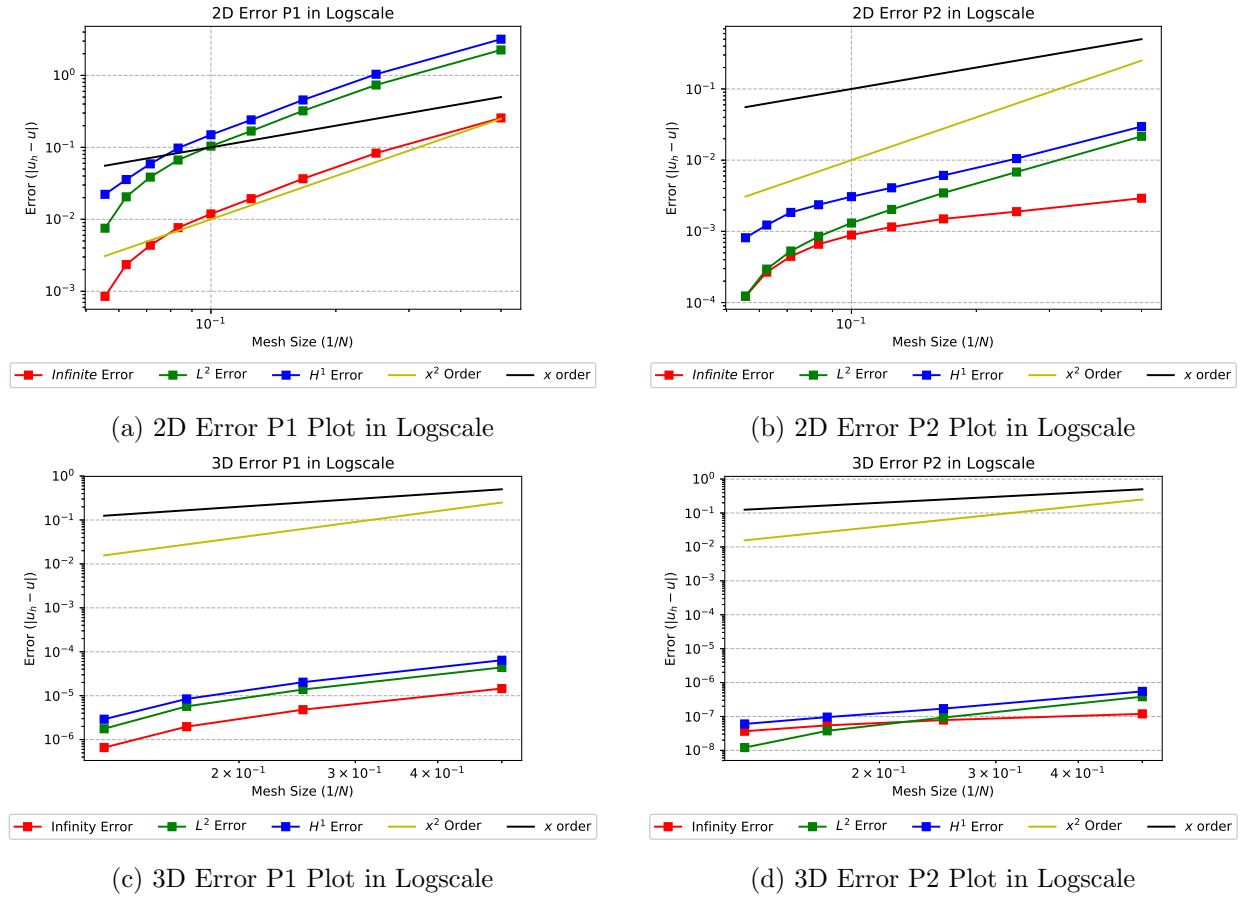


Figure 6: Error Plot in Logscale

5 Manufactured Solution

We define error estimation

$$error := ||u_n - u_{exact}|| = O(h^\alpha) \quad \alpha = \text{EOC(Experimental Order of Convergence)} \quad (11)$$

We try to solve linear elasticity based on exact solution defined by:

$$u_{exact} = \begin{pmatrix} u1 \\ u2 \end{pmatrix} = \begin{pmatrix} \sin(x_1) \\ -\sin(x_2) \end{pmatrix} \quad (12)$$

Using (12) we build linear elasticity problem as follows:

$$\begin{cases} -div \sigma[u] = f(x), & f(x) = \begin{pmatrix} \lambda \sin x_1 + 2\mu \sin x_1 \\ -\lambda \sin x_2 - 2\mu \sin x_2 \end{pmatrix} \text{ in } \Omega \\ u = g(x), & g(x) = \begin{pmatrix} \sin(x_1) \\ -\sin(x_2) \end{pmatrix} = \begin{pmatrix} \pm 1 \\ -\sin(x_2) \end{pmatrix} \text{ on } \Gamma_D \\ \sigma[u]\nu = q(x), & q(x) = \begin{pmatrix} 0 \\ \pm \lambda \cos(x_1) \end{pmatrix} \text{ on } \Gamma_N \end{cases} \quad (13)$$

Solving (13) we get the result as shown in Figure 7 and 8

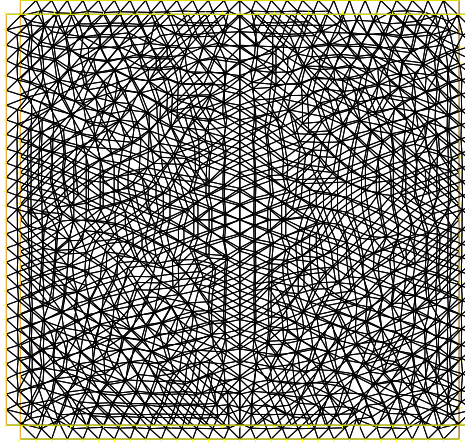


Figure 7: Deformation of Manufactured Solution

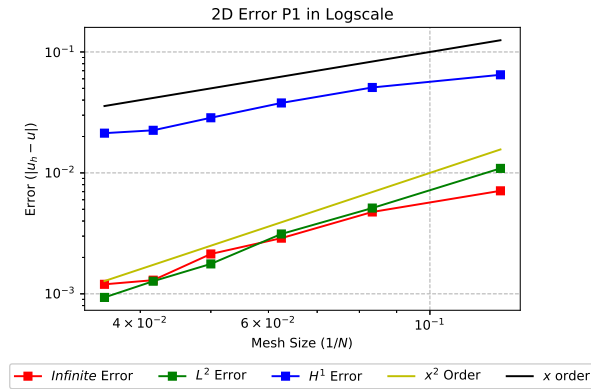


Figure 8: Error plot in Logscale

6 The Convergence of variational Approach to Crack Propagation

6.1 Simulation

We solved problem (6) using model as shown in figure 9 and numerical parameter like in table 1, the difference from linear elasticity problem is the domain. In the crack propagation case, we disturb the domain with adding some so called "crack path" into the domain. For this case, we use $n(\text{division number}) = 32$ and $d(\text{width of the crack}) = 0.01$ [m]. The result of our simulation shown the figure 10.

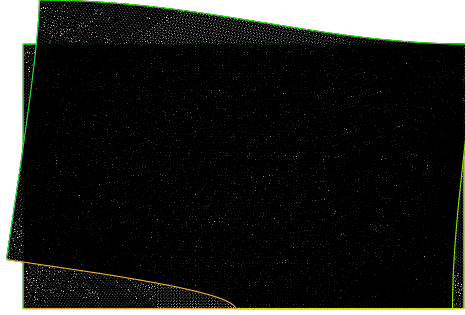


Figure 9: Deformation of Cracked Domain

6.1.1 Convergence Analysis

Since we don't have an exact solution for problem (6), we will define

$$u_h := u^k \iff \max |u^k - u^{k-1}| \leq \epsilon$$

k is index of the current solution and ϵ is a small number, $\epsilon > 0$.

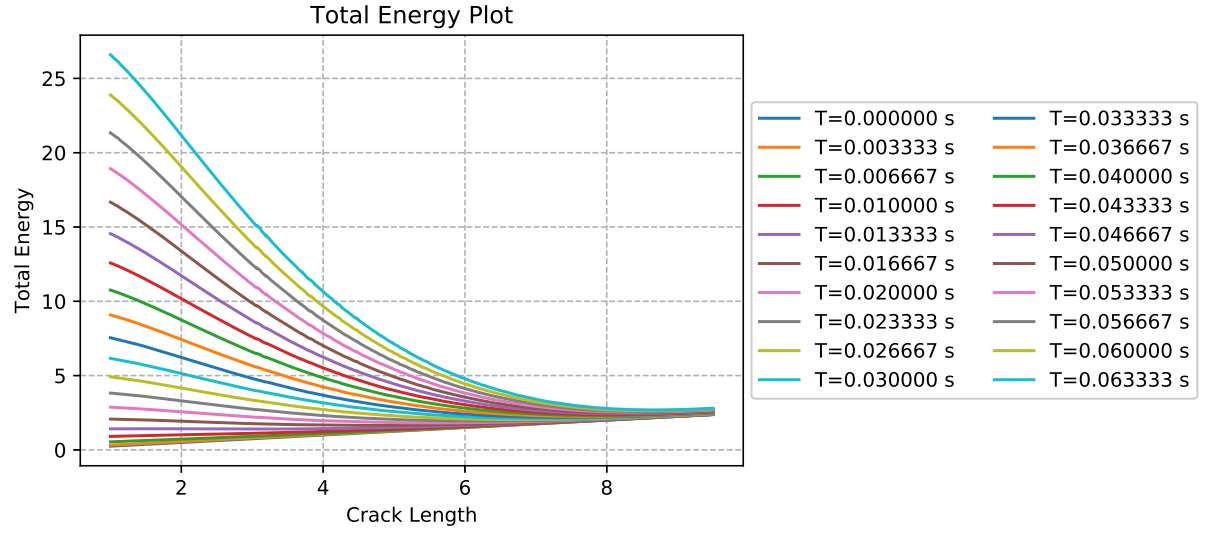
There are three types of error that we will compute, Infinity Error, $H^1(\Omega)$ and $L^2(\Omega)$, each of them defined by:

$$\|u_h - u\|_\infty = \max |u_h - u| \quad (14)$$

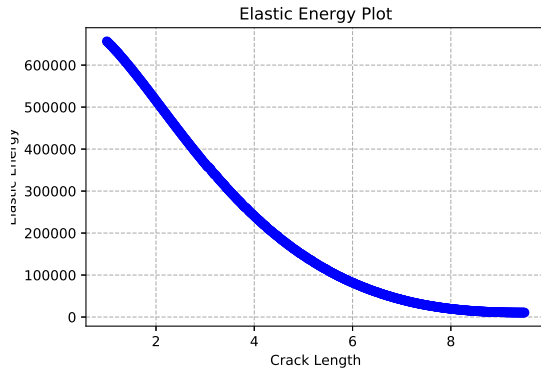
$$\|u\|_{H^1(\Omega)}^2 = \int_\Omega |u|^2 dx + \int_\Omega |\nabla u|^2 dx \quad (15)$$

$$\|u\|_{L^2(\Omega)}^2 = \int_\Omega |u|^2 dx \quad (16)$$

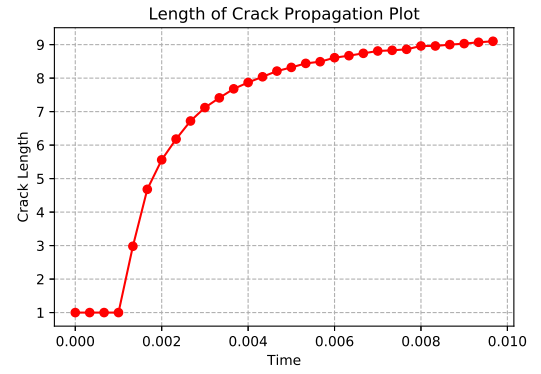
We can see the result from 2D case in the figure 11 below:



(a) Total Energy



(b) Elastic Energy



(c) Crack Length

Figure 10: Elastic Energy, Crack Length, and Total Energy

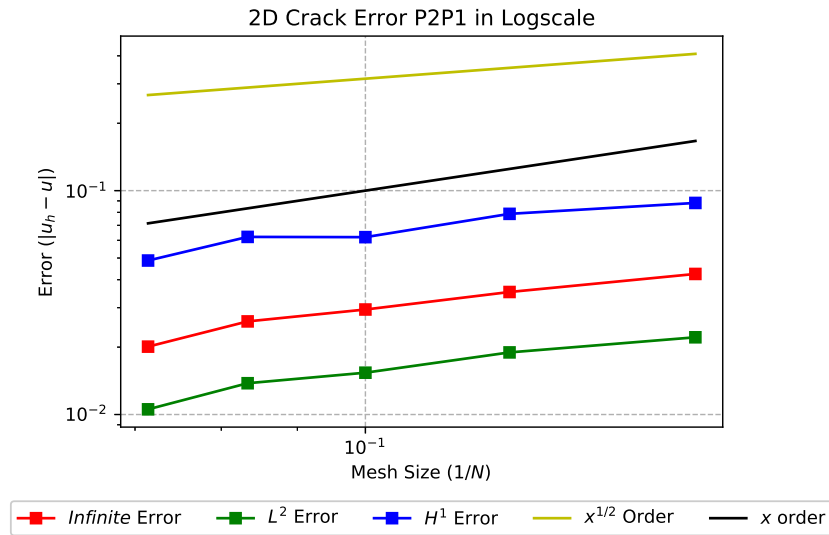


Figure 11: Convergence on 2D Cracked Domain

6.2 Case Study

We solved problem (6) using model as shown in figure 12 and numerical parameter like in table 1, the difference from simulation case is the domain. In this case study, we change the width of

the cantilever beam to 0.3[m]. The result is shown the figure 13.

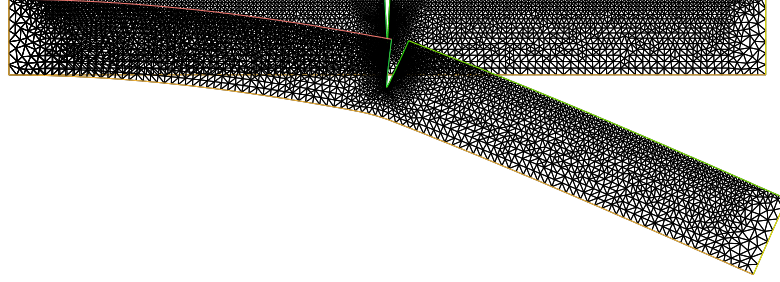


Figure 12: Deformation of Cracked Domain

6.3 Convergence Analysis

Since we don't have an exact solution for problem (6), we will define

$$u_h := u^k \iff \max |u^k - u^{k-1}| \leq \epsilon$$

k is index of the current solution and ϵ is a small number, $\epsilon > 0$.

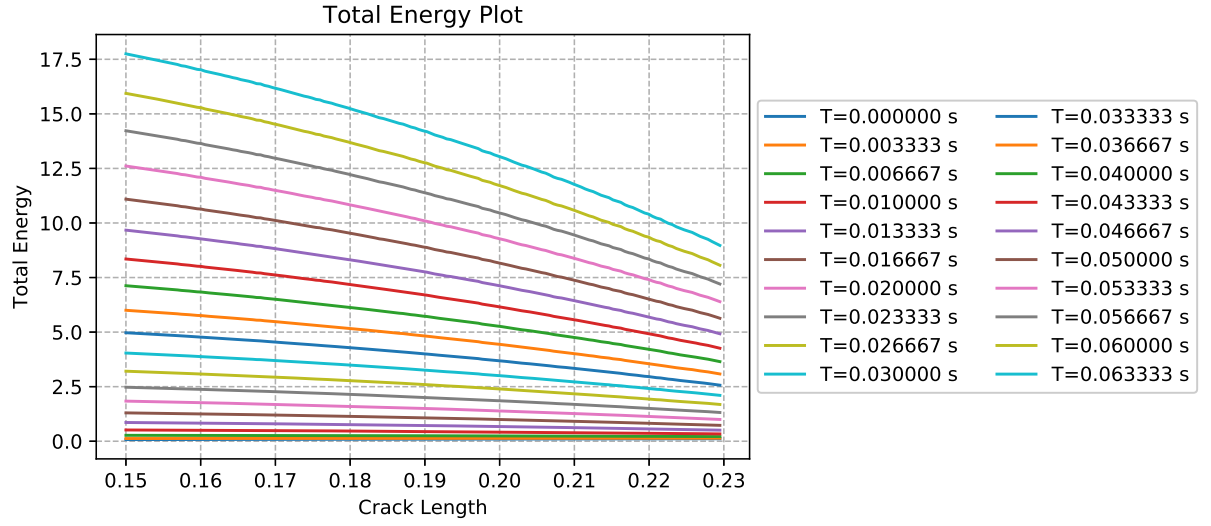
There are three types of error that we will compute, Infinity Error, $H^1(\Omega)$ and $L^2(\Omega)$, each of them defined by:

$$\|u_h - u\|_\infty = \max |u_h - u| \quad (17)$$

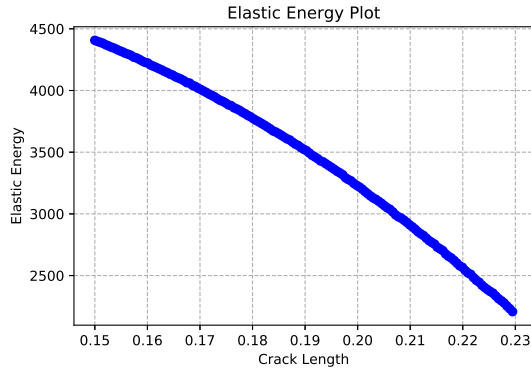
$$\|u\|_{H^1(\Omega)}^2 = \int_\Omega |u|^2 dx + \int_\Omega |\nabla u|^2 dx \quad (18)$$

$$\|u\|_{L^2(\Omega)}^2 = \int_\Omega |u|^2 dx \quad (19)$$

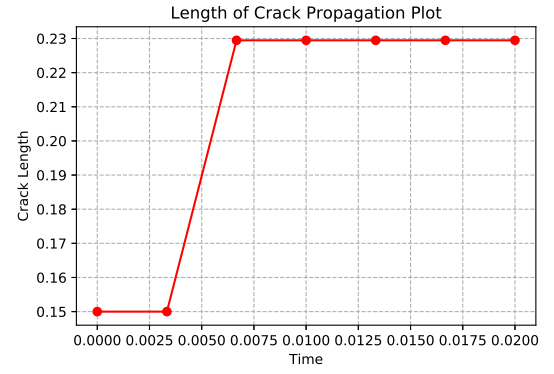
We can see the result from 2D case in the figure 14a below:



(a) Total Energy

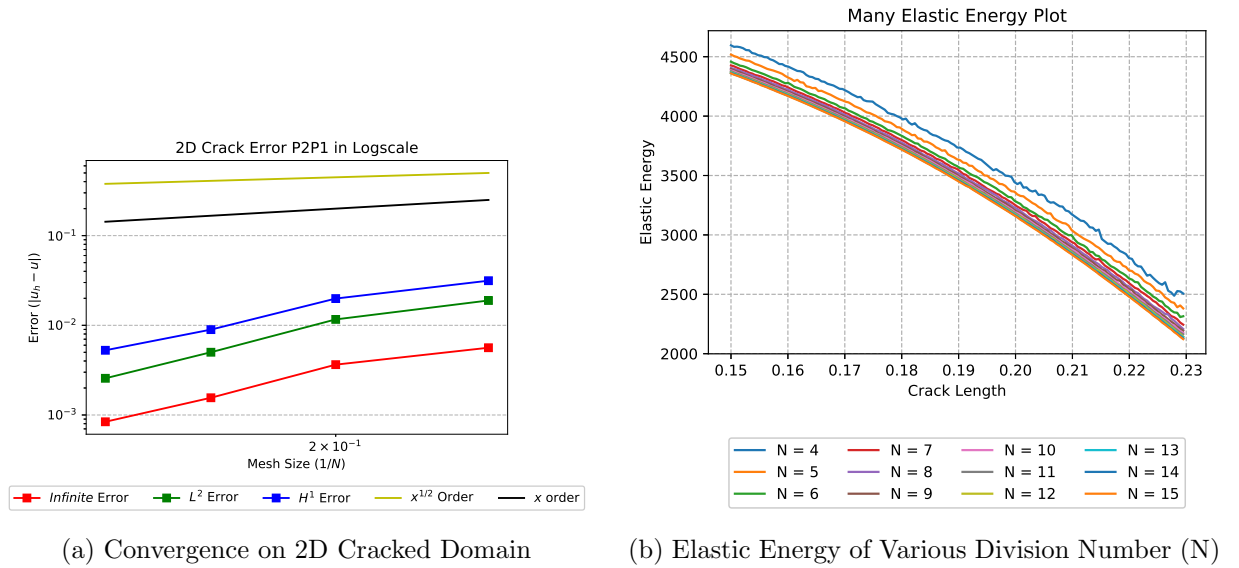


(b) Elastic Energy



(c) Crack Length

Figure 13: Elastic Energy, Crack Length, and Total Energy



(a) Convergence on 2D Cracked Domain

(b) Elastic Energy of Various Division Number (N)

Figure 14: Convergence Analysis

6.4 3D Crack Propagation Study Case

We solved problem (6) in 3D using model as shown in figure 15 and numerical parameter like in table 1. The result is shown the figure 16.

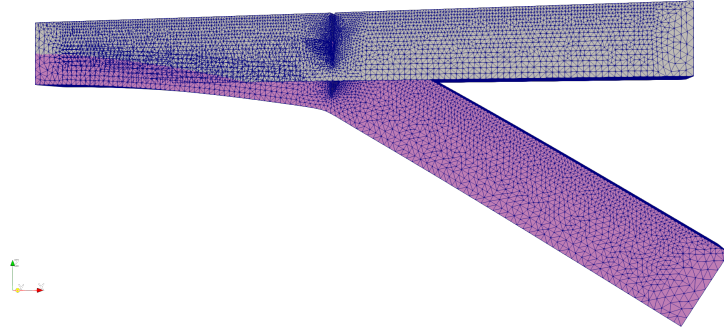


Figure 15: Deformation of Cracked Domain in 3D

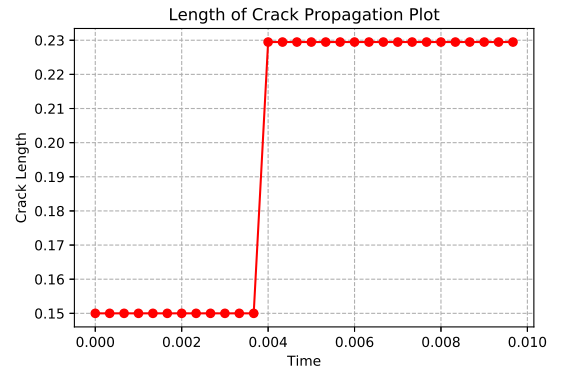
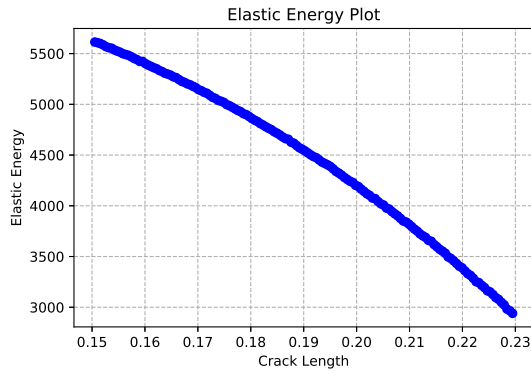
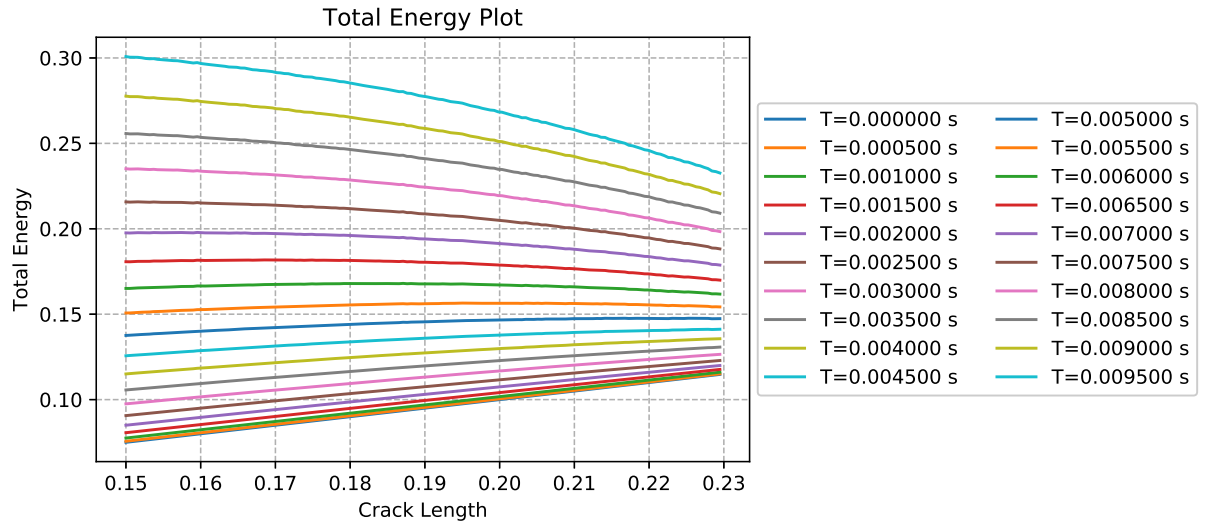


Figure 16: Elastic Energy, Crack Length, and Total Energy

6.5 Crack Location and Geometry Analysis

Figure 18 show the result from various type of crack location based on table 2 and crack dimension in figure 17.

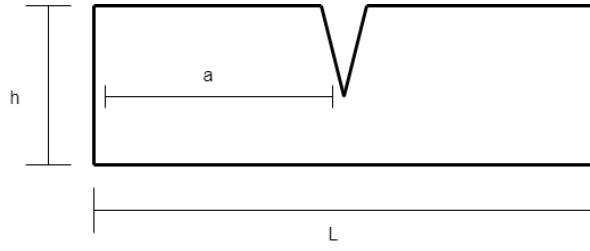


Figure 17: Crack Dimension

Number (i)	a[m]
0	1.5
1	1.4
2	1.3
3	1.2
4	1.1
5	1
6	0.9
7	0.8
8	0.7
9	0.6
10	0.5
11	0.4
12	0.3
13	0.2

Table 2: Variation of Crack Location (a)

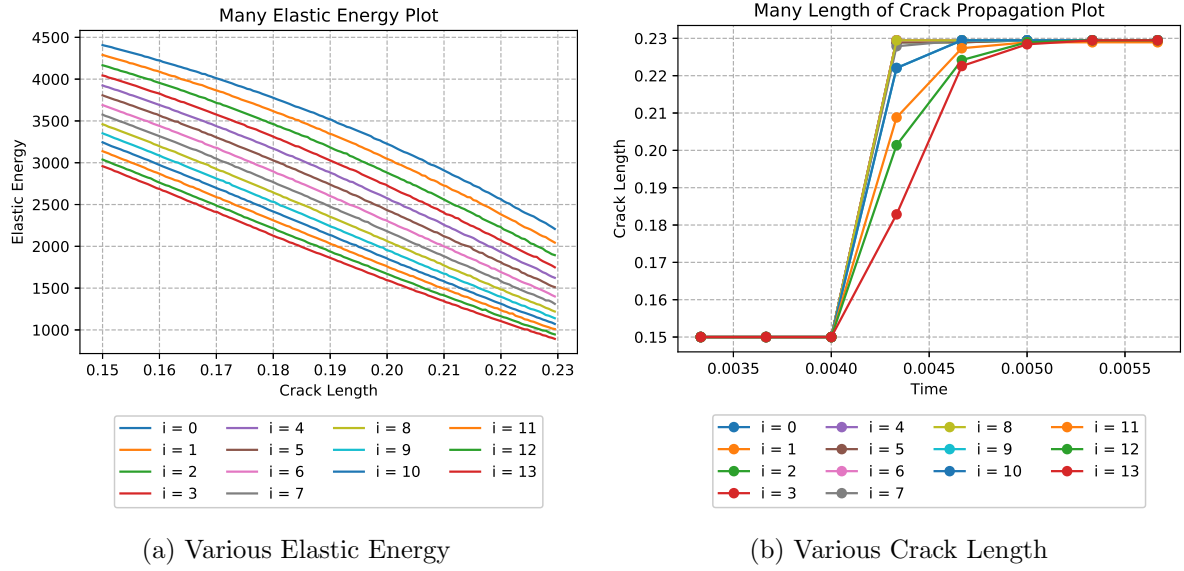


Figure 18: Elastic Energy, Crack Length of Various Crack Location

While figure 19 show the result of various geometry ratio in table 3.

Number (i)	L (m)	h (m)	Ratio
0	3	0.3	10
1	5	0.3	16.67
2	7	0.3	23.33
3	9	0.3	30
4	11	0.3	36.67

Table 3: Variation of Geometry Ratio

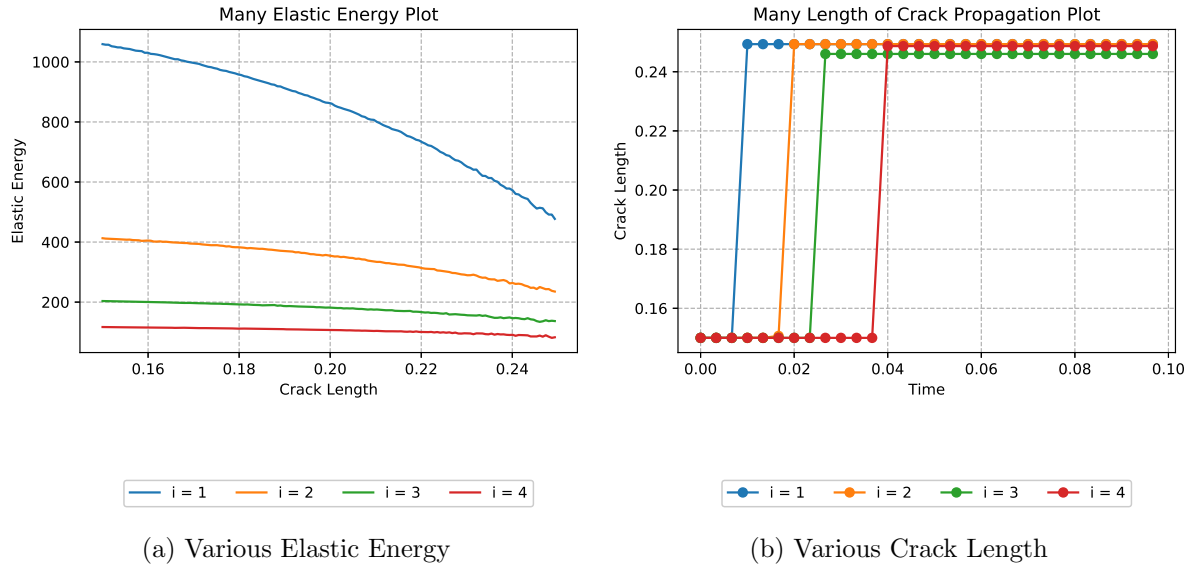


Figure 19: Elastic Energy, Crack Length of Various Geometry Ratio