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THESIS

Flood Modeling with Deep Learning

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Declaration of Authorship

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Abstract

The abstract is like a miniature version of the entire manuscript. Structure it similarly: Begin with the context and motivation for the project, a brief description of the method and available data, your findings, and conclusions. Limit yourself to one page! [**Reference1**]

Acknowledgements

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Contents

List of Abbreviations

CA **C**ellular **A**utomata **NCA**
Neural **C**ellular **A**utomata **CNN** **C**onvolutional **N**eural **N**etwork **DEM**
Digital **E**levation **M**ap

For/Dedicated to/To my…

Chapter 1

Introduction

1.0.1 Background

The topic of flood modeling has become increasingly important increased risk of flooding all over the world. Limitations of previous flood models are made increasingly apparent as the need for rapid modeling is required. Larger areas that were never at risk of flooding are now at risk. Severe, unexpected pluvial rainfall requires rapid modeling for prevension mechanisms. Unfortunately older methods just aren't fast enough. More recent CA models have allowed for faster modeling. Although these models are faster than traditional methods. They are still too slow, taking hours to model. This thesis proposes a machine learning approach to model flooding. Deep learning has revolutionized many fields and many modeling tasks. The benefits of these models is how fast they perform computation. Often only requiring O(n) time complexity. Another feature of using deep learning approaches is that it can uitilize the GPU (graphical processing unit) to further increase the speed of computation.

1.0.2 Related Work

Here I need to discuss The work Joao did and the problems they faced.

1.0.3 Objective

The objectives for this thesis are as follows:

1. Normalize the data in such a way that data remains bound to physics
2. Investigate different CNN model architectures and hyper parameters for predicting water depth.
3. Create a custom loss function that constrains the model to adhere to mass conservation.
4. Can the model project arbitrarily into the future and predict multiple time steps ahead?
5. Is the trained model computationally faster than the model proposed by [3]?

Chapter 2

Methods

In this chapter, the relevant literature and background information is discussed so the reader is more easily able to follow the thesis.

2.1 Cellular Automata

2.1.1 What Are Cellular Automata

A Cellular Automaton (CA) is a system that typically consists of a discrete lattice or grid of cells. Each cell can have a discrete state, such as alive or dead, 0 or 1, etc. The cells in the grid are updated based on simple rules that depend on the cells' local neighbourhood. The entire system is typically updated simultaneously. What makes these systems fascinating is that even with very simple rules, complex emergent behavior can arise. There are two well-known CA models that I will briefly mention:

1. Wolfram's elementary CA is a one-dimensional CA with a local neighborhood of size 3, which includes the cell itself and its right and left neighbors. Some of these rules result in simple behavior, while others exhibit incredibly complex behaviors, such as the famous Rule 30 or the Turing-complete Rule 110.

2. John Conway's Game of Life is perhaps the most famous CA model. It is a two-dimensional CA on a square lattice that has been extensively studied, with new discoveries still being made to this day. This system uses the Moore's neighborhood, which is a 3x3 neighborhood that includes the central cell. The Game of Life produces incredible emergent behavior and is also Turing-complete.

2.1.2 Why are they useful?

In the above section, although beautiful and impressive, the exampels are not practival. But the same motivation and methods can be extrapolated to model physicals systems. Especially many differentiable equations that can be descritized in time and space can be modeled using CA. Many examples have been demostrated over the years. In the following 3, the CADDIE2D model is discussed in detail. But other examples include: particle simulation, chemical reactions, fire modeling, human and animal dynaimcs to name a few. [must find references for this.]

2.2 Deep Learning

2.2.1 What is deep learning

2.2.2 Optimizers for backpropogation

2.2.3 Common loss functions for regression tasks

2.2.4 Convolutional Neural Network

2.2.5 Neural Cellular automata

Chapter 3

Flood Modeling

3.1 Classical Approaches

3.1.1 1D

A 1D flood model represents the river network by connecting cross-sections taken through the river and the land and simulating the water level as it flows through the model. It gives us a precise definition of the riverbed. The drawback to this model is it gives us limited information about the flow dynamics and the general topography of the river and flood plain [1].

3.1.2 2D

A 2D flood model represents the river network in a mesh, which provides information about the river topography and allows to define very precisely the topography of the floodplain. A drawback to this type of model is the computational time required. It is very slow is not feasible to be used over large areas. 2D models are also often linked to a 1D model of the channel between the riverbanks [1].

In a study by [2], A comparison of 1D and 2D models was made regarding Flood simulation on the Medjerda Riverin Tunisia. They found very similar information. 1D models they used (HEC RAS and MIKE 11 models) were much faster and less computationally intensive than the 2D counterpart (TELEMAC). However, the overall results were much better in the 2D model. They also found overall that the results from the 1D models and 2D models were in good agreement but was quite dependent on the accuracy of the data available to them.

3.1.3 The Problem With Classical Approaches

Although classical approaches are extremely accurate, they are extremely computationally expensive. Solving partial differential equations is quite taxing and is limited to smaller, areas at lower resolution.

3.2 CA Approach

The purpose of this review is to see how cellular automata are being implemented for flood modeling. There are a few problems with conventional flood modeling techniques. 1D models give us limited information about flow dynamics and 2D models are extremely computationally taxing, which limits their use. Cellular automata (CA) could be implemented to remedy this problem by reducing the computational time and cost to run these models. The focus of this review will be primarily on the CA created by [3] as the paper is very well laid-out and the CA used is well defined, albeit, complex. This CA uses regular grid cells as a discrete space for the CA setup and applies generic rules to local neighbourhood cells to simulate the progression of pluvial floods.

CA Formulation by Ghimire, et al

This model consists of five essential features of a true cellular automation: discrete space; neighbourhood (NH); cell state; discrete time step; transition rule. The square grid digital elevation model (DEM) provides the discrete space for the CA set up. A DEM is just a representation of the bare ground (bare earth) topographic surface of the Earth which excludes trees, buildings, and any other surface objects [4]. The NH used for this model consists of the central cell itself and its four cardinal adjacent cells (five cells in total). This is known as the von Neumann type NH. The Moore NH, consisting of eight surrounding cells and the central cell similar to the NH in John Conway's Game of Life, is an alternative. Precipitation occurs over the whole area of the terrain being considered. Movement of the water is mainly driven by the slopes between cells and limited by the transferrable volume and the hydraulic equations. The transferrable volume is the minimum of the total volume within the giving cell and the space available in the receiving cells. The transferrable volume is the minimum of the total volume within the giving cell and the space available in the receiving cells. Manning's equation and the critical flow equation are applied to restrict the flow velocity. The assumption here is that water can only flow from one cell to its local NH, according to the hydraulic gradients in one computing time step. In the calculation the NH cells are ranked according to the water level, as 1 for the cell with the lowest level and 5 for the highest one, to determine the direction of flow between cells. Only the outflow fluxes (Flux as flow rate per unit area) from the central cell to its neighbours with lower ranks are calculated. Any inflow to the cells under consideration is eventually calculated as the outflow from its neighbor that has a higher water level on the opposite of the cell interface. The fluxes through the interfaces of the central cell are determined by the states of NH cells in previous time steps and stored as intermediate buffers for updating the states of cells. The states of flood depths of all cells are updated simultaneously when all interface fluxes are determined.

Main Algorithm:

Program start

1. Initialize variables –depth, water surface elevation, input(terrain, rainfall)
2. Start time loop{
3. Add precipitation depth directly to the water depth on the cells
4. Computation starts in the local NH {
    (a) Ascending cell rankings based on the water surface elevations

(b) Layer-wise claculation of outflows from central cell

(c) Distribution of layer-wise fluxes within the NH

(d) Calculate the cell interfacial velocities

5. End of local NH loop }

6. Determine time step Δt required for the distriibutions appiled

7. Update simulation time: t = t + Δt

8. Update the states (depths, water surface elevation) for the new time step

9. Apply boundary conditions to suit the flow conditions

10. Data outputs for visualization and analysis

11. Repeat until the end of simulation time

12. End of time loop }

Program end

Outflow Flux Calculation

The calculation process starts with cell ranking, based on the water surface elevation in the local NH with five discrete states of cell ranks {r=1, 2, 3, 4, 5}. This is the height of each cell. Space between the water levels (water levels added on top of the height) of the cells are divided into four layers. Li ¬ is the free space between the water levels of cells ranked i and i + 1 that can accommodate the water volume from cells with higher ranks. If the rank of the central cell is r¬c (e.g. rank 3) there can be at most rc -1 number of cells receiving water as flux (because the central cell obviously can't receive water from itself) through the NH cell boundaries, if enough water is available in the central cell. Outflow volume to the layer i can be given by the following formula that is applied locally for each cell considered:

$$\Delta V_i = \min\left\{ V_c - \sum_{k=1}^{i-1} \Delta V_k, \Delta W L_i \sum_{k=1}^{i} A_k \right\} \tag{3.1}$$

Where V_c is the water volume of the central cell in the previous time step; Δ V_k is the volume distributed to layer k, $\sum_{k=1}^{i-1} \Delta V_k$ total volume has has been distributed to layers 1 to i-1; V_c - $\sum_{k=1}^{i-1} \Delta V_k$ represents the remaining volume available for distributing to layer i after filling i-1 layers. $\Delta W L_i$ is the water level difference between cells ranked i and i+1; $\sum_{k=1}^{i} \Delta A_k$ is the total surface area of layer i; $\Delta W L_i$ $\sum_{k=1}^{i} \Delta A_k$ is the available space for storage in layer i. For the layer adjacent to the central cell, an additional term

$$\sum_{k=1}^{i} A_k / A_c + \sum_{k=1}^{i} A_k \left(V_c - \sum_{k=1}^{i-1} \Delta V_k \right)$$

is applied to limit ΔV_i, which assumes that the water levels for all cells will reach an equivalent level. Thus, a cell with rank r receives water only from cells with higher ranks and the water received is added on top of its own water level. Thus, the total outflow flux from the central cell to a neighbouring cell ranked i is calculated as:

$$F_i = \sum_{k=i}^{r_c-1} \frac{\Delta V_k}{k} \tag{3.2}$$

For a regular grid, the areas of the central cell, A_c and the neighboring cells, A_k are constant over the domain. However, the methodology is applicable to different grid settings. Therefore, a cell containing buildings that do not allow water to flow in can be described using a variable cell area to reflect the reduced space occupied by buildings.

Depth updating

A very important step in the CA approach is the execution of the state transition rule. In the resent CA calculations, the global continuous state is the flow depth in a grid cell, which is updated for every new time step. This is done by algebraically summing the water depth from all its four neighbours. The following transition rule is used to update the flow depth:

$$d^{t+\Delta t} = d^t + \theta \frac{\sum F}{A} \tag{3.3}$$

Where θ is a non-dimensional flow relaxation parameter that can take values between 0 and 1, F is the total volume transferred to the cell under consideration as calculated from Equation 3.2 and A is the cell area. The purpose of the relaxation parameter is to damp oscillations that would appear otherwise. The effect of the relaxation parameter does not impart any effect on mass conservation rather it makes the flow smooth and gradual. The values of θ are determined by numerical experiments and calibration.

Time-Step Calculation

For most 2D hydraulic modelling, higher resolution DEM data are being used, the required time steps will be shorter to ensure the stability of model computations, which often leads to large computational burden, such that many studies have been focused on reducing the computational time of simulations. The time increment, determined as the largest that satisfies the stability criteria anywhere in the whole domain, implies that for most of the cells only a fraction of the locally allowable time steps is used to integrate the solution in time. This represents a waste of computational effort and limits the use of the method. A spatially varying time step can increase solution accuracy and reduce computer run time. In this implementation we use maximum permissible velocity which ensures the minimum time steps required to distribute the applied flux. The interfacial velocity v* is determined based on the flux transferred through a cell boundary given by:

$$v^* = \frac{F}{d^* \Delta x \Delta t} \tag{3.4}$$

Where, $d*$ is the water depth of flow available at the interface, which is the difference between higher water level and higher ground elevation of the central cell and its neighbour cell to the interface

$$d* = \max\{WL_C, WL_N\} - \max\{z_C, z_N\} \tag{3.5}$$

Where, WL and z are the water levels and ground elevation respectively and the subscripts C and N represent central and neighbouring cells repectively To prevent the velocity from over shooting, a cap on the localallowable velocity is applied as given by Equation 3.6 based on the Manning's formula and critical flow condition as:

$$v = \min\{\frac{1}{n}R^{\frac{2}{3}}S^{\frac{1}{2}}, \sqrt{gd}\} \tag{3.6}$$

where, the hydraulic radius R is taken to be equal to the water depth d and S is the slope of water surface elevation and is always positive for outflow calculation. If v is less than $v*$, the interfacial flux F is recalculated by replacing $v*$ with v in Equation 3.4 The global time step is then calculated based on the global maximum velocity to satisfy the conventional CFL criteria. Therefore, each time the state transition rule is applied, the global time step is updated using maximum velocity calculated from all cell interfaces, as given by:

$$\Delta t = \frac{\Delta x}{\max\{v_j\}} \tag{3.7}$$

Where v_j is the velocity calculated for the jth cell interface for the entire domain.

Chapter 4

Methodology

4.1 Data

4.1.1 data acquisition

The data was provided by [Eaweg, institute of Aquatic Sciences]. It was generated using CADDIE2D software, whichsed in /refChapter3. Multiple catchment areas are part of this data. Work was done on two seperate sets of data.

features

The dataset contains the WaterDepth (in m), the DEM (digital elevation map - goes into abbreviations). Timesteps, (in seconds) and rainfall events (in mm/hour).

4.1.2 data preprocessing

4.1.3 dataset creation

validation and test set

4.2 Model creation

4.2.1 classical CNN

This was a baseline model. A generic approximation of grid search was done to find the optimial parameters.

4.2.2 gradient filters

A hardcoded kernel with sobel x, sobel y, and identity filters used as a way to to get gradients and information about the neighbourhood. which is then followed up with 1x1 convolutions for the computation. This filter comes straight from the [gorwing nca reference]

4.2.3 deepthwise layer

Instead of a 'hardcoded' filter to get gradients, the model learns should learn this filter for each feature and improves the models performance.

4.2.4 NCA model and adaptions

Here I should discuss the NCA model proposed in Growing NCA paper.

4.3 custom loss functions

4.3.1 custom mse and mae

Due to the nature of the data (extremely small values), the loss starts off extrememly small (example of loss here based on real values). The model also really liked to predict 0 and try to just predict the last timestep. so we create a loss function to weight the 0's according to how many 0s appear in the data. We also try to mask it so that the model cannot predict less than 0 (constraint model)

4.4 Proposed evaluation

The proposed evaluation of the model is based on a very simple heuristic. Can the model perform better than prediciting the previous timestep? (i.e. $\delta Xt = \delta Xt - 1$)

4.5 pipeline creation

The creation of a pipline for training became extremely important for testing many parameters at once with different configurations for the dataset, training reigime, model creation. It allowed us to test multiple things in parrelel.

Chapter 5

Results

5.1 NCA

5.2 simple CNN

5.3 depthwise layer

5.4 performance of models on the different datasets

Chapter 6

Discussion

6.1 Interpretation of results

Chapter 7

Conclusion

7.1 Conclusion

7.2 Outlook and future work

Appendix A

Frequently Asked Questions

A.1 How do I change the colors of links?

The color of links can be changed to your liking using:

`\hypersetup{urlcolor=red}`, or

`\hypersetup{citecolor=green}`, or

`\hypersetup{allcolor=blue}`.

If you want to completely hide the links, you can use:

`\hypersetup{allcolors=.}`, or even better:

`\hypersetup{hidelinks}`.

If you want to have obvious links in the PDF but not the printed text, use:

`\hypersetup{colorlinks=false}`

A.2 How can I add a Figure in the Appendix?

You can refer to a figure in the Appendix (like A.1) and it will show up as expected.

FIGURE A.1: Bart Simpson. (2023, May 17). In Wikipedia. https://en.wikipedia.org/wiki/Bart_Simpson

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