

**REPORT ON DRILLED SHAFT
LOAD TESTING (OSTERBERG METHOD)**

**Test Shaft #1 - Highway 81 over Missouri River
Yankton, South Dakota (LT-9152)**

Prepared for: Jensen Construction Co.
5550 NE 22nd Street
Des Moines, Iowa 50316

Attention: Mr. Dan Timmons

PROJECT NUMBER: LT-9152, July 2, 2007

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DEEP FOUNDATION TESTING, EQUIPMENT & SERVICES • SPECIALIZING IN OSTERBERG CELL (O-cell®) TECHNOLOGY



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July 2, 2007

Jensen Construction Co.
5550 NE 22nd Street
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Attention: Mr. Dan Timmons

Load Test Report: Test Shaft #1 - Highway 81 over Missouri River
Location: Yankton, South Dakota (LT-9152)

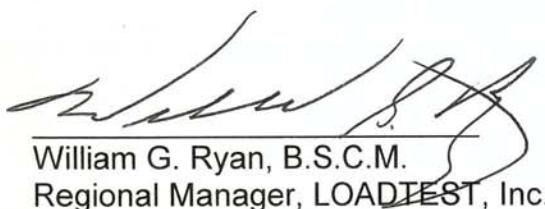
Dear Mr. Timmons,

The enclosed report contains the data and analysis summary for the O-cell test performed on Test Shaft #1 - Highway 81 over Missouri River, on June 27, 2007. For your convenience, we have included an executive summary of the test results in addition to our standard detailed data report.

We would like to express our gratitude for the on-site and off-site assistance provided by your team and we look forward to working with you on future projects.

We trust that the information contained herein will suit your current project needs. If you have any questions or require further technical assistance, please do not hesitate to contact us at 800-368-1138.

Best Regards,



William G. Ryan, B.S.C.M.
Regional Manager, LOADTEST, Inc.



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EXECUTIVE SUMMARY

On June 27, 2007, we tested a 96-inch (2,438-mm) diameter dedicated test shaft constructed by Jensen Construction Co. Mr. Michael D. Ahrens and Mr. Rick Usab of LOADTEST, Inc. carried out the test. Jensen Construction Co. completed construction of the 115.1-foot (35.08-meter) deep shaft on June 20, 2007. Sub-surface conditions at the test shaft location consist primarily of loose to medium dense sand underlain by hard silty shale (Carlile Shale). Representatives of the Nebraska Department of Roads observed construction and testing of the shaft.

The maximum sustained bi-directional load applied to the shaft was 4,522 kips (20.11 MN). At the maximum load, the displacements above and below the O-cell were 0.207 inches (5.25 mm) and 0.504 inches (12.81 mm), respectively. Unit shear data calculated from strain gages indicated average values of 4.9 ksf (234 kPa) and 6.4 ksf (305 kPa) in the zones directly above and below the O-cell at the noted displacements. The strain gage data also indicates that none of the applied load was transferred to end bearing.

Using the procedures described in the report text and in Appendix C, we constructed an equivalent top load curve for the test shaft. For a top loading of 1,850 kips (8.2 MN), the adjusted test data indicate this shaft would settle approximately 0.08 inches (2.0 mm) of which 0.06 inches (1.6 mm) is estimated elastic compression.

LIMITATIONS OF EXECUTIVE SUMMARY

We include this executive summary to provide a very brief presentation of some of the key elements of this O-cell test. It is by no means intended to be a comprehensive or stand-alone representation of the test results. The full text of the report and the attached appendices contain important information which the engineer can use to come to more informed conclusions about the data presented herein.

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SITE CONDITIONS AND SHAFT CONSTRUCTION

Site Sub-surface Conditions: The sub-surface stratigraphy at the general location of the test shaft is reported to consist primarily of loose to medium dense sand underlain by hard silty shale (Carlile Shale). The generalized subsurface profile is included in Figure A and a boring log indicating conditions near the shaft is presented in Appendix E. More detailed geologic information can be obtained from the Nebraska Department of Roads.

Test Shaft Construction: Jensen Construction Co. constructed the dedicated test shaft between June 11 and 20, 2007. We understand that the 96-inch (2,438-mm) diameter test shaft was excavated wet to a tip elevation of +1041.8 feet (+317.53 meters) using an auger. After cleaning the base with a cleanout bucket, the reinforcing cage with attached O-cell assembly was inserted into the excavation and temporarily supported at the surface. Concrete was then delivered by tremie through a nominal 12-inch (300-mm) O.D. pipe into the base of the shaft until reaching the cutoff elevation. Representatives of the Nebraska Department of Roads observed construction of the shaft.

OSTERBERG CELL TESTING

Shaft Instrumentation: Test shaft instrumentation and assembly was carried out under the direction of Mr. Rick Usab and Aaron King of LOADTEST, Inc. on June 18 through 20, 2007. The loading assembly consisted of three 21-inch diameter (540-mm) O-cells located 29.3 feet (8.92 meters) above the tip of shaft. Calibrations of O-cells and instrumentation used for this test are included in Appendix B.

O-cell testing instrumentation included four Linear Vibrating Wire Displacement Transducers (LWWDTs - Geokon Model 4450 series) positioned between the lower and upper plates of the O-cell assembly to measure expansion (Appendix A, Page 4). Two telltale casings (nominal ½-inch steel pipe) were attached to the reinforcing cage, diametrically opposed, extending from the top of the O-cell assembly to beyond the top of concrete. Compression of the shaft below the O-cell was measured by one section of Embedded Compression Telltales (ECT), consisting of telltale rods in nominal ½-inch steel pipe casings with an attached LWWDT (Appendix A, Page 3).

Strain gages were used to assess the side shear load transfer of the shaft above and below the Osterberg cell assembly. One level of four sister bar vibrating wire strain gages (Geokon Model 4911 Series) was installed in the shaft below the base of the O-cell assembly and two levels of two were installed in the shaft above it. Details concerning the strain gage placement appear in Table B and Figures A & B. The strain gages were positioned as recommended by LOADTEST, Inc.



Three lengths of steel pipe were also installed, extending from the top of the shaft to the top of the bottom plate, to vent the break in the shaft formed by the expansion of the O-cells. The pipes also provide access for post-test grouting of the annular void surrounding the O-cell assembly as described in Appendix H.

Test Arrangement: Throughout the load test, key elements of shaft response were monitored using the equipment and instruments described herein. Shaft compression was measured using $\frac{1}{4}$ -inch telltales installed in the $\frac{1}{2}$ -inch steel pipes (described under Shaft Instrumentation) and monitored by Linear Variable Displacement Transducers (LVDTs - RDP DCW Series). Two LVDTs attached to a reference system were used to monitor the top of shaft movement (Appendix A, Page 2). An automated digital survey level (Leica NA3003) was used to independently monitor the top of shaft movement during testing from a distance of approximately 35.8 feet (10.91 meters).

The reference system consisted of a steel wide flange section supported on wooden crane mats. The supports were located approximately three shaft diameters from the center of the test shaft. The beam was partially shaded for the duration of the test. A second automated digital survey level (Leica NA3003) was used to monitor the reference beam for movement during testing from a distance of approximately 36.5 feet (11.11 meters). A maximum range of vertical movement of 0.044 inches (1.12 mm) was observed for the reference beam. The top of shaft movements have been corrected for movement of the reference system (Appendix A, Page 2).

Both a Bourdon pressure gage and electronic pressure transducers were used to measure the pressure applied to the O-cell at each load interval. We used the Bourdon pressure gage for setting and maintaining loads and for data analysis. The transducer readings were used for real time plotting and as a check on the Bourdon gage. There was close agreement between the Bourdon gage and the pressure transducers.

Data Acquisition: All instrumentation were connected through a data logger (Campbell Scientific CR-10) to a laptop computer allowing data to be recorded and stored automatically at 30-second intervals and displayed in real time. The same laptop computer synchronized to the data logging system was used to acquire the Leica NA3003 data sets.

Testing Procedures: As with all of our tests, we begin by pressurizing the O-cells in order to break the tack welds that hold them closed (for handling and for placement in the shaft) and to form the fracture plane in the concrete surrounding the base of the O-cells. After the break occurs, we immediately release the pressure and then begin the loading procedure. Zero readings for all instrumentation are taken prior to the preliminary weld-breaking load-unload cycle, which in this case involved a maximum applied pressure of 500 psi (3.45 MPa) to the O-cells.

The Osterberg cell load test was conducted as follows: We pressurized the 21-inch (540-mm) diameter O-cell assembly, with its base located 29.3 feet (8.92 meters) above the base of shaft to assess the combined end bearing and lower side shear below the O-cell and the upper side shear above. We pressurized the O-cell in 12 loading increments to 6,000 psi (41.37 MPa) resulting in a bi-directional gross O-cell load of 4,522 kips (20.11 MN). The loading was halted after load interval 1L-12 because the rate of displacement of the upper side shear was beginning to increase. The O-cell was then depressurized in four decrements. The O-cell was re-pressurized in three loading increments to a bi-directional gross O-cell load of 3,395 kips (15.10 MN) at 2L-3 in order to demonstrate that the test shaft was suitable for use as a production shaft. The O-cell was then unloaded in one decrement and the test was concluded.

We applied the load increments using the Quick Load Test Method for Individual Piles (ASTM D1143 *Standard Test Method for Piles Under Static Axial Load*), holding each successive load increment constant for eight minutes by manually adjusting the O-cell pressure. The data logger automatically recorded the instrument readings every 30 seconds, but herein we report only the 1, 2, 4 and 8-minute readings (where applicable) during each increment of maintained load.

TEST RESULTS AND ANALYSES

General: The loads applied by the O-cell act in two opposing directions, resisted by the capacity of the shaft above and below. Theoretically, the O-cell does not impose an additional upward load until its expansion force exceeds the buoyant weight of the shaft above the O-cell. Therefore, *net load*, which is defined as gross O-cell load minus the buoyant weight of the shaft above, is used to determine side shear resistance above the O-cells and to construct the equivalent top-loaded load-settlement curve. For this test we calculated a buoyant weight of shaft of 391 kips (1.74 MN) above the O-cells.

Upper Side Shear Resistance: The maximum upward applied *net load* to the upper side shear was 4,131 kips (18.37 MN) which occurred at load interval 1L-12. At this loading, the upward movement of the O-cell top was 0.207 inches (5.25 mm).

In order to assess the side shear resistance of the test shaft, loads are calculated based on the strain gage data ([Appendix A, Page 6](#)) and estimates of shaft stiffness (AE) which are presented below. We used the ACI formula ($E_c=57000\sqrt{f'_c}$) to calculate an elastic modulus for the concrete, where the average f'_c was reported to be 3,256 psi (22.45 MPa) on the day of the test. This, combined with the area of reinforcing steel and nominal shaft diameter, provided an average shaft stiffness (AE) of 32,100,000 kips (143,000 MN) in the permanently cased zone, 25,800,000 kips (115,000 MN) above the O-cell in the nominal 96-inch shaft and 25,300,000

kips (113,000 MN) below the O-cell. Net unit shear curves are presented in Appendix F. Net unit shear values for loading increment 1L-12 follow in Table A:

TABLE A: Average Net Unit Side Shear Values for 1L-12

Load Transfer Zone	Displacement *	Net Unit Side Shear **
Top of Shaft to Strain Gage Level 3	↑ 0.176"	0.1 ksf (4 kPa)
Strain Gage Level 3 to Strain Gage Level 2	↑ 0.182"	2.4 ksf (113 kPa)
Strain Gage Level 2 to O-cell	↑ 0.197"	4.9 ksf (234 kPa)
O-cell to Strain Gage Level 1	↓ 0.495"	6.4 ksf (305 kPa)

* Average displacement of load transfer zone.

** For upward-loaded shear, the buoyant weight of shaft in each zone has been subtracted from the load shed in the respective zone above the O-cell.

NOTE: Net unit shear values derived from the strain gages above the O-cell assembly may not be ultimate values. See Appendix F for net unit shear vs. average shear zone displacement plots.

Combined End Bearing And Lower Side Shear Resistance: The maximum O-cell load applied to the combined end bearing and lower side shear was 4,522 kips (20.11 MN) which occurred at load interval 1L-12 (Appendix A, Page 5, Figure 1). At this loading, the average downward movement of the O-cell base was 0.504 inches (12.81 mm). A graphical extrapolation of the strain gage load distribution in Figure 3 indicates that none of the applied load was transferred to end bearing at the above noted displacement.

Creep Limit: See Appendix D for our O-cell method for determining creep limit. The combined end bearing and lower side shear creep data (Appendix A, Page 5) indicate that no apparent creep limit was reached at a downward displacement of 0.50 inches (12.8 mm) (Figure 4). The upper side shear creep data (Appendix A, Page 5) also indicate that no apparent creep limit was reached at an upward displacement of 0.21 inches (5.3 mm) (Figure 5). The creep limit for a top loaded shaft will not be reached until both components reach their respective creep limits. This will occur at the maximum of the movements required to reach the creep limit for each component. One interpretation of the data presented herein is that significant creep for this shaft will not begin until a top loading exceeds 9,196 kips (40.9 MN) by some unknown amount. The engineer should come to his own conclusions with regard to the suitability of the creep limit analysis to address long-term creep which may be an important design consideration.

Equivalent Top Load: Figure 2 presents the equivalent top-loaded load-settlement curves. The lighter curve, described in Procedure Part I of Appendix C, was generated by using the measured upward top of O-cell and downward base of O-cell data. The curve is extended out to a settlement of 0.5 inches (13 mm) by extrapolating the top of O-cell data (Appendix G). Because it is often an important component of the settlements involved, the equivalent top load curve requires an adjustment for the additional elastic compression that would occur in a top-load test.



The darker curve as described in Procedure Part II of Appendix C includes this adjustment.

For a top loading of 1,850 kips (8.2 MN), the adjusted test data indicate this shaft would settle approximately 0.08 inches (2.0 mm) of which 0.06 inches (1.6 mm) is estimated elastic compression. For a top loading of 3,700 kips (16.5 MN) the adjusted test data indicate this shaft would settle approximately 0.16 inches (4.0 mm) of which 0.12 inches (3.0 mm) is estimated elastic compression.

Note that, as explained previously, the equivalent top load curve applies to incremental loading durations of eight minutes. Creep effects will reduce the ultimate resistance of both components and increase shaft top movement for a given loading over longer times. The Engineer can estimate such additional creep effects by suitable extrapolation of time effects using the creep data presented herein.

Shaft Compression Comparison: The measured maximum shaft compression, averaged from two telltales, is 0.032 inches (0.82 mm) at 1L-12 (Appendix A, Page 2). Using an average shaft stiffness of 28,000,000 kips (125,000 MN) and the load distribution in Figure 3 at 1L-12, we calculated an elastic compression of 0.037 inches (0.93 mm) over the length of the compression telltales. We believe this close agreement provides good evidence that the values of the estimated shaft stiffness are reasonable.

POST-TEST O-CELL GROUTING

Since the test shaft is intended to carry structural loading (i.e., a production shaft), the contractor needs to fill the O-cell and annular void in the shaft created as a result of the expansion of the cell. The shaft includes the hoses and piping to permit filling the O-cell and void with grout. If not already grouted, we recommend that this be done as soon as possible according to the procedures in Appendix H.

LIMITATIONS AND STANDARD OF CARE

The instrumentation, testing services and data analysis provided by LOADTEST, Inc., outlined in this report, were performed in accordance with the accepted standards of care recognized by professionals in the drilled shaft and foundation engineering industry.

Please note that some of the information contained in this report is based on data (i.e. shaft diameter, elevations and concrete strength) provided by others. The engineer, therefore, should come to his or her own conclusions with regard to the analyses as they depend on this information. In particular, LOADTEST, Inc. typically does not observe and record drilled shaft construction details to the level of precision that the project engineer may require. In many cases, we may not be present for the entire duration of shaft construction. Since construction technique can play a significant role in determining the load bearing capacity of a drilled shaft, the engineer should pay close attention to the drilled shaft construction details that were recorded elsewhere.

We trust that this information will meet your current project needs. If you have any questions, please do not hesitate to contact us at 800-368-1138.

Prepared for LOADTEST, Inc. by



Michael D. Ahrens, M.Eng., P.E.
Geotechnical Engineer

Reviewed by



David J. Jakstis, P.E.
Geotechnical Engineer

TABLE B:
SUMMARY OF DIMENSIONS, ELEVATIONS & SHAFT PROPERTIES

Shaft:

Nominal shaft diameter (EL +1149.1 ft to +1121.7 ft)	=	102 in	2591 mm
Nominal shaft diameter (EL +1121.7 ft to +1041.8 ft)	=	96 in	2438 mm
O-cells: 21-6-00071, 21-6-00072, 21-6-00073	=	21 in	540 mm
Bouyant weight of pile above base of O-cell	=	391 kips	1.74 MN
Estimated average shaft stiffness, AE (EL +1149.1 ft to +1121.7 ft)	=	32,100,000 kips	143,000 MN
Estimated average shaft stiffness, AE (EL +1121.7 ft to +1071.0 ft)	=	25,800,000 kips	115,000 MN
Estimated average shaft stiffness, AE (EL +1071.0 ft to +1044.1 ft)	=	25,300,000 kips	113,000 MN
Estimated average shaft stiffness, AE (EL +1044.1 ft to +1041.8 ft)	=	23,500,000 kips	105,000 MN

Elevation of ground surface

= +1156.9 ft

+352.61 m

Elevation of water table

= +1151.0 ft

+350.82 m

Elevation of top of shaft concrete

= +1149.1 ft

+350.25 m

Elevation of base of O-cell (The break between upward and downward movement.)

= +1071.0 ft

+326.45 m

Elevation of shaft tip

= +1041.8 ft

+317.53 m

Casings:

Elevation of top of permanent casing (102.0 in O.D., 101.1 in I.D.)	=	+1156.9 ft	+352.61 m
Elevation of bottom of permanent casing (102.0 in O.D., 101.1 in I.D.)	=	+1121.7 ft	+341.89 m

Compression Sections:

Elevation of top of telltale used for upper shaft compression	=	+1156.9 ft	+352.61 m
Elevation of bottom of telltale used for upper shaft compression	=	+1072.2 ft	+326.79 m
Elevation of top of ECTs used for lower shaft compression	=	+1070.9 ft	+326.40 m
Elevation of bottom of ECTs used for lower shaft compression	=	+1045.0 ft	+318.52 m

Strain Gages:

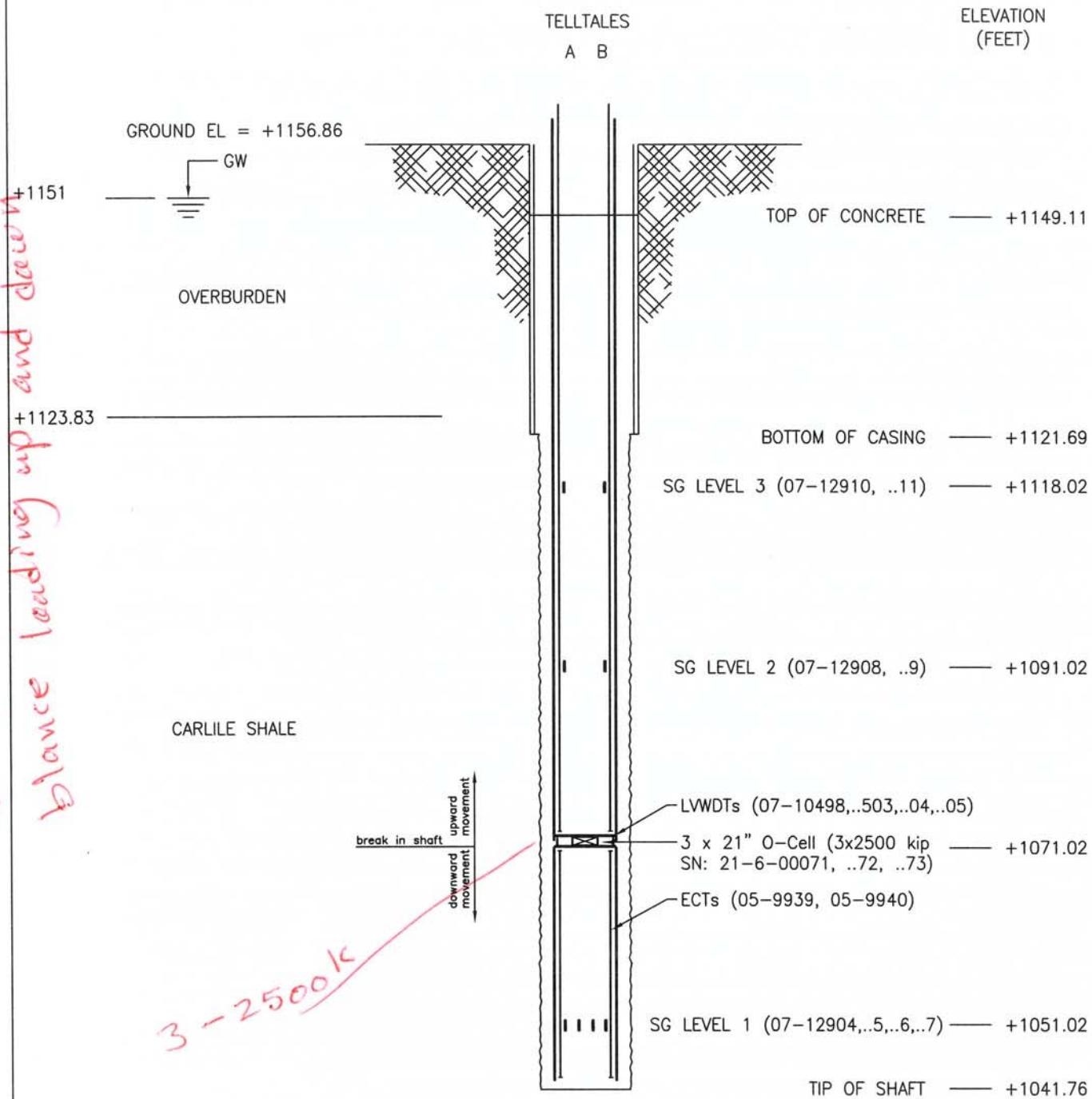
Elevation of strain gage Level 3 (AE = 25,500,000 kips)	=	+1118.0 ft	+340.77 m
Elevation of strain gage Level 2 (AE = 25,500,000 kips)	=	+1091.0 ft	+332.54 m
Elevation of strain gage Level 1 (AE = 24,500,000 kips)	=	+1051.0 ft	+320.35 m

Miscellaneous:

Top plate diameter (2 in thickness)	=	85.5 in	2172 mm
Bottom plate diameter (2 in thickness)	=	85.5 in	2172 mm
ReBar size (48 No.)	=	# 11	M 35
Spiral size (12 in spacing)	=	# 5	M 16
ReBar cage diameter	=	90 in	2286 mm
Average unconfined compressive concrete strength	=	3,256 psi	22.4 MPa
O-cell LVWDTs @ 0°, 120°, 180° and 240° with radius	=	42.0 in	1067 mm

*The location of cell is chosen based on
balance loading up and down*

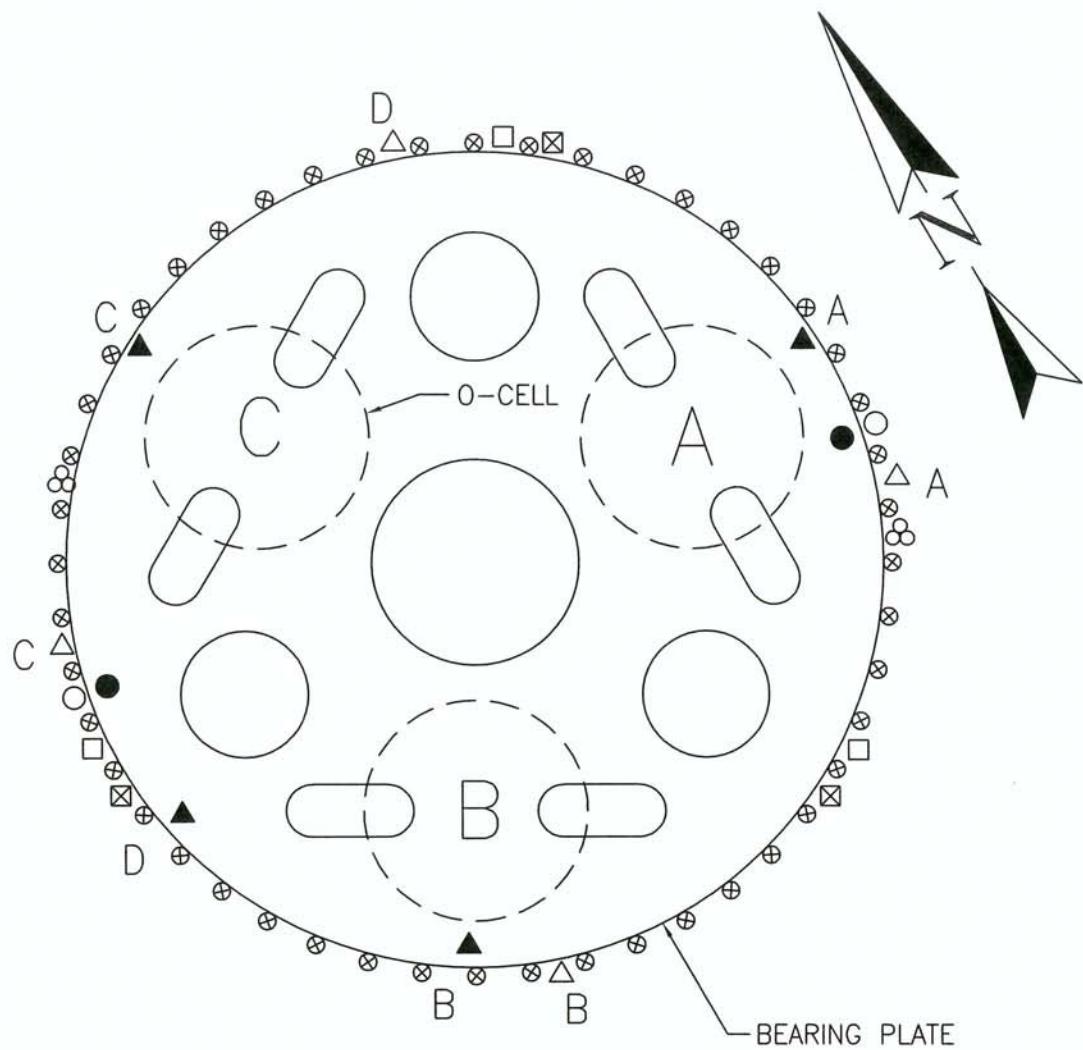
NOTE:
 - NOMINAL OUTER CASING 102"Ø OD
 - NOMINAL SOCKET 96"Ø OD



2631-D NW 41st St.
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SCHEMATIC SECTION OF TEST SHAFT
 US Highway 81 over Missouri River – Yankton, SD

DRAWN BY: WGR	DATE: 06/15/07	CHECKED BY: MDA	LT-9152
REVISED BY: MDA	DATE: 07/02/07	SCALE: NTS	FIGURE A



LEGEND:

- STRAIN GAGE
 - LVWDT
 - TELLTALE
 - ECT
 - VENT PIPE
 - HYDRAULIC HOSES
 - REBAR
 - CABLE BUNDLE
- | | |
|-----|-----|
| △ | ▲ |
| ○ | ● |
| □ | ⊗ |
| ⊗ | ⊗ |
| ○○○ | ○○○ |



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INSTRUMENTATION LAYOUT

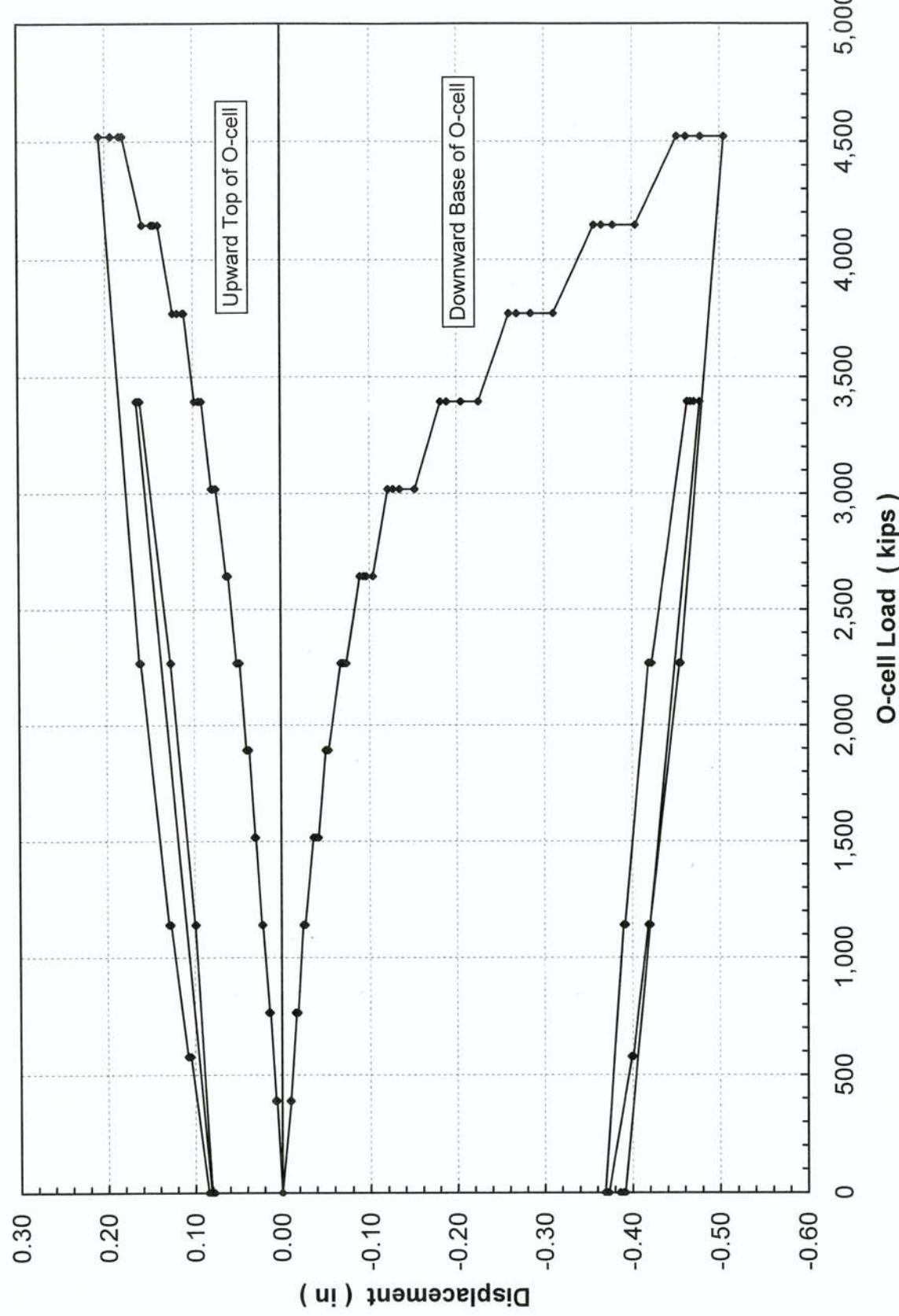
US Highway 81 over Missouri River - Yankton, SD

DWN BY: JFU	DATE: 6/25/07	CHECKED BY:	LT-9152
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REVISED BY:	DATE:	SCALE: NTS	FIGURE B
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Osterberg Cell Load-Movement Curves

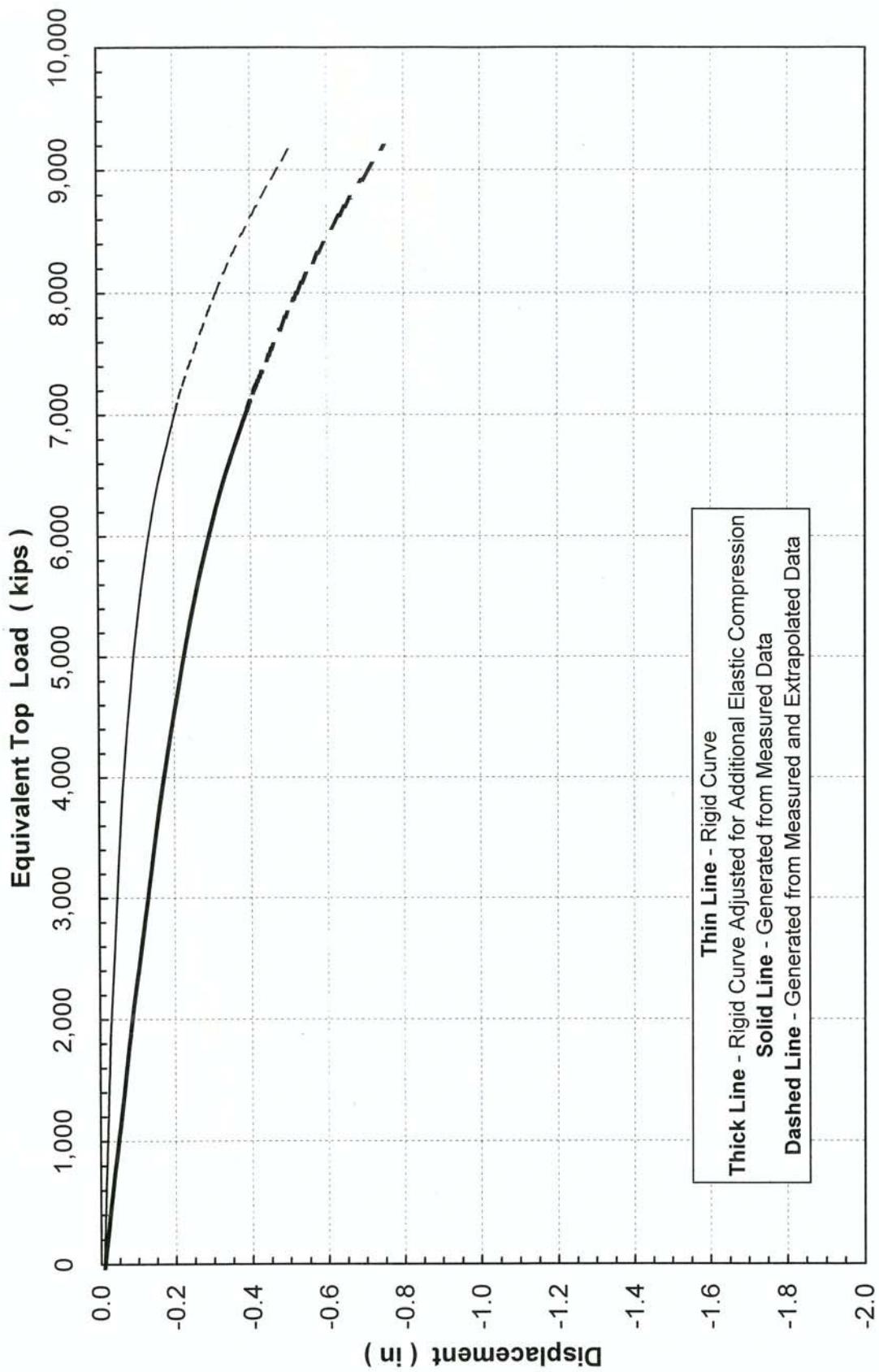
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota



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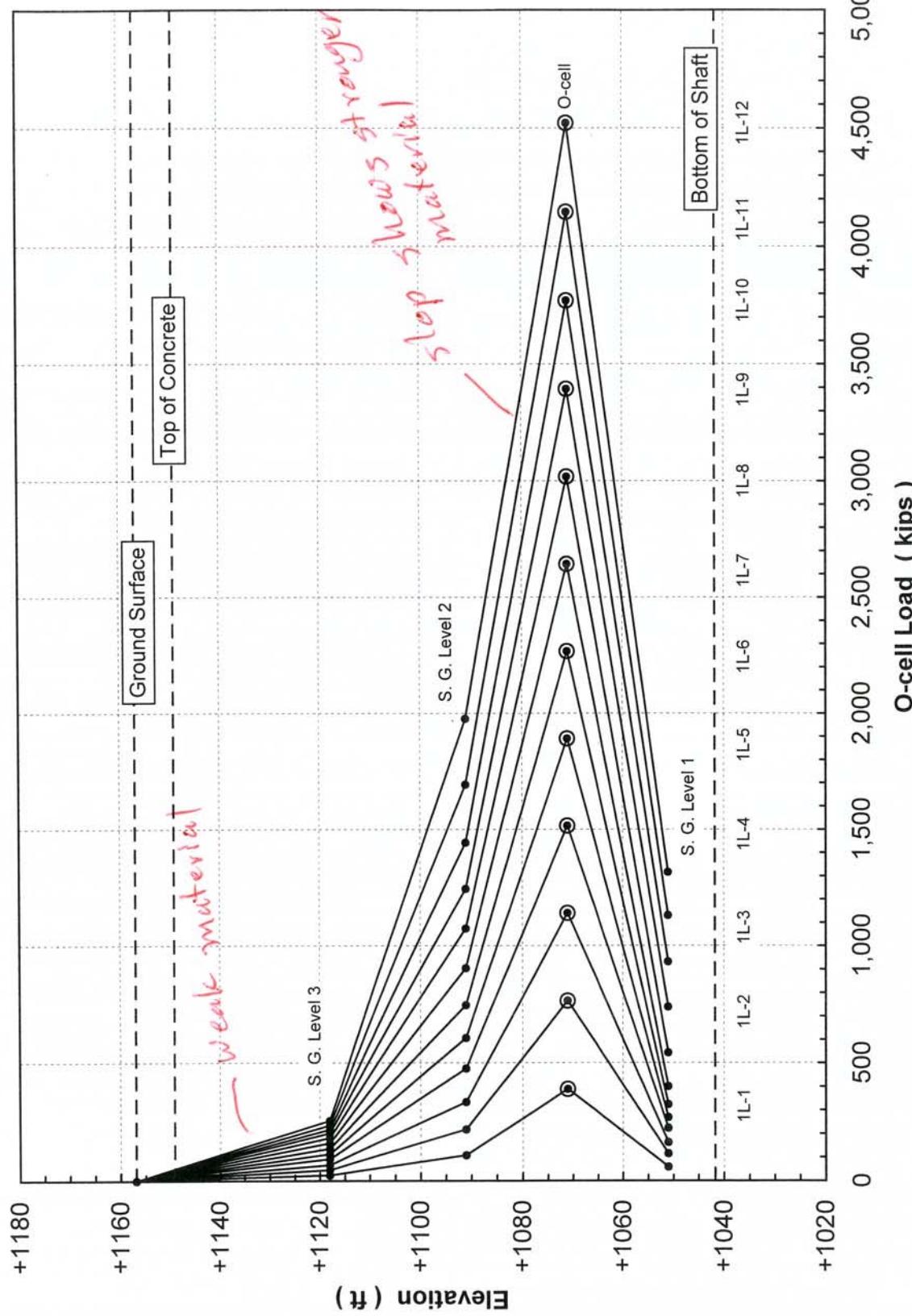
Equivalent Top Load Load-Movement Curve

Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota



Strain Gage Load Distribution Curves

Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota



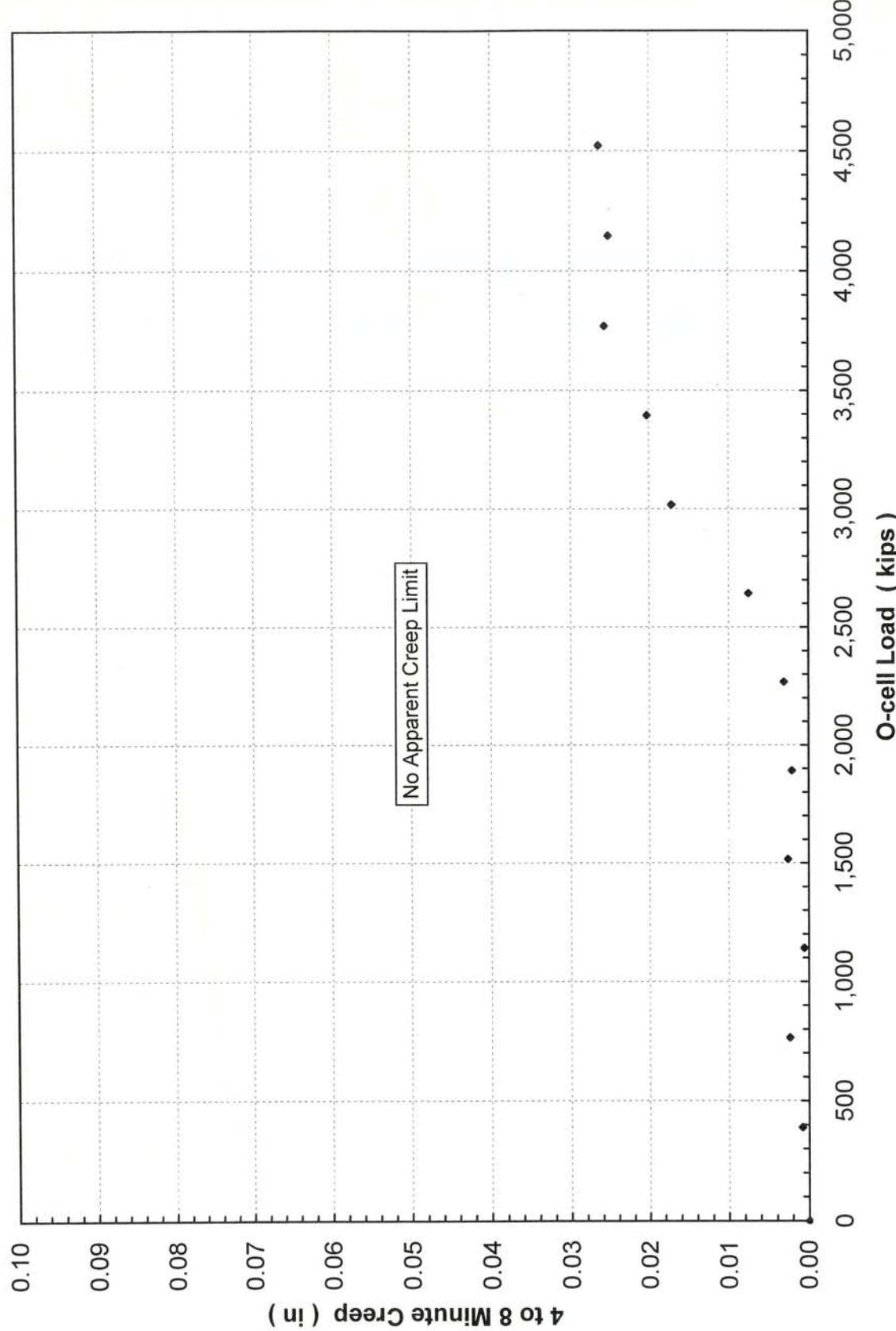
LOADTEST, Inc. Project No. LT-9152

Figure 3 of 5



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Combined End Bearing and Lower Side Shear Creep Limit
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota



LOADTEST, Inc. Project No. LT-9152

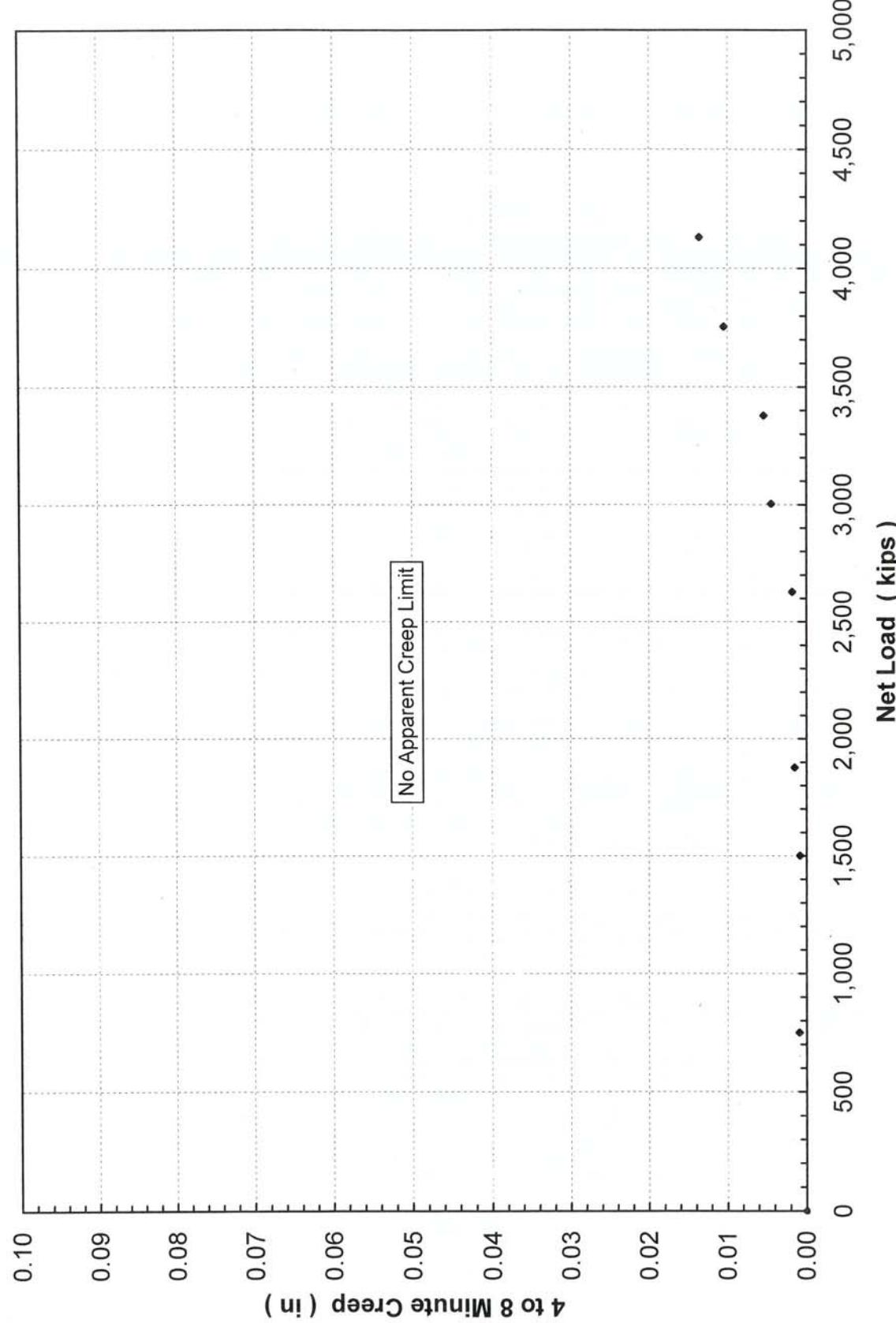
Figure 4 of 5



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Upper Side Shear Creep Limit

Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota



Test Shaft #1 - Highway 81 over Missouri River
Yankton, South Dakota (LT-9152)

APPENDIX A

FIELD DATA & DATA REDUCTION



DEEP FOUNDATION TESTING, EQUIPMENT & SERVICES • SPECIALIZING IN OSTERBERG CELL (O-cell®) TECHNOLOGY
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O-cell Loads
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota

Load Test Increment	Hold Time (minutes)	Time (h:m:s)	O-cell				
			Pressure (psi)	21-6-00071 (kips)	21-6-00072 (kips)	21-6-00073 (kips)	Load (kips)
1 L - 0	-	17:31:00	0	0	0	0	0
1 L - 1	1	17:45:30	500	131	129	129	389
1 L - 1	2	17:46:30	500	131	129	129	389
1 L - 1	4	17:48:30	500	131	129	129	389
1 L - 1	8	17:52:30	500	131	129	129	389
1 L - 2	1	17:54:30	1,000	256	254	255	765
1 L - 2	2	17:55:30	1,000	256	254	255	765
1 L - 2	4	17:57:30	1,000	256	254	255	765
1 L - 2	8	18:01:30	1,000	256	254	255	765
1 L - 3	1	18:03:30	1,500	381	380	380	1,141
1 L - 3	2	18:04:30	1,500	381	380	380	1,141
1 L - 3	4	18:06:30	1,500	381	380	380	1,141
1 L - 3	8	18:10:30	1,500	381	380	380	1,141
1 L - 4	1	18:12:30	2,000	507	505	505	1,516
1 L - 4	2	18:13:30	2,000	507	505	505	1,516
1 L - 4	4	18:15:30	2,000	507	505	505	1,516
1 L - 4	8	18:19:30	2,000	507	505	505	1,516
1 L - 5	1	18:22:30	2,500	632	630	630	1,892
1 L - 5	2	18:23:30	2,500	632	630	630	1,892
1 L - 5	4	18:25:30	2,500	632	630	630	1,892
1 L - 5	8	18:29:30	2,500	632	630	630	1,892
1 L - 6	1	18:32:00	3,000	757	755	756	2,268
1 L - 6	2	18:33:00	3,000	757	755	756	2,268
1 L - 6	4	18:35:00	3,000	757	755	756	2,268
1 L - 6	8	18:39:00	3,000	757	755	756	2,268
1 L - 7	1	18:41:30	3,500	882	880	881	2,643
1 L - 7	2	18:42:30	3,500	882	880	881	2,643
1 L - 7	4	18:44:30	3,500	882	880	881	2,643
1 L - 7	8	18:48:30	3,500	882	880	881	2,643
1 L - 8	1	18:51:00	4,000	1,008	1,006	1,006	3,019
1 L - 8	2	18:52:00	4,000	1,008	1,006	1,006	3,019
1 L - 8	4	18:54:00	4,000	1,008	1,006	1,006	3,019
1 L - 8	8	18:58:00	4,000	1,008	1,006	1,006	3,019
1 L - 9	1	19:01:00	4,500	1,133	1,131	1,131	3,395
1 L - 9	2	19:02:00	4,500	1,133	1,131	1,131	3,395
1 L - 9	4	19:04:00	4,500	1,133	1,131	1,131	3,395
1 L - 9	8	19:08:00	4,500	1,133	1,131	1,131	3,395
1 L - 10	1	19:10:30	5,000	1,258	1,256	1,257	3,771
1 L - 10	2	19:11:30	5,000	1,258	1,256	1,257	3,771
1 L - 10	4	19:13:30	5,000	1,258	1,256	1,257	3,771
1 L - 10	8	19:17:30	5,000	1,258	1,256	1,257	3,771
1 L - 11	1	19:20:30	5,500	1,383	1,381	1,382	4,146
1 L - 11	2	19:21:30	5,500	1,383	1,381	1,382	4,146
1 L - 11	4	19:23:30	5,500	1,383	1,381	1,382	4,146
1 L - 11	8	19:27:30	5,500	1,383	1,381	1,382	4,146
1 L - 12	1	19:31:30	6,000	1,509	1,506	1,507	4,522
1 L - 12	2	19:32:30	6,000	1,509	1,506	1,507	4,522
1 L - 12	4	19:34:30	6,000	1,509	1,506	1,507	4,522
1 L - 12	8	19:38:30	6,000	1,509	1,506	1,507	4,522
1 U - 1	1	19:42:30	3,000	757	755	756	2,268
1 U - 1	2	19:43:30	3,000	757	755	756	2,268
1 U - 1	4	19:45:30	3,000	757	755	756	2,268
1 U - 2	1	19:48:00	1,500	381	380	380	1,141
1 U - 2	2	19:49:00	1,500	381	380	380	1,141
1 U - 2	4	19:51:00	1,500	381	380	380	1,141
1 U - 3	1	19:53:30	750	193	192	192	577
1 U - 3	2	19:54:30	750	193	192	192	577
1 U - 3	4	19:56:30	750	193	192	192	577
1 U - 4	1	19:58:30	0	0	0	0	0
1 U - 4	2	19:59:30	0	0	0	0	0
1 U - 4	4	20:01:30	0	0	0	0	0
2 L - 0	-	20:02:00	0	0	0	0	0
2 L - 1	1	20:05:30	1,500	381	380	380	1,141
2 L - 1	2	20:06:30	1,500	381	380	380	1,141
2 L - 1	4	20:08:30	1,500	381	380	380	1,141
2 L - 2	1	20:12:00	3,000	757	755	756	2,268
2 L - 2	2	20:13:00	3,000	757	755	756	2,268
2 L - 2	4	20:15:00	3,000	757	755	756	2,268
2 L - 3	1	20:19:00	4,500	1,133	1,131	1,131	3,395
2 L - 3	2	20:20:00	4,500	1,133	1,131	1,131	3,395
2 L - 3	4	20:22:00	4,500	1,133	1,131	1,131	3,395
2 L - 3	8	20:26:00	4,500	1,133	1,131	1,131	3,395
2 U - 1	1	20:30:30	0	0	0	0	0
2 U - 1	2	20:31:30	0	0	0	0	0
2 U - 1	4	20:33:30	0	0	0	0	0
2 U - 1	8	20:37:30	0	0	0	0	0

Top of Shaft Displacement
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota

Load Test Increment	Hold Time (minutes)	Time (h:m:s)	O-cell		Ref. Beam (in)	Top of Shaft			
			Pressure (psi)	Load (kips)		A (in)	B (in)	Avg. * (in)	Leica (in)
1L-0	-	17:31:00	0	0	0.000	0.000	0.000	0.000	0.000
1L-1	1	17:45:30	500	389	-0.004	0.008	0.007	0.004	0.005
1L-1	2	17:48:30	500	389	-0.005	0.009	0.010	0.005	0.004
1L-1	4	17:48:30	500	389	-0.005	0.010	0.010	0.005	0.003
1L-1	8	17:52:30	500	389	-0.006	0.011	0.011	0.005	0.005
1L-2	1	17:54:30	1,000	765	-0.005	0.015	0.014	0.009	0.008
1L-2	2	17:55:30	1,000	765	-0.005	0.016	0.015	0.011	0.010
1L-2	4	17:57:30	1,000	765	-0.007	0.017	0.017	0.011	0.008
1L-2	8	18:01:30	1,000	765	-0.007	0.016	0.017	0.010	0.008
1L-3	1	18:03:30	1,500	1,141	-0.008	0.022	0.024	0.016	0.013
1L-3	2	18:04:30	1,500	1,141	-0.008	0.022	0.024	0.016	0.013
1L-3	4	18:06:30	1,500	1,141	-0.009	0.024	0.024	0.016	0.015
1L-3	8	18:10:30	1,500	1,141	-0.009	0.025	0.025	0.016	0.015
1L-4	1	18:12:30	2,000	1,516	-0.009	0.030	0.029	0.021	0.020
1L-4	2	18:13:30	2,000	1,516	-0.010	0.032	0.031	0.022	0.020
1L-4	4	18:15:30	2,000	1,516	-0.011	0.032	0.032	0.021	0.018
1L-4	8	18:19:30	2,000	1,516	-0.013	0.033	0.035	0.021	0.020
1L-5	1	18:22:30	2,500	1,892	-0.013	0.038	0.041	0.026	0.025
1L-5	2	18:23:30	2,500	1,892	-0.014	0.041	0.043	0.028	0.026
1L-5	4	18:25:30	2,500	1,892	-0.015	0.042	0.045	0.028	0.028
1L-5	8	18:29:30	2,500	1,892	-0.020	0.047	0.052	0.029	0.028
1L-6	1	18:32:00	3,000	2,268	-0.022	0.054	0.059	0.034	0.034
1L-6	2	18:33:00	3,000	2,268	-0.022	0.055	0.060	0.035	0.035
1L-6	4	18:35:00	3,000	2,268	-0.023	0.057	0.062	0.036	0.035
1L-6	8	18:39:00	3,000	2,268	-0.026	0.061	0.066	0.038	0.036
1L-7	1	18:41:30	3,500	2,643	-0.026	0.069	0.073	0.045	0.043
1L-7	2	18:42:30	3,500	2,643	-0.026	0.070	0.074	0.046	0.044
1L-7	4	18:44:30	3,500	2,643	-0.027	0.072	0.076	0.047	0.045
1L-7	8	18:48:30	3,500	2,643	-0.030	0.074	0.079	0.047	0.046
1L-8	1	18:51:00	4,000	3,019	-0.028	0.082	0.086	0.056	0.054
1L-8	2	18:52:00	4,000	3,019	-0.028	0.083	0.087	0.057	0.055
1L-8	4	18:54:00	4,000	3,019	-0.028	0.086	0.089	0.059	0.057
1L-8	8	18:58:00	4,000	3,019	-0.029	0.089	0.090	0.060	0.058
1L-9	1	19:01:00	4,500	3,395	-0.028	0.098	0.097	0.070	0.067
1L-9	2	19:02:00	4,500	3,395	-0.028	0.100	0.100	0.072	0.068
1L-9	4	19:04:00	4,500	3,395	-0.028	0.102	0.100	0.073	0.070
1L-9	8	19:08:00	4,500	3,395	-0.028	0.106	0.104	0.077	0.073
1L-10	1	19:10:30	5,000	3,771	-0.027	0.115	0.112	0.086	0.083
1L-10	2	19:11:30	5,000	3,771	-0.028	0.117	0.114	0.088	0.084
1L-10	4	19:13:30	5,000	3,771	-0.028	0.122	0.119	0.093	0.089
1L-10	8	19:17:30	5,000	3,771	-0.028	0.127	0.124	0.098	0.093
1L-11	1	19:20:30	5,500	4,146	-0.027	0.141	0.137	0.112	0.108
1L-11	2	19:21:30	5,500	4,146	-0.027	0.145	0.142	0.117	0.112
1L-11	4	19:23:30	5,500	4,146	-0.027	0.147	0.145	0.119	0.115
1L-11	8	19:27:30	5,500	4,146	-0.026	0.157	0.155	0.129	0.124
1L-12	1	19:31:30	6,000	4,522	-0.027	0.177	0.175	0.149	0.143
1L-12	2	19:32:30	6,000	4,522	-0.028	0.182	0.179	0.153	0.148
1L-12	4	19:34:30	6,000	4,522	-0.027	0.190	0.188	0.162	0.157
1L-12	8	19:38:30	6,000	4,522	-0.028	0.204	0.201	0.175	0.171
1U-1	1	19:42:30	3,000	2,268	-0.029	0.172	0.168	0.140	0.134
1U-1	2	19:43:30	3,000	2,268	-0.029	0.172	0.167	0.140	0.133
1U-1	4	19:45:30	3,000	2,268	-0.029	0.170	0.167	0.139	0.133
1U-2	1	19:48:00	1,500	1,141	-0.031	0.148	0.142	0.114	0.109
1U-2	2	19:49:00	1,500	1,141	-0.031	0.145	0.142	0.113	0.107
1U-2	4	19:51:00	1,500	1,141	-0.032	0.146	0.141	0.112	0.106
1U-3	1	19:53:30	750	577	-0.032	0.132	0.126	0.097	0.092
1U-3	2	19:54:30	750	577	-0.032	0.132	0.126	0.097	0.091
1U-3	4	19:56:30	750	577	-0.033	0.129	0.125	0.094	0.090
1U-4	1	19:58:30	0	0	-0.033	0.114	0.109	0.078	0.072
1U-4	2	19:59:30	0	0	-0.034	0.112	0.108	0.076	0.071
1U-4	4	20:01:30	0	0	-0.034	0.112	0.106	0.075	0.070
2L-0	-	20:02:00	0	0	-0.034	0.112	0.106	0.075	0.069
2L-1	1	20:05:30	1,500	1,141	-0.034	0.125	0.120	0.088	0.083
2L-1	2	20:06:30	1,500	1,141	-0.035	0.125	0.119	0.087	0.083
2L-1	4	20:08:30	1,500	1,141	-0.035	0.126	0.121	0.088	0.084
2L-2	1	20:12:00	3,000	2,268	-0.035	0.146	0.142	0.109	0.103
2L-2	2	20:13:00	3,000	2,268	-0.036	0.146	0.142	0.108	0.104
2L-2	4	20:15:00	3,000	2,268	-0.036	0.147	0.142	0.109	0.105
2L-3	1	20:19:00	4,500	3,395	-0.035	0.174	0.170	0.136	0.131
2L-3	2	20:20:00	4,500	3,395	-0.036	0.175	0.171	0.137	0.131
2L-3	4	20:22:00	4,500	3,395	-0.037	0.178	0.174	0.139	0.133
2L-3	8	20:26:00	4,500	3,395	-0.037	0.180	0.175	0.140	0.135
2U-1	1	20:30:30	0	0	-0.041	0.119	0.114	0.075	0.070
2U-1	2	20:31:30	0	0	-0.041	0.119	0.113	0.075	0.070
2U-1	4	20:33:30	0	0	-0.041	0.119	0.114	0.075	0.069
2U-1	8	20:37:30	0	0	-0.044	0.119	0.113	0.072	0.067

* Average top of shaft includes reference beam correction.

Shaft Compression
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota

Load Test Increment	Hold Time (minutes)	Time (h:m:s)	O-cell		ECTs - Level 1			Telltale - Level 2		
			Pressure (psi)	Load (kips)	A - 9939 (in)	B - 9940 (in)	Average (in)	A (in)	B (in)	Average (in)
1 L - 0	-	17:31:00	0	0	0.000	0.000	0.000	0.000	0.000	0.000
1 L - 1	1	17:45:30	500	389	0.001	0.001	0.001	0.002	0.002	0.002
1 L - 1	2	17:46:30	500	389	0.001	0.001	0.001	0.003	0.002	0.002
1 L - 1	4	17:48:30	500	389	0.001	0.001	0.001	0.002	0.002	0.002
1 L - 1	8	17:52:30	500	389	0.001	0.001	0.001	0.002	0.002	0.002
1 L - 2	1	17:54:30	1,000	765	0.002	0.003	0.002	0.004	0.003	0.004
1 L - 2	2	17:55:30	1,000	765	0.003	0.003	0.003	0.005	0.003	0.004
1 L - 2	4	17:57:30	1,000	765	0.002	0.003	0.002	0.004	0.003	0.004
1 L - 2	8	18:01:30	1,000	765	0.003	0.003	0.003	0.005	0.003	0.004
1 L - 3	1	18:03:30	1,500	1,141	0.003	0.004	0.004	0.007	0.005	0.006
1 L - 3	2	18:04:30	1,500	1,141	0.003	0.004	0.004	0.007	0.005	0.006
1 L - 3	4	18:06:30	1,500	1,141	0.003	0.004	0.004	0.007	0.005	0.006
1 L - 3	8	18:10:30	1,500	1,141	0.004	0.004	0.004	0.007	0.006	0.006
1 L - 4	1	18:12:30	2,000	1,516	0.005	0.006	0.005	0.009	0.007	0.008
1 L - 4	2	18:13:30	2,000	1,516	0.005	0.006	0.005	0.010	0.007	0.009
1 L - 4	4	18:15:30	2,000	1,516	0.005	0.006	0.005	0.010	0.008	0.009
1 L - 4	8	18:19:30	2,000	1,516	0.005	0.006	0.005	0.010	0.008	0.009
1 L - 5	1	18:22:30	2,500	1,892	0.006	0.007	0.006	0.012	0.010	0.011
1 L - 5	2	18:23:30	2,500	1,892	0.006	0.007	0.007	0.012	0.010	0.011
1 L - 5	4	18:25:30	2,500	1,892	0.006	0.007	0.007	0.012	0.010	0.011
1 L - 5	8	18:29:30	2,500	1,892	0.006	0.007	0.007	0.012	0.010	0.011
1 L - 6	1	18:32:00	3,000	2,268	0.007	0.009	0.008	0.014	0.012	0.013
1 L - 6	2	18:33:00	3,000	2,268	0.007	0.009	0.008	0.015	0.012	0.013
1 L - 6	4	18:35:00	3,000	2,268	0.007	0.009	0.008	0.015	0.012	0.013
1 L - 6	8	18:39:00	3,000	2,268	0.008	0.009	0.008	0.015	0.012	0.013
1 L - 7	1	18:41:30	3,500	2,643	0.009	0.010	0.010	0.017	0.014	0.016
1 L - 7	2	18:42:30	3,500	2,643	0.009	0.011	0.010	0.017	0.014	0.016
1 L - 7	4	18:44:30	3,500	2,643	0.009	0.011	0.010	0.017	0.014	0.016
1 L - 7	8	18:48:30	3,500	2,643	0.009	0.011	0.010	0.018	0.014	0.016
1 L - 8	1	18:51:00	4,000	3,019	0.010	0.012	0.011	0.020	0.016	0.018
1 L - 8	2	18:52:00	4,000	3,019	0.011	0.013	0.012	0.020	0.016	0.018
1 L - 8	4	18:54:00	4,000	3,019	0.011	0.013	0.012	0.021	0.016	0.018
1 L - 8	8	18:58:00	4,000	3,019	0.011	0.013	0.012	0.021	0.017	0.019
1 L - 9	1	19:01:00	4,500	3,395	0.013	0.015	0.014	0.023	0.019	0.021
1 L - 9	2	19:02:00	4,500	3,395	0.013	0.016	0.014	0.023	0.019	0.021
1 L - 9	4	19:04:00	4,500	3,395	0.013	0.016	0.015	0.024	0.019	0.021
1 L - 9	8	19:08:00	4,500	3,395	0.013	0.017	0.015	0.024	0.019	0.022
1 L - 10	1	19:10:30	5,000	3,771	0.015	0.019	0.017	0.026	0.021	0.024
1 L - 10	2	19:11:30	5,000	3,771	0.016	0.019	0.017	0.026	0.021	0.024
1 L - 10	4	19:13:30	5,000	3,771	0.016	0.020	0.018	0.027	0.022	0.024
1 L - 10	8	19:17:30	5,000	3,771	0.016	0.020	0.018	0.028	0.022	0.025
1 L - 11	1	19:20:30	5,500	4,146	0.018	0.022	0.020	0.030	0.024	0.027
1 L - 11	2	19:21:30	5,500	4,146	0.019	0.023	0.021	0.031	0.024	0.028
1 L - 11	4	19:23:30	5,500	4,146	0.019	0.023	0.021	0.031	0.024	0.028
1 L - 11	8	19:27:30	5,500	4,146	0.019	0.023	0.021	0.032	0.025	0.028
1 L - 12	1	19:31:30	6,000	4,522	0.021	0.025	0.023	0.034	0.027	0.031
1 L - 12	2	19:32:30	6,000	4,522	0.021	0.025	0.023	0.034	0.027	0.031
1 L - 12	4	19:34:30	6,000	4,522	0.021	0.026	0.023	0.035	0.028	0.031
1 L - 12	8	19:38:30	6,000	4,522	0.021	0.026	0.024	0.036	0.028	0.032
1 U - 1	1	19:42:30	3,000	2,268	0.014	0.019	0.016	0.025	0.020	0.022
1 U - 1	2	19:43:30	3,000	2,268	0.014	0.019	0.016	0.024	0.020	0.022
1 U - 1	4	19:45:30	3,000	2,268	0.014	0.019	0.016	0.024	0.020	0.022
1 U - 2	1	19:48:00	1,500	1,141	0.009	0.014	0.011	0.016	0.014	0.015
1 U - 2	2	19:49:00	1,500	1,141	0.009	0.014	0.011	0.016	0.013	0.015
1 U - 2	4	19:51:00	1,500	1,141	0.009	0.013	0.011	0.016	0.013	0.015
1 U - 3	1	19:53:30	750	577	0.007	0.011	0.009	0.012	0.010	0.011
1 U - 3	2	19:54:30	750	577	0.007	0.011	0.009	0.012	0.010	0.011
1 U - 3	4	19:56:30	750	577	0.006	0.011	0.009	0.012	0.010	0.011
1 U - 4	1	19:58:30	0	0	0.004	0.007	0.006	0.007	0.006	0.007
1 U - 4	2	19:59:30	0	0	0.004	0.007	0.006	0.007	0.006	0.007
1 U - 4	4	20:01:30	0	0	0.004	0.007	0.005	0.007	0.006	0.006
2 L - 0	-	20:02:00	0	0	0.004	0.007	0.005	0.007	0.005	0.006
2 L - 1	1	20:05:30	1,500	1,141	0.007	0.011	0.009	0.012	0.010	0.011
2 L - 1	2	20:06:30	1,500	1,141	0.007	0.011	0.009	0.012	0.010	0.011
2 L - 1	4	20:08:30	1,500	1,141	0.007	0.011	0.009	0.012	0.010	0.011
2 L - 2	1	20:12:00	3,000	2,268	0.011	0.015	0.013	0.020	0.015	0.018
2 L - 2	2	20:13:00	3,000	2,268	0.011	0.015	0.013	0.020	0.016	0.018
2 L - 2	4	20:15:00	3,000	2,268	0.011	0.015	0.013	0.020	0.015	0.018
2 L - 3	1	20:19:00	4,500	3,395	0.015	0.020	0.018	0.027	0.022	0.024
2 L - 3	2	20:20:00	4,500	3,395	0.015	0.020	0.018	0.027	0.022	0.025
2 L - 3	4	20:22:00	4,500	3,395	0.015	0.020	0.018	0.028	0.022	0.025
2 L - 3	8	20:26:00	4,500	3,395	0.015	0.021	0.018	0.028	0.022	0.025
2 U - 1	1	20:30:30	0	0	0.003	0.007	0.005	0.007	0.005	0.006
2 U - 1	2	20:31:30	0	0	0.003	0.007	0.005	0.007	0.005	0.006
2 U - 1	4	20:33:30	0	0	0.003	0.007	0.005	0.007	0.005	0.006
2 U - 1	8	20:37:30	0	0	0.003	0.007	0.005	0.007	0.005	0.006

O-cell Expansion
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota

Load Test Increment	Hold Time (minutes)	Time (h:m:s)	O-cell		O-cell Expansion				Average * (in)
			Pressure (psi)	Load (kips)	A - 10498 - 0° (in)	B - 10503 - 120° (in)	C - 10504 - 180° (in)	D - 10505 - 240° (in)	
1 L - 0	-	17:31:00	0	0	0.000	0.000	0.000	0.000	0.000
1 L - 1	1	17:45:30	500	369	0.023	0.018	0.014	0.026	0.016
1 L - 1	2	17:46:30	500	369	0.023	0.018	0.014	0.026	0.016
1 L - 1	4	17:48:30	500	389	0.023	0.018	0.015	0.026	0.016
1 L - 1	8	17:52:30	500	389	0.024	0.019	0.015	0.026	0.017
1 L - 2	1	17:54:30	1,000	765	0.038	0.032	0.028	0.040	0.030
1 L - 2	2	17:55:30	1,000	765	0.038	0.033	0.028	0.039	0.030
1 L - 2	4	17:57:30	1,000	765	0.039	0.033	0.029	0.040	0.031
1 L - 2	8	18:01:30	1,000	765	0.041	0.035	0.031	0.042	0.033
1 L - 3	1	18:03:30	1,500	1,141	0.054	0.048	0.044	0.055	0.046
1 L - 3	2	18:04:30	1,500	1,141	0.055	0.049	0.045	0.056	0.047
1 L - 3	4	18:06:30	1,500	1,141	0.057	0.050	0.046	0.058	0.048
1 L - 3	8	18:10:30	1,500	1,141	0.058	0.052	0.048	0.059	0.050
1 L - 4	1	18:12:30	2,000	1,516	0.075	0.068	0.064	0.075	0.066
1 L - 4	2	18:13:30	2,000	1,516	0.077	0.070	0.066	0.077	0.068
1 L - 4	4	18:15:30	2,000	1,516	0.079	0.072	0.068	0.079	0.070
1 L - 4	8	18:19:30	2,000	1,516	0.081	0.074	0.070	0.081	0.072
1 L - 5	1	18:22:30	2,500	1,892	0.097	0.089	0.086	0.097	0.088
1 L - 5	2	18:23:30	2,500	1,892	0.099	0.090	0.088	0.099	0.089
1 L - 5	4	18:25:30	2,500	1,892	0.101	0.092	0.090	0.102	0.091
1 L - 5	8	18:29:30	2,500	1,892	0.105	0.095	0.094	0.105	0.094
1 L - 6	1	18:32:00	3,000	2,268	0.125	0.116	0.114	0.126	0.115
1 L - 6	2	18:33:00	3,000	2,268	0.127	0.118	0.117	0.129	0.118
1 L - 6	4	18:35:00	3,000	2,268	0.131	0.121	0.120	0.132	0.121
1 L - 6	8	18:39:00	3,000	2,268	0.136	0.126	0.124	0.136	0.125
1 L - 7	1	18:41:30	3,500	2,643	0.160	0.150	0.149	0.160	0.149
1 L - 7	2	18:42:30	3,500	2,643	0.165	0.156	0.154	0.164	0.155
1 L - 7	4	18:44:30	3,500	2,643	0.170	0.160	0.158	0.170	0.159
1 L - 7	8	18:48:30	3,500	2,643	0.178	0.168	0.166	0.177	0.167
1 L - 8	1	18:51:00	4,000	3,019	0.206	0.195	0.195	0.205	0.195
1 L - 8	2	18:52:00	4,000	3,019	0.213	0.202	0.201	0.212	0.202
1 L - 8	4	18:54:00	4,000	3,019	0.224	0.213	0.212	0.223	0.212
1 L - 8	8	18:58:00	4,000	3,019	0.243	0.231	0.232	0.243	0.231
1 L - 9	1	19:01:00	4,500	3,395	0.284	0.272	0.272	0.283	0.272
1 L - 9	2	19:02:00	4,500	3,395	0.295	0.282	0.283	0.294	0.282
1 L - 9	4	19:04:00	4,500	3,395	0.311	0.298	0.301	0.311	0.299
1 L - 9	8	19:08:00	4,500	3,395	0.338	0.322	0.326	0.335	0.324
1 L - 10	1	19:10:30	5,000	3,771	0.384	0.367	0.373	0.383	0.370
1 L - 10	2	19:11:30	5,000	3,771	0.395	0.378	0.383	0.394	0.380
1 L - 10	4	19:13:30	5,000	3,771	0.417	0.399	0.406	0.415	0.403
1 L - 10	8	19:17:30	5,000	3,771	0.448	0.430	0.437	0.447	0.434
1 L - 11	1	19:20:30	5,500	4,146	0.511	0.491	0.501	0.511	0.496
1 L - 11	2	19:21:30	5,500	4,146	0.526	0.504	0.515	0.525	0.510
1 L - 11	4	19:23:30	5,500	4,146	0.541	0.521	0.531	0.541	0.526
1 L - 11	8	19:27:30	5,500	4,146	0.577	0.556	0.567	0.578	0.561
1 L - 12	1	19:31:30	6,000	4,522	0.648	0.624	0.638	0.646	0.631
1 L - 12	2	19:32:30	6,000	4,522	0.662	0.638	0.652	0.660	0.645
1 L - 12	4	19:34:30	6,000	4,522	0.689	0.664	0.678	0.687	0.671
1 L - 12	8	19:38:30	6,000	4,522	0.728	0.704	0.718	0.728	0.711
1 U - 1	1	19:42:30	3,000	2,268	0.633	0.613	0.623	0.633	0.618
1 U - 1	2	19:43:30	3,000	2,268	0.630	0.610	0.621	0.630	0.615
1 U - 1	4	19:45:30	3,000	2,268	0.629	0.610	0.620	0.629	0.615
1 U - 2	1	19:48:00	1,500	1,141	0.563	0.545	0.554	0.565	0.550
1 U - 2	2	19:49:00	1,500	1,141	0.561	0.544	0.553	0.564	0.548
1 U - 2	4	19:51:00	1,500	1,141	0.557	0.540	0.549	0.560	0.545
1 U - 3	1	19:53:30	750	577	0.521	0.504	0.514	0.523	0.509
1 U - 3	2	19:54:30	750	577	0.518	0.501	0.511	0.521	0.506
1 U - 3	4	19:56:30	750	577	0.516	0.499	0.508	0.518	0.504
1 U - 4	1	19:58:30	0	0	0.462	0.453	0.462	0.466	0.457
1 U - 4	2	19:59:30	0	0	0.458	0.449	0.458	0.462	0.454
1 U - 4	4	20:01:30	0	0	0.455	0.446	0.455	0.458	0.450
2 L - 0	-	20:02:00	0	0	0.454	0.445	0.454	0.459	0.450
2 L - 1	1	20:05:30	1,500	1,141	0.501	0.485	0.493	0.502	0.489
2 L - 1	2	20:06:30	1,500	1,141	0.502	0.487	0.494	0.504	0.490
2 L - 1	4	20:08:30	1,500	1,141	0.503	0.487	0.495	0.505	0.491
2 L - 2	1	20:12:00	3,000	2,268	0.558	0.540	0.549	0.559	0.545
2 L - 2	2	20:13:00	3,000	2,268	0.560	0.542	0.551	0.562	0.547
2 L - 2	4	20:15:00	3,000	2,268	0.563	0.544	0.554	0.565	0.549
2 L - 3	1	20:19:00	4,500	3,395	0.640	0.618	0.630	0.638	0.624
2 L - 3	2	20:20:00	4,500	3,395	0.644	0.622	0.634	0.643	0.628
2 L - 3	4	20:22:00	4,500	3,395	0.649	0.628	0.640	0.649	0.634
2 L - 3	8	20:26:00	4,500	3,395	0.659	0.636	0.649	0.658	0.642
2 U - 1	1	20:30:30	0	0	0.478	0.468	0.477	0.480	0.473
2 U - 1	2	20:31:30	0	0	0.475	0.466	0.475	0.478	0.470
2 U - 1	4	20:33:30	0	0	0.471	0.462	0.471	0.475	0.467
2 U - 1	8	20:37:30	0	0	0.468	0.459	0.468	0.472	0.464

* Average of 0° and 180° gages.

Upward and Downward O-cell Plate Movement and Creep (calculated)
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota

Load Test Increment	Hold Time (minutes)	Time (h:m:s)	O-cell		Top of Shaft (in)	Upper Comp. (in)	Top Plate Movement (in)	O-cell Expansion (in)	Bot. Plate Movement (in)	Lower Comp. (in)	Base of Shaft (in)	Creep Up Per Hold (in)	Creep Dn Per Hold (in)
			Pressure (psi)	Load (kips)									
1 L - 0	-	17:31:00	0	0	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	
1 L - 1	1	17:45:30	500	389	0.004	0.002	0.006	0.016	-0.010	0.001	-0.009		
1 L - 1	2	17:46:30	500	389	0.005	0.002	0.007	0.016	-0.010	0.001	-0.008	0.001	-0.001
1 L - 1	4	17:48:30	500	389	0.005	0.002	0.007	0.016	-0.009	0.001	-0.008	0.001	-0.001
1 L - 1	8	17:52:30	500	389	0.005	0.002	0.007	0.017	-0.010	0.001	-0.009	-0.001	0.001
1 L - 2	1	17:54:30	1,000	765	0.009	0.004	0.013	0.030	-0.017	0.002	-0.015		
1 L - 2	2	17:55:30	1,000	765	0.011	0.004	0.015	0.030	-0.015	0.003	-0.013	0.002	-0.002
1 L - 2	4	17:57:30	1,000	765	0.011	0.004	0.014	0.031	-0.016	0.002	-0.014	-0.001	0.001
1 L - 2	8	18:01:30	1,000	765	0.010	0.004	0.014	0.033	-0.019	0.003	-0.016	0.000	0.002
1 L - 3	1	18:03:30	1,500	1,141	0.016	0.006	0.021	0.046	-0.025	0.004	-0.021		
1 L - 3	2	18:04:30	1,500	1,141	0.016	0.006	0.022	0.047	-0.025	0.004	-0.022	0.000	0.001
1 L - 3	4	18:06:30	1,500	1,141	0.016	0.006	0.022	0.048	-0.027	0.004	-0.023	0.000	0.001
1 L - 3	8	18:10:30	1,500	1,141	0.016	0.006	0.023	0.050	-0.027	0.004	-0.023	0.001	0.001
1 L - 4	1	18:12:30	2,000	1,516	0.021	0.008	0.029	0.066	-0.037	0.005	-0.032		
1 L - 4	2	18:13:30	2,000	1,516	0.022	0.009	0.031	0.068	-0.038	0.005	-0.032	0.001	0.001
1 L - 4	4	18:15:30	2,000	1,516	0.021	0.009	0.030	0.070	-0.040	0.005	-0.035	-0.001	0.002
1 L - 4	8	18:19:30	2,000	1,516	0.021	0.009	0.030	0.072	-0.042	0.005	-0.037	0.000	0.003
1 L - 5	1	18:22:30	2,500	1,892	0.026	0.011	0.037	0.088	-0.051	0.006	-0.044		
1 L - 5	2	18:23:30	2,500	1,892	0.028	0.011	0.038	0.089	-0.051	0.007	-0.044	0.002	0.000
1 L - 5	4	18:25:30	2,500	1,892	0.028	0.011	0.039	0.091	-0.052	0.007	-0.045	0.001	0.001
1 L - 5	8	18:29:30	2,500	1,892	0.029	0.011	0.040	0.094	-0.054	0.007	-0.047	0.001	0.002
1 L - 6	1	18:32:00	3,000	2,268	0.034	0.013	0.047	0.115	-0.068	0.008	-0.060		
1 L - 6	2	18:33:00	3,000	2,268	0.035	0.013	0.048	0.118	-0.069	0.008	-0.061	0.001	0.002
1 L - 6	4	18:35:00	3,000	2,268	0.036	0.013	0.050	0.121	-0.071	0.008	-0.063	0.001	0.002
1 L - 6	8	18:39:00	3,000	2,268	0.038	0.013	0.051	0.125	-0.074	0.008	-0.066	0.002	0.003
1 L - 7	1	18:41:30	3,500	2,643	0.045	0.016	0.060	0.149	-0.089	0.010	-0.079		
1 L - 7	2	18:42:30	3,500	2,643	0.046	0.016	0.061	0.155	-0.093	0.010	-0.083	0.001	0.004
1 L - 7	4	18:44:30	3,500	2,643	0.047	0.016	0.063	0.159	-0.096	0.010	-0.086	0.001	0.003
1 L - 7	8	18:48:30	3,500	2,643	0.047	0.016	0.063	0.167	-0.104	0.010	-0.094	0.000	0.007
1 L - 8	1	18:51:00	4,000	3,019	0.056	0.018	0.074	0.195	-0.121	0.011	-0.109		
1 L - 8	2	18:52:00	4,000	3,019	0.057	0.018	0.075	0.202	-0.127	0.012	-0.115	0.001	0.006
1 L - 8	4	18:54:00	4,000	3,019	0.059	0.018	0.078	0.212	-0.135	0.012	-0.123	0.003	0.008
1 L - 8	8	18:58:00	4,000	3,019	0.060	0.019	0.080	0.231	-0.152	0.012	-0.139	0.002	0.017
1 L - 9	1	19:01:00	4,500	3,395	0.070	0.021	0.090	0.272	-0.181	0.014	-0.167		
1 L - 9	2	19:02:00	4,500	3,395	0.072	0.021	0.093	0.282	-0.189	0.014	-0.175	0.003	0.007
1 L - 9	4	19:04:00	4,500	3,395	0.073	0.021	0.094	0.299	-0.205	0.015	-0.191	0.001	0.016
1 L - 9	8	19:08:00	4,500	3,395	0.077	0.022	0.099	0.324	-0.225	0.015	-0.210	0.004	0.020
1 L - 10	1	19:10:30	5,000	3,771	0.086	0.024	0.110	0.370	-0.260	0.017	-0.243		
1 L - 10	2	19:11:30	5,000	3,771	0.088	0.024	0.111	0.380	-0.269	0.017	-0.252	0.001	0.009
1 L - 10	4	19:13:30	5,000	3,771	0.093	0.024	0.117	0.403	-0.285	0.018	-0.268	0.006	0.016
1 L - 10	8	19:17:30	5,000	3,771	0.098	0.025	0.123	0.434	-0.311	0.018	-0.293	0.005	0.026
1 L - 11	1	19:20:30	5,500	4,146	0.112	0.027	0.139	0.496	-0.357	0.020	-0.337		
1 L - 11	2	19:21:30	5,500	4,146	0.117	0.028	0.144	0.510	-0.366	0.021	-0.345	0.005	0.009
1 L - 11	4	19:23:30	5,500	4,146	0.119	0.028	0.147	0.526	-0.379	0.021	-0.358	0.003	0.013
1 L - 11	8	19:27:30	5,500	4,146	0.129	0.028	0.158	0.561	-0.404	0.021	-0.383	0.010	0.025
1 L - 12	1	19:31:30	6,000	4,522	0.149	0.031	0.180	0.631	-0.451	0.023	-0.428		
1 L - 12	2	19:32:30	6,000	4,522	0.153	0.031	0.183	0.645	-0.461	0.023	-0.438	0.004	0.010
1 L - 12	4	19:34:30	6,000	4,522	0.162	0.031	0.193	0.671	-0.478	0.023	-0.455	0.010	0.017
1 L - 12	8	19:38:30	6,000	4,522	0.175	0.032	0.207	0.711	-0.504	0.024	-0.481	0.014	0.026
1 U - 1	1	19:42:30	3,000	2,268	0.140	0.022	0.163	0.618	-0.456	0.016	-0.439		
1 U - 1	2	19:43:30	3,000	2,268	0.140	0.022	0.162	0.615	-0.454	0.016	-0.437		
1 U - 1	4	19:45:30	3,000	2,268	0.139	0.022	0.161	0.615	-0.454	0.016	-0.438		
1 U - 2	1	19:48:00	1,500	1,141	0.114	0.015	0.130	0.550	-0.420	0.011	-0.409		
1 U - 2	2	19:49:00	1,500	1,141	0.113	0.015	0.128	0.548	-0.421	0.011	-0.409		
1 U - 2	4	19:51:00	1,500	1,141	0.112	0.015	0.127	0.545	-0.418	0.011	-0.407		
1 U - 3	1	19:53:30	750	577	0.097	0.011	0.108	0.509	-0.401	0.009	-0.392		
1 U - 3	2	19:54:30	750	577	0.097	0.011	0.108	0.506	-0.398	0.009	-0.390		
1 U - 3	4	19:56:30	750	577	0.094	0.011	0.105	0.504	-0.399	0.009	-0.390		
1 U - 4	1	19:58:30	0	0	0.078	0.007	0.085	0.457	-0.373	0.006	-0.367		
1 U - 4	2	19:59:30	0	0	0.076	0.007	0.083	0.454	-0.371	0.006	-0.365		
1 U - 4	4	20:01:30	0	0	0.075	0.006	0.081	0.450	-0.369	0.005	-0.364		
2 L - 0	-	20:02:00	0	0	0.075	0.006	0.081	0.450	-0.369	0.005	-0.363		
2 L - 1	1	20:05:30	1,500	1,141	0.088	0.011	0.099	0.489	-0.390	0.009	-0.381		
2 L - 1	2	20:06:30	1,500	1,141	0.087	0.011	0.098	0.490	-0.392	0.009	-0.384		
2 L - 1	4	20:08:30	1,500	1,141	0.088	0.011	0.100	0.491	-0.392	0.009	-0.383		
2 L - 2	1	20:12:00	3,000	2,268	0.109	0.018	0.127	0.545	-0.418	0.013	-0.405		
2 L - 2	2	20:13:00	3,000	2,268	0.108	0.018	0.126	0.547	-0.421	0.013	-0.408		
2 L - 2	4	20:15:00	3,000	2,268	0.109	0.018	0.127	0.549	-0.422	0.013	-0.409		
2 L - 3	1	20:19:00	4,500	3,395	0.136	0.024	0.161	0.624	-0.463	0.018	-0.446		
2 L - 3	2	20:20:00	4,500	3,395	0.137	0.025	0.161	0.628	-0.467	0.018	-0.449		
2 L - 3	4	20:22:00	4,500	3,395	0.139	0.025	0.163	0.634	-0.471	0.018	-0.453		
2 L - 3	8	20:26:00	4,500	3,395	0.140	0.025	0.165	0.642	-0.477	0.018	-0.459		
2 U - 1	1	20:30:30	0	0	0.075	0.006	0.081	0.473	-0.391	0.005	-0.386		
2 U - 1	2	20:31:30	0	0	0.075	0.006	0.081	0.470	-0.389	0.005	-0.384		
2 U - 1	4	20:33:30	0	0	0.075	0.006	0.081	0.467	-0.385	0.005	-0.380		
2 U - 1	8	20:37:30	0	0	0.072	0.006	0.078	0.464	-0.385	0.005	-0.380		

Strain Gage Readings and Loads at Levels 1, 2 and 3
Test Shaft #1 - Highway 81 over Missouri River - Yankton, South Dakota

Load Test Increment	Hold Time (minutes)	Time (h:m:s)	O-cell		Level 1					Level 2			Level 3		
			Pressure (psi)	Load (kips)	A - 12904 ($\mu\epsilon$)	B - 12905 ($\mu\epsilon$)	C - 12906 ($\mu\epsilon$)	D - 12907 ($\mu\epsilon$)	Av. Load (kips)	A - 12908 ($\mu\epsilon$)	B - 12909 ($\mu\epsilon$)	Av. Load (kips)	A - 12910 ($\mu\epsilon$)	B - 12911 ($\mu\epsilon$)	Av. Load (kips)
1L-0	-	17:31:00	0	0	0.0	0.0	0.0	0.0	0	0.0	0.0	0	0.0	0.0	0
1L-1	1	17:45:30	500	389	2.7	2.0	2.7	2.5	60	4.4	3.9	105	0.8	1.1	24
1L-1	2	17:46:30	500	389	2.6	1.7	2.7	2.5	58	4.5	3.9	107	0.7	0.1	10
1L-1	4	17:48:30	500	389	2.7	2.0	2.7	2.5	60	4.9	3.9	112	0.7	1.1	23
1L-1	8	17:52:30	500	389	2.7	1.9	2.4	2.5	58	4.5	3.9	108	0.7	1.1	23
1L-2	1	17:54:30	1,000	765	4.8	3.8	4.8	4.8	111	8.5	7.3	201	1.4	1.9	43
1L-2	2	17:55:30	1,000	765	4.8	3.4	4.7	4.8	108	8.5	7.3	202	1.4	2.1	44
1L-2	4	17:57:30	1,000	765	5.2	3.3	4.5	4.8	109	8.6	7.4	203	1.4	1.5	36
1L-2	8	18:01:30	1,000	765	4.9	3.7	5.0	5.1	115	9.2	7.9	218	1.4	2.1	45
1L-3	1	18:03:30	1,500	1,141	7.3	5.3	6.8	7.8	166	12.8	11.5	309	2.0	3.0	64
1L-3	2	18:04:30	1,500	1,141	7.3	5.2	6.8	7.1	162	13.3	11.7	318	2.1	3.0	64
1L-3	4	18:06:30	1,500	1,141	7.4	5.2	6.8	7.2	163	13.6	12.0	326	2.1	3.0	65
1L-3	8	18:10:30	1,500	1,141	7.5	4.9	6.8	7.3	162	13.9	12.2	333	2.2	3.0	66
1L-4	1	18:12:30	2,000	1,516	10.1	7.3	8.9	10.0	222	18.0	16.7	442	2.8	3.9	85
1L-4	2	18:13:30	2,000	1,516	10.9	7.3	8.9	10.0	228	18.4	17.2	454	3.1	3.9	89
1L-4	4	18:15:30	2,000	1,516	10.1	7.2	8.8	10.2	223	19.2	17.5	468	3.1	4.0	90
1L-4	8	18:19:30	2,000	1,516	10.8	6.8	8.8	10.0	223	19.5	17.8	476	3.1	4.0	91
1L-5	1	18:22:30	2,500	1,892	11.9	8.9	10.5	12.4	268	23.7	21.5	576	3.8	4.7	109
1L-5	2	18:23:30	2,500	1,892	11.9	8.9	10.6	12.2	267	23.9	21.7	582	3.6	4.8	107
1L-5	4	18:25:30	2,500	1,892	11.9	8.8	10.8	12.6	270	24.4	22.1	592	3.9	4.8	111
1L-5	8	18:29:30	2,500	1,892	11.8	8.7	10.6	12.7	268	24.8	22.7	606	4.0	4.9	113
1L-6	1	18:32:00	3,000	2,268	14.5	10.2	12.8	15.2	323	29.5	26.7	717	4.6	5.7	132
1L-6	2	18:33:00	3,000	2,268	14.4	10.2	12.8	15.2	323	29.7	27.0	722	4.5	5.8	132
1L-6	4	18:35:00	3,000	2,268	14.5	10.5	12.9	15.3	326	30.1	27.6	737	4.6	5.9	134
1L-6	8	18:39:00	3,000	2,268	14.2	10.1	13.0	15.5	323	30.5	28.0	747	4.7	6.0	136
1L-7	1	18:41:30	3,500	2,643	16.9	13.0	15.4	18.3	390	35.7	32.5	869	5.2	6.9	155
1L-7	2	18:42:30	3,500	2,643	17.1	12.2	16.0	18.7	392	36.2	33.3	886	5.3	7.0	156
1L-7	4	18:44:30	3,500	2,643	17.0	12.2	15.7	18.9	390	36.6	33.3	890	5.3	7.1	157
1L-7	8	18:48:30	3,500	2,643	16.9	13.2	16.0	19.2	400	37.2	33.7	904	5.3	7.1	159
1L-8	1	18:51:00	4,000	3,019	20.2	15.9	19.5	22.6	479	42.1	38.5	1028	5.8	7.9	175
1L-8	2	18:52:00	4,000	3,019	20.6	16.4	20.1	23.1	491	42.7	38.9	1040	5.8	8.0	176
1L-8	4	18:54:00	4,000	3,019	21.2	17.0	21.1	23.9	510	43.2	39.3	1053	5.8	8.1	178
1L-8	8	18:58:00	4,000	3,019	22.3	17.9	22.6	25.8	543	44.3	40.0	1074	5.9	8.2	181
1L-9	1	19:01:00	4,500	3,395	26.9	22.7	27.6	30.7	680	49.2	44.0	1188	6.1	9.0	193
1L-9	2	19:02:00	4,500	3,395	27.6	23.8	28.5	31.5	682	49.4	44.4	1196	6.2	9.1	195
1L-9	4	19:04:00	4,500	3,395	28.2	25.0	29.4	32.5	705	50.3	44.8	1212	6.2	9.2	196
1L-9	8	19:08:00	4,500	3,395	29.1	26.5	31.2	33.8	739	51.8	45.8	1244	6.4	9.4	201
1L-10	1	19:10:30	5,000	3,771	33.6	31.2	36.0	38.2	852	56.4	49.8	1355	6.4	10.0	209
1L-10	2	19:11:30	5,000	3,771	34.1	31.9	36.6	38.6	865	57.1	50.1	1367	6.4	10.2	211
1L-10	4	19:13:30	5,000	3,771	35.7	33.6	38.0	39.6	900	58.7	51.7	1408	6.4	10.3	214
1L-10	8	19:17:30	5,000	3,771	36.7	35.2	39.5	40.6	932	60.1	53.1	1443	6.5	10.5	217
1L-11	1	19:20:30	5,500	4,146	42.6	41.5	45.5	45.8	1074	66.3	58.6	1592	6.7	11.2	228
1L-11	2	19:21:30	5,500	4,146	43.2	42.3	46.3	46.3	1091	67.2	59.4	1615	6.7	11.3	229
1L-11	4	19:23:30	5,500	4,146	43.3	42.6	46.4	46.4	1094	68.1	60.0	1633	6.7	11.4	230
1L-11	8	19:27:30	5,500	4,146	44.7	44.2	48.0	47.7	1131	70.6	62.2	1693	6.8	11.6	234
1L-12	1	19:31:30	6,000	4,522	49.6	49.8	53.3	51.8	1252	76.4	68.2	1843	6.9	12.3	245
1L-12	2	19:32:30	6,000	4,522	50.1	50.4	54.0	52.8	1269	77.4	69.2	1869	6.9	12.4	247
1L-12	4	19:34:30	6,000	4,522	50.9	51.3	54.7	53.5	1290	79.1	71.1	1914	7.0	12.6	251
1L-12	8	19:38:30	6,000	4,522	51.9	52.3	56.1	54.5	1316	81.7	73.3	1976	7.0	12.9	254
1U-1	1	19:42:30	3,000	2,268	33.8	35.1	39.5	37.8	895	57.7	52.6	1406	3.5	7.6	141
1U-1	2	19:43:30	3,000	2,268	33.1	34.3	39.0	37.4	881	57.3	52.0	1394	3.4	7.5	139
1U-1	4	19:45:30	3,000	2,268	32.7	33.9	38.8	37.3	874	57.3	52.0	1393	3.6	7.4	141
1U-2	1	19:48:00	1,500	1,141	22.7	23.7	29.6	27.3	633	39.8	37.7	988	1.7	4.5	79
1U-2	2	19:49:00	1,500	1,141	22.7	23.6	29.7	27.9	636	39.9	37.8	992	1.7	4.5	79
1U-2	4	19:51:00	1,500	1,141	22.0	22.7	29.2	27.5	620	39.1	37.2	973	1.6	4.4	76
1U-3	1	19:53:30	750	577	17.1	17.4	24.1	22.7	498	30.0	29.3	756	0.9	3.2	52
1U-3	2	19:54:30	750	577	16.8	17.2	23.8	22.5	492	29.7	28.9	747	0.8	3.2	51
1U-3	4	19:56:30	750	577	16.5	16.8	23.7	22.4	486	29.3	28.6	738	0.8	3.1	49
1U-4	1	19:58:30	0	0	10.8	10.4	17.5	16.8	340	18.2	19.6	482	-0.1	1.6	20
1U-4	2	19:59:30	0	0	10.6	10.3	17.3	16.7	337	17.9	19.3	475	-0.1	1.1	13
1U-4	4	20:01:30	0	0	10.6	10.1	17.2	16.6	334	17.5	18.7	461	-0.2	1.4	16
2L-0	-	20:02:00	0	0	10.3	10.5	17.1	16.6	334	17.8	19.0	469	-0.2	1.4	16
2L-1	1	20:05:30	1,500	1,141	17.7	16.6	24.4	23.5	504	30.0	29.9	764	1.7	3.9	71
2L-1	2	20:06:30	1,500	1,141	17.7	16.6	24.4	23.6	504	30.0	30.2	768	1.8	3.9	72
2L-1	4	20:08:30	1,500	1,141	18.0	16.4	24.3	23.6	504	30.1	30.3	770	1.7	3.9	71
2L-2	1	20:12:00	3,000	2,268	26.3	23.5	32.4	31.9	699	45.2	43.3	1129	4.0	6.3	131
2L-2	2	20:13:00	3,000	2,268	26.3	24.6	32.4	32.0	706	45.4	43.5	1134	4.0	6.7	137
2L-2	4	20:15:00	3,000	2,268	26.4	23.5	32.4	32.2	701	45.9	44.0	1146	4.1	6.8	139
2L-3	1	20:19:00	4,500	3,395	35.4	34.4	41.4	40.6	929	62.2	58.0	1533	5.7	9.4	193
2L-3	2	20:20:00	4,500	3,395	35.6	34.7	41.5	40.8	934	62.8	58.4	1546	5.8	9.7	198
2L-3	4	20:22:00	4,500	3,395	35.7	34.7	41.5	41.0	937	63.5	58.8	1560	5.9	9.8	200
2L-3	8	20:26:00	4,500	3,395	35.9	34.8	41.7	41.2	941						

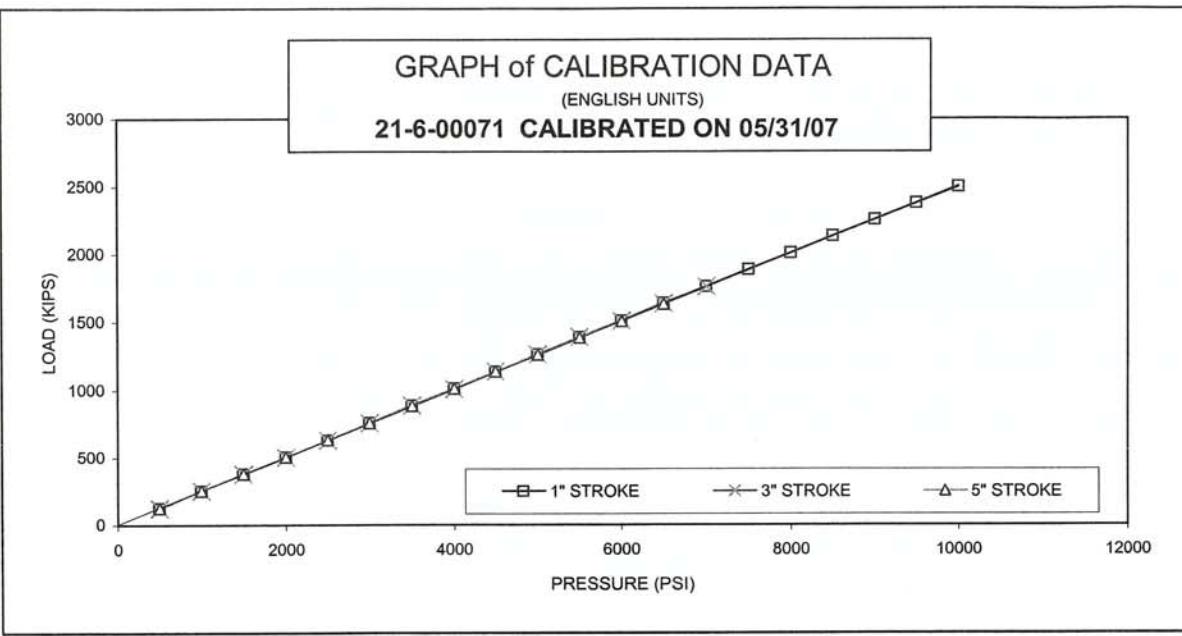
Test Shaft #1 - Highway 81 over Missouri River
Yankton, South Dakota (LT-9152)

APPENDIX B

O-CELL AND INSTRUMENTATION CALIBRATION SHEETS



DEEP FOUNDATION TESTING, EQUIPMENT & SERVICES • SPECIALIZING IN OSTERBERG CELL (O-cell®) TECHNOLOGY
O-cell® is a registered trademark.



STROKE: 1 INCH 3 INCH 5 INCH **21" O-CELL, SERIAL # 21-6-00071**

PRESSURE PSI	LOAD KIPS	LOAD KIPS	LOAD KIPS
0	0	0	0
500	125	125	125
1000	253	253	253
1500	379	378	379
2000	506	504	506
2500	632	632	633
3000	759	758	761
3500	887	886	885
4000	1011	1007	1011
4500	1136	1137	1137
5000	1262	1259	1259
5500	1388	1388	1385
6000	1510	1508	1508
6500	1639	1636	1634
7000	1761	1759	
7500	1887		
8000	2007		
8500	2133		
9000	2255		
9500	2377		
10000	2497		

LOAD CONVERSION FORMULA

$$\text{LOAD} = \text{PRESSURE} * 0.2505 + (5.50)$$

Regression Output:

Constant	5.4981 kips
X Coefficient	0.2505 kip / psi
R Square	1.0000
No. of Observations	47
Degrees of Freedom	45
Std Err of Y Est	3.95
Std Err of X Coeff	0.0002

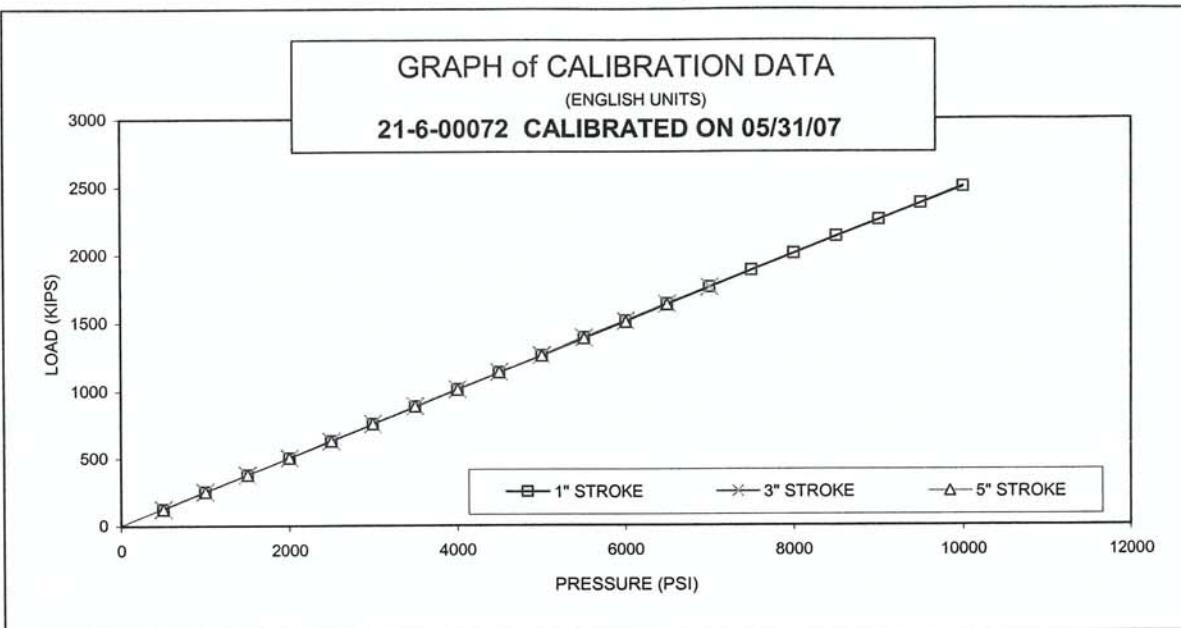
CALIBRATION STANDARDS:

All data presented are derived from 6" dia. certified hydraulic pressure gauges and electronic load transducer, manufactured and calibrated by the University of Illinois at Champaign, Illinois. All calibrations and certifications are traceable through the Laboratory Master Deadweight Gauges directly to the National Institute of Standards and Technology. No specific guidelines exist for calibration of load test jacks and equipment but procedures comply with similar guidelines for calibration of gages, ANSI specifications B40.1.

* AE & FC CUSTOMER: LOADTEST Inc
* AE & FC JOB NO: S02181
* CUSTOMER P.O. NO.: LT-9152

* CONTRACTOR.: JESEN CONSTRUCTION
* JOB LOCATION: YANKTON, SD
* DATED: 12/06/65

SERVICE ENGINEER: G. Raef DATE: 7 June 2005



STROKE: 1 INCH 3 INCH 5 INCH 21" O-CELL, SERIAL # 21-6-00072

PRESSURE PSI	LOAD KIPS	LOAD KIPS	LOAD KIPS
0	0	0	0
500	124	125	124
1000	251	251	251
1500	378	377	377
2000	503	504	504
2500	632	630	629
3000	757	757	756
3500	884	884	884
4000	1009	1008	1007
4500	1136	1135	1133
5000	1258	1258	1256
5500	1389	1386	1384
6000	1510	1508	1503
6500	1636	1634	1629
7000	1759	1757	
7500	1882		
8000	2007		
8500	2131		
9000	2253		
9500	2374		
10000	2495		

LOAD CONVERSION FORMULA

LOAD = PRESSURE * 0.2504 + (4.00)
{KIPS} {PSI}

Regression Output:

Constant	3.9963 kips
X Coefficient	0.2504 kip / psi
R Square	1.0000
No. of Observations	47
Degrees of Freedom	45
Std Err of Y Est	3.95
Std Err of X Coeff	0.0002

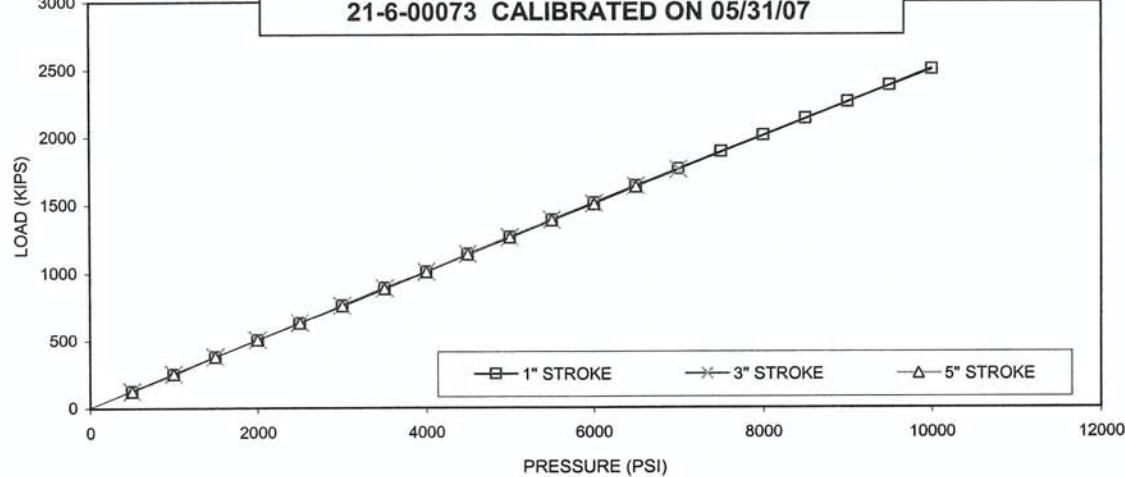
CALIBRATION STANDARDS:

All data presented are derived from 6" dia. certified hydraulic pressure gauges and electronic load transducer, manufactured and calibrated by the University of Illinois at Champaign, Illinois. All calibrations and certifications are traceable through the Laboratory Master Deadweight Gauges directly to the National Institute of Standards and Technology. No specific guidelines exist for calibration of load test jacks and equipment but procedures comply with similar guidelines for calibration of gages. ANSI specifications B40.1.

* AE & FC CUSTOMER: LOADTEST Inc
* AE & FC JOB NO: SO2181
* CUSTOMER P.O. NO : LT-9152

* CONTRACTOR.: JENSEN CONTRUCTION
* JOB LOCATION: YANKTON, SD
* DATED: 06/06/07

GRAPH of CALIBRATION DATA
 (ENGLISH UNITS)
21-6-00073 CALIBRATED ON 05/31/07



STROKE: 1 INCH 3 INCH 5 INCH **21" O-CELL, SERIAL # 21-6-00073**

PRESSURE PSI	LOAD KIPS	LOAD KIPS	LOAD KIPS
0	0	0	0
500	125	125	124
1000	249	251	251
1500	378	377	377
2000	504	503	504
2500	630	632	630
3000	759	758	757
3500	887	885	884
4000	1007	1007	1006
4500	1138	1136	1133
5000	1260	1259	1257
5500	1386	1386	1384
6000	1511	1506	1503
6500	1637	1635	1631
7000	1761	1757	
7500	1887		
8000	2007		
8500	2132		
9000	2254		
9500	2375		
10000	2495		

LOAD CONVERSION FORMULA

$$\text{LOAD} = \text{PRESSURE} * 0.2505 + (4.04)$$

{KIPS} {PSI}

Regression Output:

Constant	4.0432 kips
X Coefficient	0.2505 kip / psi
R Square	1.0000
No. of Observations	47
Degrees of Freedom	45
Std Err of Y Est	4.04
Std Err of X Coeff	0.0002

CALIBRATION STANDARDS:

All data presented are derived from 6" dia. certified hydraulic pressure gauges and electronic load transducer, manufactured and calibrated by the University of Illinois at Champaign, Illinois. All calibrations and certifications are traceable through the Laboratory Master Deadweight Gauges directly to the National Institute of Standards and Technology. No specific guidelines exist for calibration of load test jacks and equipment but procedures comply with similar guidelines for calibration of gages, ANSI specifications B40.1.

* AE & FC CUSTOMER: LOADTEST Inc
 * AE & FC JOB NO: S02181
 * CUSTOMER P.O. NO.: LT-9152

* CONTRACTOR.: JENSEN CONSTRUCTION
 * JOB LOCATION: YANKTON, SD
 * DATED: 06/06/07

SERVICE ENGINEER: G. Abel DATE: 7 June 2007



Certificate of Calibration

Certificate No.:

9748

Page 1 of 2

Date of Calibration: 7/5/2006

Customer

: FLOW TECHNOLOGY

P.O. BOX 8889
JACKSONVILLE, FL 32239-8889

Pressure and Temperature
Measurement

WIKA Instrument Corporation
1000 Wiegand Boulevard
Lawrenceville, Georgia 30043

Tel 770-513-8200
Fax 770-338-5118
www.wika.com
info@wika.com

Order No. : 288276

Specification of the device under test

Object : Dial Gauge
Manufacturer : WIKA
Model : 232.50 6
Serial No. : -
Tag : -
Pressure range : 0 ... 15000 psi
Accuracy : 1 % (of span)
Scale division / Resolution : 200 psi
Method of measurement : Gauge pressure
Output signal : -

Working Standard (WS)

Name : Electr. Gauge
Pressure range : 0 ... 1000 bar
Calibration-number : 31158 12-01-2005
Accuracy : 0.025 % (of span)
Identity : SS 204
Recal Interval : 1 year

Calibration parameters

Place of calibration : Cal-Lab (Lawrenceville)
Test temperature (in °F) : 71.5
Humidity (in %) : 52.0
Amb. pressure (in inHg) : 29.0
Pressure medium : water
Angle position : vertical
local gravity (in m/s²) : 9.79541

Used auxiliary instruments

Multimeter : -
Resistor : -

Comments :

Kuldeep Patel

Quality Assurance

: K. Patel

Nadim Dokara

Calibration technician : N. Dokara