

Long-term Response Analysis of Floating Bridges

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Chained Floating Bridge

- Development project organized by Multiconsult
- PhD at NTNU
 - Supervisors
 - Bernt Leira – Dept. of Marine Technology
 - Ole Øiseth – Dept. of Structural Engineering
- So far
 - Stochastic description of wave loads
 - Development of a new method for faster computations
- Today
 - Long-term extreme response of floating bridges



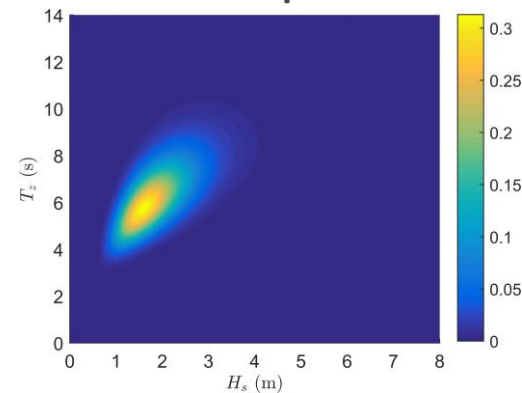
Short-term extreme value CDF

- Environmental parameters $\mathbf{S} = [H_s, T_z]$ considered constant for a short-term period \tilde{T} , typically $\tilde{T} = 3\text{h}$ for waves
- Given \mathbf{S} we assume a stationary response process $R(t)$
 - $\tilde{R}|\mathbf{S} = \max\{R(t)|\mathbf{S}; 0 \leq t \leq \tilde{T}\}$
- Assuming independent upcrossings of high levels r
 - $F_{\tilde{R}|\mathbf{S}}(r|\mathbf{s}) = \exp\{-\nu_R^+(r)\tilde{T}\}$
- Also assuming Gaussian response process
 - $F_{\tilde{R}|\mathbf{S}}(r|\mathbf{s}) = \exp\left\{-\frac{\sigma_{\dot{R}}}{2\pi\sigma_R} \exp\left(-\frac{r^2}{2\sigma_R^2}\right)\right\}$



Long-term extreme response modelling

- Sequence of N short-term states, each of duration \tilde{T}
- Joint distribution $f_S(\mathbf{s})$ for the environmental parameters
 - $H_S \sim \ln \mathcal{N}(\lambda_{H_S}, \xi_{H_S}^2)$
 - $T_Z | H_S \sim \ln \mathcal{N}(\lambda_{T_Z}(h), \xi_{T_Z}^2)$
 - $f_{H_S, T_Z}(h, t) = f_{H_S}(h) f_{T_Z | H_S}(t | h)$
- \hat{R} – largest response during the long-term period $T = N\tilde{T}$
- \tilde{R} – largest response in a randomly chosen short-term state
 - $F_{\hat{R}}(r) = F_{\tilde{R}}(r)^N$



Long-term CDF of the short-term extreme value

- Exact long-term CDF obtained by ergodic average
 - $F_{\tilde{R}}(r) = \exp\left\{\int_{\mathcal{S}} (\ln F_{\tilde{R}|\mathcal{S}}(r|\mathbf{s})) f_{\mathcal{S}}(\mathbf{s}) d\mathbf{s}\right\}$
- Common approximation using the population mean
 - $F_{\tilde{R}}(r) \approx \bar{F}_{\tilde{R}}(r) = \int_{\mathcal{S}} F_{\tilde{R}|\mathcal{S}}(r|\mathbf{s}) f_{\mathcal{S}}(\mathbf{s}) d\mathbf{s}$
- This integral can be solved approximately by using FORM
 - $$\begin{aligned}\bar{F}_{\tilde{R}}(r) &= 1 - \int_{g_r(\mathbf{u}) \leq 0} f_U(\mathbf{u}) d\mathbf{u} \\ &\approx 1 - \Phi(-\beta)\end{aligned}$$

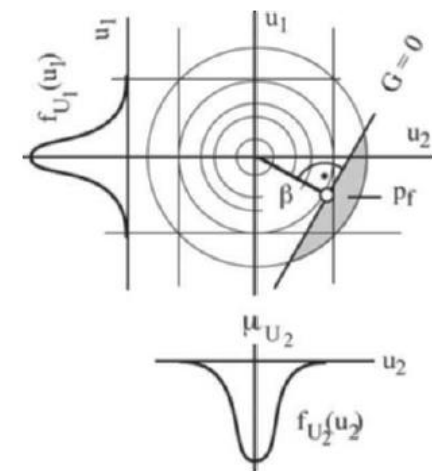


Figure from Schneider 1997,
Introduction to Safety and
Reliability of structures

Forward FORM and inverse FORM

- Forward FORM
 - Calculate $\bar{F}_{\tilde{R}}(r) \approx 1 - \Phi(-\beta)$ for a given value of r
 - β found by solving a minimization problem
 - Iteration required for each value of r
- Inverse FORM
 - Calculate r_p such that $\bar{F}_{\tilde{R}}(r_p) \approx 1 - p$
 - $\beta = -\Phi^{-1}(p)$ is given
 - r_p found by solving a maximization problem
 - Iteration to find r_p directly



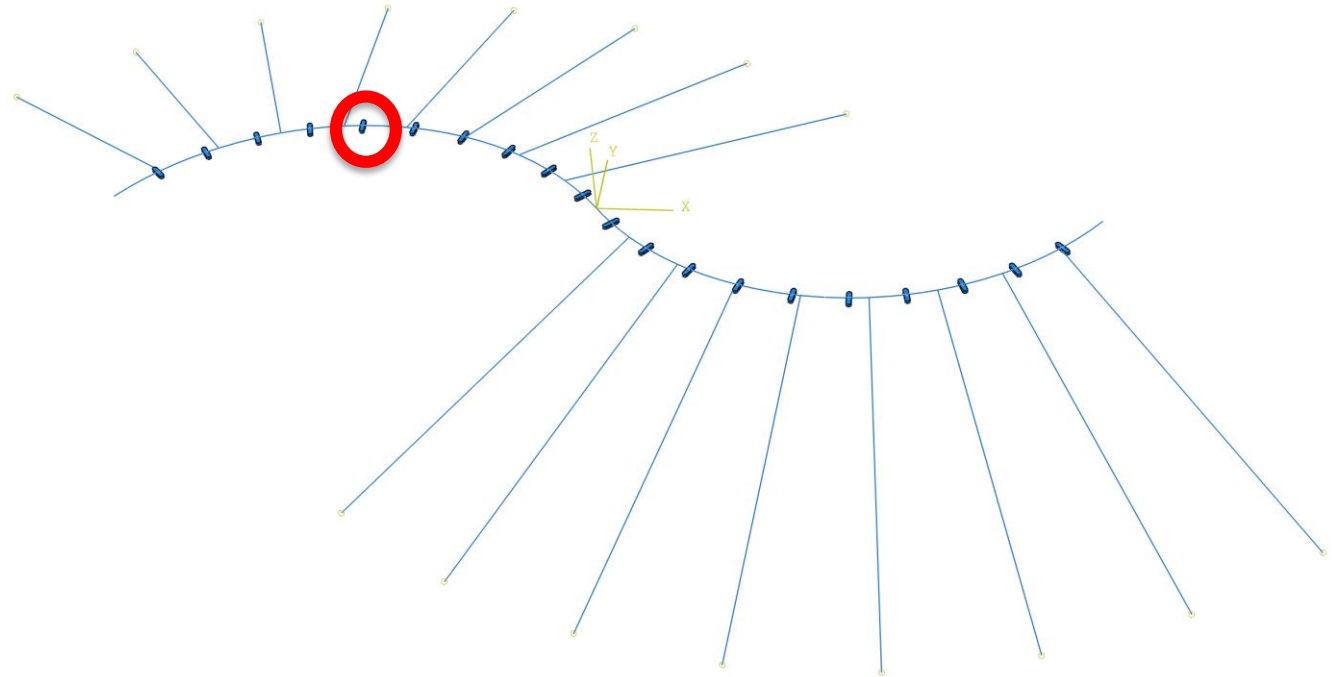
Response value with 100-yr return period

- Seek the value r_q which has a probability q of being exceeded per long-term period
 - Require $\bar{F}_{\tilde{R}}(r_q) = (1 - q)^{1/N} \approx 1 - q/N$
- Response value with 100-yr return period
 - $T = 1 \text{ yr}$, $\tilde{T} = 3 \text{ hr}$, $N = 365 \cdot 8 = 2920$ and $q = 1/100$.
 - $\bar{F}_{\tilde{R}}(r_q) = 1 - 1/292000$
 - $p = 1/292000 \Rightarrow \beta = -\Phi^{-1}(p) = 4.5$



Response value with 100-yr return period

- Simplified linear model of the chained floating bridge
- $R(t)$ – Horizontal transverse displacement of pontoon 5
- Not realistic environmental model



Response value with 100-yr return period

- Full numerical integration
 - $r_q = 13.64$ m
 - 7350 short-term response calculations
- IFORM
 - $r_q = 13.83$ m
 - 32 short-term response calculations



Thank you!

