Seismic Performance Limit-States of Steel Bridge Piers with Socket-type Connections

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Introduction

- Steel pile-to-cap element welded connection was found to exhibit premature brittle failure under cyclic loading (Fulmer et al., 2011).
- **The GSS connection**, short for **G**routed **S**hear **S**tud connection was developed to relocate damage to columns (Fulmer et al., 2015).
- Need for **displacement-based design recommendations** for socket-type connections was realized.
- This study (Jayaprakash, 2020) was undertaken to address this need.

Methods

Experimental:
We performed large-scale structural tests on two-column steel bridge pier specimens with GSS connections (see photograph to your right).

Numerical Modeling:
We developed fiber-based OpenSees models (see below) to capture full cyclic behavior of these piers.

2-D Global model

Fiber Sections
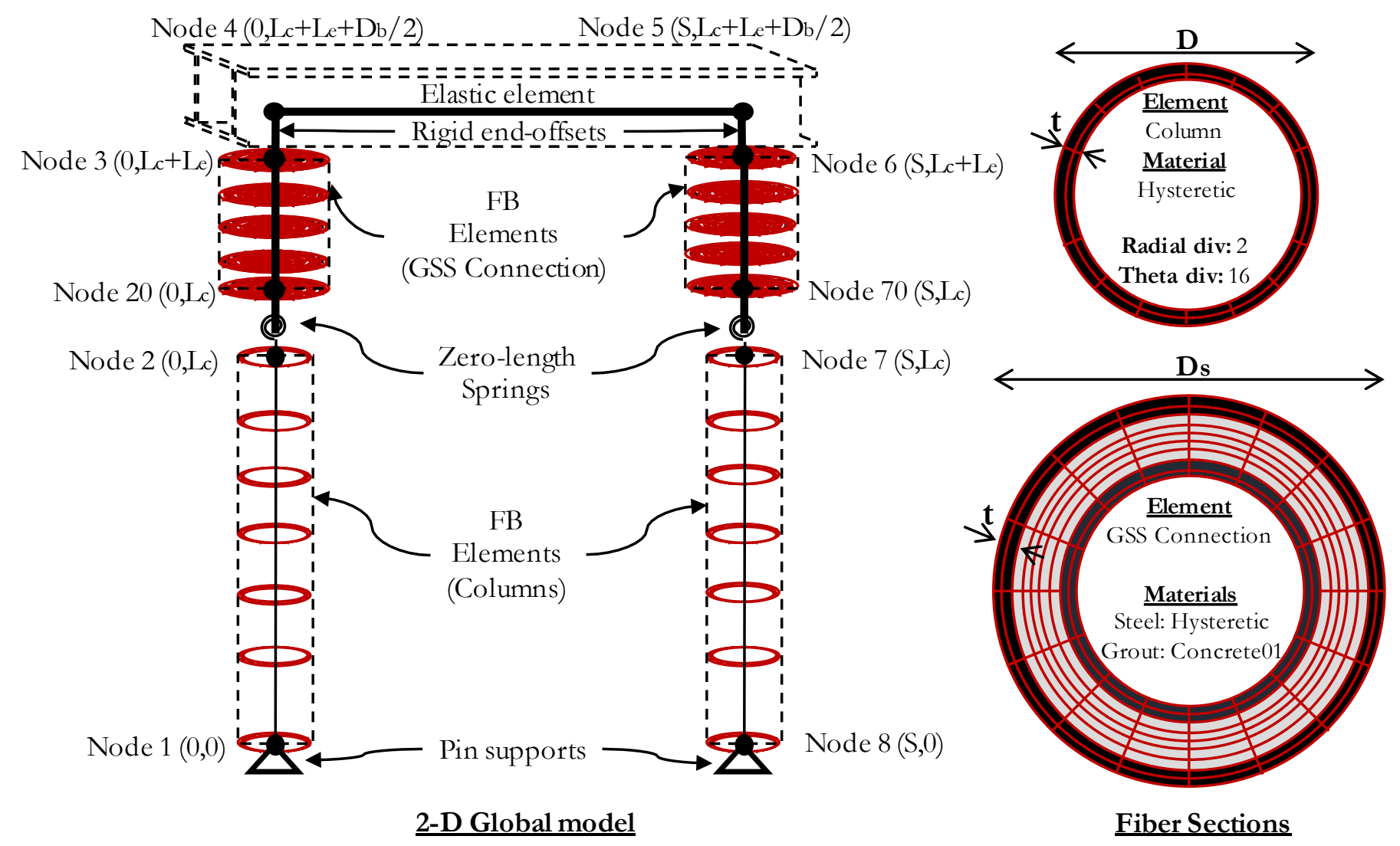
Parametric Studies:
We performed parametric studies using calibrated numerical models to develop semi-empirical equations.

Result and Broader Impact

A set of equations that can **predict** the **force and displacement** of these types of systems at various **key performance limit-states**.

This model is a **crucial component** in **displacement-based seismic design and assessment** of **steel bridge piers** with **socket connections**.

We propose a **simple hand-calculation** model that can estimate the **complete non-linear response** of two-column **steel bridge piers** under cyclic lateral loading.
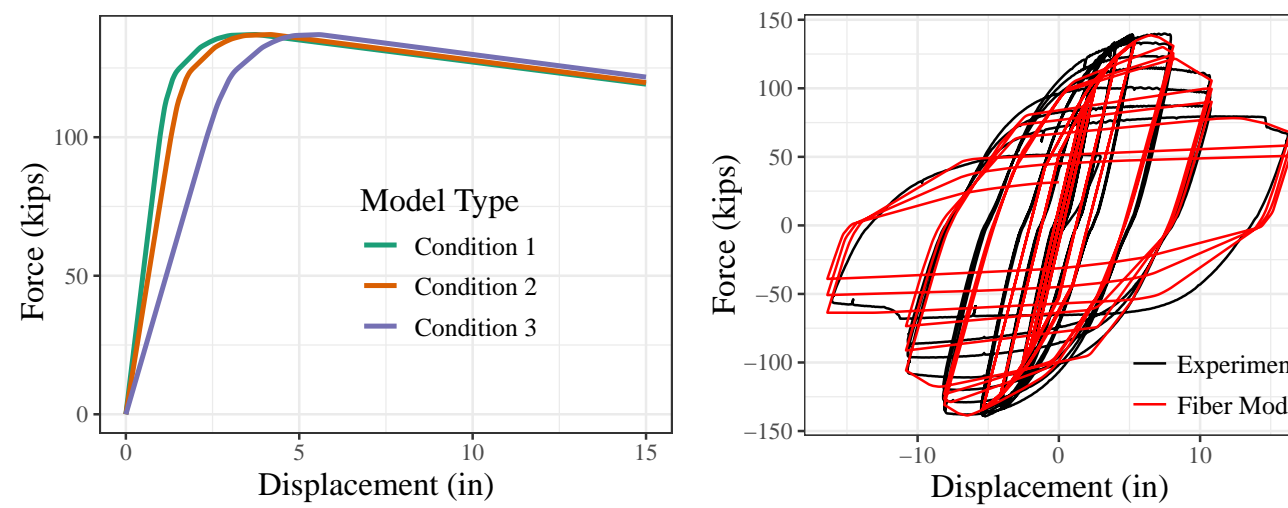

GSS Connections

Cantilever Column — Moment Diagram — Curvature Diagram — Deformed Shape
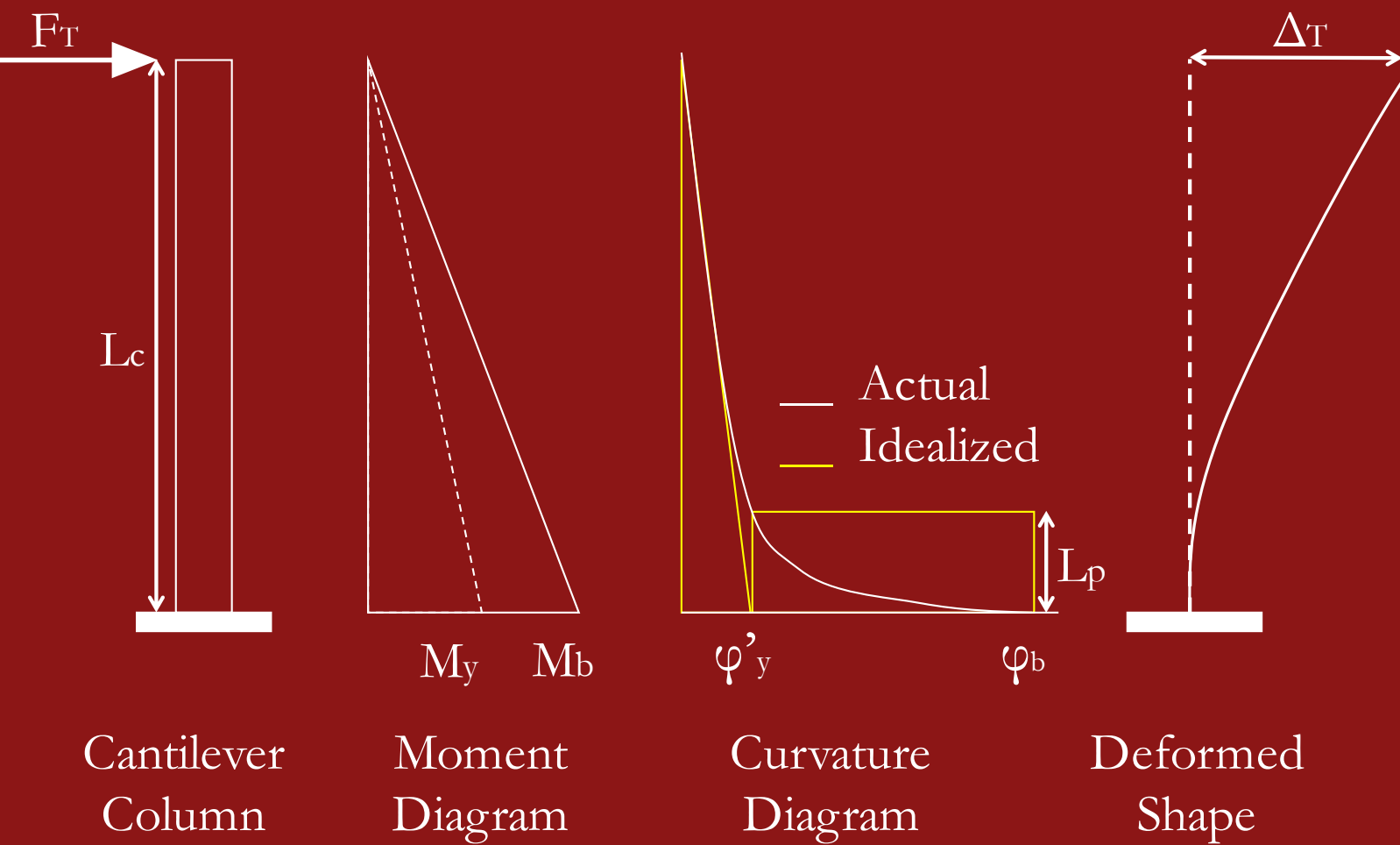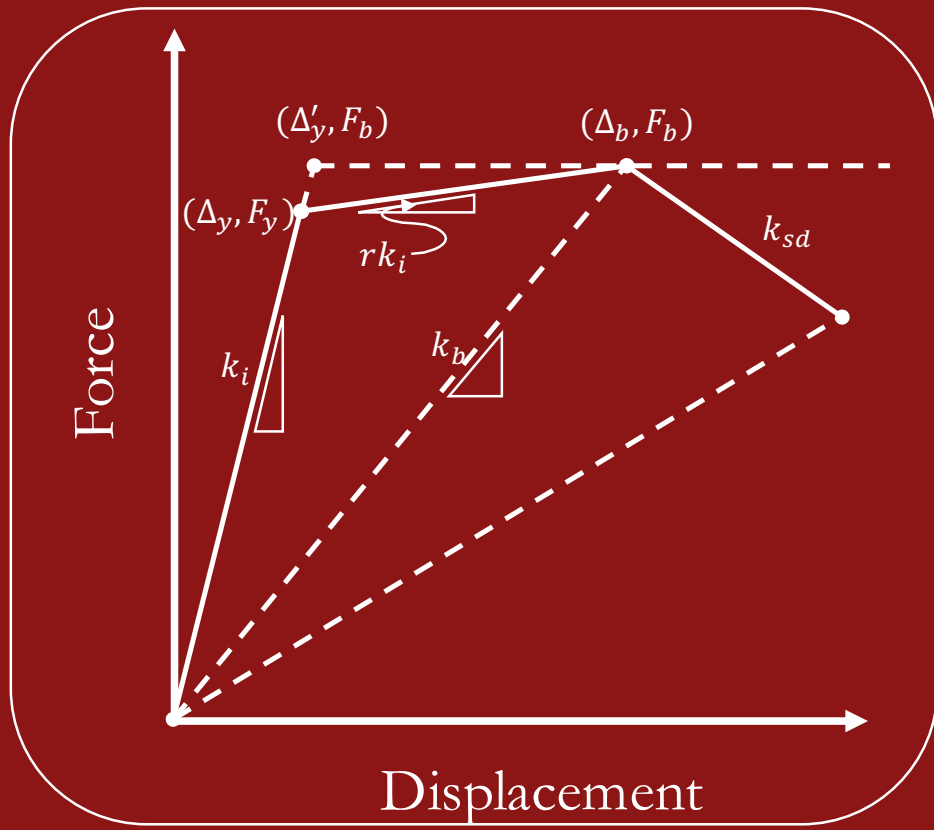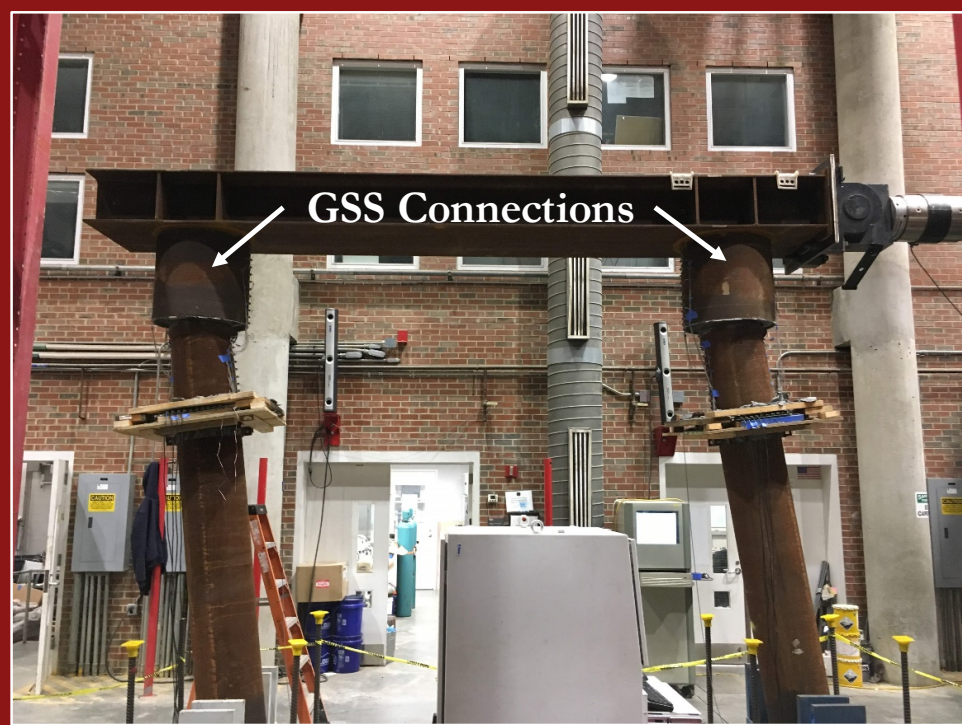
Scan the QR-code to get the model equations & more

Supplemental Information

Flexibility Correction Coefficients, γ_{cb} and γ_{sc}:
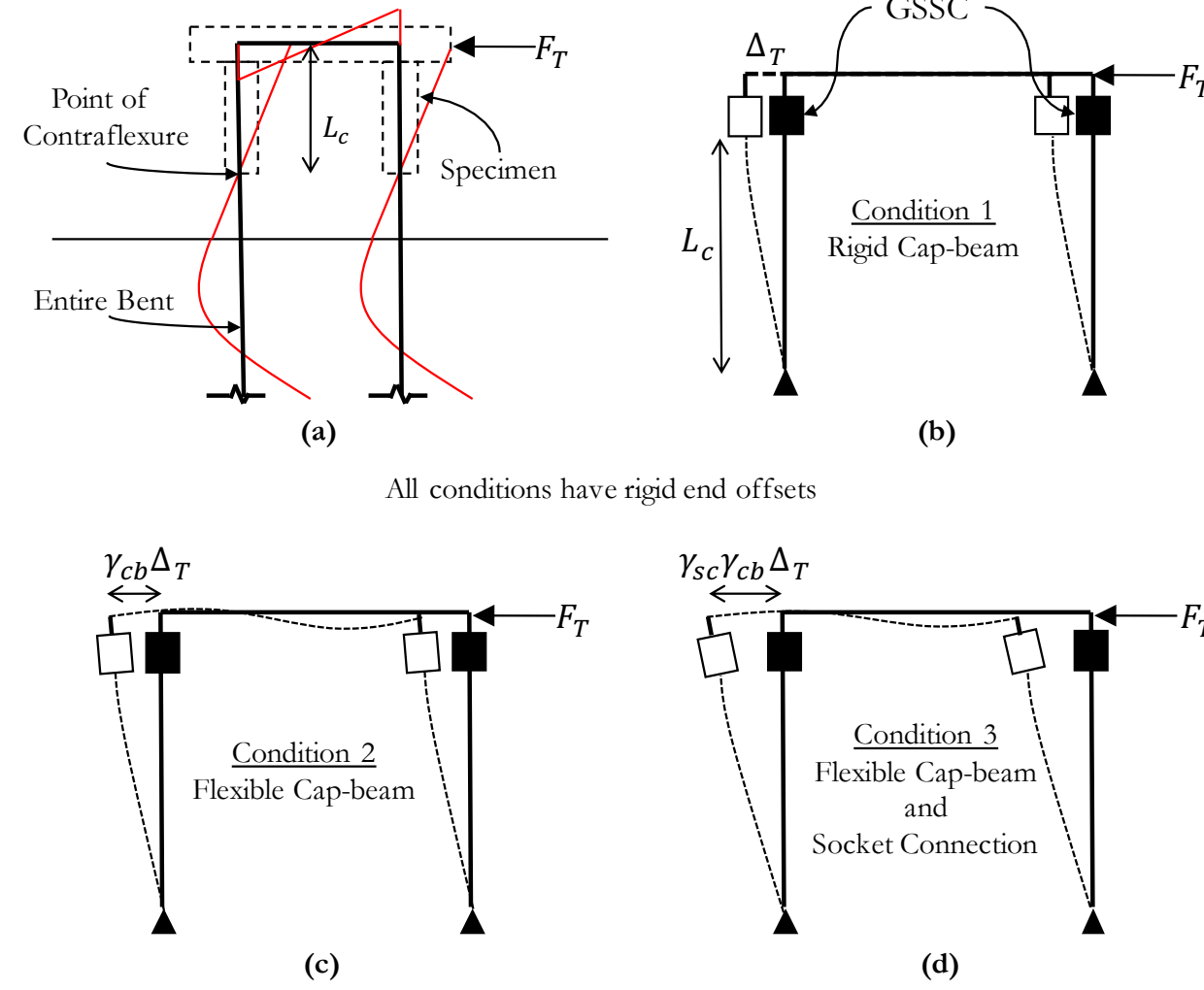
Model Calibration:

Yield Limit State, ϵ_y:
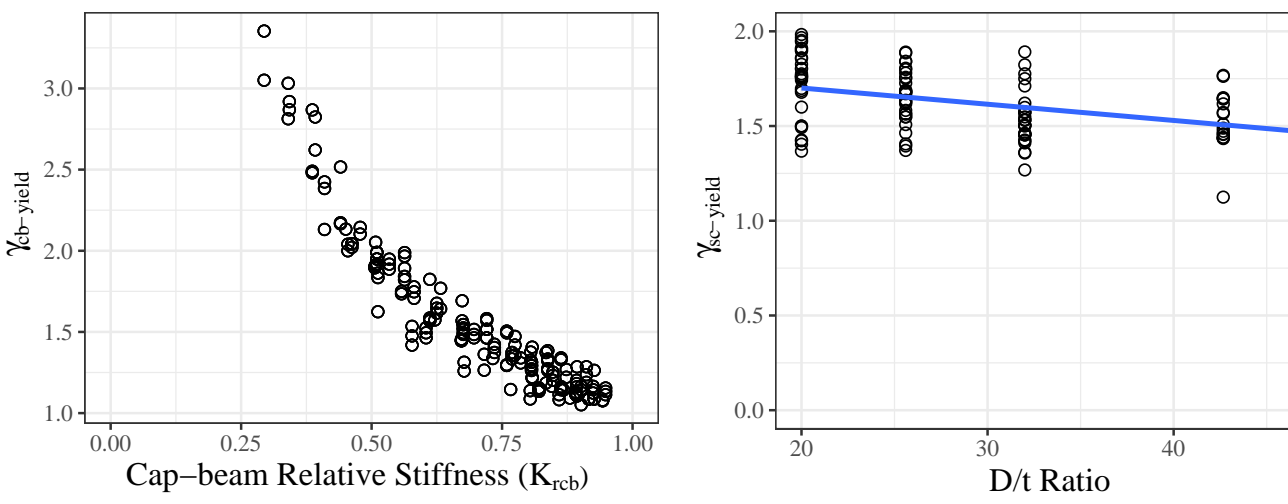
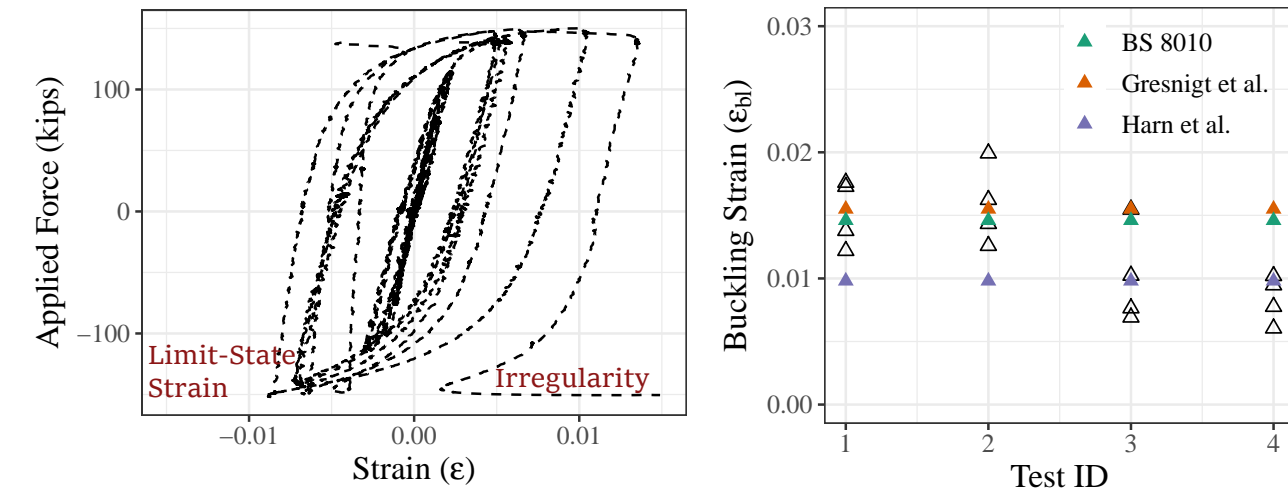### Buckling Limit State, ϵ_b:

Strength Degradation Rate, k_{sd}:
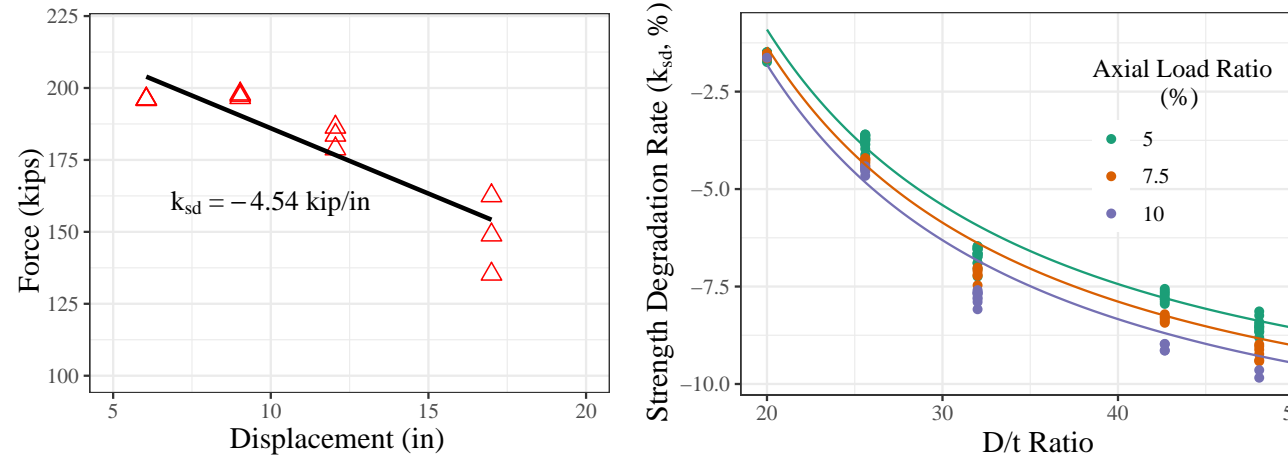
What the model looks like: