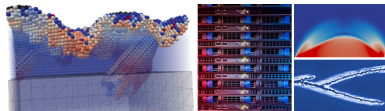


# Absorbing Boundary Layers in Time Domain Elastodynamics Stability Analysis

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- 1 Introduction
  - Absorbing boundaries
  - Perfectly matched layers
- 2 Formulation
  - Propagation of Elastic Waves in Solids
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# Absorbing boundaries

- Common practice to solve numerically wave propagation on unbounded domains.
- Important topic for many research and engineering applications.
- Simulation of earthquake ground motion, for soil-structure, geophysical, subsurface sensing, waveguides problems.
- 2 kinds of method :
  - Absorbing boundary conditions : specific conditions at the model boundaries.
  - Absorbing layers (e.g. Perfectly Matched Layer (PML))

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# Perfectly matched layers

- split-field PML :

- Introduced by Bérenger in the context of electromagnetics.
- Extended by Hastings et al to elastodynamics.
- Use on complex-valued coordinate stretching.
- To avoid convolutional operations in the time domain.
- Field splitting : partition of the variables into two components parallel and perpendicular to the truncation boundary.

- unsplit-field PML :

- Introduced by Wang in the context of elastodynamics (CPML) → Complexity
- Basu and Chopra : unsplit-field PML for time-harmonic elastodynamics.  
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# Stability of PML in the literature

- split-field PML :
  - Stability analysis using Slowness diagrams and wave fronts.
  - Definitions of sufficient and necessary conditions of stability [?].
  - First order finite difference discretization : Prone to instability in anisotropic media.
  - Second order discretization : stable [?]
- unsplit-field PML :
  - Stability proved for electromagnetism (Maxwell's equations).
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# Equations

- System of equations :

$$\begin{cases} \sum_j \frac{\partial \sigma_{ij}}{\partial x_j} = \rho \frac{\partial^2 u_i}{\partial t^2} \\ \sigma_{ij} = \sum_{k,l} C_{ijkl} \epsilon_{kl} \\ \epsilon_{ij} = \frac{1}{2} \left( \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) \end{cases} \quad (1)$$

- Complex coordinates :  $x_i \rightarrow \tilde{x}_i : \mathbb{R} \rightarrow \mathbb{C}$

$$\frac{\partial \tilde{x}_i}{\partial x_i} = \lambda_i(x_i) = 1 + f_i^e(x_i) - i \frac{f_i^p(x_i)}{b k_s} \quad (2)$$

$b$  : characteristic length of the physical problem.

$k_s = \frac{\omega}{c_s}$  : wavenumber.

$c_s$  : shear wave velocity.

- System of equations in frequency domain :

$$\begin{cases} \sum_j \frac{1}{\lambda_j(x_j)} \frac{\partial \sigma_{ij}}{\partial x_j} = -\rho \omega^2 u_j \\ \sigma_{ij} = \sum_{k,l} C_{ijkl} \epsilon_{ij} \\ \epsilon_{ij} = \frac{1}{2} \left( \frac{1}{\lambda_j(x_j)} \frac{\partial u_i}{\partial x_j} + \frac{1}{\lambda_i(x_i)} \frac{\partial u_j}{\partial x_i} \right) \end{cases} \quad (3)$$

# Strong Form of PML

## • Inverse Fourier Transform :

$$\left\{ \begin{array}{ll} \operatorname{div}(\underline{\underline{\sigma}}\tilde{F}^e + \underline{\underline{\Sigma}}\tilde{F}^p) = \rho f_m \ddot{\underline{u}} + \rho \frac{c_s}{b} f_c \dot{\underline{u}} + \frac{\mu}{b^2} f_k \underline{u}, & \text{In } \Omega_{PML} \times J \\ \underline{\underline{\sigma}} = \underline{C} : \underline{\underline{\epsilon}}, & \text{In } \Omega_{PML} \times J \\ F^{eT} \underline{\underline{\dot{\epsilon}}} F^e + F^{pT} \underline{\underline{\epsilon}} F^e + F^{eT} \underline{\underline{\epsilon}} F^p + F^{pT} \underline{\underline{E}} F^p = ... \\ \frac{1}{2}(\nabla \dot{\underline{u}}^T F^e + F^{eT} \nabla \dot{\underline{u}}) + \frac{1}{2}(\nabla \underline{u}^T F^p + F^{pT} \nabla \underline{u}), & \text{In } \Omega_{PML} \times J \end{array} \right. \quad (4)$$

With homogeneous initial conditions :

$$\left\{ \begin{array}{ll} \underline{u} = 0, & \text{on } \Gamma_{PML}^D \\ (\underline{\underline{\sigma}}\tilde{F}^e + \underline{\underline{\Sigma}}\tilde{F}^p) \cdot \underline{n}, & \text{on } \Gamma_{PML}^N \end{array} \right. \quad (5)$$



# Descriptions of the components

$$\begin{aligned}
 F^e &= \begin{bmatrix} 1 + f_1^e(x_1) & 0 \\ 0 & 1 + f_2^e(x_2) \end{bmatrix}, F^p = \begin{bmatrix} \frac{c_s}{b} f_1^p(x_1) & 0 \\ 0 & \frac{c_s}{b} f_2^p(x_2) \end{bmatrix} \\
 \tilde{F}^e &= \begin{bmatrix} 1 + f_2^e(x_2) & 0 \\ 0 & 1 + f_1^e(x_1) \end{bmatrix}, \tilde{F}^p = \begin{bmatrix} \frac{c_s}{b} f_2^p(x_2) & 0 \\ 0 & \frac{c_s}{b} f_1^p(x_1) \end{bmatrix} \\
 \begin{cases} f_m = (1 + f_1^e(x_1))(1 + f_2^e(x_2)) \\ f_c = (1 + f_1^e(x_1))f_2^p(x_2) + (1 + f_2^e(x_2))f_1^p(x_1) \\ f_k = f_1^p(x_1)f_2^p(x_2) \end{cases}
 \end{aligned}$$

Integral of stress and strain :

$$\underline{\underline{\Sigma}} = \int_0^t \underline{\underline{\sigma}} dt, \underline{\underline{E}} = \int_0^t \underline{\underline{\epsilon}} dt$$

Attenuation function :

$$f_i^\alpha = a_\alpha \left( \frac{x_i - x_0}{L_p} \right)^n$$

# Discrete form of PML

After :

- Multiplying 5 test functions  $v$  belonging to an appropriate space.
- Integrating over the computational domain.
- Discretization in time and space.

$$M\ddot{U}_{n+1} + (C + \tilde{C})\dot{U}_{n+1} + (K + \tilde{K})U_{n+1} + P(\epsilon_n, E_n, \Sigma_n) = F_{ext} \quad (6)$$

with

$$m^e = \int_{\Omega_e} \rho f_m N_I N_J d\Omega_e I_d$$

$$\tilde{c}^e = \frac{1}{dt} \int_{\Omega_e} \tilde{B}^T D B^\epsilon d\Omega_e$$

$$c^e = \int_{\Omega_e} \rho f_c \frac{c_s}{b} N_I N_J d\Omega_e I_d$$

$$\tilde{k}^e = \frac{1}{dt} \int_{\Omega_e} \tilde{B}^T D B^Q d\Omega_e$$

$$k^e = \int_{\Omega_e} \frac{\mu}{b^2} f_k N_I N_J d\Omega_e I_d$$

# Internal Forces

The last point is the Internal Forces term  $P(\epsilon_n, E_n, \Sigma_n)$

$$P^e(\epsilon_n, E_n, \Sigma_n) = \int_{\Omega_e} \tilde{B}^T \frac{D}{dt} \left[ \frac{1}{dt} \hat{F}^\epsilon \hat{\epsilon} - \hat{F}^Q \hat{E}_n \right] + \tilde{B}^P \hat{\Sigma}_n d\Omega_e \quad (7)$$

with

$$\tilde{B}_I^e = \begin{bmatrix} \tilde{N}_{I1}^e & 0 \\ 0 & \tilde{N}_{I2}^e \\ \tilde{N}_{I2}^e & \tilde{N}_{I1}^e \end{bmatrix}, \tilde{B}_I^p = \begin{bmatrix} \tilde{N}_{I1}^p & 0 \\ 0 & \tilde{N}_{I2}^p \\ \tilde{N}_{I2}^p & \tilde{N}_{I1}^p \end{bmatrix}$$

$$\tilde{N}_{ii}^e = \tilde{F}_{ji}^e N_{I,j}, \tilde{N}_{ii}^p = \tilde{F}_{ji}^p N_{I,j}$$

And

$$\tilde{B}^T = \tilde{B}^{eT} + dt \tilde{B}^{pT}$$

$$B_I^\epsilon = \begin{bmatrix} F_{11}^\epsilon N_{I1}' & F_{21}^\epsilon N_{I1}' \\ F_{12}^\epsilon N_{I2}' & F_{22}^\epsilon N_{I2}' \\ F_{11}^\epsilon N_{I2}' + F_{12}^\epsilon N_{I1}' & F_{21}^\epsilon N_{I2}' + F_{22}^\epsilon N_{I1}' \end{bmatrix}$$

following

with

$$F^I = \left[ F^P + \frac{F^e}{dt} \right]^{-1}, F^\epsilon = F^e F^I, F^Q = F^P F^I$$

and

$$N_{li}^I = F_{ij}^I N_{lj}$$

and

$$\hat{F}_I^\epsilon = \begin{bmatrix} (F_{11}^\epsilon)^2 & (F_{21}^\epsilon)^2 & F_{11}^\epsilon F_{21}^\epsilon \\ (F_{12}^\epsilon)^2 & (F_{22}^\epsilon)^2 & F_{12}^\epsilon F_{22}^\epsilon \\ 2F_{11}^\epsilon F_{12}^\epsilon & 2F_{21}^\epsilon F_{22}^\epsilon & F_{11}^\epsilon F_{22}^\epsilon + F_{12}^\epsilon F_{21}^\epsilon \end{bmatrix}$$

# Principle

- Stable direct integration scheme :

$$\exists h_0 > 0 \quad \text{such as} \quad \forall h \in [0, h_0]$$

a finite perturbation of the state vector at  $t_n$  gives a non increasing variation of the state vector at a subsequent time  $t_{n+j}$ .

- Initial disturbance :

$$\delta X_0 = X'_0 - X_0 \quad (8)$$

- The non-perturbed solution :

$$X_{n+1} = HX_n + g_{n+1} \quad (9)$$

$$= H^2X_{n-1} + Hg_n + g_{n+1} \quad (10)$$

$$\vdots \quad (11)$$

$$= H^{n+1}X_0 + \sum_{j=0}^{n+1} H^{n-j+1}g_j \quad (12)$$

- Perturbed solution :

$$X'_{n+1} = H^{n+1}X'_0 + \sum_{j=0}^{n+1} H^{n-j+1}g_j \quad (13)$$

- Effect of initial disturbance at time  $t_{n+1}$  :

$$\delta X_{n+1} = H^{n+1}\delta X_0 \quad (14)$$

- Associated eigenvalue problem :

$$\det(H - \lambda I) = 0 \quad (15)$$

- $\lambda_r, x_{(r)}$  associated eigenvalues and eigenvectors.

- Modal expansion :

$$\delta X_0 = \sum_{s=1}^{2N} a_s X_{(s)} \quad (16)$$

- Transform the recurrence relation to :

$$\delta X_{n+1} = H^{n+1} \sum_{s=1}^{2N} a_s X_{(s)} \quad (17)$$

$$= \sum_{s=1}^{2N} a_s \lambda_s^{n+1} X_{(s)} \quad (18)$$

- Disturbance will be amplified if eigenvalues are higher than unity.

- Equation of motion at time  $t_n$  and  $t_{n+1}$  :

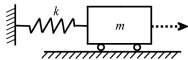
$$\begin{cases} M\ddot{U}^n = -C\dot{U}^n - KU^n + P_{int}^n \\ M\ddot{U}^{n+1} = -C\dot{U}^{n+1} - KU^{n+1} + P_{int}^{n+1} \end{cases} \quad (19)$$

- And the recurrence relationships by Newmark method (it could be another method) :

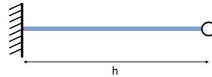
$$\begin{cases} \dot{U}_{n+1} = \dot{U}_n + (1 - \gamma)dt\ddot{U}_n + \gamma dt\ddot{U}_{n+1} \\ U_{n+1} = U_n + dt\dot{U}_n + dt^2 \left(\frac{1}{2} - \beta\right) \ddot{U}_n + dt^2 \beta \ddot{U}_{n+1} \end{cases} \quad (20)$$



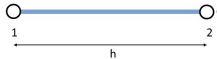
# 1D : four cases



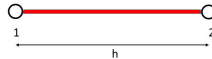
Single degree of freedom spring



Fixed extremity 1D bar element



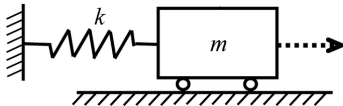
1D bar element



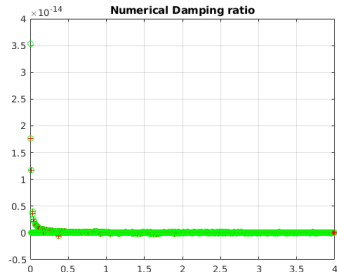
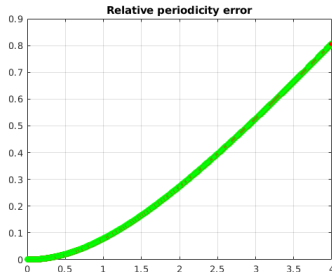
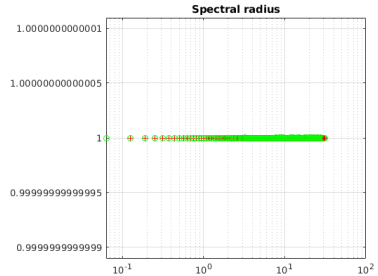
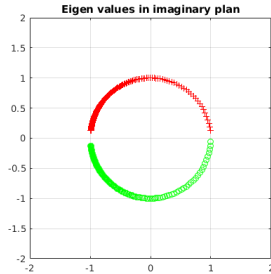
1D PML bar element

# SDOF spring

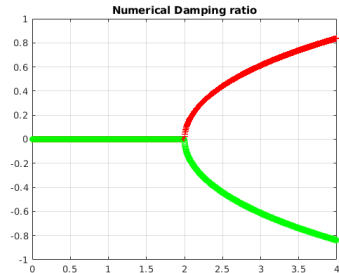
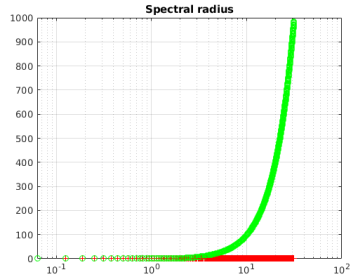
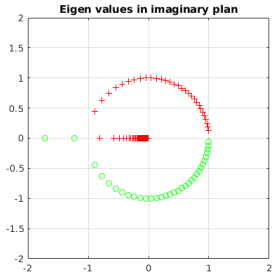
- Single degree of freedom spring :



# SDOF spring : Implicit

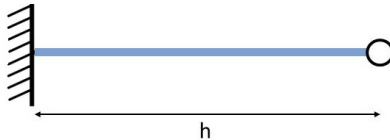


# SDOF spring : Explicit

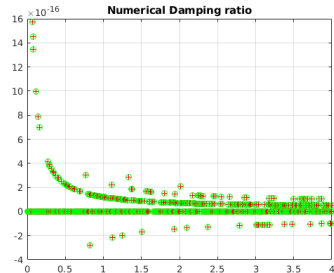
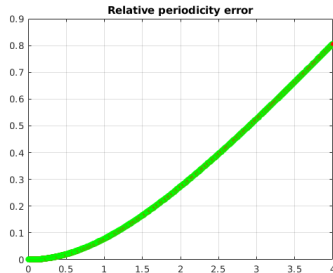
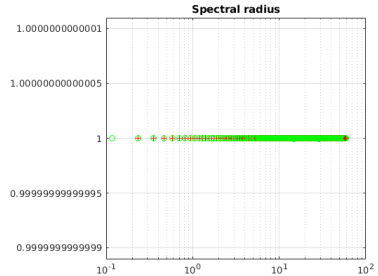
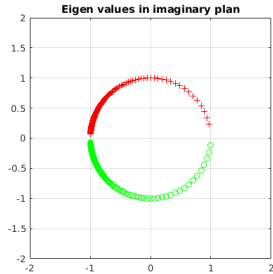


# Encastred bar element

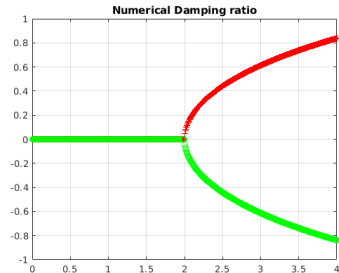
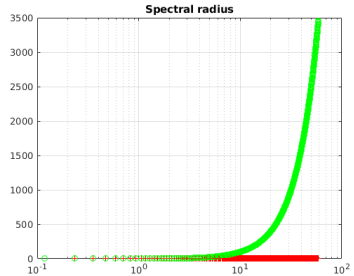
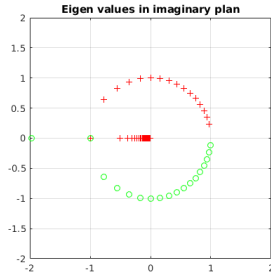
- Encastred bar element :



# 1D encastred element : Implicit

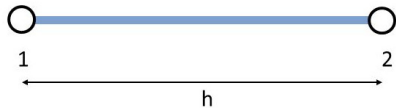


# 1D Encastred element : Explicit



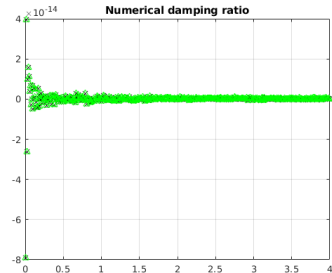
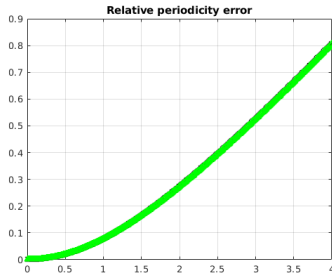
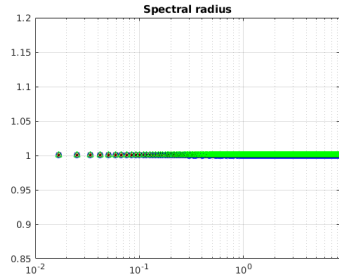
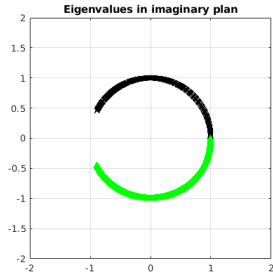
# 1D bar element

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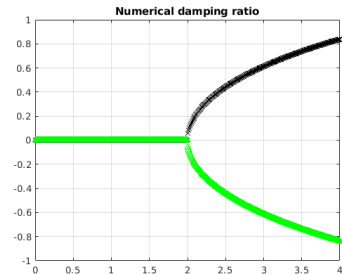
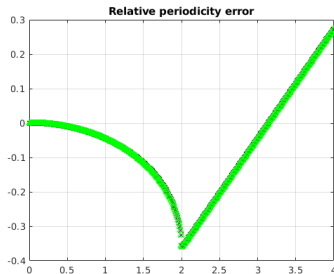
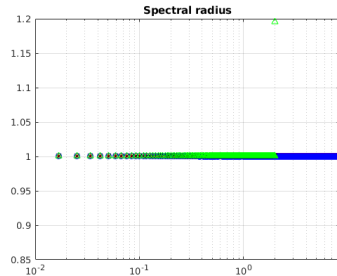
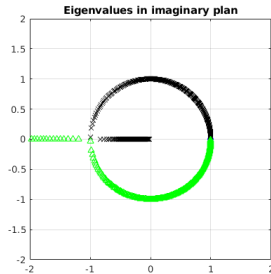




# 1D bar element : Implicit

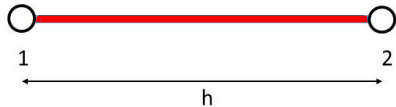


# 1D bar element : Explicit

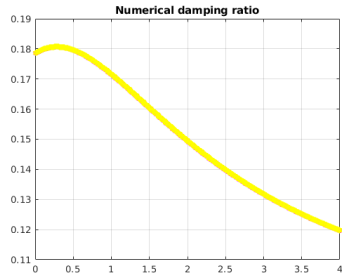
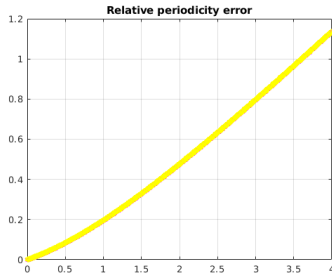
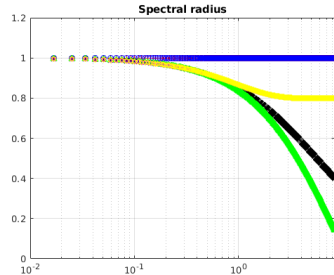
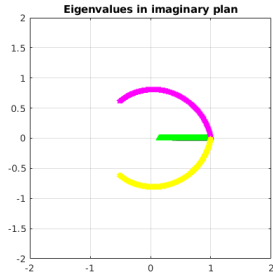


# 1D PML bar element

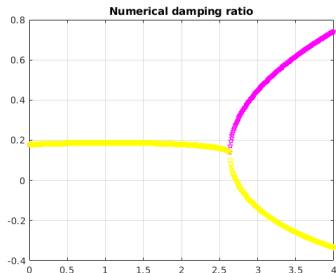
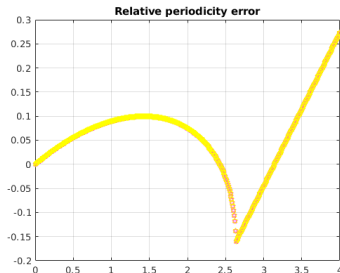
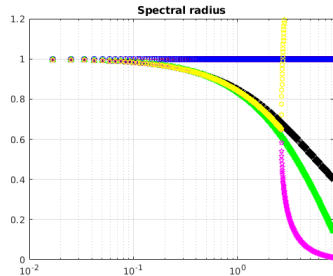
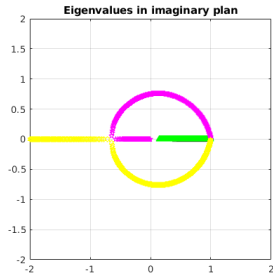
- 1D PML bar element :



# 1D PML bar element : Implicit

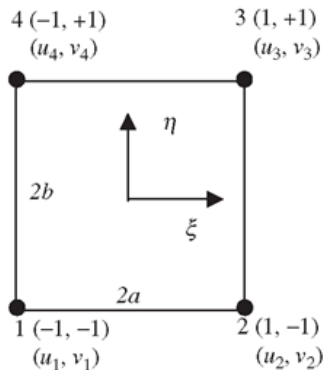


# 1D PML bar element : Explicit

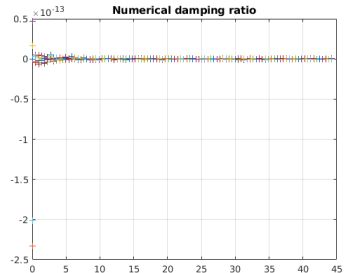
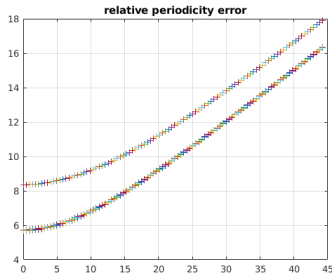
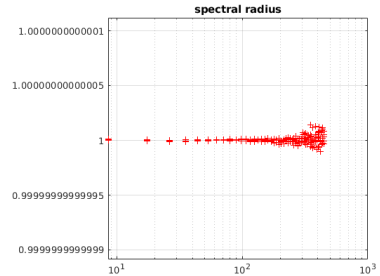
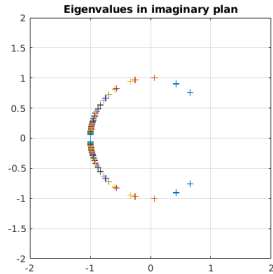


## 2D element stability

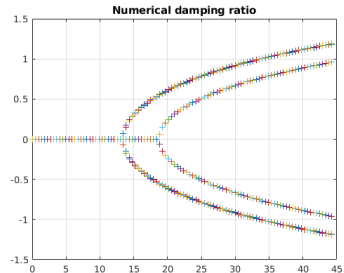
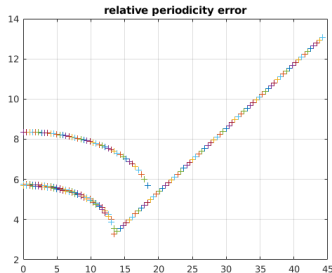
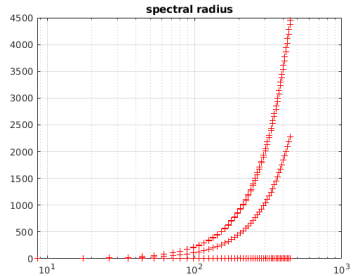
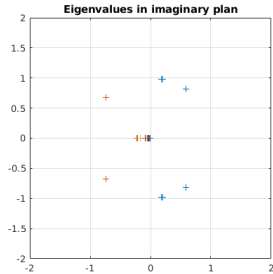
- 2D linear 4-noded element :



# 2D element : Implicit

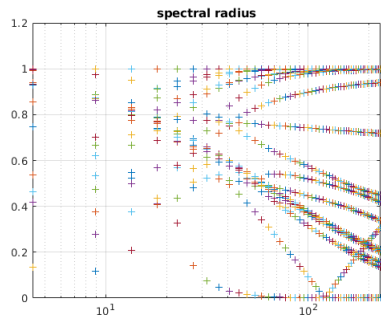
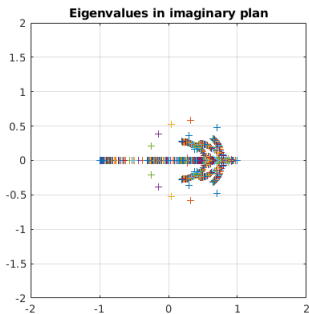


# 2D element : Explicit

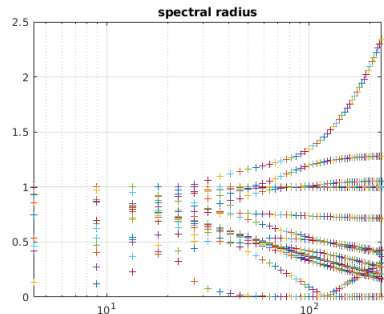
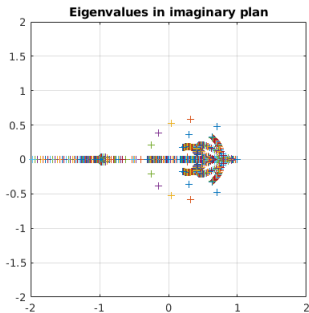




# 2D element : Implicit



# 2D element : Explicit



# Conclusion

- Overview of the analysis of stability for integration method.
- Stability of 1D and 2D perfectly matched layer (for implicit time integration method).
- Properties of attenuation and delay of unstability for 1D PML.
- Further work :
  - Same analysis of relative periodicity error and numerical damping for 2D PML.
  - Implementation of 2D Perfectly matched layer within Akantu.