

FUNDAMENTALS OF FLUID MECHANICS

Chapter 8 Pipe Flow

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MAIN TOPICS



- ❖ General Characteristics of Pipe Flow
- ❖ Fully Developed Laminar Flow
- ❖ Fully Developed Turbulent Flow
- ❖ Dimensional Analysis of Pipe Flow
- ❖ Pipe Flow Examples
- ❖ Pipe Flowrate Measurement

Introduction

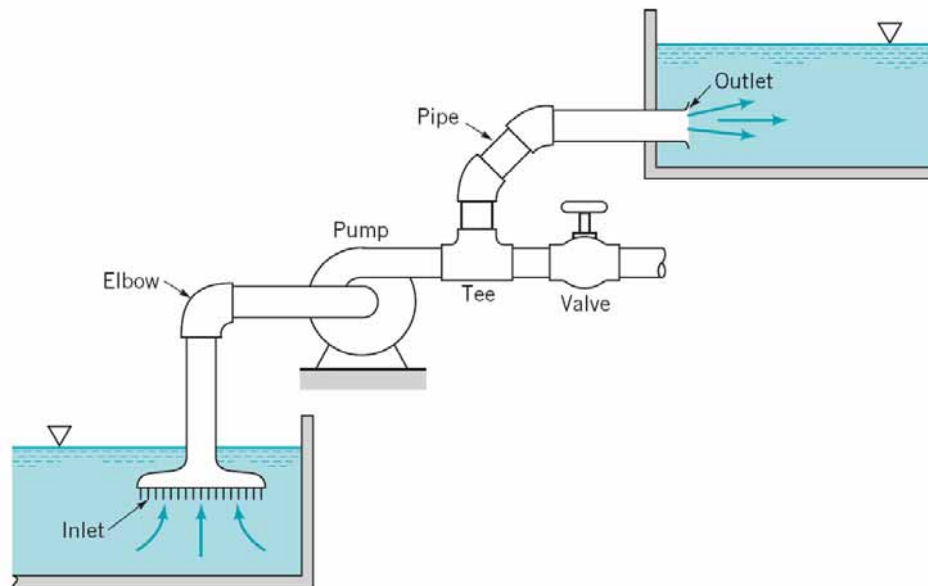
- ❖ Flows completely bounded by solid surfaces are called **INTERNAL FLOWS** which include flows through **pipes** (Round cross section), **ducts** (**NOT** Round cross section), nozzles, diffusers, sudden contractions and expansions, valves, and fittings.
- ❖ The basic principles involved are independent of the cross-sectional shape, although the details of the flow may be dependent on it.
- ❖ The flow regime (laminar or turbulent) of internal flows is primarily a function of the Reynolds number.
 - ⇒ Laminar flow: Can be solved analytically.
 - ⇒ Turbulent flow: Rely Heavily on semi-empirical theories and experimental data.

General Characteristics of Pipe Flow

The slide features several decorative circles. A large, light purple circle is positioned behind the text. Below this, there are two solid light purple circles on the left and one hollow light purple circle on the right.

Pipe System

- ❖ A pipe system include the pipes themselves (perhaps of more than one diameter), the various fittings, the flowrate control devices valves) , and the pumps or turbines.



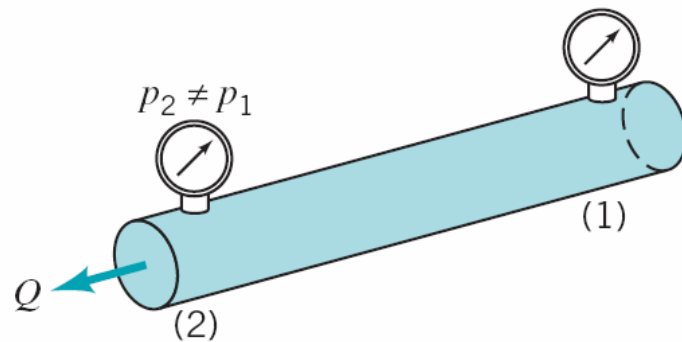
Pipe Flow vs. Open Channel Flow

❖ Pipe flow: Flows completely filling the pipe. (a)

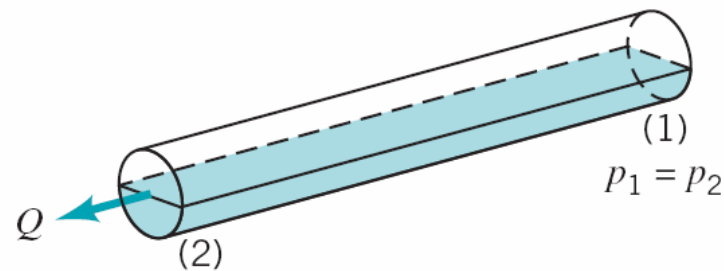
The pressure gradient along the pipe is main driving force.

❖ Open channel flow: Flows without completely filling the pipe. (b)

The gravity alone is the driving force.



(a)

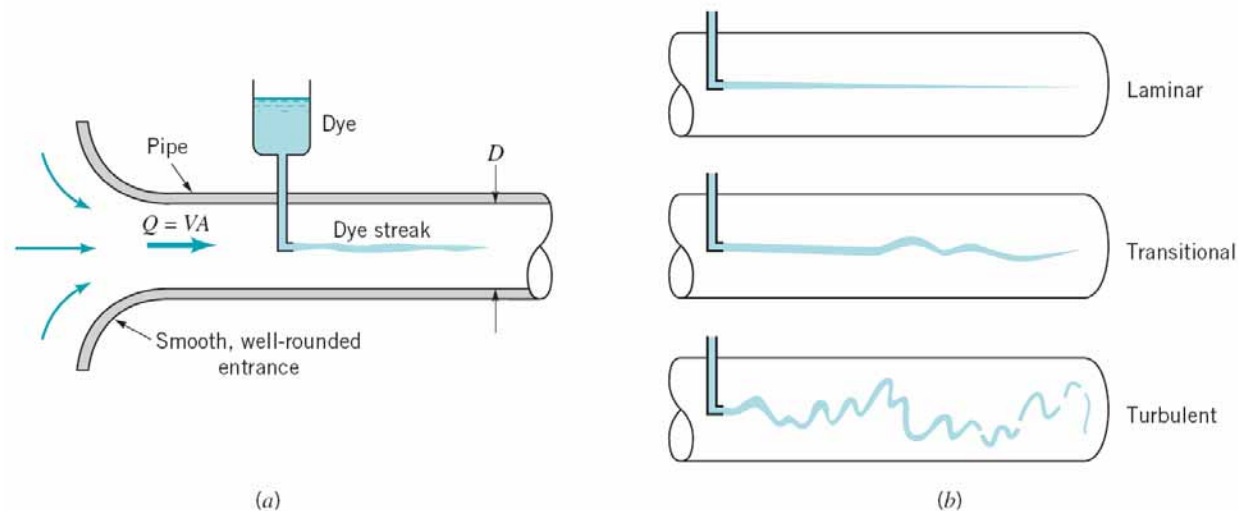


(b)

Laminar or Turbulent Flow ^{1/2}

❖ The flow of a fluid in a pipe may be **Laminar** ?
Turbulent ?

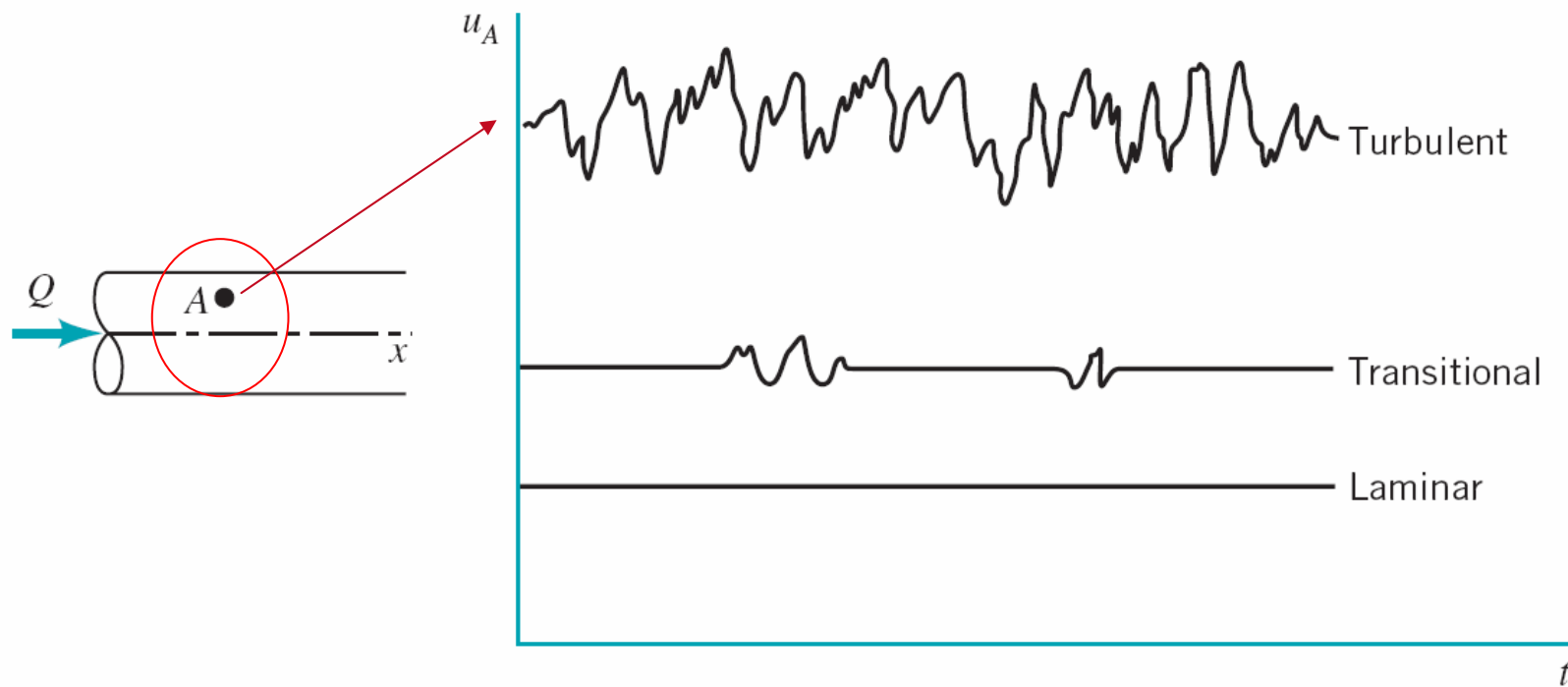
❖ **Osborne Reynolds**, a British scientist and mathematician, was the first to distinguish the difference between these classification of flow by using a **simple apparatus** as shown.



Laminar or Turbulent Flow 2/2

- ⇒ For “**small enough flowrate**” the dye streak will remain as a well-defined line as it flows along, with only slight blurring due to molecular diffusion of the dye into the surrounding water.
- ⇒ For a somewhat larger “**intermediate flowrate**” the dye fluctuates in time and space, and intermittent bursts of irregular behavior appear along the streak.
- ⇒ For “**large enough flowrate**” the dye streak almost immediately become blurred and spreads across the entire pipe in a random fashion.

Time Dependence of Fluid Velocity at a Point



Indication of Laminar or Turbulent Flow

- ❖ The term **flowrate** should be replaced by Reynolds number, $R_e = \rho V D / \mu$, where V is the average velocity in the pipe.
- ❖ It is **not only the fluid velocity** that determines the character of the flow – its density, viscosity, and the pipe size are of equal importance.
- ❖ For general engineering purpose, the flow in a round pipe
 - ⇒ **Laminar** $R_e < 2100$
 - ⇒ **Transitional**
 - ⇒ **Turbulent** $R_e > 4000$


Example 8.1 Laminar or Turbulent Flow

- Water at a temperature of 50 °F flows through a pipe of diameter $D = 0.73$ in. (a) Determine the minimum time taken to fill a 10-oz glass (volume = 0.125 ft³) with water if the flow in the pipe is to be laminar. (b) Determine the maximum time taken to fill the glass if the flow is to be turbulent. Repeat the calculation if the water temperature is 140 °F.

Example 8.1 Solution

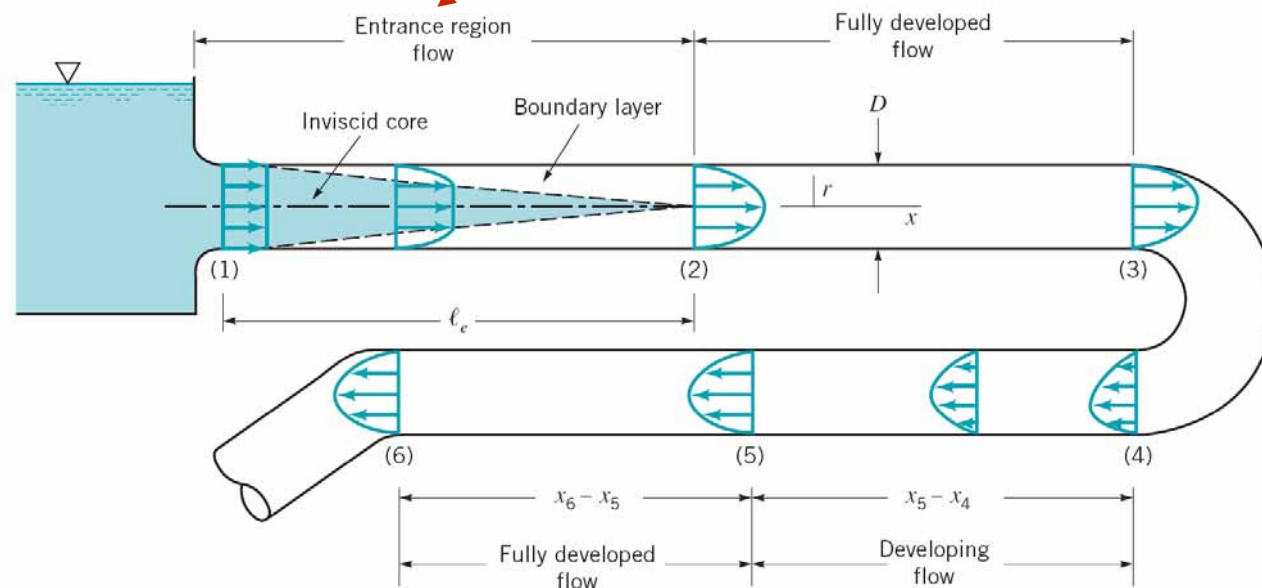
- ❖ If the flow in the pipe is to maintain laminar, the minimum time to fill the glass will occur if the Reynolds number is the maximum allowed for laminar flow, typically $Re=2100$. Thus

$$V = 2100 \mu / \rho D = 0.486 \text{ ft / s}$$

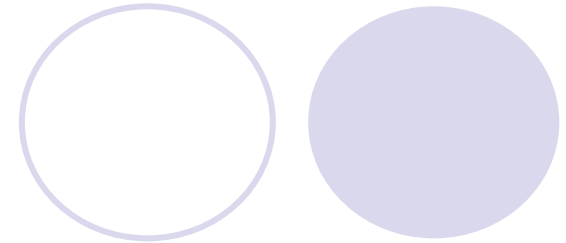

$$t = \frac{V}{Q} = \frac{V}{(\rho \pi / 4) D^2 V} = \dots = 8.85 \text{ s}$$

Entrance Region and Fully Developed Flow ^{1/5}

- ❖ Any fluid flowing in a pipe had to enter the pipe at some location.
- ❖ The region of flow near where the fluid enters the pipe is termed the entrance region.



Entrance Region and Fully Developed Flow ^{2/5}



- ❖ The fluid typically enters the pipe with a nearly uniform velocity profile at section (1).
- ❖ The region of flow near where the fluid enters the pipe is termed the entrance region.
- ❖ As the fluid moves through the pipe, viscous effects cause it to stick to the pipe wall (**the no slip boundary condition**).

Entrance Region and Fully Developed Flow ^{3/5}

- ❖ A **boundary layer** in which viscous effects are important is produced along the pipe wall such that the initial velocity profile changes with distance along the pipe, x , until the fluid reaches the end of the **entrance length, section (2)**, beyond which the velocity profile does not vary with x .
- ❖ The boundary layer has grown in thickness to completely fill the pipe.

Entrance Region and Fully Developed Flow ^{4/5}

- ❖ Viscous effects are of considerable importance within the boundary layer. Outside the boundary layer, the viscous effects are negligible.
- ❖ The shape of the velocity profile in the pipe depends on whether the flow is laminar or turbulent, as does the length of the entrance region, ℓ_ℓ .

For laminar flow

$$\frac{\ell_\ell}{D} = 0.06R_e$$

Dimensionless entrance length

For turbulent flow

$$\frac{\ell_\ell}{D} = 4.4R_e^{1/6}$$

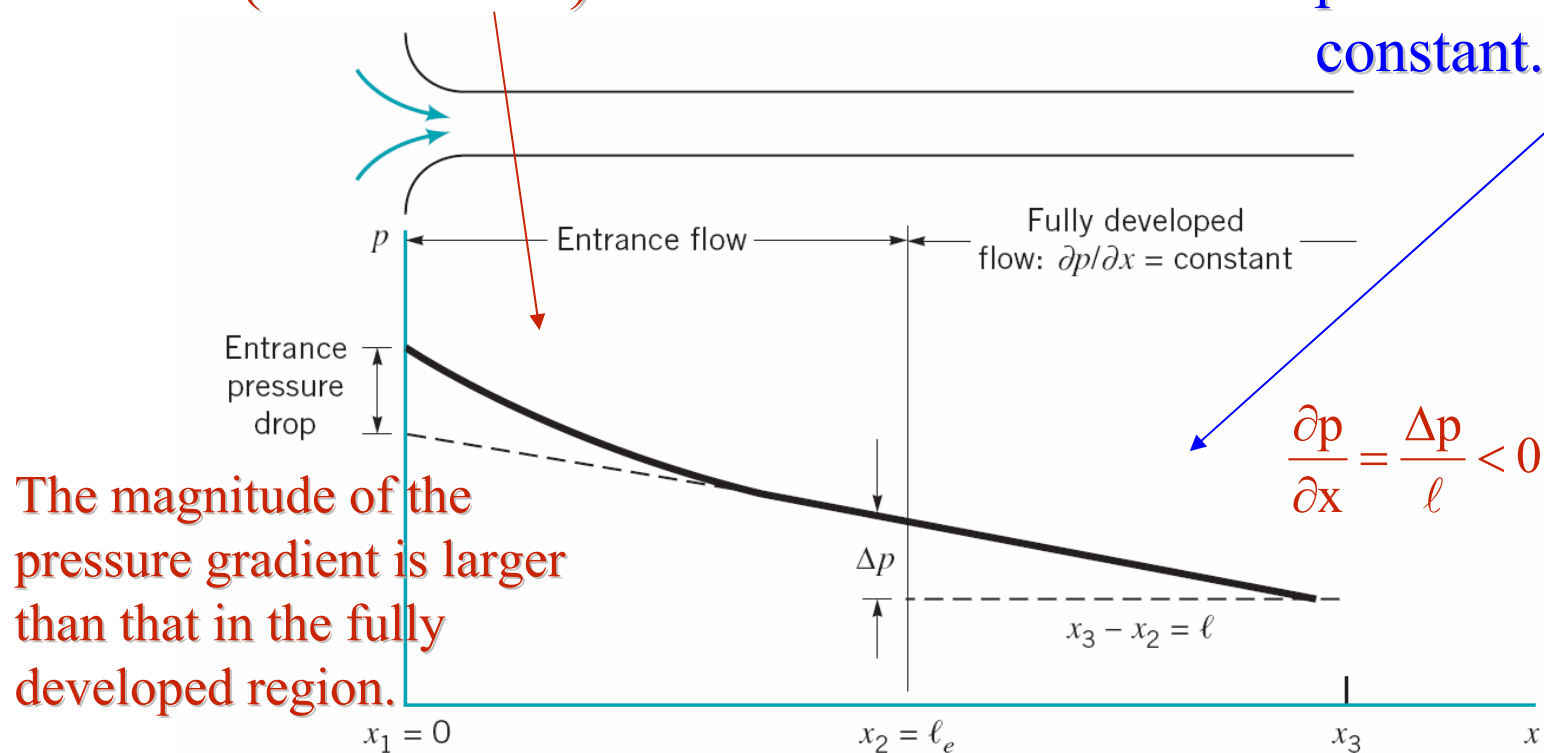
Entrance Region and Fully Developed Flow 5/5

- ❖ Once the fluid reaches the end of the entrance region, section (2), the flow is simpler to describe because *the velocity is a function of only the distance from the pipe centerline, r , and independent of x .*
- ❖ The flow between (2) and (3) is termed **fully developed**.

Pressure Distribution along Pipe

In the entrance region of a pipe, the fluid accelerates or decelerates as it flows. There is a balance between pressure, viscous, and inertia (acceleration) force.

The magnitude of the pressure gradient is constant.



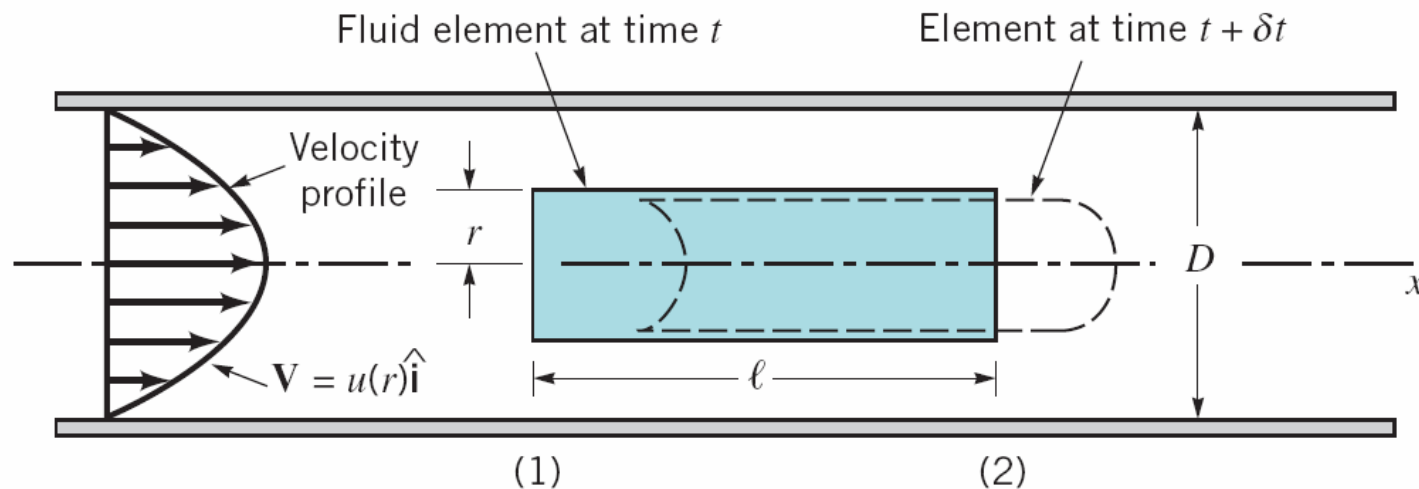
Fully Developed Laminar Flow

There are numerous ways to derive important results pertaining to fully developed laminar flow:

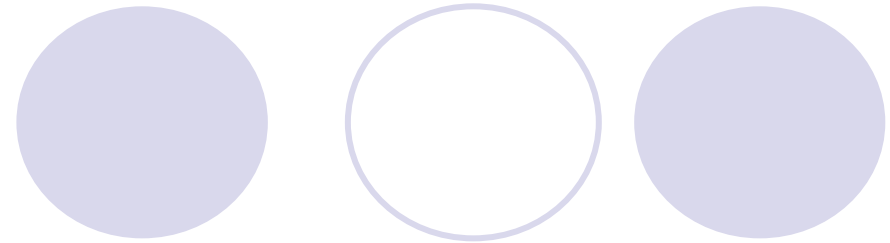
- ⇒ From $F=ma$ applied directly to a fluid element.
- ⇒ From the Navier-Stokes equations of motion
- ⇒ From dimensional analysis methods

From $F=ma$ ^{1/8}

- ❖ Considering a fully developed axisymmetric laminar flow in a long, straight, constant diameter section of a pipe.
- ❖ **The Fluid element** is a circular cylinder of fluid of length ℓ and radius r centered on the axis of a horizontal pipe of diameter D .



From $F=ma$ ^{2/8}



- ❖ Because the velocity is not uniform across the pipe, the initially flat end of the cylinder of fluid **at time t become distorted at time $t+\delta t$** when the fluid element has moved to its new location along the pipe.
- ❖ If the flow is fully developed and steady, the distortion on each end of the fluid element is the same, and no part of the fluid experiences any acceleration as it flows.

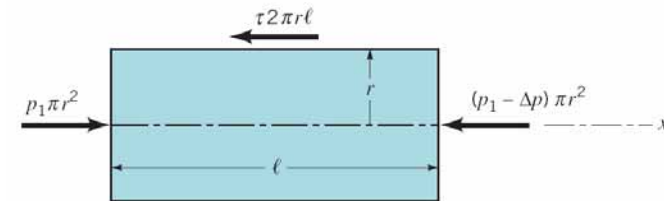
Steady $\frac{\partial \vec{V}}{\partial t} = 0$

Fully developed $\vec{V} \cdot \nabla \vec{V} = u \frac{\partial u}{\partial x} \vec{i} = 0$

From $F=ma$ ^{3/8}

Apply the Newton's second Law to the cylinder of fluid

$$F_x = ma_x$$



The force balance

$$p_1 \pi r^2 - (p_1 - \Delta p) \pi r^2 - \tau \ell (2\pi r) = 0 \Rightarrow \frac{\Delta p}{\ell} = \frac{2\tau}{r}$$

⇒ Basic balance in forces needed to drive each fluid particle along the pipe with constant velocity

Not function of r

$$\frac{\Delta p}{\ell} = \frac{2\tau}{r}$$

Not function of r

Independent of r

$$\tau \Rightarrow \tau = Cr$$

$$\text{B.C. } r=0 \quad \tau=0$$

$$r=D/2 \quad \tau = \tau_w$$



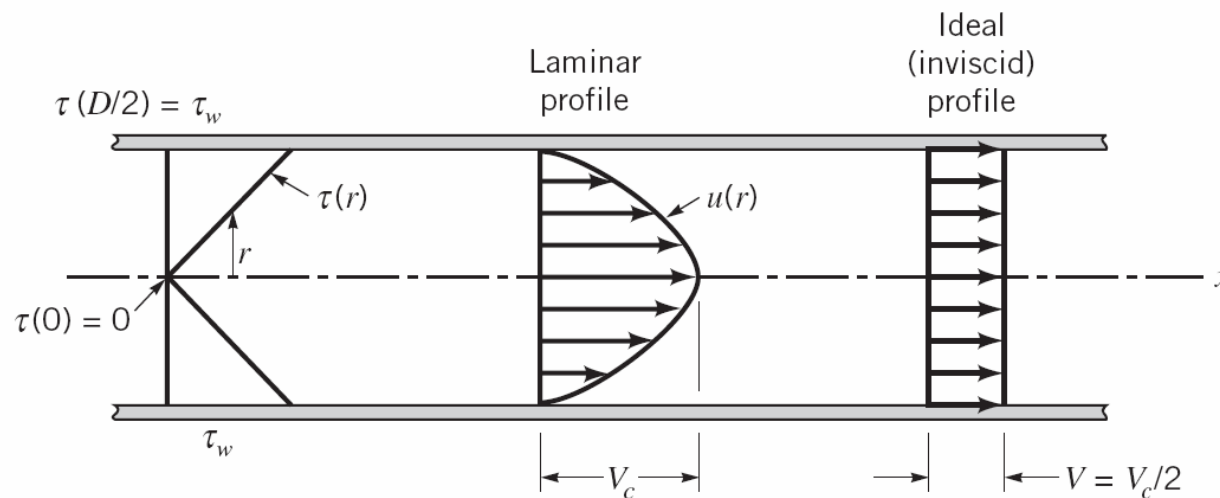
$$\tau = \frac{2\tau_w r}{D}$$

From $F=ma$ ^{4/8}

The pressure drop and wall shear stress are related by

$$\tau = \frac{2\tau_w r}{D} \oplus \frac{\Delta p}{\ell} = \frac{2\tau}{r} \longrightarrow \Delta p = \frac{4\ell\tau_w}{D}$$

Valid for both laminar and turbulent flow.



Laminar

$$\tau = -\mu \frac{du}{dr}$$

From $F=ma$ ^{5/8}

Since $\tau = -\mu \frac{du}{dr}$



$$\frac{du}{dr} = -\left(\frac{\Delta p}{2\mu\ell}\right)r$$

Laminar

$$\int du = -\frac{\Delta p}{2\mu\ell} \int r dr \Rightarrow u = -\left(\frac{\Delta p}{4\mu\ell}\right)r^2 + C_1$$

With the boundary conditions: $u=0$ at $r=D/2$

$$C_1 = -\frac{\Delta p D^2}{16\mu\ell}$$

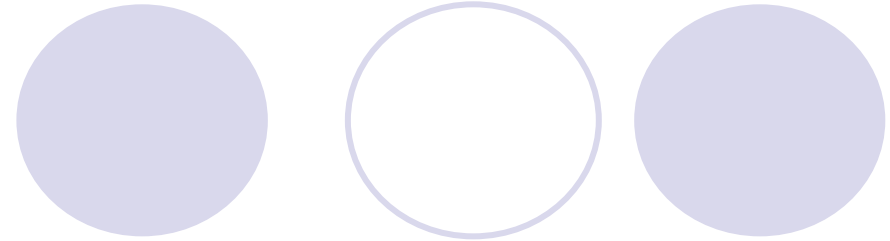
Velocity distribution

$$\Delta p = \frac{4\ell\tau_w}{D}$$

$$u(r) = \frac{\Delta p D^2}{16\mu\ell} \left[1 - \left(\frac{2r}{D} \right)^2 \right] = V_c \left[1 - \left(\frac{2r}{D} \right)^2 \right]$$

$$u(r) = \frac{\tau_w D}{4\mu} \left(1 - \left(\frac{r}{R} \right)^2 \right)$$

From $F=ma$ ^{6/8}



❖ The shear stress distribution

$$\tau = \mu \frac{du}{dr} = \frac{r\Delta p}{2\ell}$$

❖ Volume flowrate

$$Q = \int_A u \cdot d\vec{A} = \int_0^R u(r) 2\pi r dr = \dots = \frac{\pi R^4 V_c}{2}$$

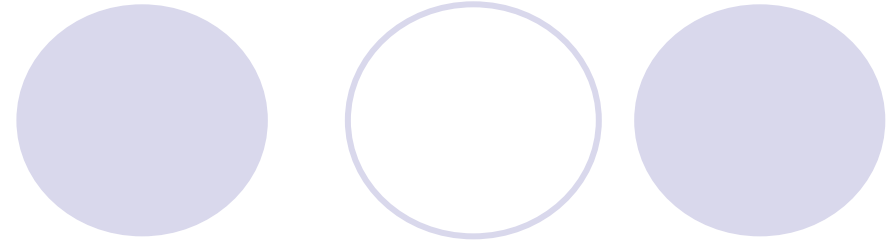
$$Q = \frac{\pi D^4 \Delta p}{128 \mu \ell}$$



Poiseuille's Law

Valid for Laminar flow only

From $F=ma$ ^{7/8}



❖ Average velocity

$$V_{\text{average}} = \frac{Q}{A} = \frac{Q}{\pi R^2} = \frac{\Delta p D^2}{32 \mu \ell}$$

❖ Point of maximum velocity

$$\frac{du}{dr} = 0 \quad \text{at } r=0$$

$$u = u_{\text{max}} = U = -\frac{R^2 \Delta p}{4 \mu \ell} = 2 V_{\text{average}}$$

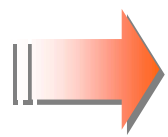
From $F=ma$ ^{8/8}

❖ Making adjustment to account for nonhorizontal pipes

$$\Delta p \rightarrow \Delta p - \gamma \ell \sin \theta$$

$\theta > 0$ if the flow is uphill

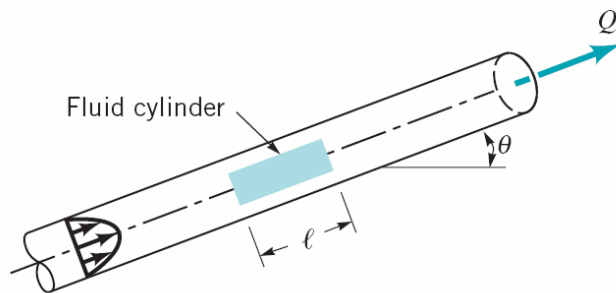
$\theta < 0$ if the flow is downhill



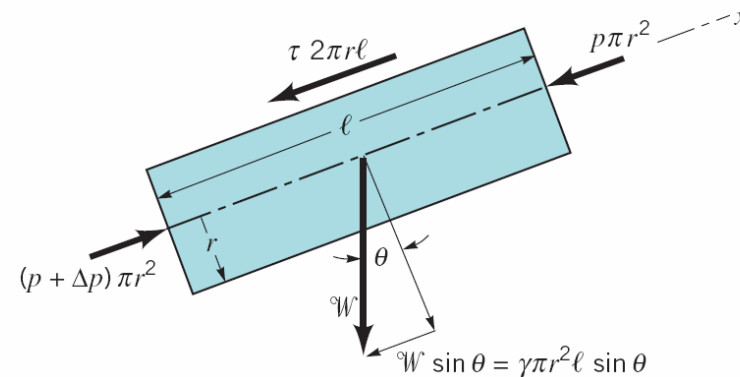
$$\frac{\Delta p - \gamma \ell \sin \theta}{\ell} = \frac{2\tau}{r}$$

$$Q = \frac{\pi(\Delta p - \gamma \ell \sin \theta)D^4}{128\mu\ell}$$

$$V_{\text{average}} = \frac{(\Delta p - \gamma \ell \sin \theta)D^2}{32\mu\ell}$$



(a)



(b)

Example 8.2 Laminar Pipe Flow

- An oil with a viscosity of $\mu = 0.40 \text{ N}\cdot\text{s}/\text{m}^2$ and density $\rho = 900 \text{ kg}/\text{m}^3$ flows in a pipe of diameter $D = 0.20 \text{ m}$. (a) What pressure drop, $p_1 - p_2$, is needed to produce a flowrate of $Q = 2.0 \times 10^{-5} \text{ m}^3/\text{s}$ if the pipe is horizontal with $x_1 = 0$ and $x_2 = 10 \text{ m}$? (b) How steep a hill, θ , must the pipe be on if the oil is to flow through the pipe at the same rate as in part (a), but with $p_1 = p_2$? (c) For the conditions of part (b), if $p_1 = 200 \text{ kPa}$, what is the pressure at section, $x_3 = 5 \text{ m}$, where x is measured along the pipe?

Example 8.2 Solution^{1/2}

$$R_e = \rho V D / \mu = 2.87 < 2100$$

$$V = \frac{Q}{A} = 0.0637 \text{ m/s}$$

The flow is laminar flow

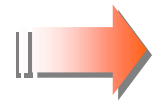
$$\Rightarrow \Delta p = p_1 - p_2 = \frac{128 \mu \ell Q}{\pi D^4} = \dots = 20.4 \text{ kPa}$$

If the pipe is on the hill of angle with $p=0$

$$\sin \theta = -\frac{128 \mu \ell Q}{\pi \rho g D^4} = \dots \Rightarrow \theta = -13.34^\circ$$

Example 8.2 Solution^{2/2}


With $p_1=p_2$ the length of the pipe, ℓ , does not appear in the flowrate equation

 $p=0$ for all ℓ

$$p_1 = p_2 = p_3 = 200 \text{ kPa}$$

From the Navier-Stokes Equations ^{1/3}

- ❖ General motion of an incompressible Newtonian fluid is governed by the continuity equation and the momentum equation


$$\begin{cases} \nabla \cdot \vec{V} = 0 \\ \frac{\partial \vec{V}}{\partial t} + \vec{V} \cdot \nabla \vec{V} = \frac{-\nabla p}{\rho} + \vec{g} + \nu \nabla^2 \vec{V} \end{cases}$$

Steady flow

$\vec{g} = -g\vec{k}$

For steady, fully developed flow in a pipe, the velocity contains only an axial component, which is a function of only the radial coordinate $\vec{V} = u(r)\vec{i}$



From the Navier-Stokes Equations ^{2/3}

⇒ Simplify the Navier-Stokes equation

$$\begin{cases} \nabla \cdot \vec{V} = 0 \\ \nabla p + \rho g \vec{k} = \mu \nabla^2 \vec{V} \end{cases}$$

The flow is governed by a balance of pressure, weight, and viscous forces in the flow direction.

From the Navier-Stokes Equations ^{3/3}

$$\vec{V} = u(r)\vec{i} \quad \Rightarrow \quad \frac{\partial p}{\partial x} + \rho g \sin \theta = \mu \frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial u}{\partial r} \right)$$

Function of, at most, only x

Function of, at most, only r

$$\frac{\partial p}{\partial x} = \text{const.} \rightarrow \frac{\partial p}{\partial x} = \frac{-\Delta p}{\ell}$$

Integrating **Velocity profile $u(r)=$**

B.C. (1) $r = R$, $u = 0$;

(2) $r = 0$, $u < \infty$ or $r = 0$ $\partial u / \partial r = 0$

From Dimensional Analysis ^{1/3}

- ❖ Assume that the pressure drop in the horizontal pipe, Δp , is a function of the average velocity of the fluid in the pipe, V , the length of the pipe, ℓ , the pipe diameter, D , and the viscosity of the fluid, μ .

$$\Delta p = F(V, \ell, D, \mu)$$

Dimensional analysis

$$\frac{D\Delta p}{\mu V} = \phi\left(\frac{\ell}{D}\right)$$

an unknown function of the length to diameter ratio of the pipe.

From Dimensional Analysis ^{2/3}

$$\frac{D\Delta p}{\mu V} = \frac{C\ell}{D} \quad \text{where } C \text{ is a constant.}$$

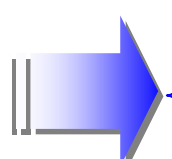
$$\Rightarrow \frac{\Delta p}{\ell} = \frac{C\mu V}{D^2} \quad Q = AV = \frac{(\pi/4C)\Delta p D^4}{\mu \ell}$$

The value of C must be determined by theory or experiment.
For a round pipe, $C=32$. For duct of other cross-sectional shapes, the value of C is different.

$$\text{For a round pipe} \quad \Delta p = \frac{32\mu \ell V}{D^2}$$

From Dimensional Analysis 3/3

For a round pipe $\frac{\Delta p}{\frac{1}{2}\rho V^2} = \frac{32\mu\ell V / D^2}{\frac{1}{2}\rho V^2} = 64 \frac{\mu}{\rho V D} \frac{\ell}{D} = \frac{64}{\text{Re}} \frac{\ell}{D}$



$$\left\{ \begin{aligned} \Delta p &= f \frac{\ell}{D} \frac{\rho V^2}{2} \\ f &= \frac{\Delta p \frac{D}{\ell}}{\frac{\rho V^2}{2}} \end{aligned} \right.$$

f is termed the friction factor, or sometimes the Darcy friction factor.

For laminar flow

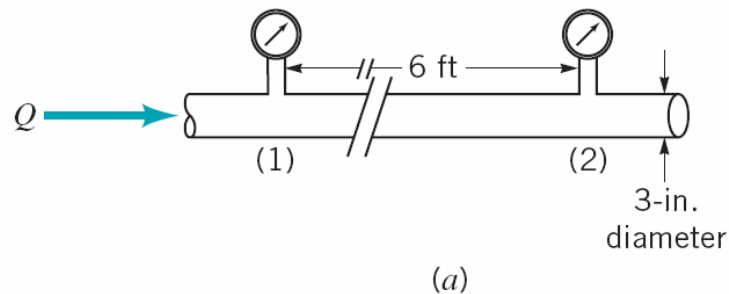
$$f = \frac{64}{\text{Re}} = \frac{8\tau_w}{\rho V^2}$$

$\Delta p = \frac{4\ell\tau_w}{D}$

Example 8.3 Laminar Pipe Flow Properties ^{1/2}

- The flowrate, Q , of corn syrup through the horizontal pipe shown in Figure E8.3 is to be monitored by measuring the pressure difference between sections (1) and (2). It is proposed that $Q = K \sqrt{p}$, where the calibration constant, K , is a function of temperature, T , because of the variation of the syrup's viscosity and density with temperature. These variations are given in Table E8.3. (a) Plot $K(T)$ versus T for $60^\circ\text{F} \leq T \leq 160^\circ\text{F}$. (b) Determine the wall shear stress and the pressure drop, $\Delta p = p_1 - p_2$, for $Q = 0.5 \text{ ft}^3/\text{s}$ and $T = 100^\circ\text{F}$. (c) For the conditions of part (b), determine the net pressure force, $(\pi D^2/4) \Delta p$, and the net shear force, $D \ell \tau_w$, on the fluid within the pipe between the sections (1) and (2).

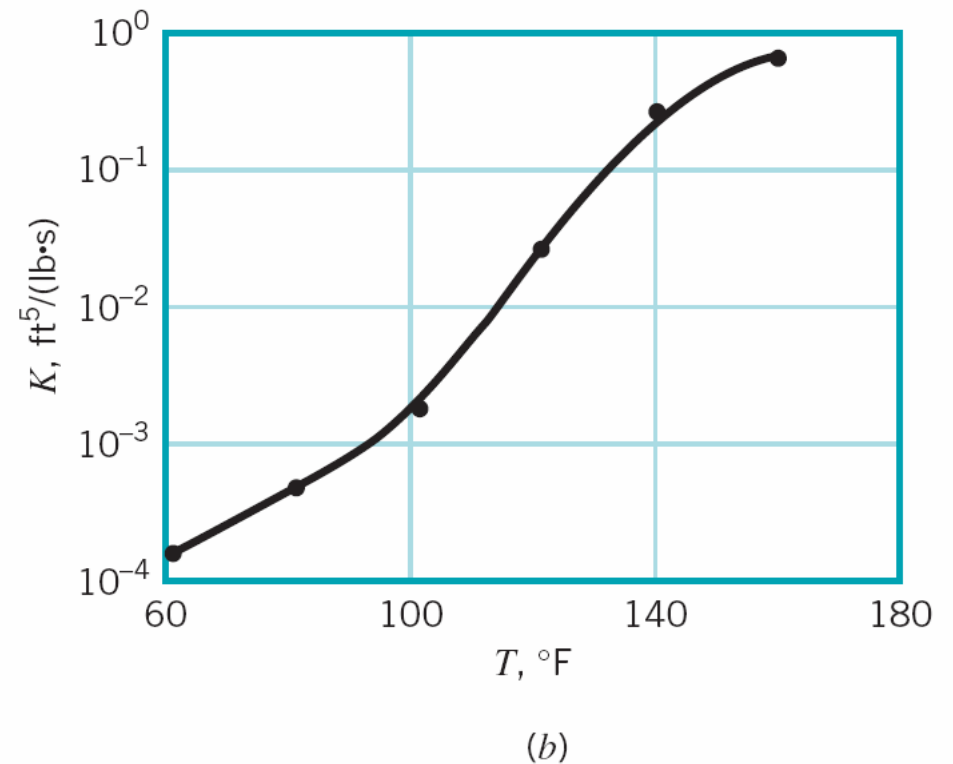
Example 8.3 Laminar Pipe Flow Properties ^{1/2}



■ FIGURE E8.3

■ TABLE E8.3

| T (°F) | ρ (slugs/ft ³) | μ (lb · s/ft ²) |
|----------|---------------------------------|---------------------------------|
| 60 | 2.07 | 4.0×10^{-2} |
| 80 | 2.06 | 1.9×10^{-2} |
| 100 | 2.05 | 3.8×10^{-3} |
| 120 | 2.04 | 4.4×10^{-4} |
| 140 | 2.03 | 9.2×10^{-5} |
| 160 | 2.02 | 2.3×10^{-5} |



Example 8.3 Solution^{1/2}

If the flow is laminar

$$Q = \frac{\pi \Delta p D^4}{128 \mu \ell} = K \Delta p \quad K = \frac{1.60 \times 10^{-5}}{\mu}$$

For $T=100^\circ\text{F}$, $\mu = 3.8 \times 10^{-3} \text{ lb}\cdot\text{s}/\text{ft}^2$, $Q=0.5 \text{ ft}^3/\text{s}$

$$\Delta p = \frac{128 \mu \ell Q}{\pi D^4} = \dots = 119 \text{ lb}/\text{ft}^2$$

$$V = \frac{Q}{A} = \dots = 10.2 \text{ ft}/\text{s} \quad R_e = \rho V D / \mu = \dots = 1380 < 2100$$

$$\Delta p = \frac{4 \ell \tau_w}{D} \Rightarrow \tau_w = \frac{\Delta p D}{4 \ell} = \dots = 1.24 \text{ lb}/\text{ft}^2$$

Example 8.3 Solution^{2/2}

The new pressure force and viscous force on the fluid within the pipe between sections (1) and (2) is

$$F_p = \frac{\pi D^2}{4} \Delta p = \dots = 5.84 \text{ lb}$$

$$F_v = 2\pi \frac{D}{2} \ell \tau_w = \dots = 5.84 \text{ lb}$$

The values of these two forces are the same. The net force is zero; there is no acceleration.

Fully Developed Turbulent Flow

- ❖ Turbulent pipe flow is actually more likely to occur than laminar flow in practical situations.
- ❖ Turbulent flow is a very complex process.
- ❖ Numerous persons have devoted considerable effort in an attempting to understand the variety of baffling aspects of turbulence. Although a considerable amount of knowledge about the topics has been developed, the field of turbulent flow still remains the least understood area of fluid mechanics.

Much remains to be learned about the nature of turbulent flow.

Transition from Laminar to Turbulent Flow in a Pipe ^{1/2}

❖ For any flow geometry, there is one (or more) dimensionless parameters such as with this parameter value below a particular value the flow is laminar, whereas with the parameter value larger than a certain value the flow is turbulent.

⇒ The important parameters involved and their critical values depend on the specific flow situation involved.

For flow in pipe : $2100 < Re < 4000$

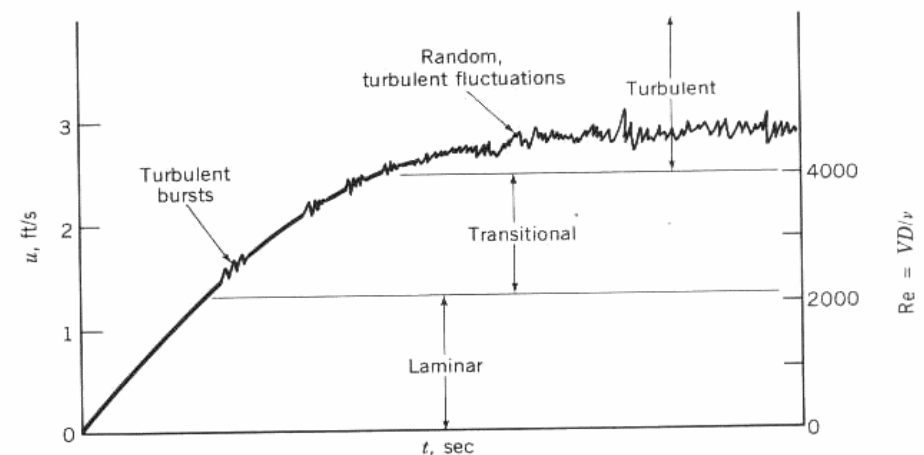
For flow along a plate $Re_x \sim 5000$

Consider a long section of pipe that is initially filled with a fluid at rest.



Transition from Laminar to Turbulent Flow in a Pipe ^{2/2}

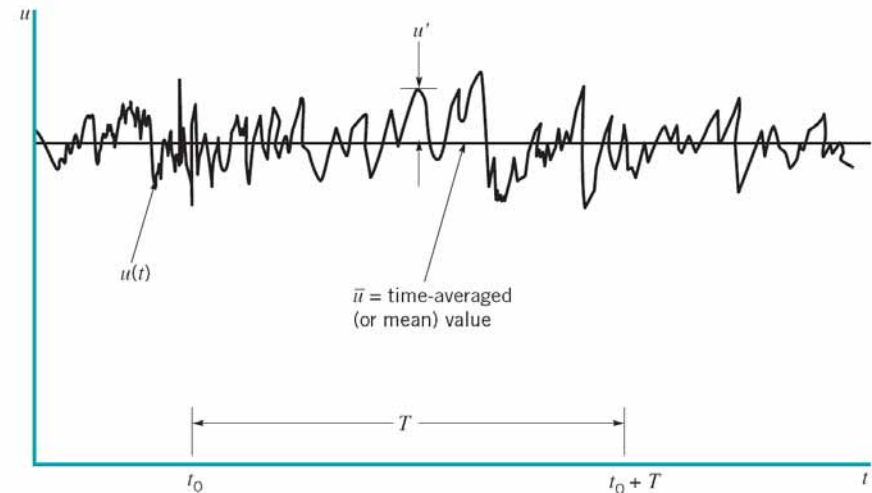
- ❖ As the valve is opened to start the flow, the flow velocity and, hence, the Reynolds number increase from zero (no flow) to their maximum steady flow values.
- ❖ For the initial time period the Reynolds number is small enough for laminar flow to occur.
- ❖ At some time the Reynolds number reaches 2100, and the flow begins its transition to turbulent conditions.
- ❖ Intermittent spots or burst appear.....



Description for Turbulent Flow ^{1/5}

- ❖ Turbulent flows involve randomly fluctuating parameters.
- ❖ The character of many of the important properties of the flow (pressure drop, heat transfer, etc.) depends strongly on the existence and nature of the turbulent fluctuations or randomness.

A typical trace of the axial component of velocity measured at a given location in the flow, $u=u(t)$.



The time-averaged, \bar{u} , and fluctuating, u' description of a parameter for tubular flow.

Description for Turbulent Flow ^{2/5}

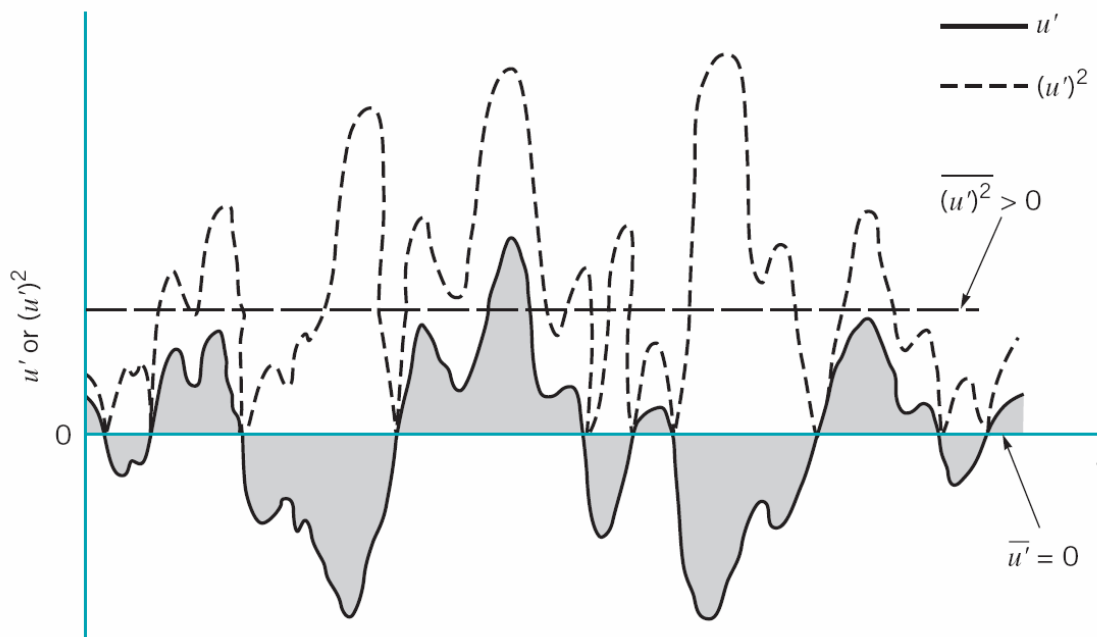
- ❖ Due to the macroscopic scale of the randomness in turbulent flow, mixing processes and heat transfer processes are considerably enhanced in turbulent flow compared to laminar flow.
- ❖ Such finite-sized random mixing is very effective in transporting energy and mass throughout the flow field.
- ❖ [Laminar flow can be thought of as very small but finite-sized fluid particles flowing smoothly in layer, one over another. The only randomness and mixing take place on the molecular scale and result in relatively small heat, mass, and momentum transfer rates.]

Description for Turbulent Flow ^{3/5}

- ❖ In some situations, turbulent flow characteristics are advantages. In other situations, laminar flow is desirable.
 - ▶ Turbulence: mixing of fluids.
 - ▶ Laminar: pressure drop in pipe, aerodynamic drag on airplane.

Description for Turbulent Flow ^{4/5}

- ❖ Turbulent flows are characterized by random, three-dimensional vorticity.
- ❖ Turbulent flows can be described in terms of their mean values on which are superimposed the fluctuations.



$$\bar{u} = \frac{1}{T} \int_{t_0}^{t_0+T} u(x, y, z, t) dt$$

$$u' = u - \bar{u}$$

$$u = \bar{u} + u'$$

Description for Turbulent Flow 5/5

- ❖ The time average of the fluctuations is zero.

$$\overline{u'} = \frac{1}{T} \int_{t_0}^{t_0+T} (u - \bar{u}) dt = \frac{1}{T} (T\bar{u} - T\bar{u}) = 0$$

- ❖ The square of a fluctuation quantity is positive.

$$\overline{(u')^2} = \frac{1}{T} \int_{t_0}^{t_0+T} (u')^2 dt > 0$$

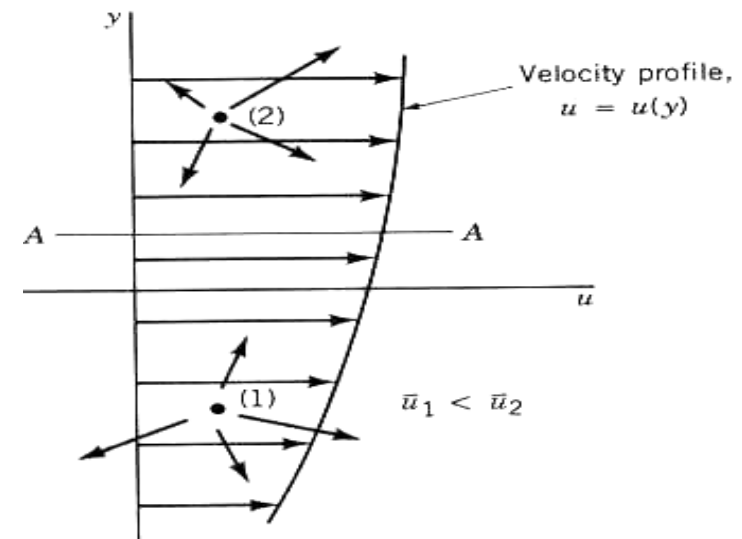
- ❖ Turbulence intensity or the level of the turbulence

$$\mathfrak{I} = \frac{\sqrt{\overline{(u')^2}}}{\bar{u}} = \frac{\left[\frac{1}{T} \int_{t_0}^{t_0+T} (u')^2 dt \right]^{1/2}}{\bar{u}}$$

The larger the turbulence intensity, the larger the fluctuations of the velocity. Well-designed wind tunnels have typical value of $\mathfrak{I}=0.01$, although with extreme care, values as low as $\mathfrak{I}=0.0002$ have been obtained.

Shear Stress for Laminar Flow ^{1/2}

- ❖ Laminar flow is modeled as fluid particles that flow smoothly along in layers, gliding past the slightly slower or faster ones on either side.
- ❖ The fluid actually consists of numerous molecules darting about in an almost random fashion. The motion is not entirely random – a slight bias in one direction.
- ❖ As the molecules dart across a given plane (plane A-A, for example), the ones moving upward have come from an area of smaller average x component of velocity than the ones moving downward, which have come from an area of large velocity.



Shear Stress for Laminar Flow ^{2/2}

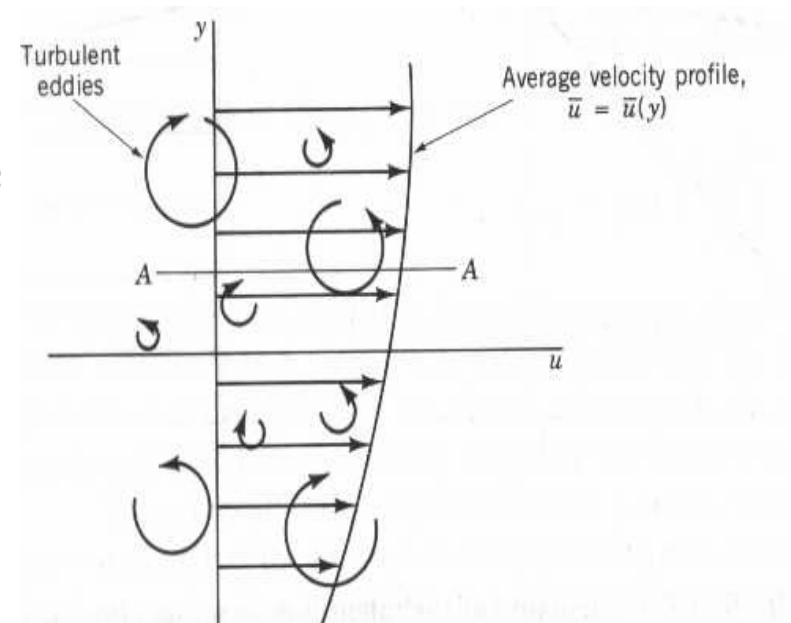
- ❖ The momentum flux in the x direction across plane A-A give rise to a drag of the lower fluid on the upper fluid and an equal but opposite effect of the upper fluid on the lower fluid. The sluggish molecules moving upward across plane A-A must accelerated by the fluid above this plane. The rate of change of momentum in this process produces a shear force. Similarly, the more energetic molecules moving down across plane A-A must be slowed down by the fluid below that plane.
- ❖ BY combining these effects, we obtain the well-known Newton viscosity law

$$\tau_{yx} = \mu \frac{du}{dy}$$

Shear stress is present only if there is a gradient in $u=u(y)$.

Shear Stress for Turbulent Flow ^{1/2}

- ❖ The turbulent flow is thought as a series of random, three-dimensional eddy type motions.
- ❖ These eddies range in size from very small diameter to fairly large diameter.
- ❖ This eddy structure greatly promotes mixing within the fluid.



Shear Stress for Turbulent Flow ^{2/2}

❖ The flow is represented by \bar{u} (time-mean velocity) plus u' and v' (time randomly fluctuating velocity components in the x and y direction).

❖ The shear stress on the plane A-A

$$\tau = \mu \frac{d\bar{u}}{dy} - \rho \overline{u'v'} = \tau_{\text{la min ar}} + \tau_{\text{turbulent}}$$

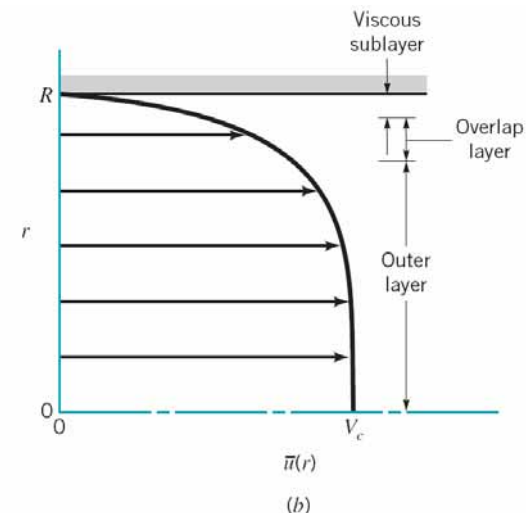
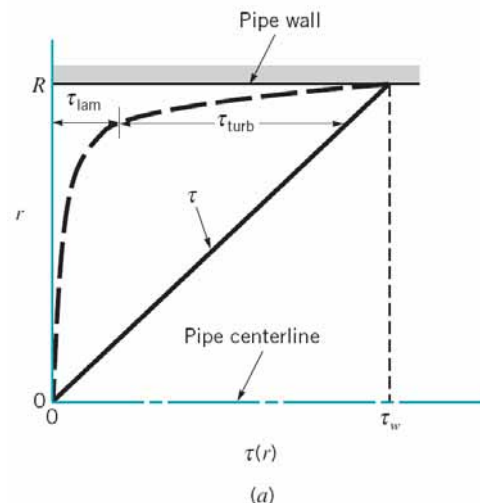
The shear stress is not merely proportional to the gradient of the time-averaged velocity, $\bar{u}(y)$.

$\overline{\rho u'v'}$ is called Reynolds stress introduced by Osborne Reynolds.

$\overline{\rho u'v'} \rightarrow 0$ As we approach wall, and is zero at the wall (the wall tends to suppress the fluctuations.)

Structure of Turbulent Flow in a Pipe ^{1/2}

- ❖ Near the wall (the viscous sublayer), the **laminar shear stress τ_{lam} is dominant.**
- ❖ Away from the wall (in the outer layer), **the turbulent shear stress τ_{turb} is dominant.**
- ❖ The transition between these two regions occurs in the overlap layer.



Structure of Turbulent Flow in a Pipe ^{2/2}

- ❖ The relative magnitude of τ_{lam} compared to τ_{turb} is a complex function dependent on the specific flow involved.
- ❖ Typically the value of τ_{turb} is 100 to 1000 times greater than τ_{lam} in the outer region.

Alternative Form of Shear Stress ^{1/2}

- ❖ τ_{turb} : requiring an accurate knowledge of the fluctuations u' and v' , or $\overline{\rho u' v'}$
- ❖ The shear stress for turbulent flow is given in terms of the eddy viscosity η .

$$\tau_{turb} = \eta \frac{d\bar{u}}{dy}$$

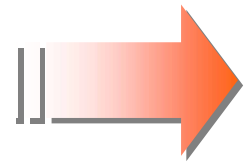
This extension of of laminar flow terminology was introduced by J. Boussubesq, a French scientist, in 1877.

$\eta?$ A semiempirical theory was proposed by **L. Prandtl** to determine the value of η

Alternative Form of Shear Stress 2/2

$$\eta = \rho \ell_m^2 \left| \frac{d\bar{u}}{dy} \right| \quad \Rightarrow \quad \tau_{\text{turb}} = \rho \ell_m^2 \left(\frac{d\bar{u}}{dy} \right)^2$$

mixing length, is not constant
throughout the flow field.



There is no general, all-encompassing, useful model that can accurately predict the shear stress throughout a general incompressible, viscous turbulent flow.

Turbulent Velocity Profile ^{1/5}

- ❖ Fully developed turbulent flow in a pipe can be broken into three region: the viscous sublayer, the overlap region, and the outer turbulent sublayer.
- ❖ Within the viscous sublayer the shear stress is dominant compared with the turbulent stress, and the random, eddying nature of the flow is essentially absent.
- ❖ In the outer turbulent layer the Reynolds stress is dominant, and there is considerable mixing and randomness to the flow.
- ❖ Within the viscous sublayer the fluid viscosity is an important parameter; the density is unimportant. In the outer layer the opposite is true.

Turbulent Velocity Profile ^{2/5}

- ❖ Considerable information concerning turbulent velocity profiles has been obtained through the use of **dimensional analysis, and semi-empirical theoretical efforts**.
- ❖ In the viscous sublayer the velocity profile can be written in dimensionless form as

$$u^+ = \frac{\bar{u}}{u^*} = \frac{yu^*}{\nu} = y^+ \quad \text{Law of the wall}$$

Where y is the distance measured from the wall $y=R-r$.

$u^* = (\tau_w / \rho)^{1/2}$ is called the friction velocity.

Is valid very near the smooth wall, for $0 \leq \frac{yu^*}{\nu} \leq 5$

Turbulent Velocity Profile ^{3/5}

- ❖ In the outer region the velocity should vary as the logarithm of y

$$\frac{\bar{u}}{u^*} = 2.5 \ln \left(\frac{yu^*}{y} \right) + 5.0 \quad \text{for} \quad \frac{yu^*}{\nu} > 30$$

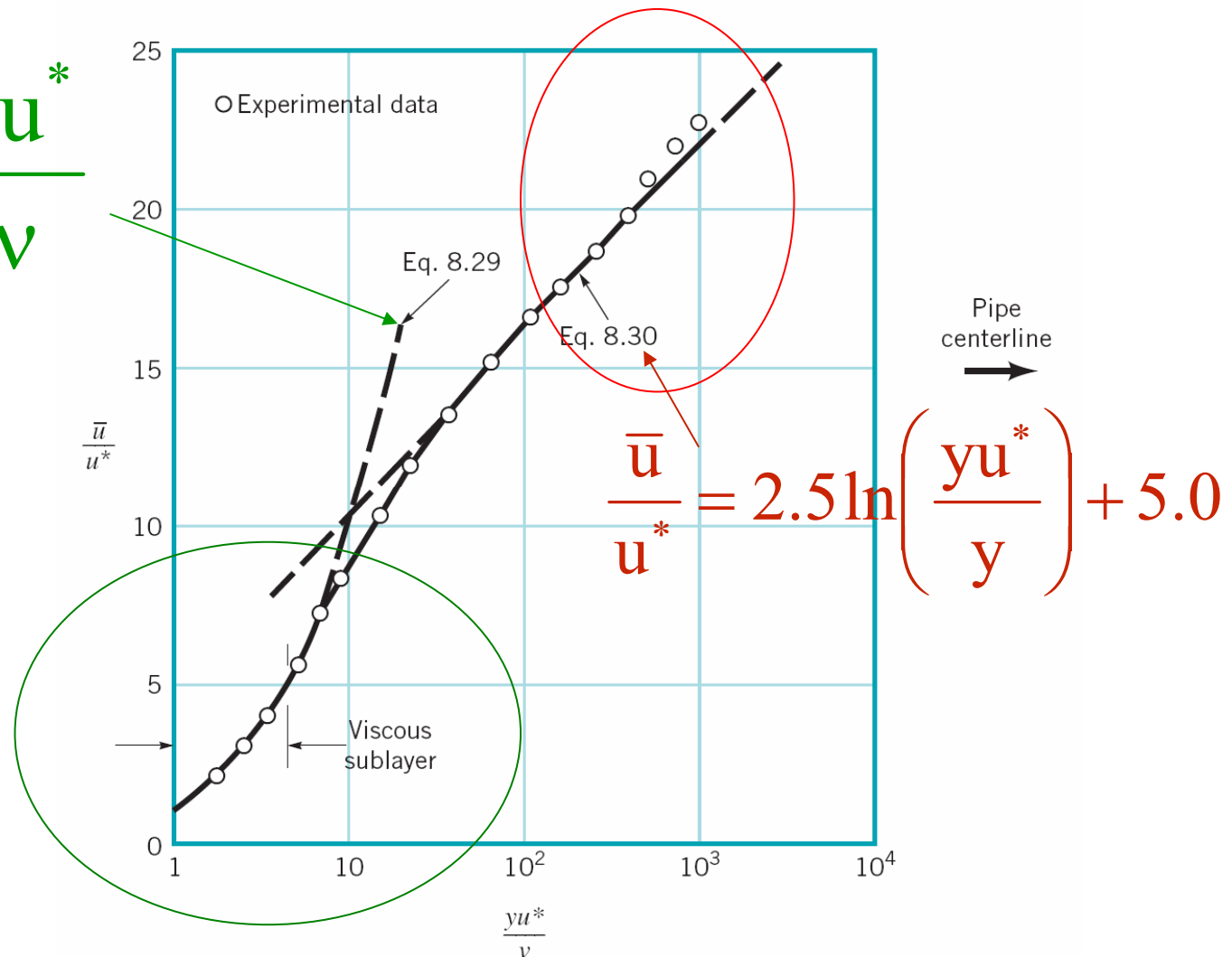
Determined experimentally

- ❖ In transition region or buffer layer

$$\frac{U - \bar{u}}{u^*} = 2.5 \ln \left(\frac{R}{y} \right) \quad \text{for} \quad 5 - 7 \leq \frac{yu^*}{\nu} \leq 30$$

Turbulent Velocity Profile 4/5

$$\frac{\bar{u}}{u^*} = \frac{yu^*}{\nu}$$



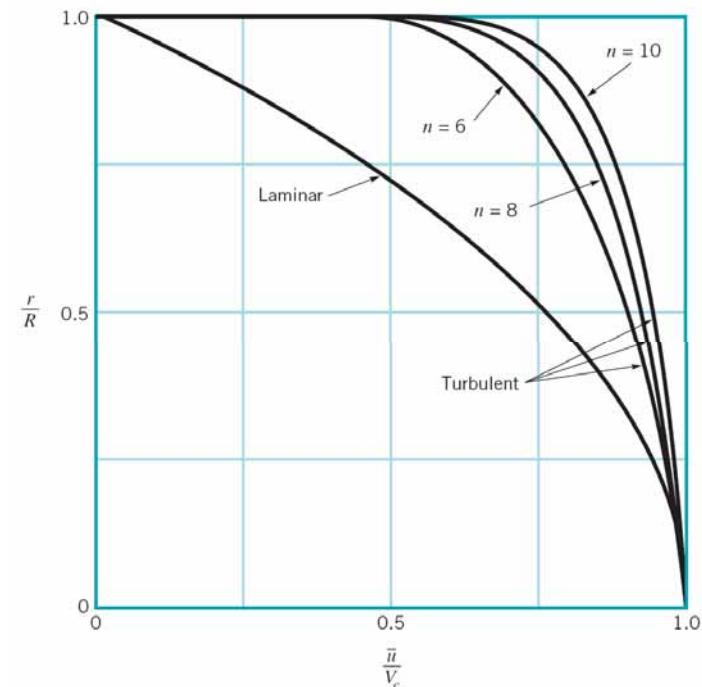
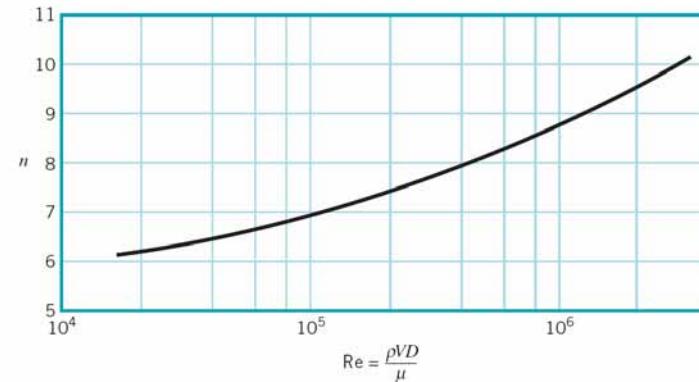
Turbulent Velocity Profile 5/5

- ❖ The velocity profile for turbulent flow through a smooth pipe may also be approximated by the empirical power-law equation

$$\frac{\bar{u}}{U} = \left(\frac{y}{R} \right)^{1/n} = \left(1 - \frac{r}{R} \right)^{1/n}$$

Where the exponent, n , varies with the Reynolds number.

- ❖ The power-law profile is not applicable close to the wall.



Example 8.4 Turbulent Pipe Flow Properties

- Water at 20 °C ($\rho = 998 \text{ kg/m}^3$ and $\mu = 1.004 \times 10^{-6} \text{ m}^2/\text{s}$) flows through a horizontal pipe of 0.1-m diameter with a flowrate of $Q = 4 \times 10^{-2} \text{ m}^3/\text{s}$ and a pressure gradient of 2.59 kPa/m. (a) Determine the approximate thickness of the viscous sublayer. (b) Determine the approximate centerline velocity, V_c . (c) Determine the ratio of the turbulent to laminar shear stress, $\tau_{\text{turb}}/\tau_{\text{lam}}$ at a point midway between the centerline and the pipe wall (i.e., at $r = 0.025 \text{ m}$)

Example 8.4 Solution^{1/3}

The thickness of viscous sublayer, δ_s , is approximately

$$\frac{\delta_s u^*}{\nu} = 5 \quad \Rightarrow \quad \delta_s = 5 \frac{\nu}{u^*} \quad \left\{ \begin{array}{l} \tau_w = \frac{D\Delta p}{4\ell} = \dots = 64.8 \text{ N/m}^2 \\ u^* = (\tau_w / \rho)^{1/2} = \dots = 0.255 \text{ m/s} \end{array} \right.$$

$$\Rightarrow \delta_s = 5 \frac{\nu}{u^*} = \dots = 1.97 \times 10^{-5} \text{ m} = 0.02 \text{ mm}$$

The centerline velocity can be obtained from the average velocity and the assumption of a power-law velocity profile

$$V = \frac{Q}{A} = \frac{0.04 \text{ m}^3/\text{s}}{\pi(0.1 \text{ m})^2/4} = 5.09 \text{ m/s} \quad R_e = VD/\nu = \dots = 5.07 \times 10^5$$

Example 8.4 Solution^{2/3}

$$\frac{\bar{u}}{U} = \left(\frac{y}{R} \right)^{1/n} = \left(1 - \frac{r}{R} \right)^{1/n}$$

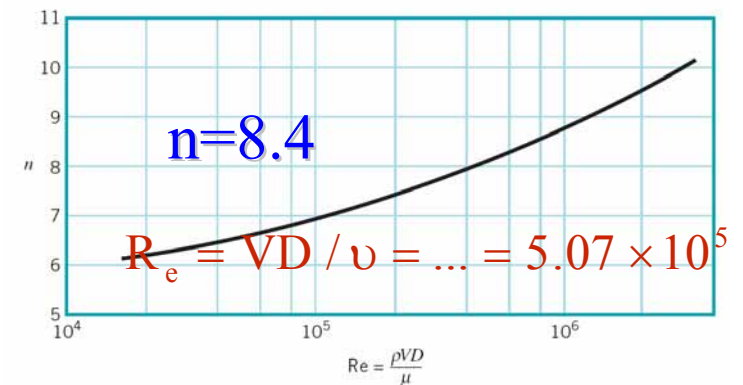
$$Q = AV = \int \bar{u} dA = \dots = 2\pi R^2 V_c \frac{n^2}{(n+1)(2n+1)} = \pi R^2 V$$

$$\frac{V}{V_c} = \frac{2n^2}{(n+1)(2n+1)} \quad V_c = \dots = 6.04 \text{ m/s}$$

$$\tau = \frac{2\tau_w r}{D} \quad \text{Valid for laminar or turbulent flow}$$

$$\tau = \frac{2\tau_w r}{D} = \frac{2(64.8 \text{ N/m}^2)(0.025 \text{ m})}{(0.1 \text{ m})}$$

$$= \tau_{\text{lam}} + \tau_{\text{turb}} = 32.4 \text{ N/m}^2$$



Example 8.4 Solution^{3/3}

$$\tau = \frac{2\tau_w r}{D} = \frac{2(64.8 \text{ N/m}^2)(0.025 \text{ m})}{(0.1 \text{ m})}$$

$$= \tau_{\text{lam}} + \tau_{\text{turb}} = 32.4 \text{ N/m}^2$$

$$\tau_{\text{lam}} = -\mu \frac{d\bar{u}}{dr} = -\mu \frac{V_c}{nR} \left(1 - \frac{r}{R}\right)^{(1-n)/n} = 0.0266 \text{ N/m}^2$$

$$\frac{\tau_{\text{turb}}}{\tau_{\text{lam}}} = \frac{\tau - \tau_{\text{lam}}}{\tau_{\text{lam}}} = \frac{32.4 - 0.0266}{0.0266} = 1220$$



Dimensional Analysis of Pipe Flow

Energy Considerations ^{1/8}

- ❖ Considering the steady flow through the piping system, including a reducing elbow. The basic equation for conservation of energy – the first law of thermodynamics

$$\dot{Q}_{\text{net in}} + \dot{W}_{\text{Shaft in}} + \int_{\text{CS}} \sigma_{\text{nn}} \vec{V} \cdot \vec{n} dA = \frac{\partial}{\partial t} \int_{\text{CV}} e \rho dV + \int_{\text{CS}} e \rho \vec{V} \cdot \vec{n} dA$$

$$\Rightarrow \dot{Q}_{\text{net in}} + \dot{W}_{\text{Shaft in}} = \frac{\partial}{\partial t} \int_{\text{CV}} e \rho dV + \int_{\text{CS}} e \rho \vec{V} \cdot \vec{n} dA - \int_{\text{CS}} \sigma_{\text{nn}} \vec{V} \cdot \vec{n} dA$$

Energy equation

$$\frac{\partial}{\partial t} \int_{\text{CV}} e \rho dV + \int_{\text{CS}} \left(\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right) \rho \vec{V} \cdot \vec{n} dA = \dot{Q}_{\text{net in}} + \dot{W}_{\text{Shaft in}}$$

$$e = u + \frac{V^2}{2} + gz$$

Energy Considerations 2/8

When the flow is steady $\frac{\partial}{\partial t} \int_{cv} e \rho dV = 0$

The integral of

$$\int_{cs} \left[\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right] \rho \vec{V} \cdot \vec{n} dA$$

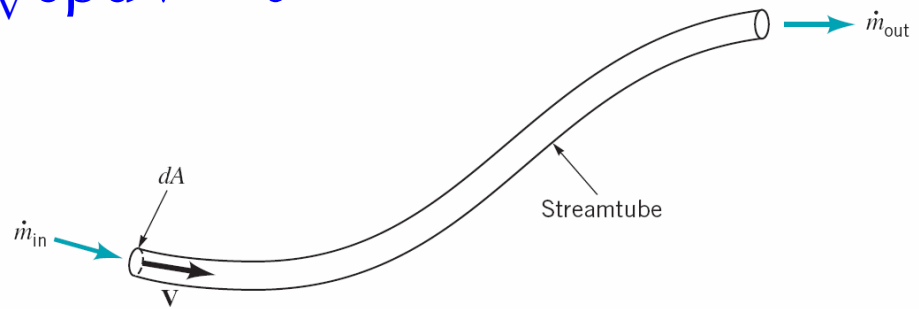
???

Uniformly distribution

$$\int_{cs} \left[\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right] \rho \vec{V} \cdot \vec{n} dA = \sum_{out} \left(\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right) \dot{m} - \sum_{in} \left(\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right) \dot{m}$$

**Only one stream
entering and leaving**

$$\begin{aligned} & \int_{cs} \left[\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right] \rho \vec{V} \cdot \vec{n} dA \\ &= \left(\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right)_{out} \dot{m}_{out} - \left(\hat{u} + \frac{p}{\rho} + \frac{V^2}{2} + gz \right)_{in} \dot{m}_{in} \end{aligned}$$



Energy Considerations ^{3/8}

If shaft work is involved....

$$\dot{m} \left[\hat{u}_{\text{out}} - \hat{u}_{\text{in}} + \left(\frac{p}{\rho} \right)_{\text{out}} - \left(\frac{p}{\rho} \right)_{\text{in}} + \frac{V_{\text{out}}^2 - V_{\text{in}}^2}{2} + g(z_{\text{out}} - z_{\text{in}}) \right]$$
$$= \dot{Q}_{\text{net in}} + \dot{W}_{\text{shaft net in}} \quad \longleftarrow \text{One-dimensional energy equation for steady-in-the-mean flow}$$

Enthalpy $\hat{h} = \hat{u} + \frac{p}{\rho}$ The energy equation is written in terms of enthalpy.

$$\dot{m} \left[\hat{h}_{\text{out}} - \hat{h}_{\text{in}} + \frac{V_{\text{out}}^2 - V_{\text{in}}^2}{2} + g(z_{\text{out}} - z_{\text{in}}) \right] = \dot{Q}_{\text{net/in}} + \dot{W}_{\text{shaft net/in}}$$

Energy Considerations 4/8

For steady, incompressible flow... One-dimensional energy equation

$$\dot{m} \left[\hat{u}_{\text{out}} - \hat{u}_{\text{in}} + \left(\frac{p_{\text{out}}}{\rho} \right) - \left(\frac{p_{\text{in}}}{\rho} \right) + \frac{V_{\text{out}}^2 - V_{\text{in}}^2}{2} + g(z_{\text{out}} - z_{\text{in}}) \right] = \dot{Q}_{\text{net in}}$$

$$\div \dot{m} \Rightarrow \frac{p_{\text{out}}}{\rho} + \frac{V_{\text{out}}^2}{2} + gz_{\text{out}} = \frac{p_{\text{in}}}{\rho} + \frac{V_{\text{in}}^2}{2} + gz_{\text{in}} - (\hat{u}_{\text{out}} - \hat{u}_{\text{in}} - q_{\text{net in}})$$

$$\text{where } q_{\text{net in}} = \dot{Q}_{\text{net in}} / \dot{m}$$

For steady, incompressible, **frictionless flow**...

$$p_{\text{out}} + \frac{\rho V_{\text{out}}^2}{2} + \gamma z_{\text{out}} = p_{\text{in}} + \frac{\rho V_{\text{in}}^2}{2} + \gamma z_{\text{in}} \quad \text{Bernoulli equation}$$

$$\Rightarrow \hat{u}_{\text{out}} - \hat{u}_{\text{in}} - q_{\text{net in}} = 0 \quad \text{Frictionless flow...}$$


Energy Considerations ^{5/8}

For steady, incompressible, **frictional flow...**

$$\hat{u}_{\text{out}} - \hat{u}_{\text{in}} - q_{\text{net in}} > 0 \quad \text{Frictional flow...}$$

Defining “useful or available energy”... $\frac{p}{\rho} + \frac{V^2}{2} + gz$

Defining “loss of useful or available energy”... $\hat{u}_{\text{out}} - \hat{u}_{\text{in}} - q_{\text{net in}} = \text{loss}$


$$\frac{p_{\text{out}}}{\rho} + \frac{V_{\text{out}}^2}{2} + gz_{\text{out}} = \frac{p_{\text{in}}}{\rho} + \frac{V_{\text{in}}^2}{2} + gz_{\text{in}} - \text{loss}$$

Energy Considerations 6/8

For steady, incompressible flow with friction and shaft work...

$$\dot{m} \left[\hat{u}_{\text{out}} - \hat{u}_{\text{in}} + \left(\frac{p_{\text{out}}}{\rho} \right) - \left(\frac{p_{\text{in}}}{\rho} \right) + \frac{V_{\text{out}}^2 - V_{\text{in}}^2}{2} + g(z_{\text{out}} - z_{\text{in}}) \right] = \dot{Q}_{\text{net in}} + \dot{W}_{\text{shaft net in}}$$

$$\div \dot{m} \Rightarrow \frac{p_{\text{out}}}{\rho} + \frac{V_{\text{out}}^2}{2} + gz_{\text{out}} = \frac{p_{\text{in}}}{\rho} + \frac{V_{\text{in}}^2}{2} + gz_{\text{in}} + w_{\text{shaft net in}} - (\hat{u}_{\text{out}} - \hat{u}_{\text{in}} - q_{\text{net in}})$$

$$\Rightarrow \frac{p_{\text{out}}}{\rho} + \frac{V_{\text{out}}^2}{2} + gz_{\text{out}} = \frac{p_{\text{in}}}{\rho} + \frac{V_{\text{in}}^2}{2} + gz_{\text{in}} + w_{\text{shaft net in}} - \text{loss}$$

$$\div g \Rightarrow \frac{p_{\text{out}}}{\gamma} + \frac{V_{\text{out}}^2}{2g} + z_{\text{out}} = \frac{p_{\text{in}}}{\gamma} + \frac{V_{\text{in}}^2}{2g} + z_{\text{in}} + h_s - h_L$$

$$\text{Shaft head } h_s = \frac{w_{\text{shaft net in}}}{g} \equiv \frac{\dot{W}_{\text{shaft net in}}}{\dot{m}g} = \frac{\dot{W}_{\text{shaft net in}}}{\gamma Q} \quad \text{Head loss } h_L = \frac{\text{loss}}{g}$$

Energy Considerations ^{7/8}

$$\frac{p_{\text{out}}}{\gamma} + \frac{V_{\text{out}}^2}{2g} + z_{\text{out}} = \frac{p_{\text{in}}}{\gamma} + \frac{V_{\text{in}}^2}{2g} + z_{\text{in}} + h_s - h_L$$

❖ For turbine $h_s = -h_T$ ($h_T > 0$) **h_T is turbine head**

❖ For pump $h_s = h_p$ **h_p is pump head**

❖ The actual head drop across the turbine

$$h_T = -(h_s + h_L)_T$$

❖ The actual head drop across the pump

$$h_p = (h_s - h_L)_p$$

Energy Considerations ^{8/8}

- ❖ Total head loss , h_L , is regarded as the sum of major losses, $h_{L \text{ major}}$, due to frictional effects in fully developed flow in constant area tubes, and minor losses, $h_{L \text{ minor}}$, resulting from entrance, fitting, area changes, and so on.

$$h_L = h_{L \text{ major}} + h_{L \text{ minor}}$$

Major Losses: Friction Factor

- ❖ The energy equation for steady and incompressible flow with zero shaft work

$$\left(\frac{p_1}{\rho g} + \frac{\alpha_1 V_1^2}{2g} + z_1 \right) - \left(\frac{p_2}{\rho g} + \frac{\alpha_2 V_2^2}{2g} + z_2 \right) = h_L$$

For fully developed flow through a constant area pipe

$$\frac{\alpha_1 \bar{V}_1^2}{2g} = \frac{\alpha_2 \bar{V}_2^2}{2g} \quad \gg \gg \gg \quad \frac{p_1 - p_2}{\rho g} = (z_2 - z_1) + h_L$$

For horizontal pipe, $z_2 = z_1$ $\gg \gg \gg \quad \frac{p_1 - p_2}{\rho g} = \frac{\Delta p}{\rho g} = h_L$

Major Losses: Laminar Flow


❖ In fully developed laminar flow in a horizontal pipe, the pressure drop

$$\Delta p = \frac{128 \mu \ell Q}{\pi D^4} = \frac{128 \mu \ell V (\pi D^2 / 4)}{\pi D^4} = 32 \frac{\ell}{D} \frac{\mu V}{D}$$

$$\frac{\Delta p}{\frac{1}{2} \rho V^2} = 64 \frac{\mu}{\rho V D} \frac{\ell}{D} = \frac{64}{Re} \frac{\ell}{D}$$

$$\Delta p = f \frac{\ell}{D} \frac{\rho V^2}{2} \gg h_L = 32 \frac{\ell}{D} \frac{\mu V}{\rho D} = \frac{\ell}{D} \frac{V^2}{2} \left(64 \frac{\mu}{\rho V D} \right) = \left(\frac{64}{Re} \right) \frac{\ell}{D} \frac{V^2}{2}$$

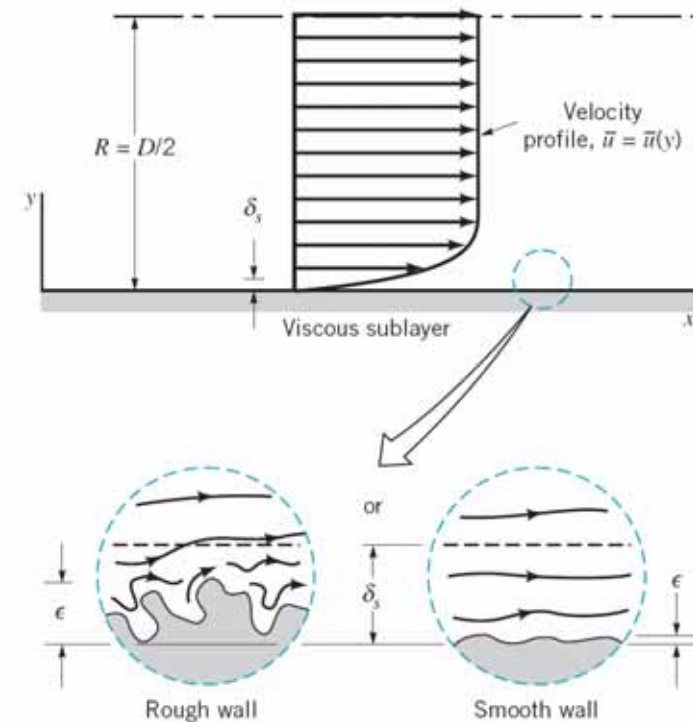
Friction Factor $f = \Delta p (D / \ell) / (\rho V^2 / 2)$


$$f_{laminar} = \frac{64}{Re}$$

Major Losses: Turbulent Flow ^{1/3}

- ❖ In turbulent flow we cannot evaluate the pressure drop analytically; we must resort to experimental results and use dimensional analysis to correlate the experimental data.
- ❖ In fully developed turbulent flow the pressure drop, Δp , caused by friction in a horizontal constant-area pipe is known to depend on pipe diameter, D , pipe length, ℓ , pipe roughness, ϵ , average flow velocity, V , fluid density ρ , and fluid viscosity, μ .

$$\Delta p = F(V, D, \ell, \epsilon, \mu, \rho)$$



Major Losses: Turbulent Flow ^{2/3}

- ❖ Applying dimensional analysis, the result were a correlation of the form

$$\frac{\Delta p}{\frac{1}{2}\rho V^2} = \bar{\phi}\left(\frac{\rho V D}{\mu}, \frac{\ell}{D}, \frac{\varepsilon}{D}\right)$$

- ❖ Experiments show that the nondimensional head loss is directly proportional to ℓ/D . Hence we can write

$$\frac{\Delta p}{\frac{1}{2}\rho V^2} = \frac{\ell}{D} \phi\left(\text{Re}, \frac{\varepsilon}{D}\right) \quad \Delta p = f \frac{\ell}{D} \frac{\rho V^2}{2}$$

$$f \equiv \phi\left(\text{Re}, \frac{\varepsilon}{D}\right)$$

Darcy-Weisbach equation

$$h_{L_{\text{major}}} \equiv f \frac{\ell}{D} \frac{V^2}{2g}$$

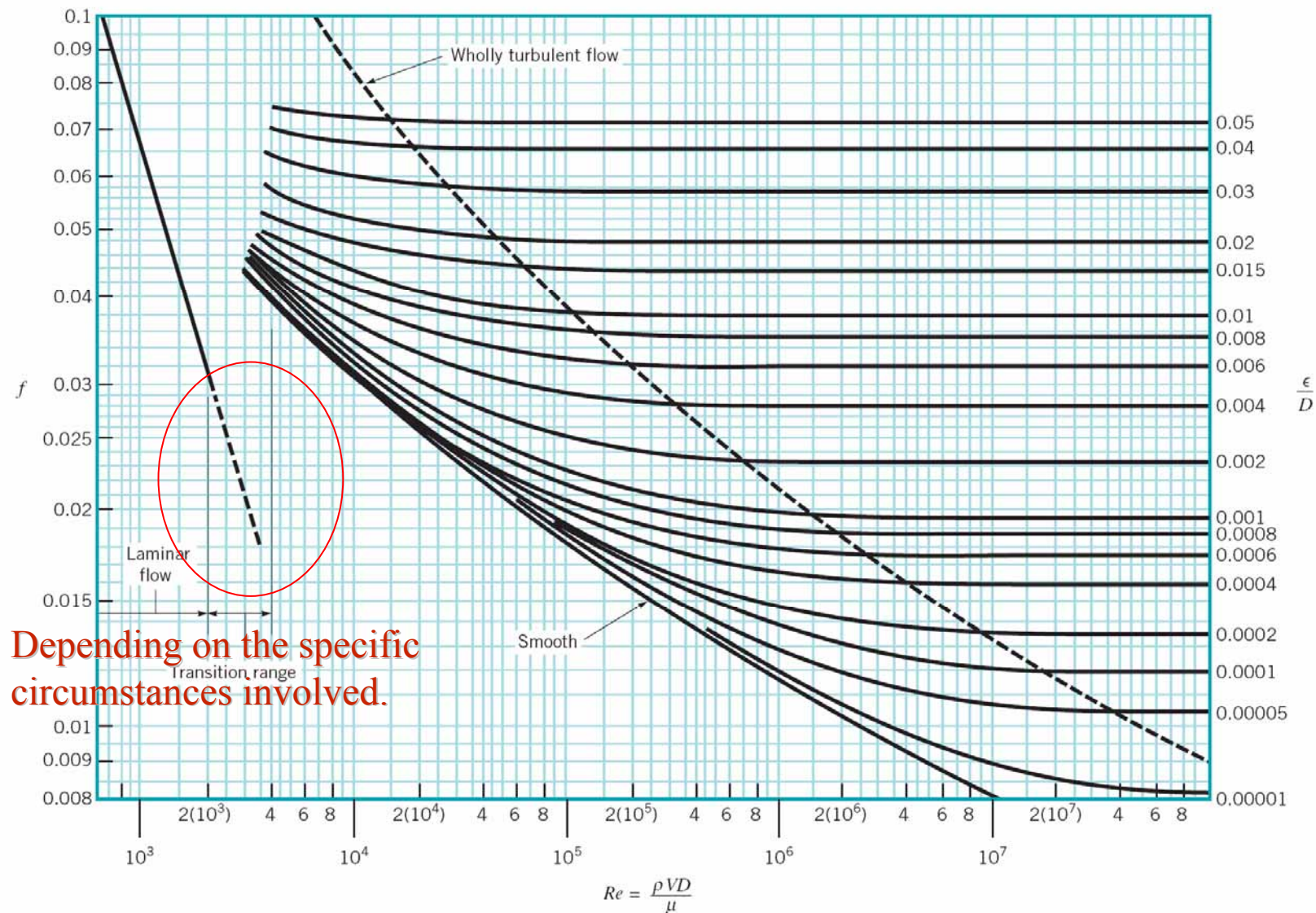
Roughness for Pipes

■ **TABLE 8.1**

Equivalent Roughness for New Pipes [From Moody (Ref. 7) and Colebrook (Ref. 8)]

| Pipe | Equivalent Roughness, ϵ | |
|-------------------------------------|----------------------------------|--------------|
| | Feet | Millimeters |
| Riveted steel | 0.003–0.03 | 0.9–9.0 |
| Concrete | 0.001–0.01 | 0.3–3.0 |
| Wood stave | 0.0006–0.003 | 0.18–0.9 |
| Cast iron | 0.00085 | 0.26 |
| Galvanized iron | 0.0005 | 0.15 |
| Commercial steel or wrought iron | 0.00015 | 0.045 |
| Drawn tubing | 0.000005 | 0.0015 |
| Plastic, glass | 0.0 (smooth) | 0.0 (smooth) |

Friction Factor by L. F. Moody



About Moody Chart

- ❖ For laminar flow, $f=64/Re$, which is independent of the relative roughness.
- ❖ For very large Reynolds numbers, $f= \left(\frac{0.316}{Re^{0.25}} \right)$, which is independent of the Reynolds numbers.
- ❖ For flows with very large value of Re , commonly termed completely turbulent flow (or wholly turbulent flow), the laminar sublayer is so thin (its thickness decrease with increasing Re) that the surface roughness completely dominates the character of the flow near the wall.
- ❖ For flows with moderate value of Re , the friction factor $f= \left(\frac{0.0791}{Re^{0.25}} \right)$.

Major Losses: Turbulent Flow ^{3/3}

- ❖ **Colebrook** – To avoid having to use a graphical method for obtaining f for turbulent flows.

Valid for the entire nonlaminar range of the Moody chart.

$$\frac{1}{\sqrt{f}} = -2.0 \log \left[\frac{\varepsilon / D}{3.7} + \frac{2.51}{\text{Re} \sqrt{f}} \right]$$

Colebrook formula

- ❖ **Miler** suggests that a single iteration will produce a result within 1 percent if the initial estimate is calculated from

$$f_0 = 0.25 \log \left[\frac{\varepsilon / D}{3.7} + \frac{5.74}{\text{Re}^{0.9}} \right]^{-2}$$

Example 8.5 Comparison of Laminar or Turbulent pressure Drop

- Air under standard conditions flows through a 4.0-mm-diameter drawn tubing with an average velocity of $V = 50$ m/s. For such conditions the flow would normally be turbulent. However, if precautions are taken to eliminate disturbances to the flow (the entrance to the tube is very smooth, the air is dust free, the tube does not vibrate, etc.), it may be possible to maintain laminar flow. (a) Determine the pressure drop in a 0.1-m section of the tube if the flow is laminar. (b) Repeat the calculations if the flow is turbulent.

Example 8.5 Solution^{1/2}

Under standard temperature and pressure conditions

$$=1.23\text{kg/m}^3, \mu =1.79\times 10^{-5}\text{N}\cdot\text{s/m}$$

The Reynolds number

$$R_e = \rho V D / \mu = \dots = 13,700 \rightarrow \text{Turbulent flow}$$

If the flow were laminar

$$f=64/\text{Re}=\dots=0.0467$$

$$\Delta p = f \frac{\ell}{D} \frac{1}{2} \rho V^2 = \dots = 0.179 \text{ kPa}$$

Example 8.5 Solution^{2/2}

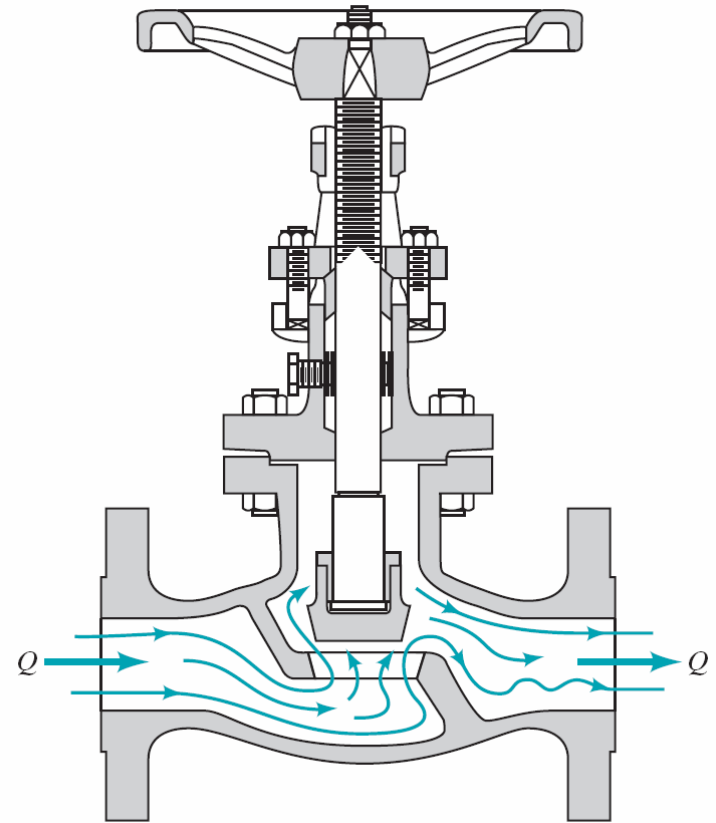
If the flow were turbulent

From Moody chart $f = \Phi(\text{Re}, \varepsilon/D) = \dots 0.028$

$$\Delta p = f \frac{\ell}{D} \frac{1}{2} \rho V^2 = \dots = 1.076 \text{ kPa}$$

Minor Losses ^{1/5}

- ❖ Most pipe systems consist of considerably more than straight pipes. These additional components (valves, bends, tees, and the like) add to the overall head loss of the system.
- ❖ Such losses are termed MINOR LOSS.



The flow pattern through a valve

Minor Losses ^{2/5}

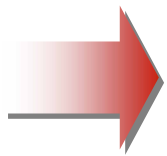
- ❖ The theoretical analysis to predict the details of flow pattern (through these additional components) is not, as yet, possible.
- ❖ The head loss information for essentially all components is given in dimensionless form and based on experimental data. The most common method used to determine these head losses or pressure drops is to specify the loss coefficient, K_L

Minor Losses ^{3/5}

$$K_L = \frac{h_{L_{\text{minor}}}}{V^2 / 2g} = \frac{\Delta p}{\frac{1}{2}\rho V^2} \Rightarrow \Delta p = K_L \frac{1}{2}\rho V^2$$

Minor losses are sometimes given in terms of an equivalent length ℓ_{eq}

$$\left\{ \begin{array}{l} h_{L_{\text{minor}}} = K_L \frac{V^2}{2g} = f \frac{\ell_{\text{eq}}}{D} \frac{V^2}{2g} \\ \ell_{\text{eq}} = K_L \frac{D}{f} \end{array} \right.$$



The actual value of K_L is strongly dependent on the geometry of the component considered. It may also dependent on the fluid properties. That is

$$K_L = \phi(\text{geometry}, \text{Re})$$

Minor Losses ^{4/5}

- ❖ For many practical applications the Reynolds number is large enough so that the flow through the component is dominated by inertial effects, with viscous effects being of secondary importance.
- ❖ In a flow that is dominated by inertia effects rather than viscous effects, it is usually found that pressure drops and head losses correlate directly with the dynamic pressure.
- ❖ This is the reason why the friction factor for very large Reynolds number, fully developed pipe flow is independent of the Reynolds number.

Minor Losses ^{5/5}

- ❖ This is true for flow through pipe components.
- ❖ Thus, in most cases of practical interest the loss coefficients for components are a function of geometry only,

$$K_L = \phi(\text{geometry})$$

Minor Losses Coefficient Entrance flow 1/3

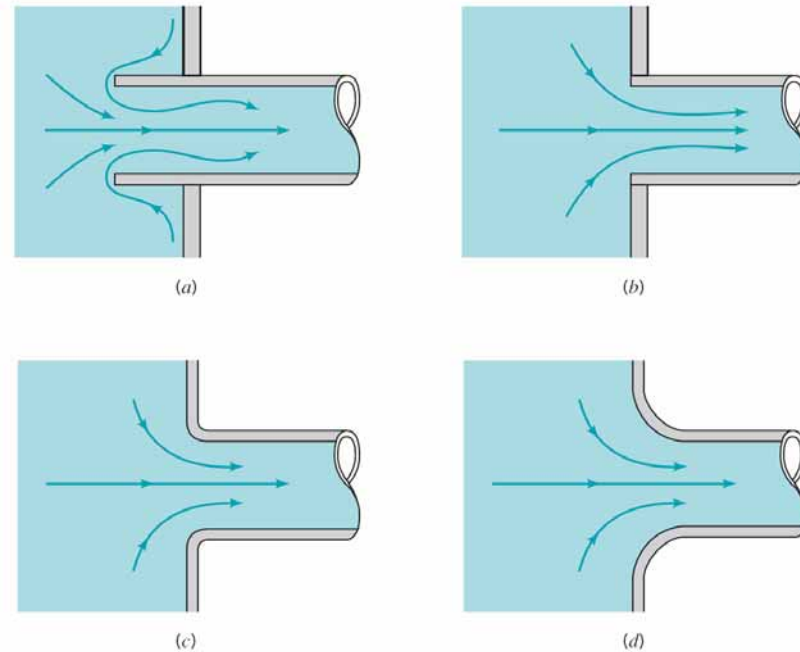
❖ Entrance flow condition and loss coefficient

(a) Reentrant, $K_L = 0.8$

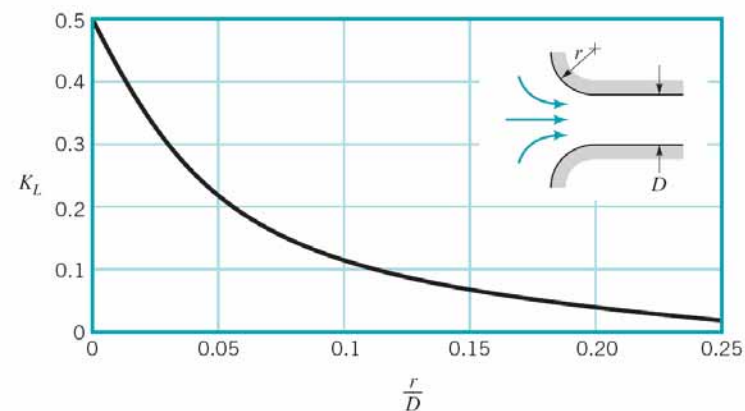
(b) sharp-edged, $K_L = 0.5$

(c) slightly rounded, $K_L = 0.2$

(d) well-rounded, $K_L = 0.04$



K_L = function of rounding of the inlet edge.

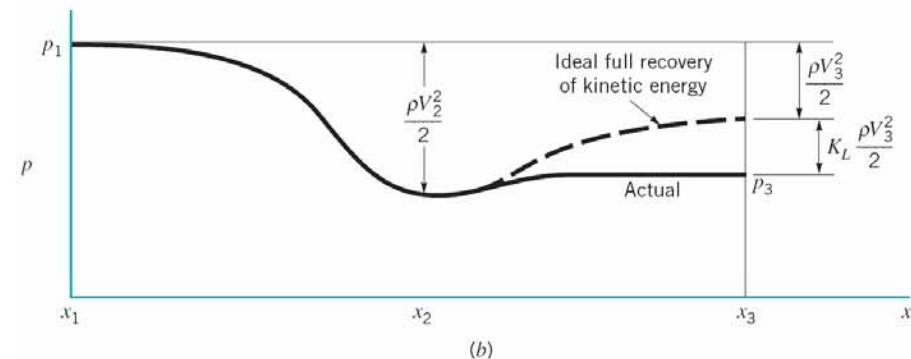
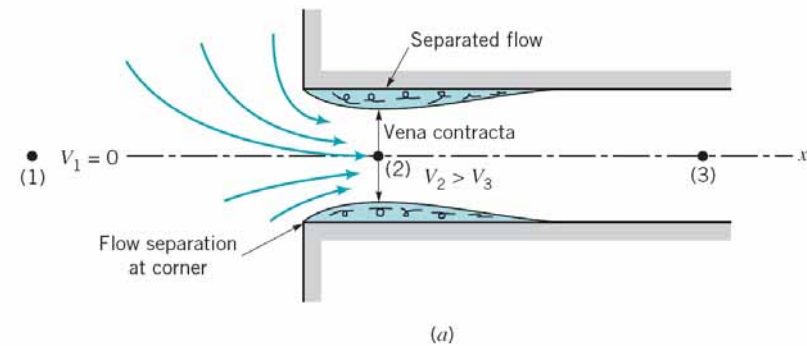


Minor Losses Coefficient Entrance flow 2/3

- ❖ A vena contracta region may result because the fluid cannot turn a sharp right-angle corner. The flow is said to separate from the sharp corner.
- ❖ The maximum velocity velocity at section (2) is greater than that in the pipe section (3), and the pressure there is lower.
- ❖ If this high speed fluid could slow down efficiently, the kinetic energy could be converted into pressure.

Minor Losses Coefficient Entrance flow 3/3

- ❖ Such is not the case. Although the fluid may be accelerated very efficiently, it is very difficult to slow down (decelerate) the fluid efficiently.
- ❖ (2)→(3) The extra kinetic energy of the fluid is partially lost because of viscous dissipation, so that the pressure does not return to the ideal value.



Flow pattern and pressure distribution for a sharp-edged entrance

Minor Losses Coefficient Exit flow

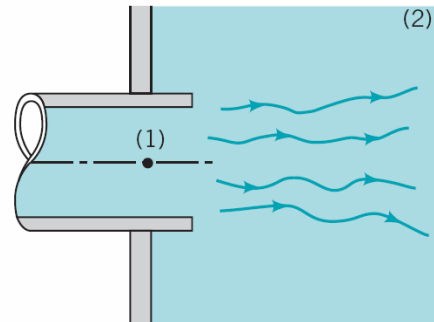
❖ Exit flow condition and loss coefficient

(a) Reentrant, $K_L = 1.0$

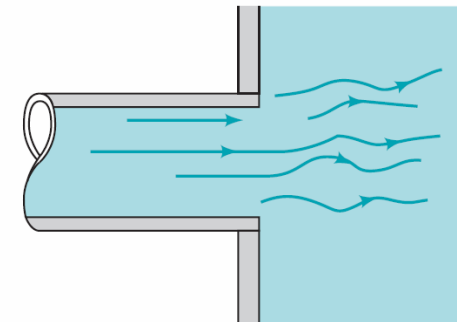
(b) sharp-edged, $K_L = 1.0$

(c) slightly rounded, $K_L = 1.0$

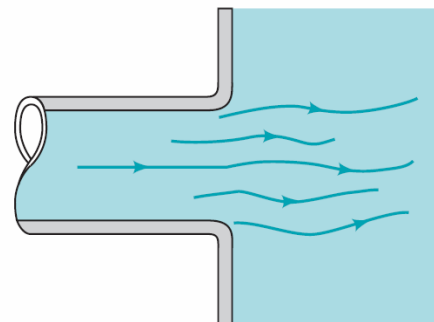
(d) well-rounded, $K_L = 1.0$



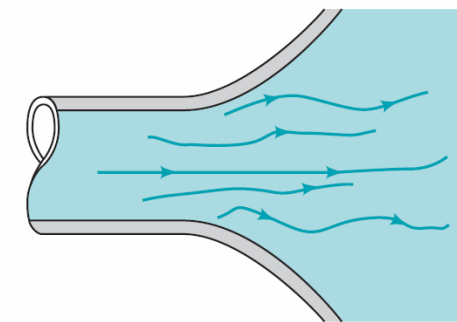
(a)



(b)



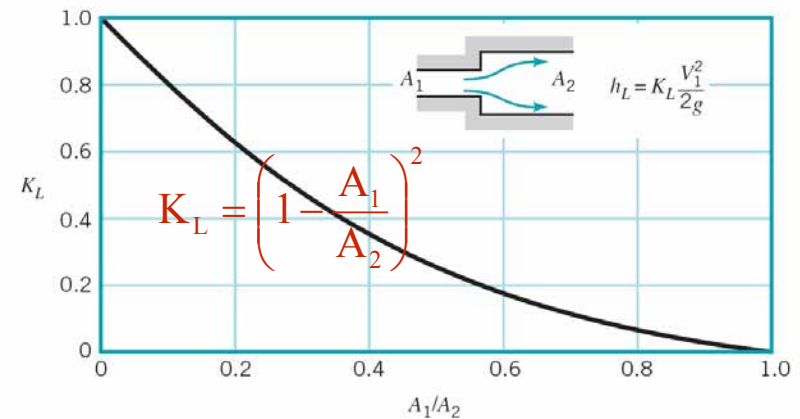
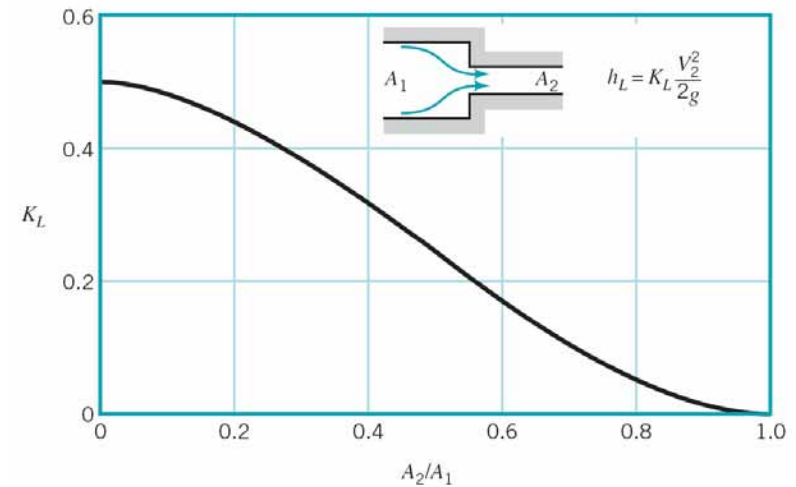
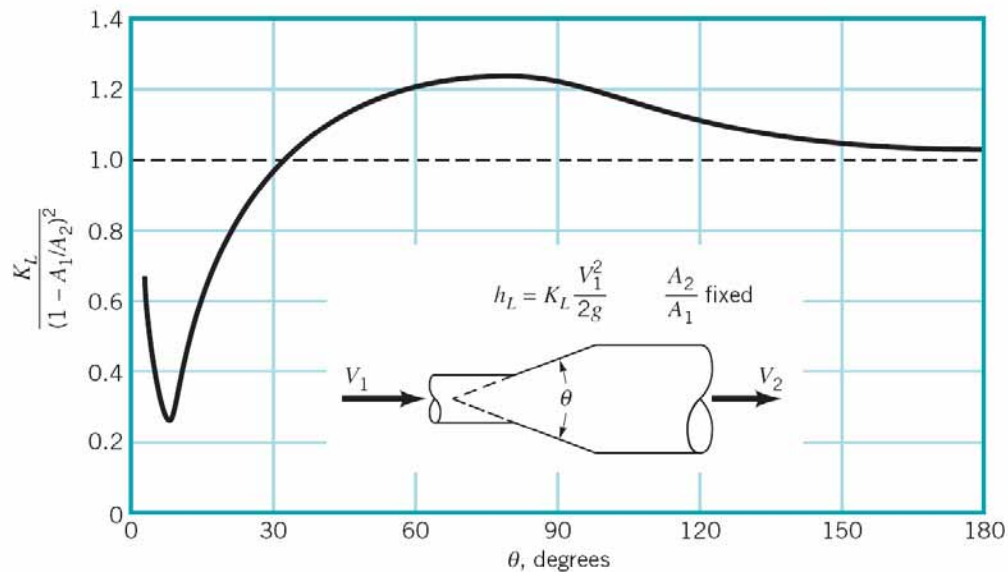
(c)



(d)

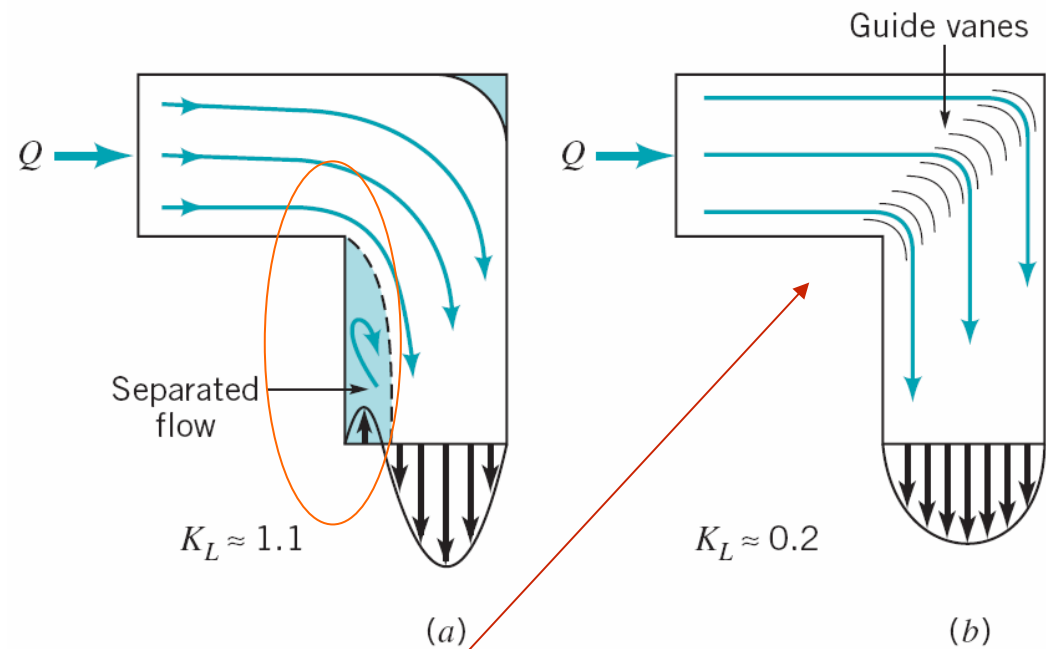
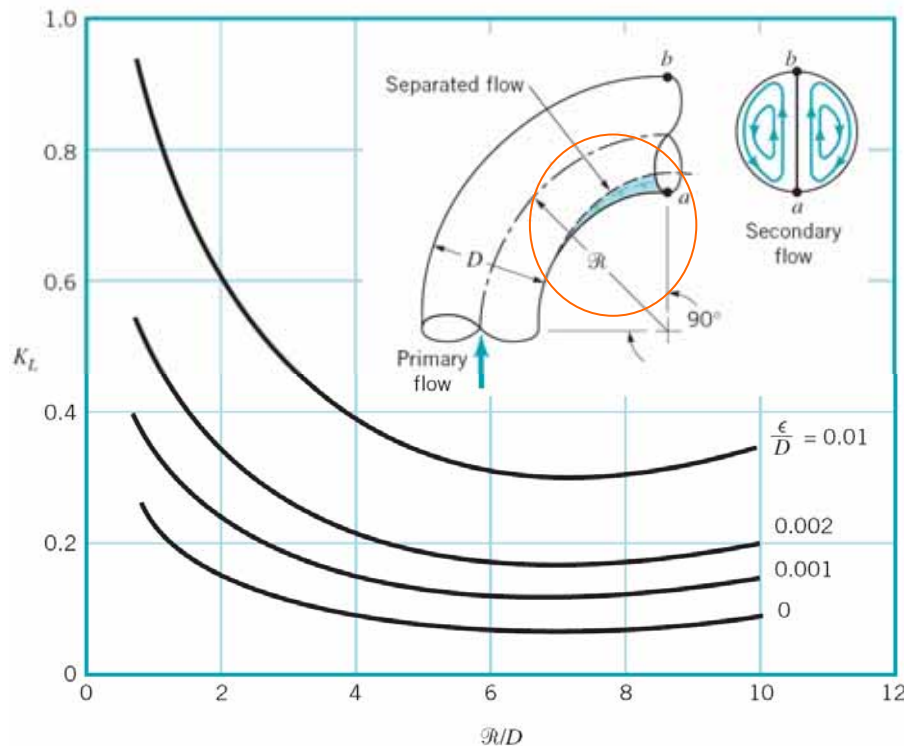
Minor Losses Coefficient varied diameter

- ❖ Loss coefficient for sudden contraction, expansion, typical conical diffuser.



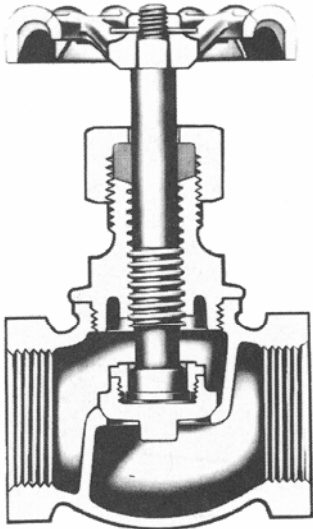
Minor Losses Coefficient Bend

- ❖ Character of the flow in bend and the associated loss coefficient.

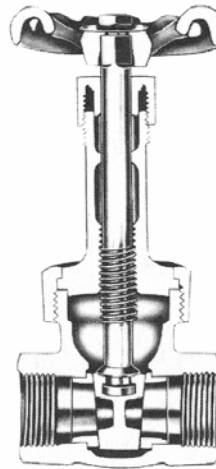


Carefully designed guide vanes help direct the flow with less unwanted swirl and disturbances.

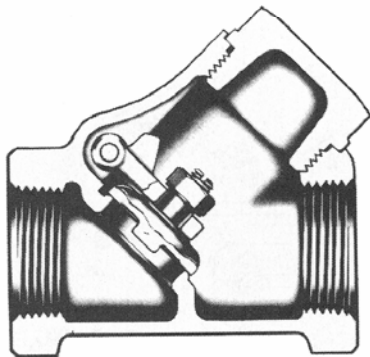
Internal Structure of Valves



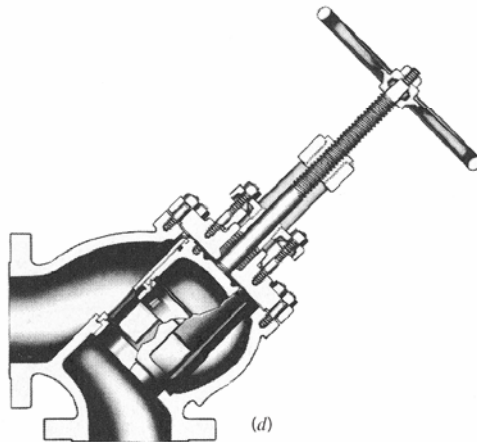
(a)



(b)



(c)



(d)

(a) globe valve

(b) gate valve

(c) swing check valve

(d) stop check valve

Loss Coefficients for Pipe Components

■ TABLE 8.2

Loss Coefficients for Pipe Components $\left(h_L = K_L \frac{V^2}{2g}\right)$ (Data from Refs. 5, 10, 27)

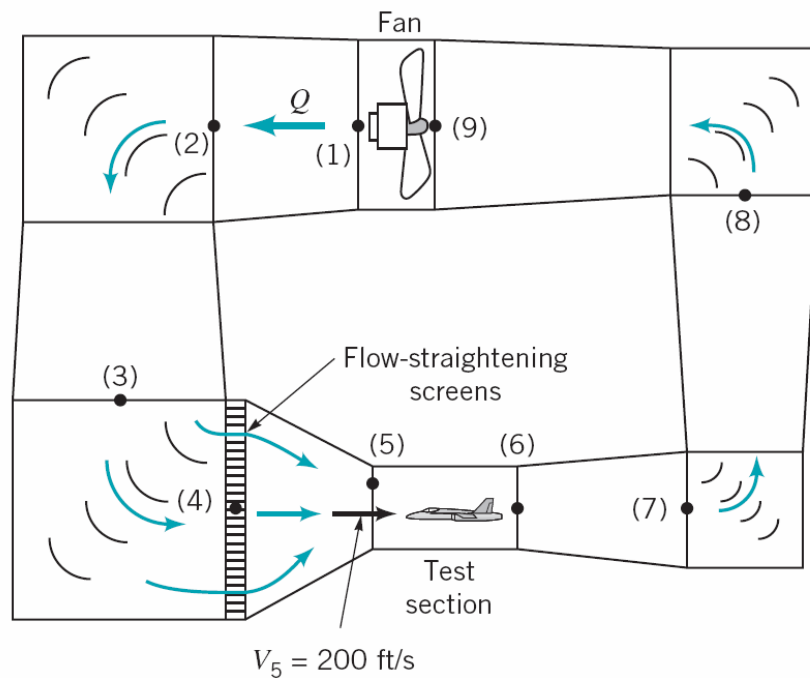
| Component | K_L | |
|----------------------------------|----------|--|
| a. Elbows | | |
| Regular 90°, flanged | 0.3 | |
| Regular 90°, threaded | 1.5 | |
| Long radius 90°, flanged | 0.2 | |
| Long radius 90°, threaded | 0.7 | |
| Long radius 45°, flanged | 0.2 | |
| Regular 45°, threaded | 0.4 | |
| b. 180° return bends | | |
| 180° return bend, flanged | 0.2 | |
| 180° return bend, threaded | 1.5 | |
| c. Tees | | |
| Line flow, flanged | 0.2 | |
| Line flow, threaded | 0.9 | |
| Branch flow, flanged | 1.0 | |
| Branch flow, threaded | 2.0 | |
| d. Union, threaded | | |
| | 0.08 | |
| e. Valves | | |
| Globe, fully open | 10 | |
| Angle, fully open | 2 | |
| Gate, fully open | 0.15 | |
| Gate, $\frac{1}{4}$ closed | 0.26 | |
| Gate, $\frac{1}{2}$ closed | 2.1 | |
| Gate, $\frac{3}{4}$ closed | 17 | |
| Swing check, forward flow | 2 | |
| Swing check, backward flow | ∞ | |
| Ball valve, fully open | 0.05 | |
| Ball valve, $\frac{1}{2}$ closed | 5.5 | |
| Ball valve, closed | 210 | |

*See Fig. 8.32 for typical valve geometry.

Example 8.6 Minor Loss ^{1/2}

- Air at standard conditions is to flow through the test section [between sections (5) and (6)] of the closed-circuit wind tunnel shown in Figure E8.6 with a velocity of 200 ft/s. The flow is driven by a fan that essentially increase the static pressure by the amount $p_1 - p_9$ that is needed to overcome the head losses experienced by the fluid as it flows around the circuit. Estimate the value of $p_1 - p_9$ and the horsepower supplied to the fluid by the fan.

Example 8.6 Minor Loss ^{2/2}



| Location | Area (ft ²) | Velocity (ft/s) |
|----------|-------------------------|-----------------|
| 1 | 22.0 | 36.4 |
| 2 | 28.0 | 28.6 |
| 3 | 35.0 | 22.9 |
| 4 | 35.0 | 22.9 |
| 5 | 4.0 | 200.0 |
| 6 | 4.0 | 200.0 |
| 7 | 10.0 | 80.0 |
| 8 | 18.0 | 44.4 |
| 9 | 22.0 | 36.4 |

Example 8.6 Solution^{1/3}

The maximum velocity within the wind tunnel occurs in the test section (smallest area). Thus, the maximum Mach number of the flow is $Ma_5 = V_5/c_5$

$$V_5 = 200 \text{ ft/s} \quad \uparrow \quad c_5 = (KRT_5)^{1/2} = 1117 \text{ ft/s}$$

The energy equation between points (1) and (9)

$$\frac{p_1}{\gamma} + \frac{V_1^2}{2g} + z_1 = \frac{p_9}{\gamma} + \frac{V_9^2}{2g} + z_9 + h_{L1-9}$$

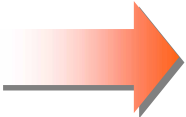

$$h_{L1-9} = \frac{p_1}{\gamma} - \frac{p_9}{\gamma} \quad \text{The total head loss from (1) to (9).}$$

Example 8.6 Solution^{2/3}

The energy across the fan, from (9) to (1)

$$\frac{p_9}{\gamma} + \frac{V_9^2}{2g} + z_9 + h_p = \frac{p_1}{\gamma} + \frac{V_1^2}{2g} + z_1$$

H_p is the actual head rise supplied by the pump (fan) to the air.


$$h_p = \frac{p_1}{\gamma} - \frac{p_9}{\gamma} = h_{L1-9}$$

The actual power supplied to the air (horsepower, P_a) is obtained from the fan head by

$$P_a = \gamma Q h_p = \gamma A_5 V_5 h_p = \gamma A_5 V_5 h_{L1-9}$$

Example 8.6 Solution^{3/3}

The total head loss

$$h_{L1-9} = h_{L_{\text{corner}7}} + h_{L_{\text{corner}8}} + h_{L_{\text{corner}2}} + h_{L_{\text{corner}3}} + h_{L_{\text{dif}}} + h_{L_{\text{noz}}} + h_{L_{\text{scr}}}$$

$$h_{L_{\text{corner}}} = K_L \frac{V^2}{2g} = 0.2 \frac{V^2}{2g} \quad h_{L_{\text{dif}}} = K_{L_{\text{dif}}} \frac{V^2}{2g} = 0.6 \frac{V^2}{2g}$$

$$K_{L_{\text{noz}}} = 0.2 \quad K_{L_{\text{scr}}} = 4.0$$


$$p_1 - p_9 = \gamma h_{L1-9} = (0.765 \text{ lb/ft}^2)(560 \text{ ft}) = \dots = 0.298 \text{ psi}$$

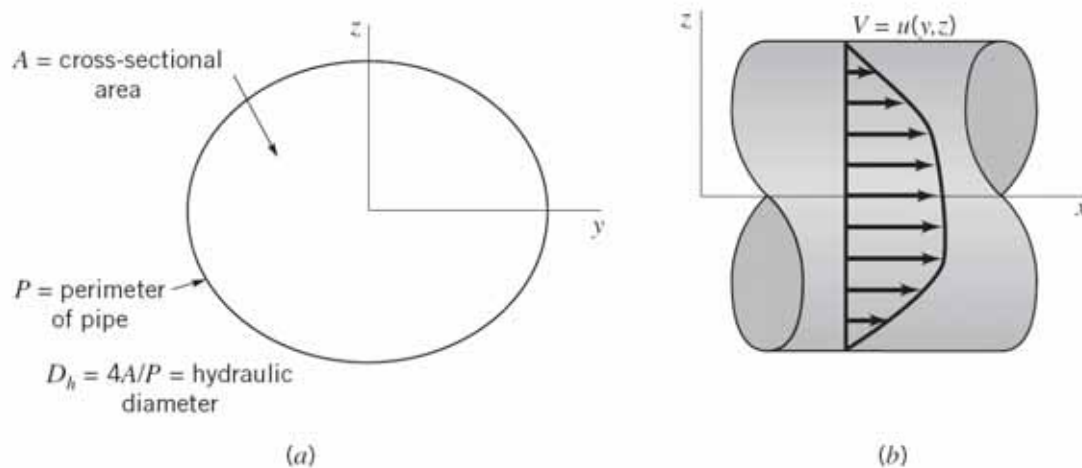
$$P_a = \dots = 34300 \text{ ft} \cdot \text{lb/s} = 62.3 \text{ hp}$$

Noncircular Ducts ^{1/4}

- ❖ The empirical correlations for pipe flow may be used for computations involving noncircular ducts, provided their cross sections are not too exaggerated.
- ❖ The correlation for turbulent pipe flow are extended for use with noncircular geometries by introducing the hydraulic diameter, defined as

$$D_h \equiv \frac{4A}{P}$$

Where A is cross-sectional area, and P is wetted perimeter.



Noncircular Ducts ^{2/4}

❖ For a circular duct

$$D_h \equiv \frac{4A}{P} = D$$

❖ For a rectangular duct of width b and height h

$$D_h \equiv \frac{4A}{P} = \frac{4bh}{2(b+h)} = \frac{2h}{1+ar} \quad ar = h/b$$

The hydraulic diameter concept can be applied in the approximate range $\frac{1}{4} < ar < 4$. So the correlations for pipe flow give acceptably accurate results for rectangular ducts.

Noncircular Ducts ^{3/4}

- ❖ The friction factor can be written as $f = C / Re_h$, where the constant C depends on the particular shape of the duct, and Re_h is the Reynolds number based on the hydraulic diameter.
- ❖ The hydraulic diameter is also used in the definition of the friction factor, $h_L = f(\ell / D_h)(V^2 / 2g)$, and the relative roughness ε / D_h .

Noncircular Ducts 4/4

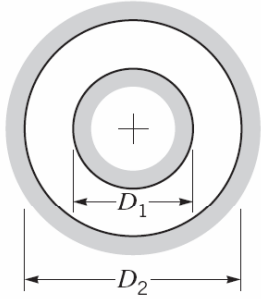
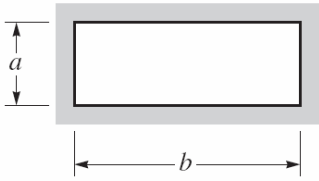
- ❖ For Laminar flow, the value of $C=f \cdot Re_h$ have been obtained from theory and/or experiment for various shapes.
- ❖ For turbulent flow in ducts of noncircular cross section, calculations are carried out by using the Moody chart data for round pipes with the diameter replaced by the hydraulic diameter and the Reynolds number based on the hydraulic diameter.

The Moody chart, developed for round pipes, can also be used for noncircular ducts.

Friction Factor for Laminar Flow in Noncircular Ducts

■ TABLE 8.3

Friction Factors for Laminar Flow in Noncircular Ducts (Data from Ref. 18)

| Shape | Parameter | $C = f Re_h$ |
|---|-----------|--------------|
| I. Concentric Annulus $D_h = D_2 - D_1$ | D_1/D_2 | |
|  | 0.0001 | 71.8 |
| | 0.01 | 80.1 |
| | 0.1 | 89.4 |
| | 0.6 | 95.6 |
| | 1.00 | 96.0 |
| II. Rectangle $D_h = \frac{2ab}{a+b}$ | a/b | |
|  | 0 | 96.0 |
| | 0.05 | 89.9 |
| | 0.10 | 84.7 |
| | 0.25 | 72.9 |
| | 0.50 | 62.2 |
| | 0.75 | 57.9 |
| | 1.00 | 56.9 |

Example 8.7 Noncircular Duct

- Air at temperature of 120°F and standard pressure flows from a furnace through an 8-in.-diameter pipe with an average velocity of 10ft/s. It then passes through a transition section and into a square duct whose side is of length a . The pipe and duct surfaces are smooth ($\epsilon = 0$). Determine the duct size, a , if the head loss per foot is to be the same for the pipe and the duct.

Example 8.7 Solution^{1/3}

The head loss per foot for the pipe

$$\frac{h_L}{\ell} = \frac{f}{D} \frac{V^2}{2g}$$

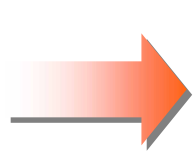
For given pressure and temperature $\nu = 1.89 \times 10^{-4} \text{ ft}^2/\text{s}$

$$\text{Re} = \frac{VD}{\nu} = 35300$$

→ **For the square duct** $\frac{h_L}{\ell} = \frac{f}{D_h} \frac{V_s^2}{2g} = 0.0512$

$$D_h = \frac{4A}{P} = a \quad V_s = \frac{Q}{A} = \frac{3.49}{a^2}$$

Example 8.7 Solution^{2/3}


$$\frac{h_L}{\ell} = \frac{f}{D_h} \frac{V_s^2}{2g} = 0.0512 = \frac{f}{a} \frac{(3.49/a^2)^2}{2(32.2)} \Rightarrow a = 1.30f^{1/5} \quad (1)$$

The Reynolds number based on the hydraulic diameter

$$Re_h = \frac{V_s D_h}{\nu} = \frac{(3.49/a^2)a}{1.89 \times 10^{-4}} = \frac{1.89 \times 10^{-4}}{a} \quad (2)$$

Have three unknown (a, f, and Re_h) and three equation – Eqs. 1, 2, and either in graphical form the Moody chart or the Colebrook equation



Find a

Example 8.7 Solution^{3/3}

Use the Moody chart



Assume the friction factor for the duct is the same as for the pipe.

That is, assume $f=0.022$.

From Eq. 1 we obtain $a=0.606$ ft.

From Eq. 2 we have $Re_h=3.05 \times 10^4$

From Moody chart we find $f=0.023$, which does not quite agree the assumed value of f .

Try again, using the latest calculated value of $f=0.023$ as our guess.

..... The final result is $f=0.023$, $Re_h=3.05 \times 10^4$, and $a=0.611$ ft.

Pipe Flow Examples ^{1/2}

- ❖ The energy equation, relating the conditions at any two points 1 and 2 for a single-path pipe system

$$\left(\frac{p_1}{\rho g} + \frac{\alpha_1 V_1^2}{2g} + z_1 \right) - \left(\frac{p_2}{\rho g} + \frac{\alpha_2 V_2^2}{2g} + z_2 \right) = h_L = \sum h_{L_{\text{major}}} + \sum h_{L_{\text{minor}}}$$

by judicious choice of points 1 and 2 we can analyze not only the entire pipe system, but also just a certain section of it that we may be interested in.

$$\text{Major loss } h_{L_{\text{major}}} \equiv f \frac{\ell}{D} \frac{V^2}{2g} \quad \text{Minor loss } h_{L_{\text{minor}}} = K_L \frac{V^2}{2g}$$

Pipe Flow Examples ^{2/2}

- ❖ Single pipe whose length may be interrupted by various components.
- ❖ Multiple pipes in different configuration
 - ⇒ Parallel
 - ⇒ Series
 - ⇒ Network

Single-Path Systems ^{1/2}

❖ Pipe flow problems can be categorized by what parameters are given and what is to be calculated.

■ TABLE 8.4

Pipe Flow Types

| Variable | Type I | Type II | Type III |
|---------------------------------|-----------|-----------|-----------|
| a. Fluid | | | |
| Density | Given | Given | Given |
| Viscosity | Given | Given | Given |
| b. Pipe | | | |
| Diameter | Given | Given | Determine |
| Length | Given | Given | Given |
| Roughness | Given | Given | Given |
| c. Flow | | | |
| Flowrate or Average Velocity | Given | Determine | Given |
| d. Pressure | | | |
| Pressure Drop or Head Loss | Determine | Given | Given |

Single-Path Systems ^{2/2}

- ❖ Given pipe (L and D), and flow rate, and Q, find pressure drop p
- ❖ Given p , D, and Q, find L.
- ❖ Given p , L, and D, find Q.
- ❖ Given p , L, and Q, find D.

Given L , D, and Q, find p

❖ The energy equation

$$\left(\frac{p_1}{\rho g} + \frac{\alpha_1 V_1^2}{2g} + z_1 \right) - \left(\frac{p_2}{\rho g} + \frac{\alpha_2 V_2^2}{2g} + z_2 \right) = h_L = \sum h_{L_{\text{major}}} + \sum h_{L_{\text{minor}}}$$

- ❖ The flow rate leads to the Reynolds number and hence the friction factor for the flow.
- ❖ Tabulated data can be used for minor loss coefficients and equivalent lengths.
- ❖ The energy equation can then be used to directly to obtain the pressure drop.

Given p , D , and Q , **find** L

❖ The energy equation

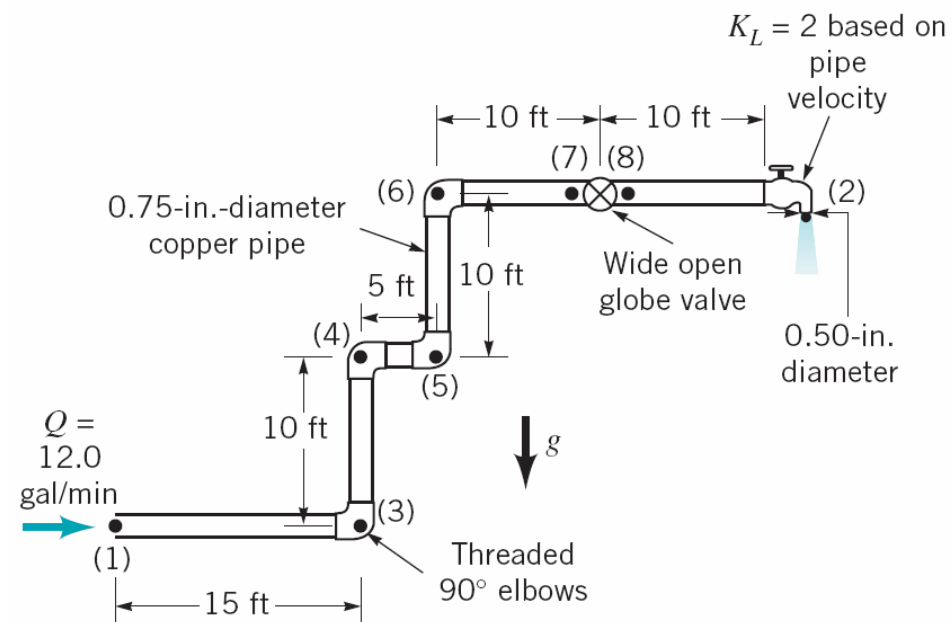
$$\left(\frac{p_1}{\rho g} + \frac{\alpha_1 V_1^2}{2g} + z_1 \right) - \left(\frac{p_2}{\rho g} + \frac{\alpha_2 V_2^2}{2g} + z_2 \right) = h_L = \sum h_{L_{\text{major}}} + \sum h_{L_{\text{minor}}}$$

- ❖ The flow rate leads to the Reynolds number and hence the friction factor for the flow.
- ❖ Tabulated data can be used for minor loss coefficients and equivalent lengths.
- ❖ The energy equation can then be rearranged and solved directly for the pipe length.

Example 8.8 Type I Determine Pressure Drop

- Water at 60°F flows from the basement to the second floor through the 0.75-in. (0.0625-ft)-diameter copper pipe (a drawn tubing) at a rate of $Q = 12.0 \text{ gal/min} = 0.0267 \text{ ft}^3/\text{s}$ and exits through a faucet of diameter 0.50 in. as shown in Figure E8.8.

Determine the pressure at point (1) if: (a) all losses are neglected, (b) the only losses included are major losses, or (c) all losses are included.



Example 8.8 Solution^{1/4}

$$V_1 = \frac{Q}{A_1} = \dots = 8.70 \text{ ft/s}$$

$$\rho = 1.94 \text{ slug/ft}^3$$

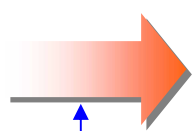
$$\mu = 2.34 \times 10^{-5} \text{ lb} \cdot \text{s/ft}^2$$

$$\text{Re} = \rho V D / \mu = 45000$$

The flow is turbulent

The energy equation

$$\frac{p_1}{\rho g} + \frac{\alpha_1 V_1^2}{2g} + z_1 = \frac{p_2}{\rho g} + \frac{\alpha_2 V_2^2}{2g} + z_2 + h_L$$



$$p_1 = \gamma z_2 + \frac{1}{2} \rho (V_2^2 - V_1^2) + \gamma h_L$$

$$z_1 = 0, z_2 = 20 \text{ ft}, p_2 = 0 \text{ (free jet)}$$

$$V_2 = Q / A_2 = \dots = 19.6 \text{ ft/s}$$

Head loss is different for each of the three cases.

Example 8.8 Solution^{2/4}

(a) If all losses are neglected ($h_L=0$)

$$p_1 = \gamma z_2 + \frac{1}{2} \rho (V_2^2 - V_1^2) = \dots = 1547 \text{ lb / ft}^2 = 10.7 \text{ psi}$$

(b) If the only losses included are the major losses, the head loss is

$$h_L = f \frac{\ell}{D} \frac{V_1^2}{2g}$$

$$\varepsilon = 0.000005 \quad \varepsilon / D = 8 \times 10^{-5} \quad \text{Re} = 45000 \quad \xrightarrow{\text{Moody chart}} \quad f = 0.0215$$

$$p_1 = \gamma z_2 + \frac{1}{2} \rho (V_2^2 - V_1^2) + \rho f \frac{\ell (= 60 \text{ ft})}{D} \frac{V_1^2}{2} = \dots = 3062 \text{ lb / ft}^2 = 21.3 \text{ psi}$$

Example 8.8 Solution^{3/4}

(c) If major and minor losses are included

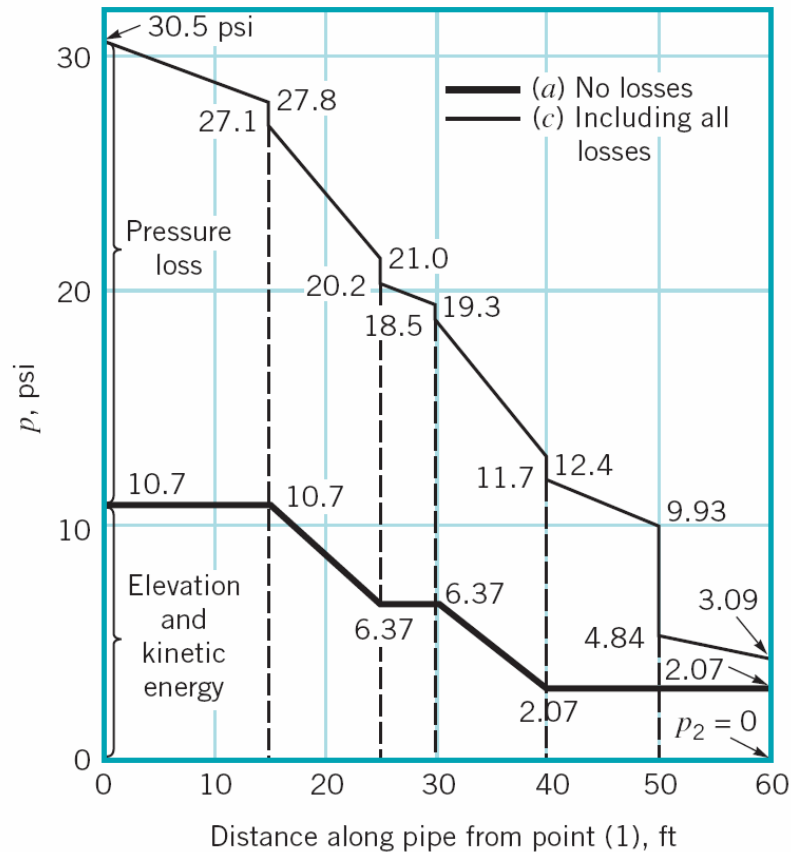
$$p_1 = \gamma z_2 + \frac{1}{2} \rho (V_2^2 - V_1^2) + f \gamma \frac{\ell}{D} \frac{V_1^2}{2g} + \sum \rho K_L \frac{V^2}{2}$$

$$p_1 = 21.3 \text{ psi} + \sum \rho K_L \frac{V^2}{2}$$

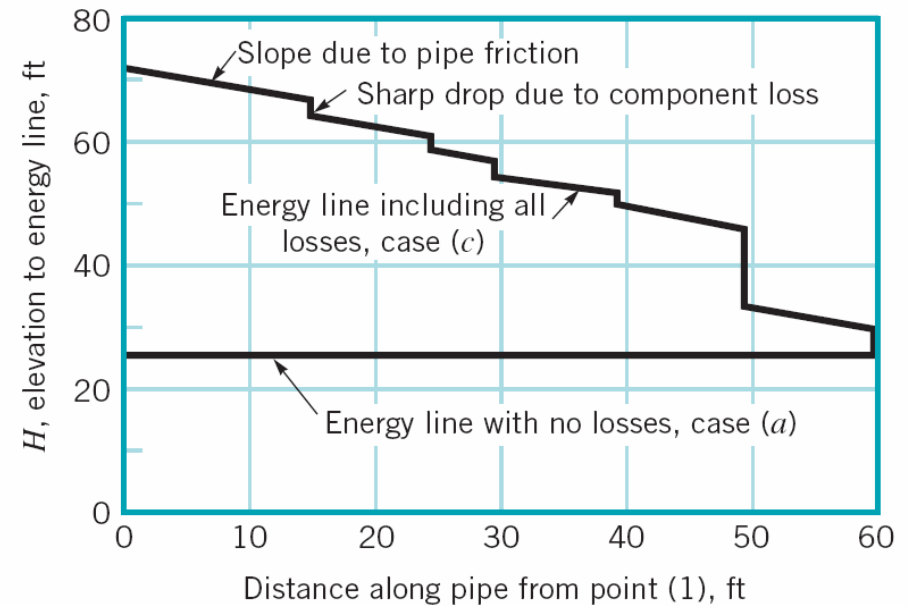
$$= 21.3 \text{ psi} + (1.94 \text{ slugs / ft}^3) \frac{(8.70 \text{ ft / s})^2}{2} [10 + 4(1.5) + 2]$$

$$p_1 = 21.3 \text{ psi} + 9.17 \text{ psi} = 30.5 \text{ psi}$$

Example 8.8 Solution^{4/4}



Location: (1) (3) (4) (5) (6) (7) (8) (2)



Example 8.9 Type I, Determine Head Loss

- Crude oil at 140°F with $\rho = 53.7 \text{ lb/ft}^3$ and $\mu = 8 \times 10^{-5} \text{ lb}\cdot\text{s/ft}^2$ (about four times the viscosity of water) is pumped across Alaska through the Alaska pipeline, a 799-mile-long, 4-ft-diameter steel pipe, at a maximum rate of $Q = 2.4 \text{ million barrel/day} = 117 \text{ ft}^3/\text{s}$, or $V = Q/A = 9.31 \text{ ft/s}$. Determine the horsepower needed for the pumps that drive this large system.

Example 8.9 Solution^{1/2}


The energy equation between points (1) and (2)

$$\frac{p_1}{\gamma} + \frac{V_1^2}{2g} + z_1 + h_p = \frac{p_2}{\gamma} + \frac{V_2^2}{2g} + z_2 + h_L$$

h_p is the head provided to the oil by the pump.

Assume that $z_1=z_2$, $p_1=p_2=V_1=V_2=0$ (large, open tank)

Minor losses are negligible because of the large length-to-diameter ratio of the relatively straight, uninterrupted pipe.


$$h_L = h_p = f \frac{\ell}{D} \frac{V^2}{2g} = \dots = 17700\text{ft}$$

$f=0.0124$ from Moody chart $\ell/D=(0.00015\text{ft})/(4\text{ft})$, $Re=\dots$

Example 8.9 Solution^{2/2}

The actual power supplied to the fluid.

$$P_a = \gamma Q h_p = \dots \left(\frac{1 \text{ hp}}{550 \text{ ft} \cdot \text{lb} / \text{s}} \right) = 202000 \text{ hp}$$

Given ρ , L , and D , find $Q^{1/2}$

- ❖ These types of problems required either manual iteration or use of a computer application.
- ❖ The unknown flow rate or velocity is needed before the Reynolds number and hence the friction factor can be found.

Repeat the iteration process

f V Re f until convergence

Given p , L , and D , find Q ^{2/2}

- ❖ First, we make a guess for f and solve the energy equation for V in terms of known quantities and the guessed friction factor f .
- ❖ Then we can compute a Reynolds number and hence obtain a new value for f .

Repeat the iteration process

f V Re f until convergence

Given p , L , and Q , find $D^{1/2}$

- ❖ These types of problems required either manual iteration or use of a computer application.
- ❖ The unknown diameter is needed before the Reynolds number and relative roughness, and hence the friction factor can be found.

Given p , L , and Q , find $D^{2/2}$

- ❖ First, we make a guess for f and solve the energy equation for D in terms of known quantities and the guessed friction factor f .
- ❖ Then we can compute a Reynolds number and hence obtain a new value for f .

Repeat the iteration process

f D Re and $/D$ f until convergence

Example 8.10 Type II, Determine Flowrate

- According to an appliance manufacturer, the 4-in-diameter galvanized iron vent on a clothes dryer is not to contain more than 20 ft of pipe and four 90° elbows. Under these conditions determine the air flowrate if the pressure within the dryer is 0.20 inches of water. Assume a temperature of 100 °F and standard pressure.

Example 8.10 Solution^{1/2}

Application of the energy equation between the inside of the dryer, point (1), and the exit of the vent pipe, point (2) gives

$$\frac{p_1}{\gamma} + \frac{V_1^2}{2g} + z_1 = \frac{p_2}{\gamma} + \frac{V_2^2}{2g} + z_2 + f \frac{\ell}{D} \frac{V^2}{2g} + \sum K_L \frac{V^2}{2g}$$

Assume that $z_1=z_2$, $p_2=0$, $V_1=0$

$$\frac{p_1}{\gamma_{H_2O}} = 0.2 \text{ in} \Rightarrow p_1 = (0.2 \text{ in.}) \left(\frac{1 \text{ ft}}{12 \text{ in.}} \right) (62.4 \text{ lb / ft}^3) = 1.04 \text{ lb / ft}^2$$

With $\gamma = 0.0709 \text{ lb/ft}^3$, $V_2=V$, and $f = 1.79 \times 10^{-4} \text{ ft}^2/\text{s}$.

$$945 = (7.5 + 60f)V^2 \quad (1)$$

f is dependent on Re , which is dependent on V , and unknown.

Example 8.10 Solution^{2/2}

$$Re = \frac{VD}{\nu} = \dots = 1860V \quad (2)$$

We have three relationships (Eq. 1, 2, and the $f/D=0.0015$ curve of the Moody chart) from which we can solve for the three unknowns f , Re , and V .

This is done easily by iterative scheme as follows.

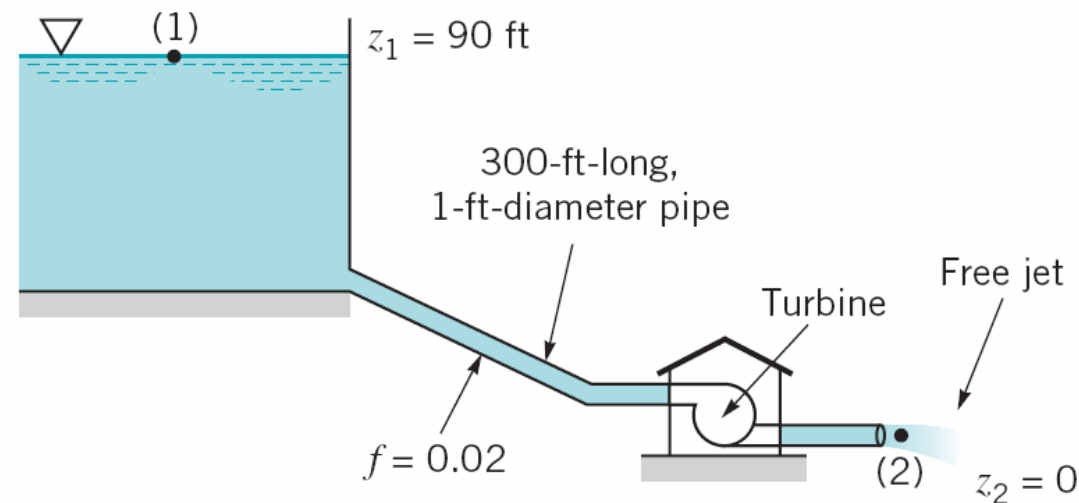
Assume $f=0.022$ $V=10.4\text{ft/s}$ (Eq. 1) $Re=19,300$ (Eq.2) $f=0.029$

Assume $f=0.029$ $V=10.1\text{ft/s}$ $Re=18,800$ $f=0.029$

$$Q = AV = \dots = 0.881\text{ft}^3/\text{s}$$

Example 8.11 Type II, Determine Flowrate

- The turbine shown in Figure E8.11 extracts 50 hp from the water flowing through it. The 1-ft-diameter, 300-ft-long pipe is assumed to have a friction factor of 0.02. Minor losses are negligible. Determine the flowrate through the pipe and turbine.



Example 8.11 Solution^{1/2}

The energy equation can be applied between the surface of the lake and the outlet of the pipe as

$$\frac{p_1}{\gamma} + \frac{V_1^2}{2g} + z_1 = \frac{p_2}{\gamma} + \frac{V_2^2}{2g} + z_2 + h_L + h_T$$

Where $p_1 = V_1 = p_2 = z_2 = 0$, $z_1 = 90\text{ft}$, and $V_2 = V$, the fluid velocity in the pipe

$$h_L = f \frac{\ell}{D} \frac{V^2}{2g} = 0.0932 V^2 \text{ft} \quad h_T = \frac{P_a}{\gamma Q} = \dots = \frac{561}{V} \text{ft}$$

$$0.107 V^3 - 90 V + 561 = 0$$

There are two real, positive roots: $V = 6.58 \text{ ft/s}$ or $V = 24.9 \text{ ft/s}$. The third root is negative ($V = -31.4 \text{ ft/s}$) and has no physical meaning for this flow.

Example 8.11 Solution^{2/2}

Two acceptable flowrates are

$$Q = \frac{\pi}{4} D^2 V = \dots = 5.17 \text{ ft}^3 / \text{s}$$

$$Q = \frac{\pi}{4} D^2 V = \dots = 19.6 \text{ ft}^3 / \text{s}$$

Example 8.12 Type III Without Minor Losses, Determine Diameter

- Air at standard temperature and pressure flows through a horizontal, galvanized iron pipe ($\epsilon = 0.0005$ ft) at a rate of $2.0 \text{ ft}^3/\text{s}$. Determine the minimum pipe diameter if the pressure drop is to be no more than 0.50 psi per 100 ft of pipe.

Example 8.12 Solution^{1/2}

Assume the flow to be incompressible with $\rho = 0.00238 \text{ slugs/ft}^3$ and $\mu = 3.74 \times 10^{-7} \text{ lb} \cdot \text{s/ft}^2$.

If the pipe were too long, the pressure drop from one end to the other, $p_1 - p_2$, would not be small relative to the pressure at the beginning, and compressible flow considerations would be required.

With $z_1 = z_2$, $V_1 = V_2$, The energy equation becomes $p_1 = p_2 + f \frac{\ell}{D} \frac{\rho V^2}{g}$

$$p_1 - p_2 = (0.5)(144) \text{ lb/ft}^2 = f \frac{(100 \text{ ft})}{D} (0.00238 \text{ slugs/ft}^3) \frac{V^2}{2g}$$

$$V = \frac{Q}{A} = \frac{2.55}{D^2}$$

$$\Rightarrow D = 0.404 f^{1/5} \quad (1)$$

Example 8.12 Solution^{2/2}

$$Re = \frac{\rho V D}{\mu} = \dots = \frac{1.62 \times 10^4}{D} \quad (2)$$

$$\frac{\varepsilon}{D} = \frac{0.0005}{D} \quad (3)$$

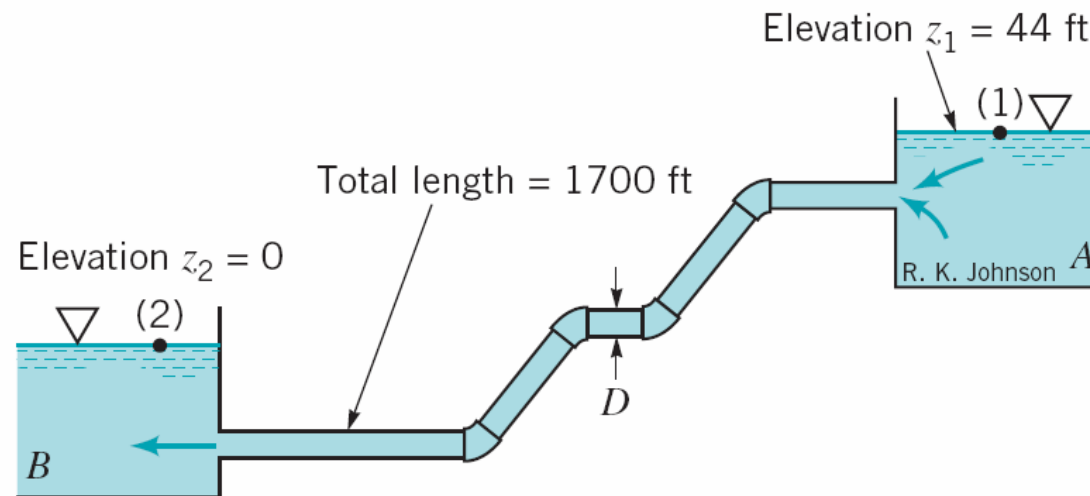
We have four equations (Eq. 1, 2, 3, and either the Moody chart or the Colebrook equation) and four unknowns (f , D , ε/D , and Re) from which the solution can be obtained by trial-and-error methods.

Repeat the iteration process

f D Re and ε/D f until convergence
(1) (2) (3)

Example 8.13 Type III With Minor Losses, Determine Diameter

- Water at 60°F ($\nu = 1.21 \times 10^{-5} \text{ ft}^2/\text{s}$) is to flow from reservoir A to reservoir B through a pipe of length 1700 ft and roughness 0.0005 ft at a rate of $Q = 26 \text{ ft}^3/\text{s}$ as shown in Figure E8.13. The system contains a sharp-edged entrance and four flanged 45° elbow. Determine the pipe diameter needed.



Example 8.13 Solution^{1/2}

The energy equation can be applied between two points on the surfaces of the reservoirs ($p_1 = V_1 = p_2 = z_2 = V_2 = 0$)

$$\frac{p_1}{\gamma} + \frac{V_1^2}{2g} + z_1 = \frac{p_2}{\gamma} + \frac{V_2^2}{2g} + z_2 + h_L$$

$$z_1 = \frac{V^2}{2g} \left(f \frac{\ell}{D} + \sum K_L \right)$$

$$V = \frac{Q}{A} = \frac{33.1}{D^2} \quad K_{Lent} = 0.5, K_{Lelbow} = 0.2, \text{ and } K_{Lexit} = 1$$

$$44\text{ft} = \frac{V^2}{2(32.2\text{ft/s}^2)} \left(f \frac{1700}{D} + [4(0.2) + 0.5 + 1] \right)$$

$$f = 0.00152D^5 - 0.00135D \quad (1)$$

Example 8.13 Solution^{2/2}

$$Re = \frac{VD}{\nu} = \dots = \frac{2.74 \times 10^6}{D} \quad (2)$$

$$\frac{\varepsilon}{D} = \frac{0.0005}{D} \quad (3)$$

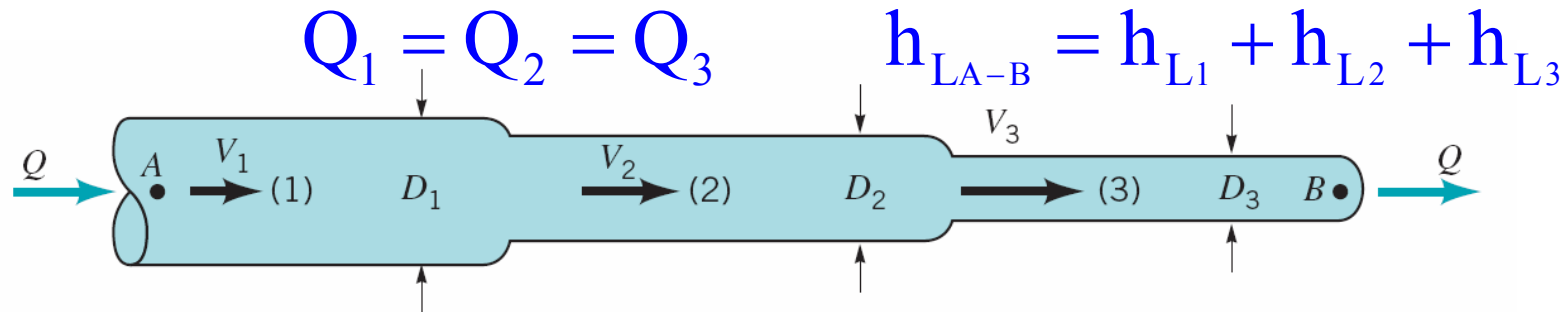
We have four equations (Eq. 1, 2, 3, and either the Moody chart or the Colebrook equation) and four unknowns (f , D , ε/D , and Re) from which the solution can be obtained by trial-and-error methods.

Repeat the iteration process

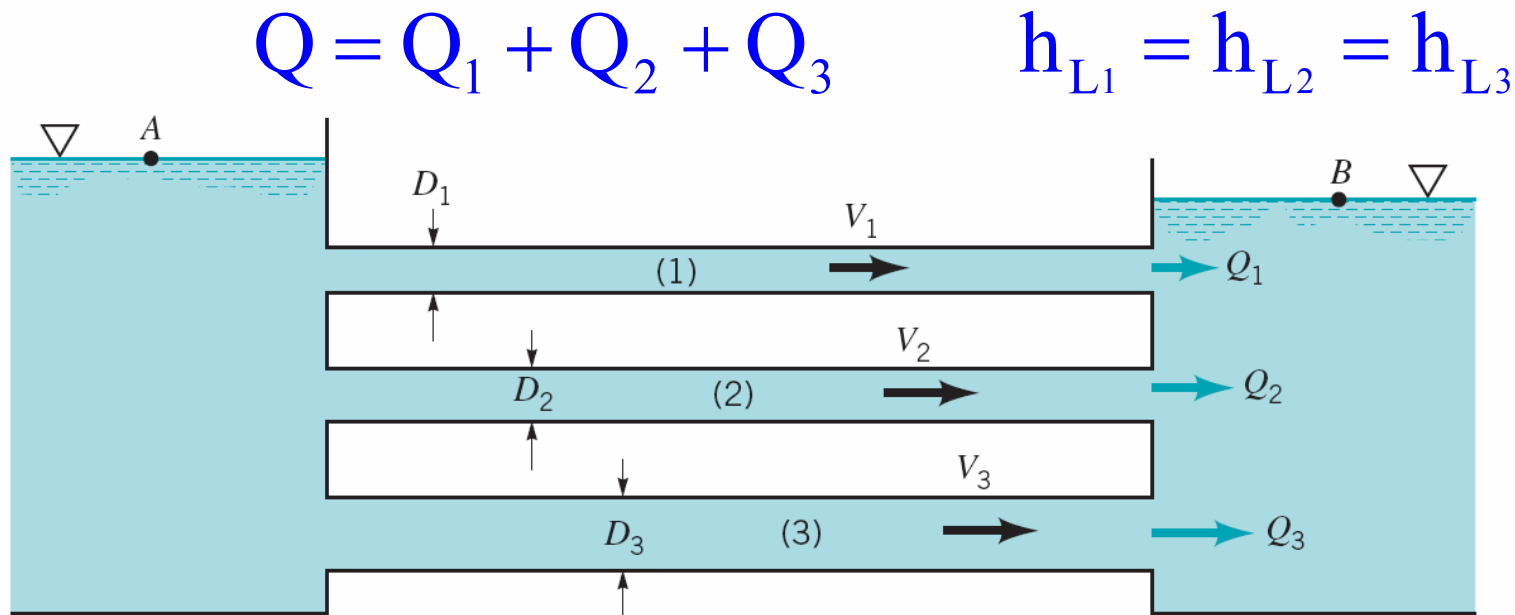
D f Re and ε/D f until convergence
(1) (2) (3)

Multiple-Path Systems

Series and Parallel Pipe System



(a)



(b)

Multiple-Path Systems

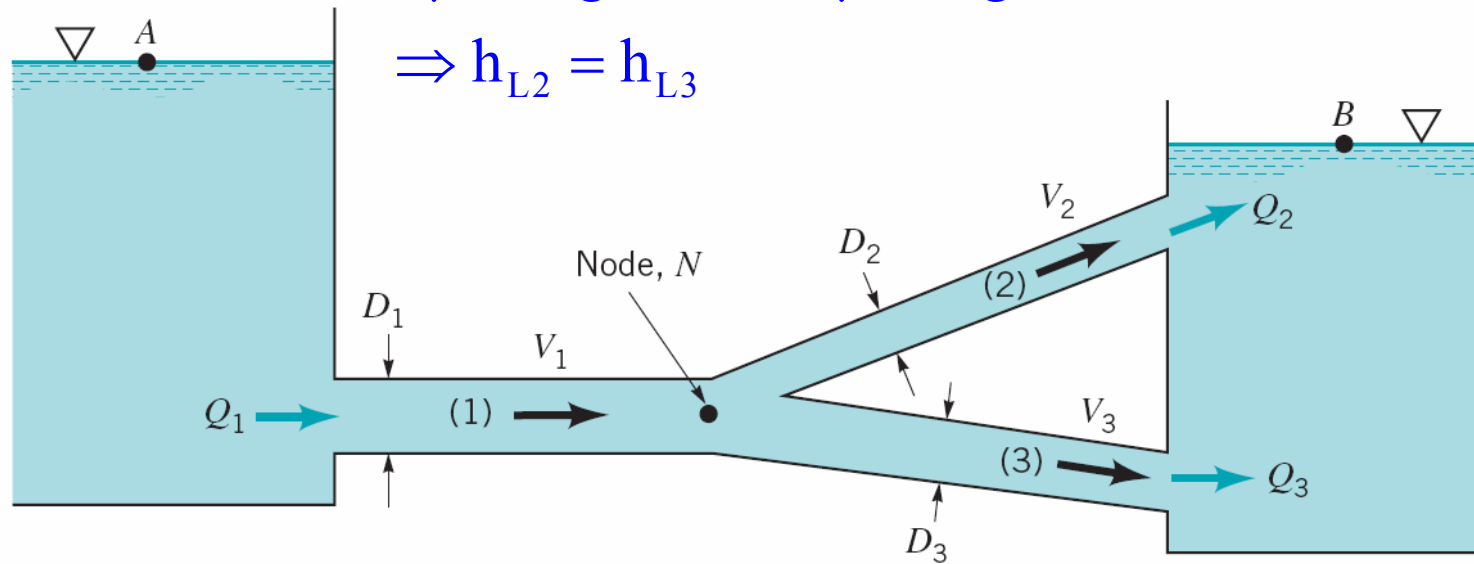
Multiple Pipe Loop System

$$Q_1 = Q_2 + Q_3$$

$$\frac{p_A}{\gamma} + \frac{V_A^2}{2g} + z_A = \frac{p_B}{\gamma} + \frac{V_B^2}{2g} + z_B + h_{L1} + h_{L2} (1 \rightarrow 2)$$

$$\frac{p_A}{\gamma} + \frac{V_A^2}{2g} + z_A = \frac{p_B}{\gamma} + \frac{V_B^2}{2g} + z_B + h_{L1} + h_{L3} (1 \rightarrow 3)$$

$$\Rightarrow h_{L2} = h_{L3}$$



Multiple-Path Systems

Three-Reservoir System

If valve (1) was closed, reservoir B \rightarrow reservoir C

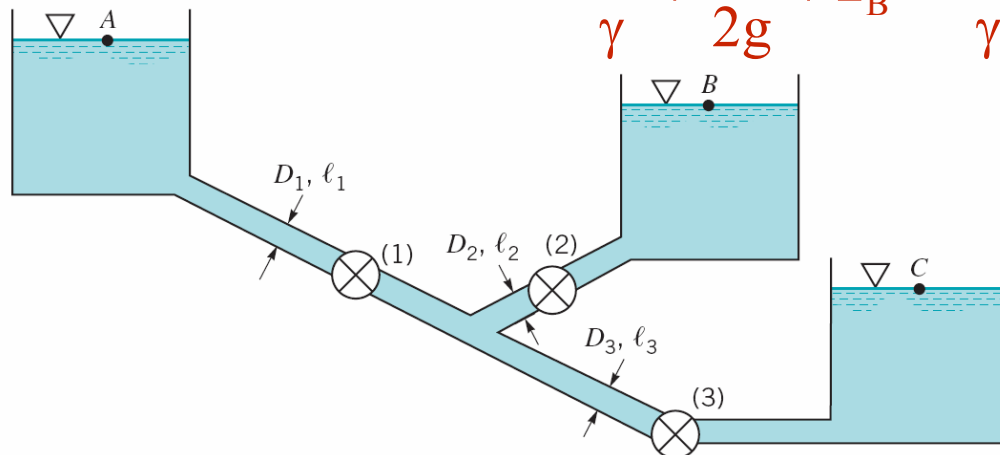
If valve (2) was closed, reservoir A \rightarrow reservoir C

If valve (3) was closed, reservoir A \rightarrow reservoir B

With all valves open... $Q_1 = Q_2 + Q_3$

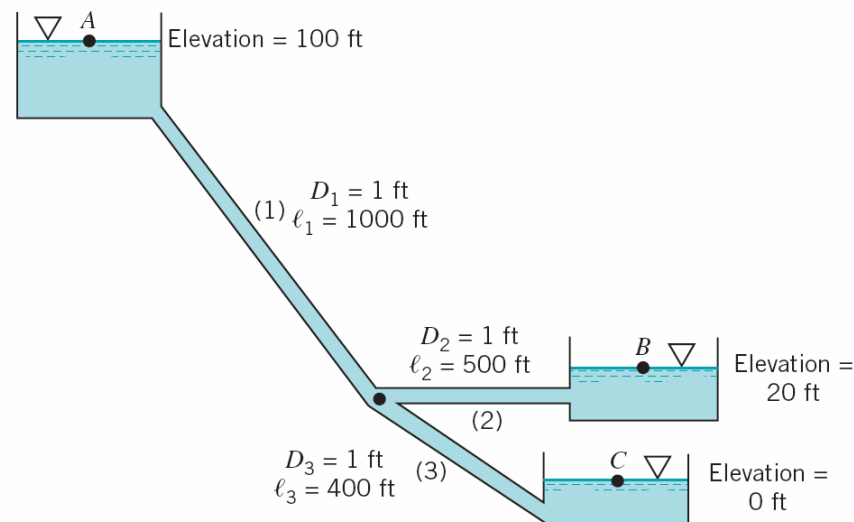
$$\frac{p_A}{\gamma} + \frac{V_A^2}{2g} + z_A = \frac{p_B}{\gamma} + \frac{V_B^2}{2g} + z_B + h_{L1} + h_{L2} (A \rightarrow B)$$

$$\frac{p_B}{\gamma} + \frac{V_B^2}{2g} + z_B = \frac{p_C}{\gamma} + \frac{V_C^2}{2g} + z_C + h_{L2} + h_{L3} (B \rightarrow C)$$



Example 8.14 Three reservoir, Multiple Pipe System

- Three reservoirs are connected by three pipes as are shown in Figure E8.14. For simplicity we assume that the diameter of each pipe is 1 ft, the frictional factor for each is 0.02, and because of the large length-to-diameter ratio, minor losses are negligible. Determine the flowrate into or out of each reservoir.



Example 8.14 Solution^{1/4}

The continuity equation requires that

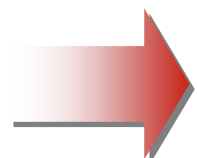
Flows out of reservoir B $Q_1 + Q_2 = Q_3 \Rightarrow V_1 + V_2 = V_3$ (1)

↑
The diameters are the same for each pipe

The energy equation for the fluid that flows from A to C in pipes (1) and (3) can be written as

$$\frac{p_A}{\gamma} + \frac{V_A^2}{2g} + z_A = \frac{p_C}{\gamma} + \frac{V_C^2}{2g} + z_C + f_1 \frac{\ell_1}{D_1} \frac{V_1^2}{2g} + f_3 \frac{\ell_3}{D_3} \frac{V_3^2}{2g}$$

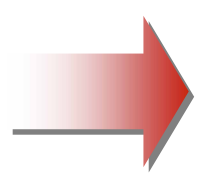
By using the fact that $p_A = p_C = p_C = V_A = V_C = z_C = 0$


$$z_A = f_1 \frac{\ell_1}{D_1} \frac{V_1^2}{2g} + f_3 \frac{\ell_3}{D_3} \frac{V_3^2}{2g} \quad 322 = V_1^2 + 0.4V_2^2 \quad (2)$$

Example 8.14 Solution^{2/4}

Similarly the energy equation for fluid following from B to C

$$\frac{p_B}{\gamma} + \frac{V_B^2}{2g} + z_B = \frac{p_C}{\gamma} + \frac{V_C^2}{2g} + z_C + f_2 \frac{\ell_2}{D_2} \frac{V_2^2}{2g} + f_3 \frac{\ell_3}{D_3} \frac{V_3^2}{2g}$$


$$z_B = f_2 \frac{\ell_2}{D_2} \frac{V_2^2}{2g} + f_3 \frac{\ell_3}{D_3} \frac{V_3^2}{2g} \quad 64.4 = 0.5V_2^2 + 0.4V_3^2 \quad (3)$$

No solution to Eqs. 1, 2, and 3 with real, positive values of V_1 , V_2 , and V_3 . **Thus, our original assumption of flow out of reservoir B must be incorrect.**

Example 8.14 Solution^{3/4}

The continuity equation requires that

**Flows into
reservoir B**

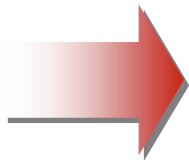
$$Q_1 = Q_2 + Q_3 \Rightarrow V_1 = V_2 + V_3 \quad (4)$$

The energy equation between points A and B and A and C

$$z_A = z_B + f_1 \frac{\ell_1}{D_1} \frac{V_1^2}{2g} + f_2 \frac{\ell_2}{D_2} \frac{V_2^2}{2g} \quad 258 = V_1^2 + 0.5V_2^2 \quad (5)$$

$$z_A = z_C + f_1 \frac{\ell_1}{D_1} \frac{V_1^2}{2g} + f_3 \frac{\ell_3}{D_3} \frac{V_3^2}{2g} \quad 322 = V_1^2 + 0.4V_3^2 \quad (6)$$

Solve



$$V_1 = 15.9 \text{ ft/s} \quad V_2 = 2.88 \text{ ft/s}$$

Example 8.14 Solution^{4/4}

The corresponding flowates are

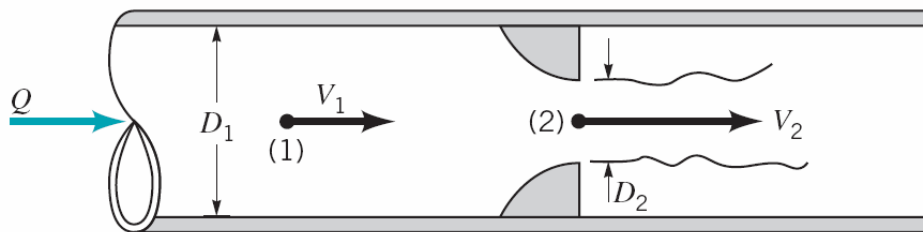
$$Q_1 = A_1 V_1 = 12.5 \text{ft}^3 / \text{s} \text{ from A}$$

$$Q_2 = A_2 V_2 = 2.26 \text{ft}^3 / \text{s} \text{ into B}$$

$$Q_3 = Q_1 - Q_2 = 10.2 \text{ft}^3 / \text{s} \text{ into C}$$

Pipe Flowrate Meters^{1/2}

- ❖ The theoretical flow rate may be related to the pressure differential between section 1 and 2 by applying the continuity and Bernoulli equations.
- ❖ Then empirical correction factors may be applied to obtain the actual flow rate.



Basic equation

$$\frac{\partial}{\partial t} \int_{CV} \rho dV + \int_{CS} \rho \vec{V} \cdot d\vec{A} = 0$$

$$\frac{p_1}{\rho} + \frac{V_1^2}{2} + gz_1 = \frac{p_2}{\rho} + \frac{V_2^2}{2} + gz_2$$



Pipe Flowrate Meters^{2/2}

$$p_1 - p_2 = \frac{\rho V_2^2}{2} \left[1 - \left(\frac{A_2}{A_1} \right)^2 \right] \quad V_2 = \sqrt{\frac{2(p_1 - p_2)}{\rho \left[1 - (A_2 / A_1)^2 \right]}}$$

┌ ──┐ **Theoretical mass flow rate** ──▶

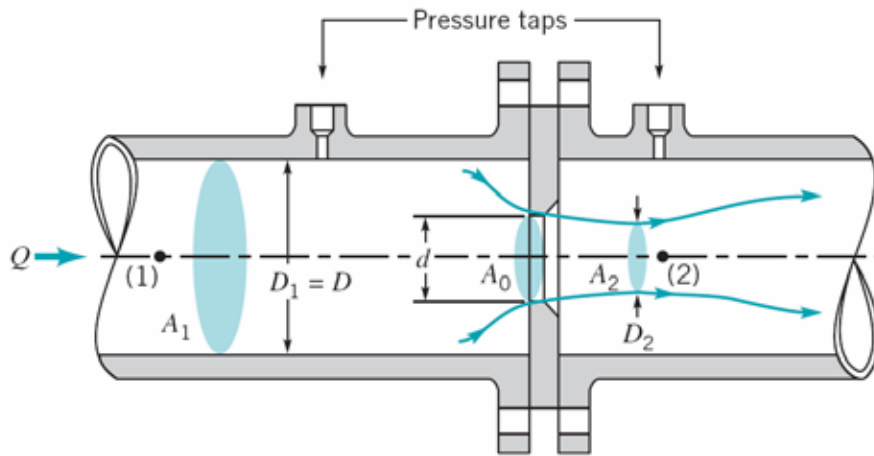
$$Q_{\text{ideal}} = V_2 A_2 = A_2 \sqrt{\frac{2(p_1 - p_2)}{\rho \left(1 - (D_2 / D_1)^4 \right)}}$$

➡ $Q_{\text{ideal}} \propto \sqrt{\Delta p}$

$Q_{\text{actual}} ??$

Pipe Flowrate Meters

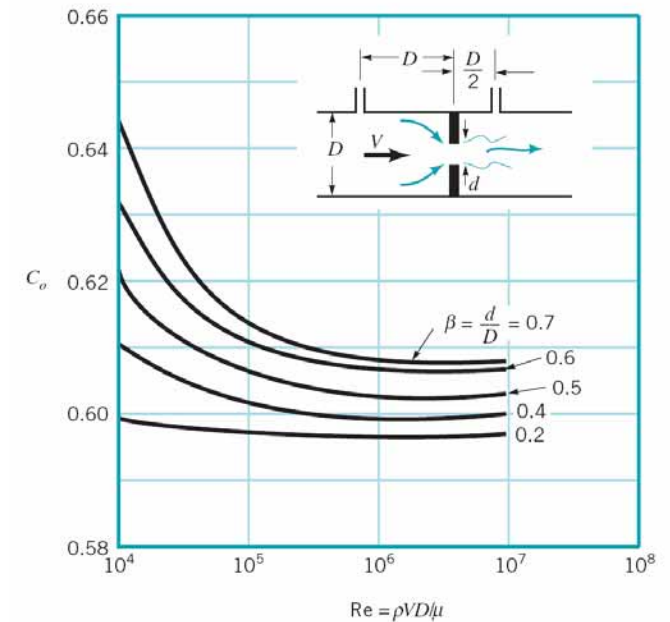
Orifice Meter



$$Q_{\text{actual}} = C_o Q_{\text{ideal}} = C_o A_o \sqrt{\frac{2(p_1 - p_2)}{\rho(1 - \beta^4)}}$$

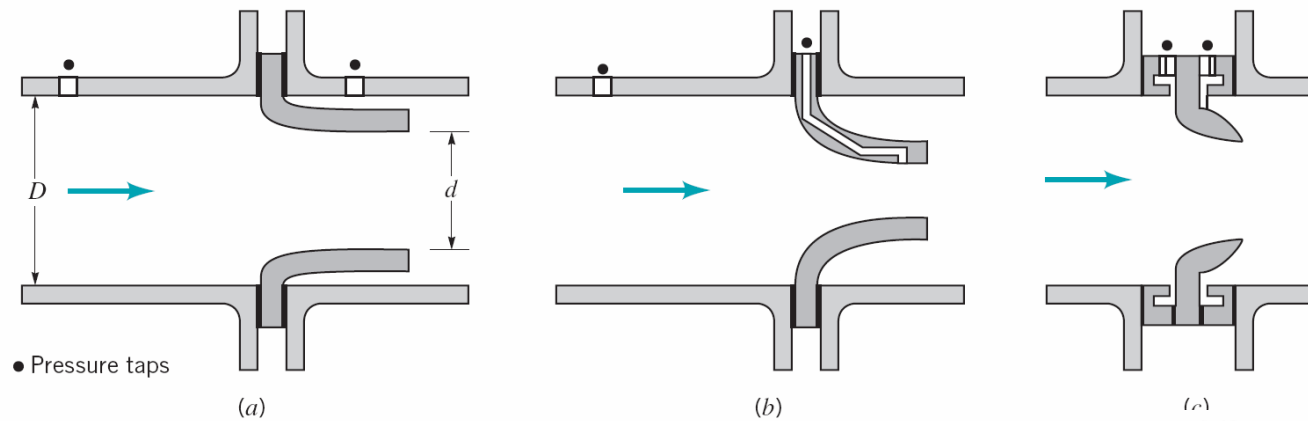
$A_o = \pi d^2 / 4$ Area of the hole in the orifice plate

$C_o = C_o(\beta = d / D, \text{Re} = \rho V D / \mu)$ Orifice meter discharge coefficient



Pipe Flowrate Meters

Nozzle Meter

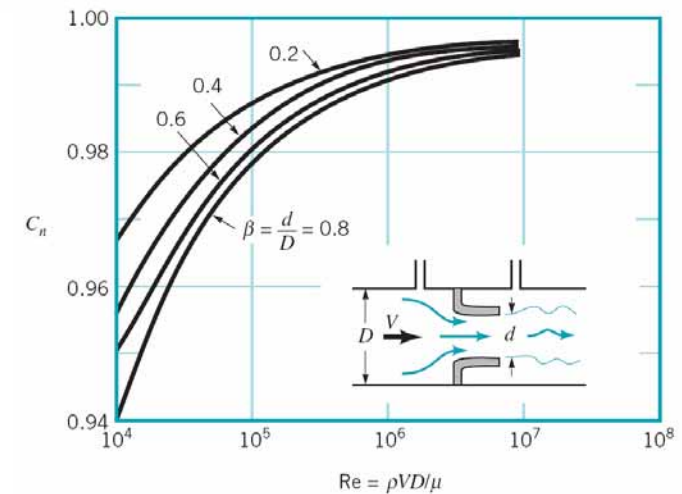


$$Q_{\text{actual}} = C_n Q_{\text{ideal}} = C_n A_n \sqrt{\frac{2(p_1 - p_2)}{\rho(1 - \beta^4)}}$$

$$A_n = \pi d^2 / 4 \quad \text{Area of the hole}$$

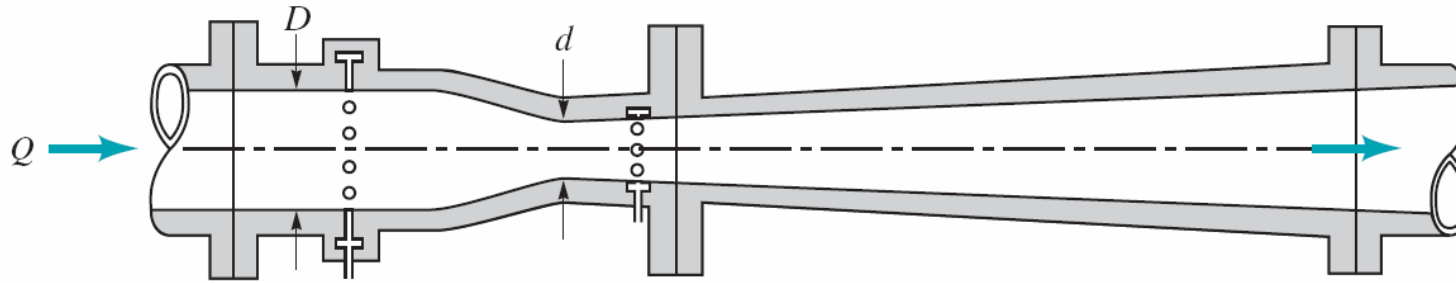
$$C_n = C_n(\beta = d / D, \text{Re} = \rho V D / \mu)$$

Nozzle meter discharge coefficient



Pipe Flowrate Meters

Venturi Meter

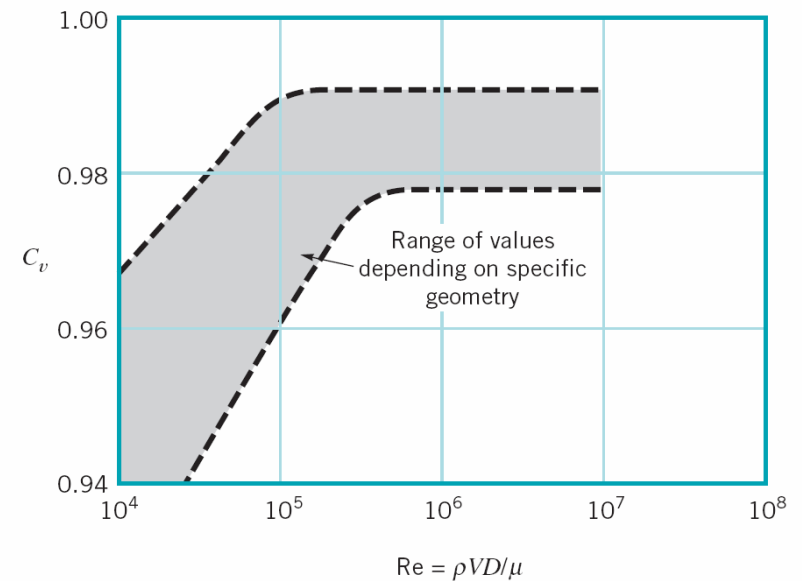


$$Q_{\text{actual}} = C_v Q_{\text{ideal}} = C_v A_T \sqrt{\frac{2(p_1 - p_2)}{\rho(1 - \beta^4)}}$$

$$A_T = \pi d^2 / 4 \quad \text{Area of the throat}$$

$$C_v = C_v(\beta = d / D, \text{Re} = \rho V D / \mu)$$

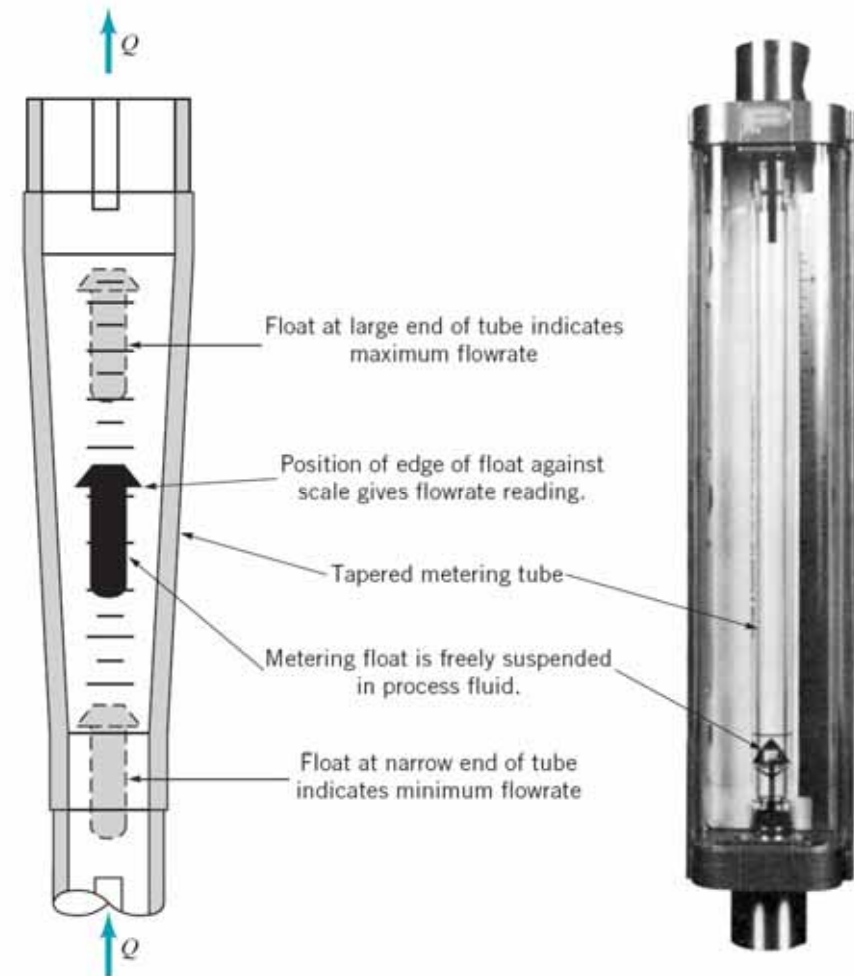
The Venturi discharge coefficient



Linear Flow Measurement

Float-type Variable-area Flow Meters

- ❖ The ball or float is carried upward in the tapered clear tube by the flowing fluid until the drag force and float weight are in equilibrium.
- ❖ Float meters often called rotameters.

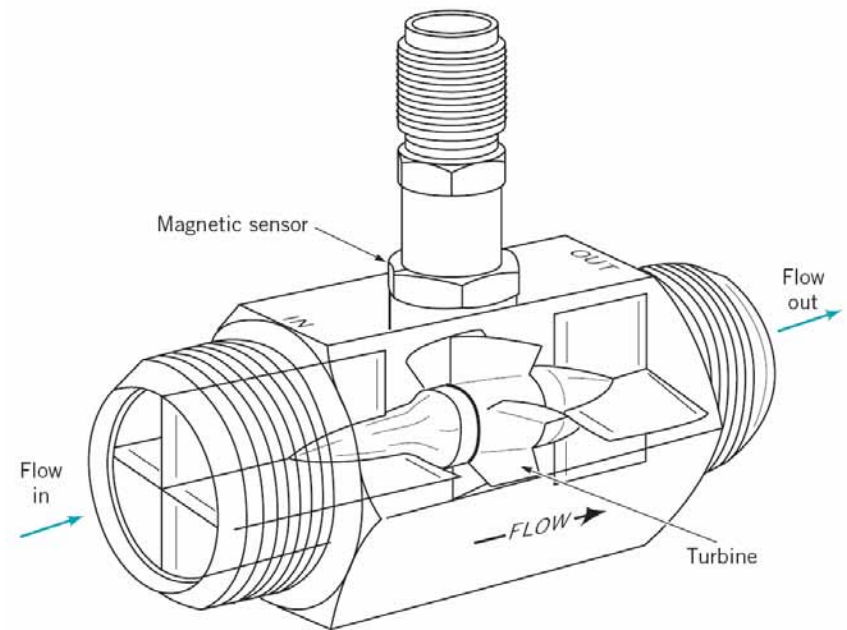


Rotameter-type flow meter

Linear Flow Measurement

Turbine Flow Meter

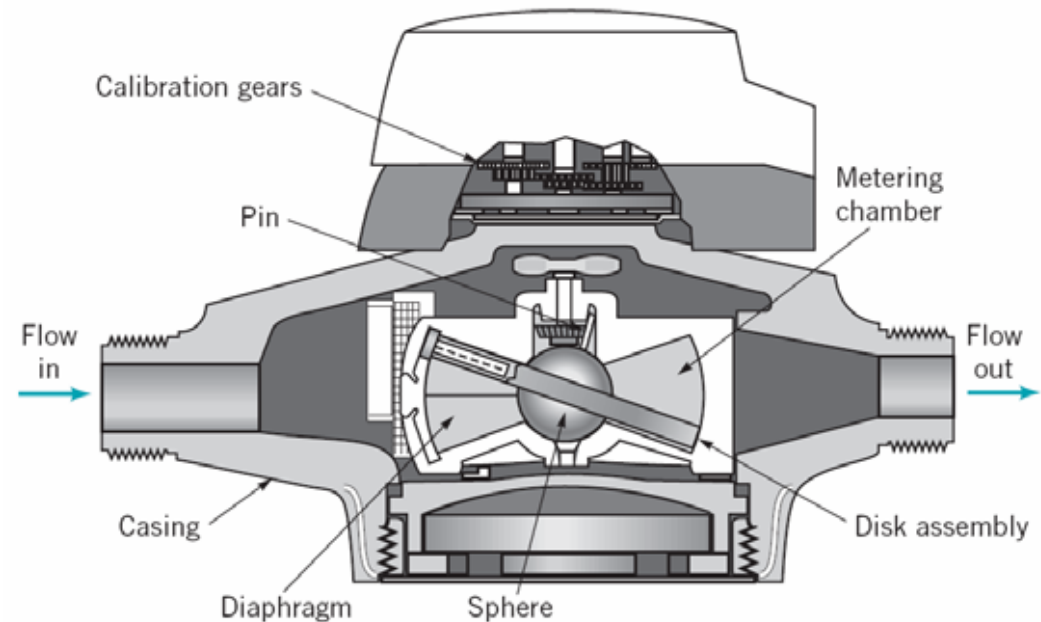
- ❖ A small, freely rotating propeller or turbine within the turbine meter rotates with an angular velocity that is function of the average fluid velocity in the pipe. This angular velocity is picked up magnetically and calibrated to provide a very accurate measure of the flowrate through the meter.



Turbine-type flow meter

Volume Flow Meters ^{1/2}

- ❖ Measuring the amount (volume or mass) of fluid that has passed through a pipe during a given time period, rather than the instantaneous flowrate.
- ❖ Nutating disk flow meters is widely used to measurement the net amount of water used in domestic and commercial water systems as well as the amount of gasoline delivered to your gas tank.



Nutating disk flow meter

Volume Flow Meters ^{2/2}

- ❖ Bellow-type flow meter is a quantity-measuring device used for gas flow measurement.
- ❖ It contains a set of bellows that alternately fill and empty as a result of the pressure of gas and the motion of a set of inlet and outlet valves.

Bellows-type flow meter.

- (a) Back case emptying, back diaphragm filling.
- (b) Front diaphragm filling, front case emptying.
- (c) Back case filling, back diaphragm emptying.
- (d) Front diaphragm emptying, front case filling.

