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# Discrete model of circular plate

According to given initial conditions structure is circle width some amount of distributed load in center. Due to the symmetry of our structure, has to be studied only half of structure(fig.1).

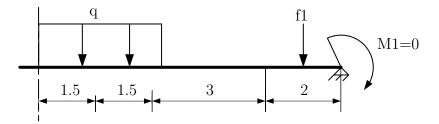


Figure 1: Schematic representation for half of structure

Developed discrete model of given structure consist 4 finite elements(fig.2), local displacements and moments of each element shown on figure 3, while global ones are on figure 4.

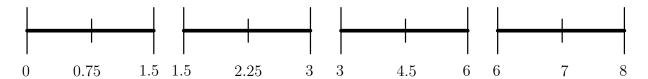


Figure 2: Coordinate scheme o finite elements

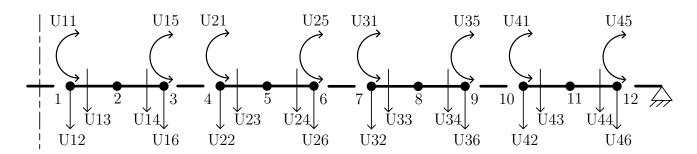


Figure 3: Local displacements and moments of each element

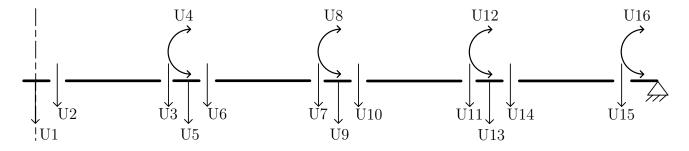


Figure 4: Global displacements and moments of each element

# Compatibility matrix of displacements

It's based on the discrete model. It represents a relationship among the local and global displacements. We used 4 finite elements for each one has 6 local displacement and 5 nodes have total number of global displacements m=16. The final compatibility matrix of displacements are shown on figure 5, where rows are global displacements and columns are local displacements.

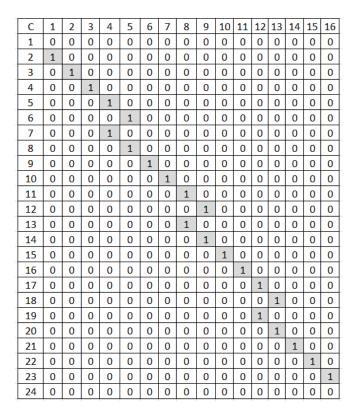


Figure 5: Compatibility matrix of displacements

# Coefficient matrix of equilibrium equations

The matrix of the coefficients of the equilibrium equations for a ring element of a round plate is given in the table.

$$[\overline{A}_{k}] = 2\pi \begin{bmatrix} \rho_{k,1} & 0 & 0 & 0 & 0 & 0 \\ 1.5 \frac{\rho_{k,1}}{b_{k}} - 1 & 1 & -2 \frac{\rho_{k,1}}{b_{k}} & 0 & \frac{\rho_{k,1}}{2b_{k}} & 0 \\ -\frac{\rho_{k,2}}{b_{k}} + 2 & -\frac{5}{6} & 2 \frac{\rho_{k,2}}{b_{k}} - 2 & \frac{2}{3} & -\frac{\rho_{k,2}}{b_{k}} & \frac{1}{6} \\ -\frac{\rho_{k,2}}{b_{k}} & -\frac{1}{6} & 2 \frac{\rho_{k,2}}{b_{k}} 2 & -\frac{2}{3} & -\frac{\rho_{k,2}}{b_{k}} - 2 & \frac{5}{6} \\ 0 & 0 & 0 & 0 & -\rho_{k,3} & 0 \\ \frac{\rho_{k,3}}{2b_{k}} & 0 & -2 \frac{\rho_{k,3}}{b_{k}} & 0 & 1 + 1.5 \frac{\rho_{k,3}}{b_{k}} & -1 \end{bmatrix}$$

$$(1)$$

 $A = [C]^T[\overline{A}]$  - equilibrium equation matrix, where:

 $[C]^T$  - is transpose of compatibility matrix of displacements [C]

[A] - is the matrix above in table

Calculation of the equilibrium equation matrix from Matlab

A =													
Columns 1	l through 1	4											
-6.2832	6.2832	0	0	1.7671	0	0	0	0	0	0	0	0	0
6.2832	-5.2360	0	4.1888	-6.2832	1.0472	0	0	0	0	0	0	0	0
-6.2832	-1.0472	25.1327	-4.1888	-18.8496	5.2360	0	0	0	0	0	0	0	0
0	0	0	0	-9.4248	0	9.4248	0	0	0	0	0	0	0
3.5343	0	-25.1327	0	25.1327	-6.2832	12.5664	6.2832	-25.1327	0	5.3014	0	0	0
0	0	0	0	0	0	-6.2832	-5.2360	25.1327	4.1888	-18.8496	1.0472	0	0
0	0	0	0	0	0	-18.8496	-1.0472	50.2655	-4.1888	-31.4159	5.2360	0	0
0	0	0	0	0	0	0	0	0	0	-18.8496	0	18.8496	0
0	0	0	0	0	0	7.0686	0	-50.2655	0	43.9823	-6.2832	12.5664	6.2832
0	0	0	0	0	0	0	0	0	0	0	0	-6.2832	-5.2360
0	0	0	0	0	0	0	0	0	0	0	0	-18.8496	-1.0472
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	28.2743	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
	l5 through												
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
-25.1327	0	21.2058	0	0	0	0	0	0	0				
25.1327	4.1888	-18.8496	1.0472	0	0	0	0	0	0				
50.2655	-4.1888	-31.4159	5.2360	0	0	0	0	0	0				
0	0		0	37.6991	0	0	0	0	0				
-50.2655	0	43.9823	-6.2832	50.2655		-75.3982	0	21.9911	0				
0	0	0		-31.4159	-5.2360	75.3982	4.1888	-43.9823	1.0472				
0	0	0	0	-43.9823	-1.0472		-4.1888	-56.5487	5.2360				
0	0	0	0	0	0	0	0	-50.2655	0				

# Flexibility matrix

To reduce the size of the matrix and ease of placing it in the report, the equations were replaced by j1, j2, j3, j4.

$$[D_{k}] = \frac{2\pi b_{k}}{15K_{k}(1-\nu_{k}^{2})} \begin{bmatrix} j_{2} & -\nu_{k}j_{2} & 2j_{1} & -2\nu_{k}j_{1} & -\rho_{k,2} & \nu_{k}\rho_{k,2} \\ j_{2} & -2\nu_{k}j_{1} & 2j_{1} & \nu_{k}\rho_{k,2} & -\rho_{k,2} \\ & 16\rho_{k,2} & -16\nu_{k}\rho_{k,2} & 2j_{3} & -2\nu_{k}j_{3} \\ & & 16\rho_{k,2} & -2\nu_{k}j_{3} & 2j_{3} \\ & & & j_{4} & -\nu_{k}j_{4} \\ symm. & & & j_{4} \end{bmatrix}$$

$$(2)$$

Where:

$$K_k = \frac{E_k t_k^3}{12(1-\nu_k^2)} \ j_1 = \rho_{k,2} - b_k$$

$$j_2 = 4\rho_{k,2} - 3b_k$$

$$j_3 = \rho_{k,2} + b_k$$

$$j_4 = 4\rho_{k,2} + 3b_k$$

Calculation of the flexibility matrix from Matlab

D_ =													
Columns 1 through 14													
0.0001	-0.0000	0	0	-0.0001	0.0000	0	0	0	0	0	0	0	0
-0.0000	0.0001	0	0	0.0000	-0.0001	0	0	0	0	0	0	0	0
0	0	0.0017	-0.0005	0.0004	-0.0001	0	0	0	0	0	0	0	0
0	0	-0.0005	0.0017	-0.0001	0.0004	0	0	0	0	0	0	0	0
-0.0001	0.0000	0.0004	-0.0001	0.0008	-0.0002	0	0	0	0	0	0	0	0
0.0000	-0.0001	-0.0001	0.0004	-0.0002	0.0008	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0.0010	-0.0003	0.0004	-0.0001	-0.0003	0.0001	0	0
0	0	0	0	0	0	-0.0003	0.0010	-0.0001	0.0004	0.0001	-0.0003	0	0
0	0	0	0	0	0	0.0004	-0.0001	0.0052	-0.0016	0.0009	-0.0003	0	0
0	0	0	0	0	0	-0.0001	0.0004	-0.0016	0.0052	-0.0003	0.0009	0	0
0	0	0	0	0	0	-0.0003	0.0001	0.0009	-0.0003	0.0016	-0.0005	0	0
0	0	0	0	0	0	0.0001	-0.0003	-0.0003	0.0009	-0.0005	0.0016	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0.0039	-0.0012
0	0	0	0	0	0	0	0	0	0	0	0	-0.0012	0.0039
0	0	0	0	0	0	0	0	0	0	0	0	0.0017	-0.0005
0	0	0	0	0	0	0	0	0	0	0	0	-0.0005	0.0017
0	0	0	0	0	0	0	0	0	0	0	0	-0.0013	0.0004
0	0	0	0	0	0	0	0	0	0	0	0	0.0004	-0.0013
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5 through												
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0	0	0	0	0	0	0	0	0	0				
0.0017	-0.0005	-0.0013	0.0004	0	0	0	0	0	0				
-0.0005	0.0017	0.0004	-0.0013	0	0	0	0	0	0				
0.0207	-0.0062	0.0034	-0.0010	0	0	0	0	0	0				

-0.0062	0.0207	-0.0010	0.0034	0	0	0	0	0	0
0.0034	-0.0010	0.0065	-0.0019	0	0	0	0	0	0
-0.0010	0.0034	-0.0019	0.0065	0	0	0	0	0	0
0	0	0	0	0.0048	-0.0014	0.0023	-0.0007	-0.0013	0.0004
0	0	0	0	-0.0014	0.0048	-0.0007	0.0023	0.0004	-0.0013
0	0	0	0	0.0023	-0.0007	0.0214	-0.0064	0.0031	-0.0009
0	0	0	0	-0.0007	0.0023	-0.0064	0.0214	-0.0009	0.0031
0	0	0	0	-0.0013	0.0004	0.0031	-0.0009	0.0059	-0.0018
0	0	0	0	0.0004	-0.0013	-0.0009	0.0031	-0.0018	0.0059

### External load vector

Vector of external forces for node described as:

$$\{F_k\} = \frac{2\pi b_k \rho_k}{3} \begin{Bmatrix} 3\rho_{k,2} - b_k \\ 3\rho_{k,2} + b_k \end{Bmatrix} = \{\eta_k\} \rho_k \tag{3}$$

 $[F] = [F_o] + [C]^T [F_p], \text{ where:}$   $[F_o] = f 12\pi \rho_{f1}$   $f 1 - \text{value of external force which correspond } u_1$   $\rho_{f1} - \text{coordinate where is } m_1$   $[F_p] - \text{represent the equivalent of distributed loads.}$   $[C]^T - \text{is transpose of compatibility matrix of displacements } [C]$ 

The results of global displacements are shown as following:

$$[U] = \{ [A][D]^{-1} \}^{-1} [A]^T [F]$$
(4)

Where:

A - coefficient matrix of equilibrium equations

 $[A]^T$  - transpose coefficient matrix of equilibrium equations

[D]: flexibility matrix

[F] - External load vector

```
Uglob =
    0.0340
    0.0383
    0.0185
    0.0020
    0.0216
    0.0235
    0.0134
   -0.0069
    0.0143
    0.0205
    0.0224
   -0.0037
    0.0185
    0.0179
    0.0026
   -0.0103
```

The results of internal forces are shown as following:

$$[S] = [D]^{-1}[A]^{T}[U]$$
(5)

#### Where:

- [D] flexibility matrix
- $[A]^T$  transpose coefficient matrix of equilibrium equations
- [U] global displacements

Results of internal forces (S) consists of  $M_{\rho}$  and moments  $M_{\varphi}$ . Schema have 24 values of internal forces (S) it's divided into: 12 values of moment  $M_{\rho}$  and 12 values of moments  $M_{\varphi}$ . In the matlab command  $M_{\rho} = S(1:2:end)$  means to take the 1st value of (S), 3rd, 5th, 7 th ..... till end. All those values belong to moment  $M_{\rho}$ . For  $M_{\varphi} = S(2:2:end)$  means to take the 2nd value of (S), 4th, 6th.....till end.

#### M\_Ro = -158.2735 -31.3524 -11.0997 -11.0997 3.5628 3.4310 3.4310 14.6141 22.3579 22.3579 9.7933 0.0000 $M_fi =$ -155.1518 50.9813 -51.6406 -10.0672 10.3970 -2.2716 -9.1624 4.6054 8.0553 11.6794 4.5640 5.7628

# Internal forces and displacements

The equilibrium of finite element method used to solve the mentioned annular plate. The results were obtained from a MATLAB commands. Based on these calculations, the results shows the internal forces and the a distribution of global displacements along our structure. The results shows that the maximum displacement was U = 36.0471 mm in the direction gravity. The allowable displacement was  $U_{allowable} = L/250 = 64$  mm so, the verification was correct based on the current geometry and material properties. Some parametric analysis were done for plate thickness as a very important parameter to reduce the displacement values.

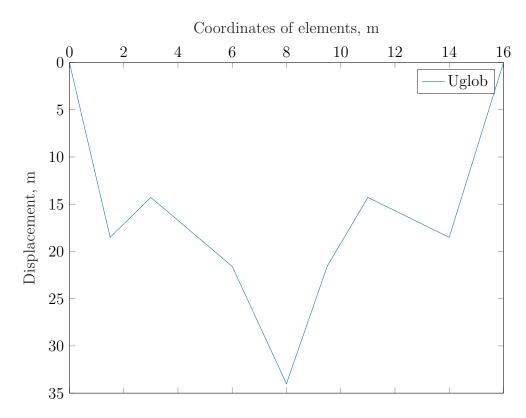


Figure 6: Global displacement across plate

# Code listing

#### Main function

```
1 close all; clc; clear;
2 format compact;
3 addpath('../matlab2tikz/');
4\% initial data
5 \text{ q.LOAD } =20; \% \text{ KN/m}
6 f = 20; \% KN
7 L=16; % span length m
8 h=0.05; \% thickness of plate
9 E=210e6; \% modulus of elastisty kPa
10 \text{ v} = .3; \% \text{ poisson 's ratio}
11 %
12 r=L/2; % radius of plate
13 B=[1.5 1.5 3 2]; %vector of elements lengths, meters
14 no_FE=length(B); % Number of plate finite elements, as length of vector with
      elements lengths
15 b=B/2; % half of elements lengths
16 coords = zeros(no_FE,3); %filling matrix (number of elements by 3) by zeros
17 coords (1,2)=B(1)/2;% coordinate of half of first element
18 coords (1,3)=B(1); % coordinate of end of first element
19 for i=2:no_FE% loop over coordinates matrix, "i" is current element, "i-1" is
      previous element
20
       coords (i,1)=coords (i-1,3); %start coordinate as end coordinate of previous
          element
21
       coords (i,2)=coords (i - 1,3) + B(i)/2; %half coordinate as end coordinate of
          previous element plus half length of current element
22
       coords (i,3) = coords (i - 1,3) + B(i); %end coordinate as end coordinate of
          previous element plus length of current element
23 end
24 delete coordsoutput.txt;
25 diary('coordsoutput.txt');
26 diary on ;
27 coords% printing coordinates matrix
28 diary off; %to avoid print other commands.
29 no_of_local_dis = 6; % Number of local displacements
30 no_of_global_dis=16; % Number of global displacements
31 % 2. Compatibality matrix C
32 % for the first FE
33 c-1stcompatipality_matrix=zeros(no_of_local_dis,no_of_global_dis);
34 c_1stcompatipality_matrix (2:6,1:5)=eye(5);
35 % for the Second FE
36 c_2ndcompatipality_matrix=zeros(no_of_local_dis,no_of_global_dis);
37 \text{ c}_2\text{ndcompatipality}_\text{matrix}(1:6,4:9) = \text{eye}(6);
38 % for the third FE
39 c_3rdcompatipality_matrix=zeros(no_of_local_dis,no_of_global_dis);
40 c_3rdcompatipality_matrix (1:6,8:13) = eye(6);
41 % for the 4th FE
42 c_4thcompatipality_matrix=zeros(no_of_local_dis,no_of_global_dis);
43 c_4thcompatipality_matrix (1:5,12:16)=eye(5);
44 % the total compatibality matrix of displacements
45 delete Cmtxoutput.txt;
46 diary ('Cmtxoutput.txt');
47 diary on ;
48 C=[c_1stcompatipality_matrix; c_2ndcompatipality_matrix; c_3rdcompatipality_matrix;
      c_4thcompatipality_matrix]
```

```
49 diary off; %to avoid print other commands.
50 %4. Matrix of equilibrium equantions A
51 for k=1:no_FE
52 \text{ A-matrix} = \text{getAmtx}(\text{coords}(k,1), \text{coords}(k,2), \text{coords}(k,3), \text{b}(k));
53 A_{-}(k*6-5:k*6,k*6-5:k*6) = 2*pi*A_{-}matrix;
54 end
55 delete Amtxoutput.txt;
56 diary ('Amtxoutput.txt');
57 diary on;
58 A=C'*A_
59 diary off; %to avoid print other commands.
60 % % 5. Flexibility MATRIX OF D
61 for k=1:no_FE
62 \text{ Rok2=coords}(k,2);
63 bk=b(k);
64 D_{\text{matrix}} = \text{getDmtx}(\text{coords}(k,2), b(k), v);
65 K. k=E*h^3/(12*(1-v^2));
66 D_{-}(k*6-5:k*6,k*6-5:k*6) = (2*pi*bk/(15*K.k*(1-v^2)))*D_{-}matrix;
67 end
68 delete Dmtxoutput.txt;
69 diary ('Dmtxoutput.txt');
70 diary on ;
71 D<sub>-</sub>
72 diary off; %to avoid print other commands.
73~\%~\%~6. EXTERNAL LOAD VICTOR F
74 Fo=zeros (no_of_global_dis ,1);
75 Rof=6:
76 Fo (13) = f *2 * \mathbf{pi} * \text{Rof};
77 % Rof coordinate where f
78 % Fkp is nodal external load vector which is equivilant to distributed load of
         the kth element
79
   q_Load_vector = [20 \ 20 \ 0 \ 0];
   for k=1:no_FE
81
         bk=b(k);
82
         q=q_Load_vector(k);
83
         Rok2 = coords(k, 2);
84
         Fk = (2 * pi * bk / 3) * q * [3 * Rok2 - bk; 3 * Rok2 + bk];
85
         Fp = [0; 0; Fk; 0; 0];
         Fp_{-}(6*k-5:k*6,1)=Fp;
86
87 end
88 delete Foutput.txt;
89 diary ('Foutput.txt');
90 diary on;
91 F=Fo+C'* Fp_
92 diary off; %to avoid print other commands.
93 delete Ugloboutput.txt;
94 diary('Ugloboutput.txt');
95 diary on ;
96 Uglob=inv(A*inv(D_-)*A')*F
97 diary off; %to avoid print other commands.
98 delete Ulocaloutput.txt;
99 diary('Ulocaloutput.txt');
100 diary on ;
101 Ulocal=inv(D_{-})*A'*Uglob
102 diary off; %to avoid print other commands.
103 delete M_Rooutput.txt;
104 diary ('M_Rooutput.txt');
105 diary on ;
106 M_Ro=Ulocal (1:2:end)
```

```
107 diary off; %to avoid print other commands.
108 delete M_fioutput.txt;
109 diary ('M_fioutput.txt');
110 diary on ;
111 M_{\text{fi}} = \text{Ulocal}(2:2:\text{end})
112 diary off; %to avoid print other commands.
113 delete um_mmoutput.txt;
114 diary ('um_mmoutput.txt');
115 diary on ;
116 \text{ um.mm} = 1000*[\text{Uglob}(1:4:\text{end});0]
117 diary off; %to avoid print other commands.
118 x coord = [0; coords(1:end,3)];
119 figure(1);
120 plot (xcoord, um.mm, 'DisplayName', 'Uglob');
121 xlabel ('Coordinates of elements, m')
122 ylabel ('Displacement, m')
123 set(gca, 'XAxisLocation', 'top', 'YAxisLocation', 'left', 'ydir', 'reverse'); 124 matlab2tikz('um.mm.tex', 'showInfo', false);
125 delete u_allowableoutput.txt;
126 diary ('u_allowableoutput.txt');
127 diary on ;
128 \text{ u-allowable} = 16/250*1000
129 diary off; %to avoid print other commands.
130 fullCoord = [0; coords(1:end,3); coords(1:end,3)+coords(end)];
131 flipedU = zeros(length(um_mm),1);
132 j = length (um\_mm);
133 for i=1:length(um_mm)
          flipedU(i) = um\_mm(j);
134
          j=j-1;
135
136 end
137 fullU = [flipedU; um.mm(2:end)];
138 figure(2);
139 plot (fullCoord, fullU, 'DisplayName', 'Uglob');
140 xlabel ('Coordinates of elements, m')
141 ylabel ('Displacement, m')
142 set(gca, 'XAxisLocation', 'top', 'YAxisLocation', 'left', 'ydir', 'reverse');
143 matlab2tikz('fullU.tex', 'showInfo', false);
    getAmtx function
  1 function A_matrix = getAmtx(Rok1, Rok2, Rok3, bk)
          A_{\text{matrix}} = \mathbf{zeros}(6,6);
  3
          A_{\text{matrix}}(1,1) = \text{Rok1};
          A_{\text{matrix}}(2,1) = 1.5 * \text{Rok}1/\text{bk}-1;
  4
  5
          A_{\text{-}}matrix (2,2)=1;
  6
          A_{\text{matrix}}(2,3) = -2*\text{Rok}1/\text{bk};
  7
          A_{\text{matrix}}(2,5) = \text{Rok}2/2*bk;
  8
          A_{\text{matrix}}(3,1) = -\text{Rok}2/\text{bk}+2;
  9
          A_{\text{matrix}}(3,2) = -5/6;
 10
          A_{\text{matrix}}(3,3) = 2*Rok2/bk-2;
 11
          A_{\text{matrix}}(3,4) = 2/3;
 12
          A_{\text{matrix}}(3,5) = -\text{Rok}2/\text{bk};
 13
          A_{\text{matrix}}(3,6) = 1/6;
 14
          A_{\text{matrix}}(4,1) = -\text{Rok}2/\text{bk};
 15
          A_{\text{matrix}}(4,2) = -1/6;
 16
          A_{\text{matrix}}(4,3) = 2*Rok2/bk+2;
 17
          A_{\text{matrix}}(4,4) = -2/3;
 18
          A_{\text{matrix}}(4,5) = -\text{Rok}2/\text{bk}-2;
 19
          A_{\text{matrix}}(4,6) = 5/6;
```

```
\begin{array}{lll} 20 & A_{\rm matrix}\left(5\,,5\right) = -{\rm Rok}3\,;\\ 21 & A_{\rm matrix}\left(6\,,1\right) = {\rm Rok}3/2*\,{\rm bk}\,;\\ 22 & A_{\rm matrix}\left(6\,,3\right) = -2*{\rm Rok}3/\,{\rm bk}\,;\\ 23 & A_{\rm matrix}\left(6\,,5\right) = 1+1.5*{\rm Rok}3/\,{\rm bk}\,;\\ 24 & A_{\rm matrix}\left(6\,,6\right) = -1;\\ 25 & \text{end} \end{array}
```

### getDmtx function

```
1 function D<sub>matrix</sub> = getDmtx(Rok2, bk, v)
 2
          j1=Rok2-bk;
 3
          i2 = 4*Rok2 - 3*bk;
 4
          j3=Rok2+bk;
          j4=4*Rok2+3*bk;
 6
          D_{\text{matrix}}(1,1)=j2;
          D_{\text{-}}matrix (1,2)=-v*j2;
 7
 8
          D_{\text{matrix}}(1,3) = 2*j1;
 9
          D_{\text{matrix}}(1,4) = -2*v*j1;
10
          D_{\text{-}}matrix (1,5) = -\text{Rok}2;
11
          D_{\text{-}}matrix (1,6)=v*Rok2;
12
          D_{\text{matrix}}(2,1) = D_{\text{matrix}}(1,2);
13
          D_{\text{matrix}}(2,2) = j2;
14
          D_{\text{matrix}}(2,3) = -2*v*j1;
15
          D_{\text{matrix}}(2,4) = 2*j1;
16
          D_{\text{matrix}}(2,5) = v * \text{Rok}2;
17
          D_{\text{-}}matrix (2,6) = -\text{Rok}2;
          D_{\text{matrix}}(3,1) = D_{\text{matrix}}(1,3);
18
19
          D_{\text{matrix}}(3,2) = D_{\text{matrix}}(2,3);
20
          D_{\text{matrix}}(3,3) = 16*Rok2;
21
          D_{\text{-matrix}}(3,4) = -16 * v * Rok2;
22
          D_{\text{matrix}}(3,5) = 2*j3;
23
          D_{\text{matrix}}(3,6) = -2 * v * j 3;
24
          D_{\text{matrix}}(4,1) = D_{\text{matrix}}(1,4);
          D_{\text{matrix}}(4,2)=D_{\text{matrix}}(2,4);
25
26
          D_{\text{matrix}}(4,3) = D_{\text{matrix}}(3,4);
27
          D_{\text{matrix}}(4,4) = 16*\text{Rok}2;
28
          D_{\text{-}}matrix (4,5)=-2*v*j3;
29
          D_{\text{matrix}}(4,6) = 2*j3;
30
          D_{\text{matrix}}(5,1) = D_{\text{matrix}}(1,5);
31
          D_{\text{matrix}}(5,2) = D_{\text{matrix}}(2,5);
32
          D_{\text{matrix}}(5,3) = D_{\text{matrix}}(3,5);
33
          D_{\text{matrix}}(5,4) = D_{\text{matrix}}(4,5);
34
          D_{\text{matrix}}(5,5)=j4;
35
          D_{\text{matrix}}(5,6) = -v * j4;
36
          D_{\text{matrix}}(6,1) = D_{\text{matrix}}(1,6);
37
          D_{\text{matrix}}(6,2) = D_{\text{matrix}}(2,6);
38
          D_{\text{matrix}}(6,3) = D_{\text{matrix}}(3,6);
39
          D_{\text{matrix}}(6,4) = D_{\text{matrix}}(4,6);
40
          D_{\text{matrix}}(6,5) = D_{\text{matrix}}(5,6);
41
          D_{\text{matrix}}(6,6)=j4;
42 end
```