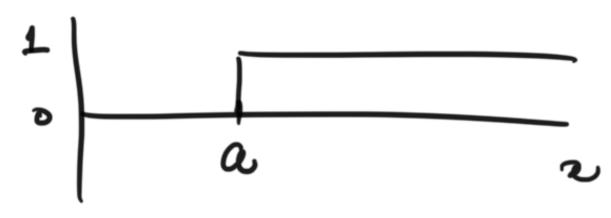
Singularity functions



represent by (2-a)

f < \varphi > imply that

\$ >0 we ignore or f=0
\$ > 0 then the value
\$ 5.

(2-a) n=1 a (2

(x-a) if x >a 0 if x < 0

(2-4)2

n=2

a

$$\langle x-a \rangle^{n} = (x-a)^{n} \quad \text{if } x \geqslant a$$

$$= 0 \quad \text{if } x < a$$

$$\int_{-\infty}^{\infty} \langle x-a \rangle^{n} dx = \langle x-a \rangle^{n+1}, \quad n \geqslant 0$$

moment M represented by M(2-a)_ in loading

dueto q(x) $\omega(x) = -9(x)(x-a)$

w(x) = - wo (x-a)

was due to ro.

 $=-\omega \langle z-a \rangle$ + wo(2-20)

7 functions to work Unforcession for W(x), integrale twice to get M(x)

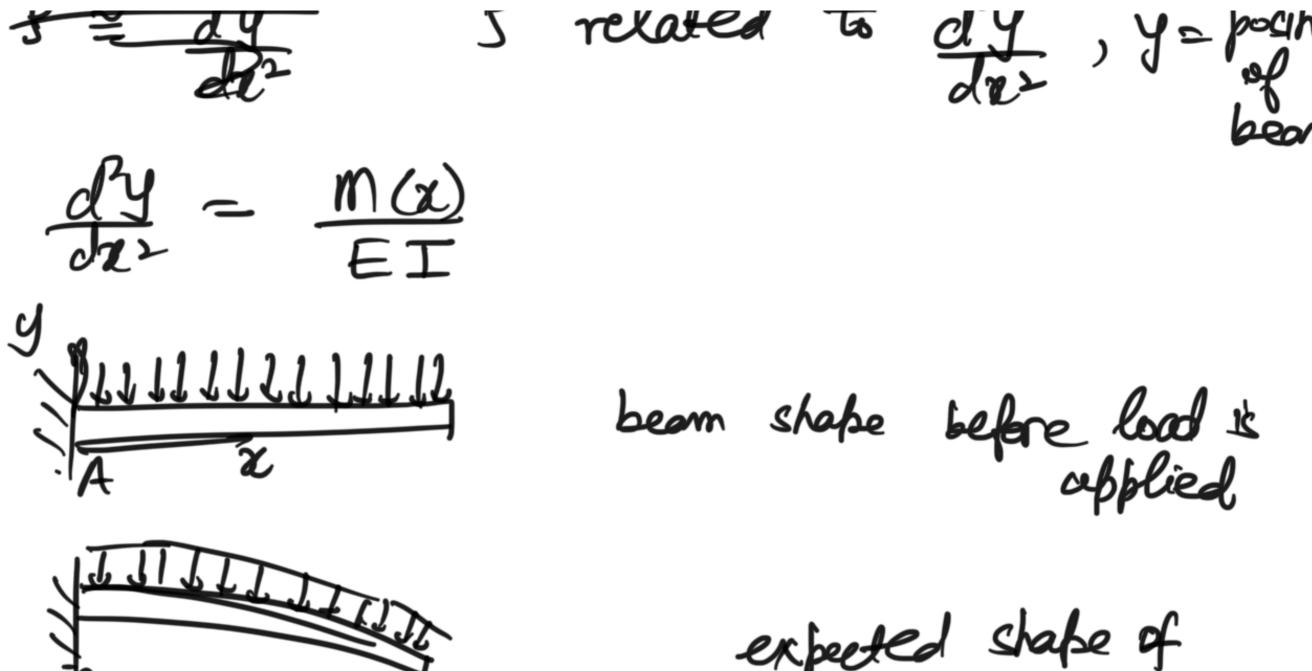
Deflection of beams

- i) Design specifications allows for a maximum value of deflection
- 2) Indeterminate beams can be solved for veryo theory of deflections
- 2) Method of superficition can be used to conviniently solve problems

When a beam unclergoes a tromsvere load

If M = M(x), S is also a function of x.

3 - curvature will helle us to determine deflection of beam.



expected shake of beam after locating

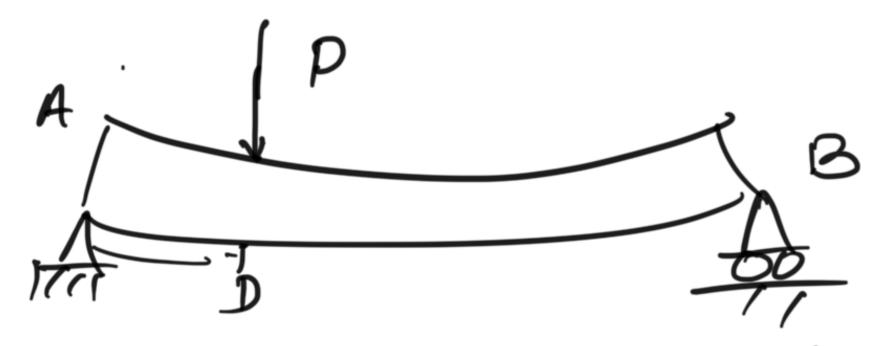
 $\frac{dy_A}{dz}$ = slope of deflection = Θ_A = 0 (for fixed, end)

- combilerer)

A ya = ya = 0 The coen after different cleftedon)

state after different

dr = M(x) EI 2 rd order ODE integrate twice.



y obtained after successive reference.

deflected beam due to load P

 $y_1(x)$ be deflection from A to D. \leftarrow based on m(x) from $y_2(x)$ be deflection from D to B \leftarrow hard D

integrate 2nd order ODE for ex AD -> 2 constats
refron DB -> 2 constats.

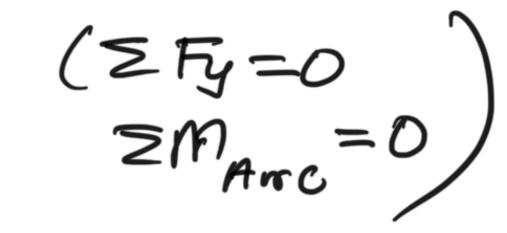
YB=0 YA =0 At D, deflecter 2 slopes are equal $y_1(D) = y_2(D)$ $\Theta_1(D) = \Theta_2(D)$

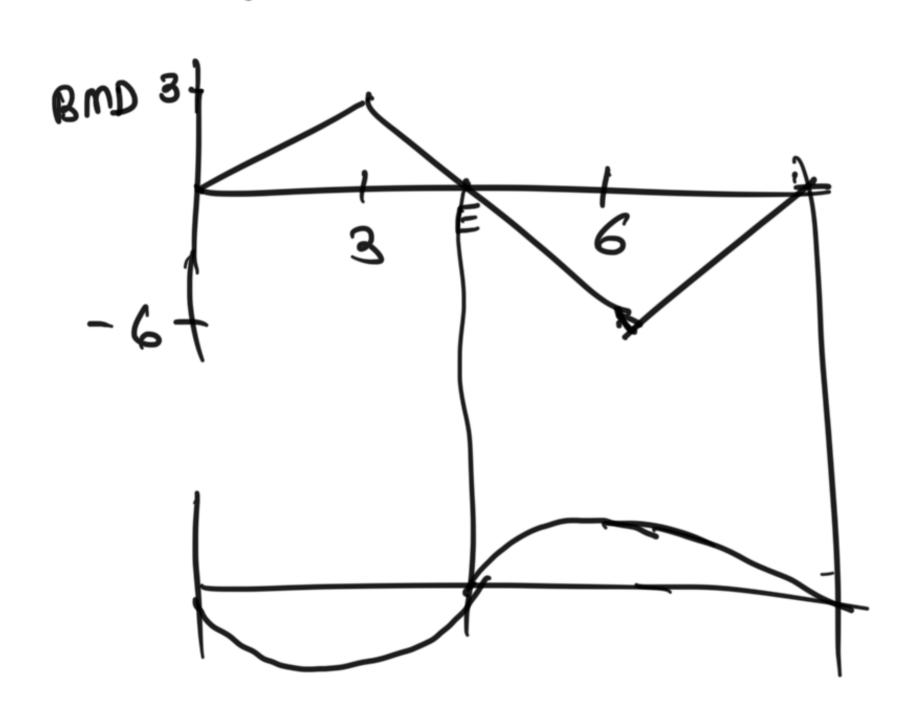
BD LAKN J ZKN

statically determinate

Cy

$$Ay = 1 kN$$
, $Cy = 5 kN$





0<2<3 m=12 3<2<6 m=2-42+12 =12-32

expected shape of beam bensed on BMD

expected chopes of deflected Beam

m >0 the comptants
concave whosels

Calculus:
$$\frac{1}{S} = \frac{d^3y}{dt^2}$$

$$\frac{1}{(H(\frac{dy}{dt})^2)^3/2}$$

$$y(x) \text{ for beam } \Rightarrow \text{ Elastic curve of beam}$$

$$\frac{dy}{dz} \ll 1 \implies \text{ denominator can be worthen as}$$

$$\frac{1}{S} = \frac{d^3y}{dt^2} = \frac{M(x)}{ET}$$

$$\frac{1}{S} = \frac{d^3y}{dt^2} = \frac{M(x)}{ET}$$

EIdy = m(x)

EI dy =
$$\int_{0}^{\infty} m(x) dx + G_{1}$$

EI $y(x) = \int_{0}^{\infty} \int_{0}^{\infty} m(x) dx + G_{1} \int_{0}^{\infty} dx$
 $\frac{dy}{dx} = \Theta(x) \rightarrow \text{slope (roadians)}$
 $\frac{dy}{dx} = \text{tan } \theta \approx \theta \text{ (if } \Theta \text{ is small.}$

dy - May dw2 - EI

$$\frac{dM}{dz} = V \implies \frac{d^3y}{dz^3} = \frac{V(z)}{EL}$$

$$\frac{dV}{dz} = -w \implies \frac{d^4y}{dz^4} = -\frac{w(z)}{EL}$$
At free end, $V = 0$
on simply supported end $M = 0$
ig. A $\frac{1}{W} \frac{P}{4u} = \frac{3L}{4u} \frac{A}{4u}$

$$\frac{1}{W} \frac{P}{4u} = \frac{3L}{4u} \frac{A}{4u}$$

from DGB:

$$m(x) = 3Px - P(x-L)$$

$$= P(L-x) = FId^{2}$$

$$= dx^{2}$$

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Statically indetermate problem eg. of statically indeminate problen.

m = (RA-P)~.

ET
$$dx^2 = (R_A - P) \times ET dx^2 = (R_A - P) \frac{2^3}{6} + C_1 \times + C_2$$

 $y(0) = 0$, $y(L) = 0$ get R_A
 $\theta(L) = 0$ C_1 , C_2