PROJECT:

CHLOE'S POINTE APARTMENTS

OGDEN, UTAH

GENERAL NOTES:

1. THE APPLICANT IS RESPONSIBLE FOR THE COMPLIANCE WITH ALL REQUIREMENTS OF THE ANSI 117.1
2. FIELD VARIFY ALL SITE ELEVATIONS – VARIFY BUILDING FLOOR FINISH ELEVATION WITH SITE GRADING AND DRAINAGE PLAN. BUILDING REFERENCE ELEVATION = 100'–0".
3. FLOOR/CEILING ASSEMBLIES BETWEEN DWELLING UNITS OR BETWEEN A DWELLING UNIT AND A PUBLIC OR SERVICE AREA WITHIN THE STRUCTURE SHALL HAVE AN IMPACT INSULATION CLASS (IIC) RATING OF NOT LESS THAN 50 (45 IF FIELD TESTED) WHEN TESTED IN ACCORDANCE WITH ASTM E 492. IBC 1207.2 AIR–BORNE SOUND.
4. WALLS, PARTITIONS AND FLOOR/CEILING ASSEMBLIES SEPARATING DWELLING UNITS FROM EACH OTHER OR FROM PUBLIC OR SERVICE AREAS SHALL HAVE A SOUND TRANSMISSION CLASS (STC) OF NOT LESS THAN 50 (45 IF FIELD TESTED) FOR AIR–BORNE NOISE WHEN TESTED IN ACCORDANCE WITH ASTM E 90. PENETRATIONS OR OPENINGS IN CONSTRUCTION ASSEMBLIES FOR PIPING; ELECTRICAL DEVICES; RECESSED CABINETS; BATHTUBS; SOFFITS; OR HEATING, VENTILATING OR EXHAUST DUCTS SHALL BE SEALED, LINED, INSULATED OR OTHERWISE TREATED TO MAINTAIN THE REQUIRED RATINGS. THIS REQUIREMENT SHALL NOT APPLY TO DWELLING UNIT ENTRANCE DOORS; HOWEVER, SUCH DOORS SHALL BE TIGHT FITTING TO THE FRAME AND SILL. IBC 1207.2 AIR–BORNE SOUND.
5. FASTENERS FOR PRESSURE–PRESERVATIVE AND FIRE–RETARDANT–TREATED WOOD SHALL BE OF HOT–DIPPED ZINC–COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER. THE COATING WEIGHTS FOR ZINC–COATED FASTENERS SHALL BE IN ACCORDANCE WITH ASTM A 153. ALL NAILS FROM THE SHEAR PANELS, SECURED INTO THE MUDSILLS WILL NEED SPECIAL NAILS WHEN IN CONTACT WITH THE NEW TYPE OF TREATED MUDSILLS.
6. SILL PLATE SIZE AND ANCHORAGE IN SEISMIC DESIGN CATEGORY D, E OR F. ANCHOR BOLTS FOR SHEAR WALLS SHALL INCLUDE STEEL PLATE WASHERS, A MINIMUM OF 0.229 INCH BY 3 INCHES BY 3 INCHES IN SIZE, BETWEEN THE SILL PLATE AND NUT. THE HOLE IN THE PLATE WASHER IS PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/16 INCH LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED 1¾ INCHES, PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. IBC 2308.3.2
7. TRUSS DESIGN DRAWINGS, THE WRITTEN, GRAPHIC AND PICTORIAL DEPICTION OF EACH INDIVIDUAL TRUSS SHALL BE PROVIDED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO INSTALLATION. IBC 2303.4.1.1
8. DRAFTSTOPPING SHALL BE PROVIDED IN ATTICS, MANSARDS, OVERHANGS OR OTHER CONCEALED ROOF SPACES OF GROUP R–2 BUILDINGS WITH THREE OR MORE DWELLING UNITS AND IN ALL GROUP R–1 BUILDINGS. DRAFTSTOPPING SHALL BE INSTALLED ABOVE, AND IN LINE WITH, SLEEPING UNIT AND DWELLING UNIT SEPARATION WALLS THAT DO NOT EXTEND TO THE UNDERSIDE OF THE ROOF SHEATHING ABOVE. EXCEPTIONS: 1. WHERE CORRIDOR WALLS PROVIDE A SLEEPING UNIT OR DWELLING UNIT SEPARATION, DRAFTSTOPPING SHALL ONLY BE REQUIRED ABOVE ONE OF THE CORRIDOR WALLS 3. IN OCCUPANCIES IN GROUP R–2 THAT DO NOT EXCEED FOUR STORIES ABOVE GRADE PLANE, THE ATTIC SPACE SHALL BE SUBDIVIDED BY DRAFTSTOPS INTO AREAS NOT EXCEEDING 3,000 SQUARE FEET OR ABOVE EVERY TWO DWELLING UNITS, WHICHEVER IS SMALLER 4. DRAFTSOPPING IS NOT REQUIRED IN BUILDINGS EQUIPPED THROUGHOUT WITH AN AUTOMATIC SPRINKLER SYSTEM IN ACCORDANCE WITH SECTION 903.3.1.2, PROVIDED THAT AUTOMATIC SPRINKLERS ARE INSTALLED IN THE COMBUSTIBLE CONCEALED SPACE WHERE DRAFTSTOPPING IS BEING OMITTED. IBC 718.4.2
9. UTAH STATE AMENDMENT IBC 1405.4 FLASHING. FLASHING SHALL BE INSTALLED IN SUCH A MANNER SO AS TO PREVENT MOISTURE FROM ENTERING THE WALL OR TO REDIRECT IT TO THE EXTERIOR. FLASHINGS SHALL BE INSTALLED AT THE PERIMETERS OF EXTERIOR DOOR AND WINDOW ASSEMBLIES, PENETRATIONS AND TERMINATIONS OF EXTERIOR WALL ASSEMBLIES, EXTERIOR WALL INTERSECTIONS WITH ROOFS, CHIMNEYS, PORCHES, DECKS, BALCONIES AND SIMILAR PROJECTIONS AND AT BUILT–IN GUTTERS AND SIMILAR LOCATIONS WHERE MOISTURE COULD ENTER THE WALL. FLASHING WITH PROJECTED FLANGES SHALL BE INSTALLED ON BOTH SIDES AND THE ENDS OF COPINGS, UNDER SILLS AND CONTINUOUSLY ABOVE PROJECTED TRIM. A FLASHING SHALL BE INSTALLED AT THE INTERSECTION OF THE FOUNDATION TO STUCCO, MASONRY, SIDING OR BRICK VENEER. THE FLASHING SHALL BE ON AN APPROVED CORROSION–RESISTANT FLASHING WITH A 1/2" DRIP LEG EXTENDING PAST EXTERIOR SIDE OF THE FOUNDATION.
10. ALL GLASS WITHIN 12 IN. OF DOORS, AT STAIRWAY LANDINGS, ADJACENT TO BATHTUBS AND LESS THAN 18 IN. ABOVE FLOOR TO BE TEMPERED.
11. BALCONIES AND DECKS. SPRINKLER PROTECTION SHALL BE PROVIDED FOR EXTERIOR BALCONIES, DECKS AND GROUND FLOOR PATIOS OF DWELLING UNITS WHERE THE BUILDING IS OF TYPE V CONSTRUCTION, PROVIDED THERE IS A ROOF OR DECK ABOVE. SIDEWALL SPRINKLERS THAT ARE USED TO PROTECT SUCH AREAS SHALL BE PERMITTED TO BE LOCATED SUCH THAT THEIR DEFLECTORS ARE WITHIN 1 INCH TO 6 INCHES BELOW THE STRUCTURAL MEMBERS AND A MAXIMUM DISTANCE OF 14 INCHES BELOW THE DECK OF THE EXTERIOR BALCONIES AND DECKS THAT ARE CONSTRUCTED OF OPEN WOOD JOIST CONSTRUCTION. IBC 903.3.1.2.1 FIRE SPRINKLER PROTECTION SHALL BE EXTENDED TO THE DECKS AS PER IBC SECT, 1406.3 EXCEPTION 3.
12. THE REQUIRED FIRE–RESISTANCE RATING OF EXTERIOR WALLS WITH A FIRE SEPARATION DISTANCE OF GREATER THAN 10 FEET SHALL BE RATED FOR EXPOSURE TO FIRE FROM THE INSIDE.
13. THE AREA OF UNPROTECTED AND PROTECTED OPENINGS IS NOT LIMITED WITH A FIRE SEPARATION DISTANCE GREATER THAN 30 FEET.
14. SIGNS SHALL BE PLACED WHERE THE EXIT OR THE PATH OF EGRESS TRAVEL IS NOT READILY VISIBLE. NO POINT IN A CORRIDOR OR PASSAGEWAY TO BE MORE THAN 100 FEET FROM AN EXIT SIGN. IBC 1013.1
15. EXIT SIGNS SHALL BE INTERNALLY OR EXTERNALLY ILLUMINATED. IBC 1013.3. EXIT SIGN SHALL BE ILLUMINATED AT ALL TIMES INCLUDING DURING LOSS OF PRIMARY POWER. IBC 1011.5 & 1013.6.3
16. THE MINIMUM WIDTH OF A STAIRWAY IS 44 INCHES IN THE STAIR ENCLOSURE IBC 1011.2. PROVIDE A HANDRAIL ON EACH SIDE OF STAIRWAYS. IBC 1011.11
17. AREAS OF REFUGE ARE NOT REQUIRED WHEN AN AUTOMATIC SPRINKLER SYSTEM IS PROVIDED. IBC 1009.4, EXCEPTION 2
18. STAIRWAY ENCLOSURES SHALL HAVE A FIRE RESISTANCE RATING OF NOT LESS THAN 2 HOURS. 1023.2. WHERE INTERIOR EXIT ENCLOSURES ARE EXTENDED TO THE EXTERIOR OF THE BUILDING BY AN EXIT PASSAGE WAY, FIRE–RESISTANCE OF THE EXIT PASSAGE WAY SHALL BE THE SAME AS THE ENCLOSURE. IBC 1023.3.1.
19. THE RATED STAIRWAY SHAFTS MUST BE BUILT AS FIRE BARRIERS PER IBC 707, FRAMING MEMBERS, BEAMS OR TRUSSES MAY NOT PENETRATE THE FIRE BARRIER.
20. WHERE EXTERIOR WALLS SERVE AS A PART OF A REQUIRED FIRE–RESISTANCE–RATED SHAFT OR EXIT ENCLOSURE, OR SEPARATION, SUCH WALLS SHALL COMPLY WITH THE REQUIREMENTS OF SECTION 705 FOR EXTERIOR WALLS AND THE FIRE–RESISTANCE–RATED ENCLOSURE OR SEPARATION REQUIREMENTS SHALL NOT APPLY. IBC 707.4
21. FIRE BARRIERS SHALL EXTEND FROM THE TOP OF THE FLOOR/CEILING ASSEMBLY BELOW TO THE UNDERSIDE OF THE FLOOR OR ROOF SHEATHING, SLAB OR DECK ABOVE AND SHALL BE SECURELY ATTACHED THERETO. SUCH FIRE BARRIERS SHALL BE CONTINUOUS THROUGH CONCEALED SPACES, SUCH AS THE SPACE ABOVE A SUSPENDED CEILING. 707.5 EXCEPT. 2. SHAFT ENCLOSURES SHALL BE PERMITTED TO TERMINATE AT A TOP ENCLOSURE COMPLYING WITH SECTION 713.12.
22. THE SUPPORTING CONSTRUCTION FOR A FIRE BARRIER SHALL BE PROTECTED TO AFFORD THE REQUIRED FIRE–RESISTANCE RATING OF THE FIRE BARRIER SUPPORTED. HOLLOW VERTICAL SPACES WITHIN A FIRE BARRIER SHALL BE FIREBLOCKED IN ACCORDANCE WITH SECTION 717.2 AT EVERY FLOOR LEVEL. 707.5.1
23. WHERE COLUMNS ARE REQUIRED TO BE FIRE–RESISTANCE RATED, THE ENTIRE COLUMN SHALL BE PROVIDED INDIVIDUAL ENCASEMENT PROTECTION BY PROTECTING IT ON ALL SIDES FOR THE FULL COLUMN LENGTH, INCLUDING CONNECTIONS TO OTHER STRUCTURAL MEMBERS, WITH MATERIALS HAVING THE REQUIRED FIRE–RESISTANCE RATING. WHERE THE COLUMN EXTENDS THROUGH A CEILING, THE ENCASEMENT PROTECTION SHALL BE CONTINUOUS FROM THE TOP OF THE FOUNDATION OR FLOOR/CEILING ASSEMBLY BELOW THROUGH THE CEILING SPACE TO THE TOP OF THE COLUMN.
24. MEMBERS OF THE PRIMARY STRUCTURAL FRAME OTHER THAN COLUMNS THAT ARE REQUIRED TO HAVE A FIRE–RESISTANCE RATING AND SUPPORT MORE THAN TWO FLOORS OR ONE FLOOR AND ROOF, OR SUPPORT A LOAD–BEARING WALL OR A NONLOAD–BEARING WALL MORE THAN TWO STORIES HIGH, SHALL BE PROVIDED INDIVIDUAL ENCASEMENT PROTECTION BY PROTECTING THEM ON ALL SIDES FOR THEIR FULL LENGTH, INCLUDING CONNECTIONS TO OTHER STRUCTURAL MEMBERS, WITH MATERIALS HAVING THE REQUIRED FIRE–RESISTANCE RATING. EXCEPTION: INDIVIDUAL ENCASEMENT PROTECTION ON ALL SIDES SHALL BE PERMITTED ON ALL EXPOSED SIDES PROVIDED THE EXTENT OF PROTECTION IS IN ACCORDANCE WITH THE REQUIRED FIRE–RESISTANCE RATING, AS DETERMINED IN SECTION 703.
25. SPRINKLER AND ALARM PLANS SHALL BE PROVIDED TO THE FIRE MARSHAL FOR HIS APPROVAL BY DEFERRED SUBMITTAL.

RADON VENTING:

1. VERIFY REQUIREMENTS WITH OGDEN CITY REGARDING IF RADON VENTING IS REQUIRED. IF REQUIRED, PROVIDE PERFORATED PIPE UNDER GRAVEL AT 40' O.C.. VENT TO ROOF.

CODE COMPLIANCE SUMMARY:

PROJECT: CHLOE'S POINTE APARTMENTS
231 WEST 12TH STREET
OGDEN, UT 84404

BUILDING OCCUPANCY = R2
BUILDING TYPE = VA SPRINKLERED
FIRE SPRINKLERS = NFPA 13 SYSTEM
BUILDING AREA 104,239 ALLOWED 252,000

TOTAL LIVING UNITS = 114
(88) 1 BEDROOM UNIT = 61,980 S.F. + 3,920 S.F. PATIOS OR DECKS
(26) 2 BEDROOM UNITS = 23,561 S.F. + 1,188 S.F. PATIOS OR DECKS
COMMON SPACE = 14,052
TOTAL LIVING AREA (EXCLUDING DECKS) = 85,541 SQ. FT.
DECKS = 5,108 SQ. FT.
TOTAL ENVELOPE = 104,239 SQ. FT.
OCCUPANCY =522

ALLOWED HEIGHT 60 FEET IBC TABLE 504.3 MAIN PARAPET HEIGHT = 43 FEET.
ALLOWED STORIES = 4. ACTUAL STORIES = 4.

NORTH SIDE = 25' TO PUBLIC WAY
SOUTH SIDE = 155' TO PROPERTY LINE
EAST SIDE = 35' TO PROPERTY LINE
WEST SIDE = 15.71' TO PROPERTY LINE. OPENINGS = 26% ALLOWED OPENINGS 75%

IBC TABLES 601 AND 602
PRIMARY STRUCTURAL FRAME= 1 HR.
EXTERIOR BEARING = 1 HR.
INTERIOR BEARING= 1 HR.
EXTERIOR NON–BEARING = 1 HR.
INTERIOR NON–BEARING = 0 HR.
FLOOR CONSTRUCTION AND ASSOCIATED SECONDARY MEMBERS= 1 HR
ROOF CONSTRUCTION AND ASSOCIATED SECONDARY MEMBERS= 1 HR
FLOOR/CEILING ASSEMBLY = 1 HR
ROOF/CEILING ASSEMBLY = 1 HR
ROOF TYPE= B IBC TABLE 1505.1

MECHANICAL NOTES:

1. PROVIDE COMBUSTION AIR SUPPLY TO FURNACE OR WATER HEATER FROM OUTSIDE PER CODE REQUIREMENTS.
2. FURNACES TO HAVE A MIN. OF 3 IN. CLEAR WORKING SPACE ON SIDES, BACK, AND TOP – 30 IN. IN FRONT.
3. ALL INTERIOR MECHANICAL VENT SHAFTS ARE 2 HR. FIRE RATED. STAIRS AND SHAFTS THAT GO ALL 4 LEVELS NEED TO BE 2 HOUR FIRE RATED, IF ONLY 3 LEVELS 1 HOUR FIRE RATED WILL BE REQUIRED.
4. VENTS SHALL TERMINATE 4 FT. BELOW, OR 4 FT. HORIZONTAL AND AT LEAST 12 IN. ABOVE A DOOR, OPERABLE WINDOW, OR A GRAVITY AIR INLET INTO A BUILDING.
5. FLUE VENTS AND EXHAUST FAN VENTS SHALL BE AT LEAST 3 FT. ABOVE AN OUTSIDE AIR INLET LOCATED WITHIN 10 FT. AND AT LEAST 4 FT. FROM A PROPERTY LINE.
6. VENT CLOTHES DRYER TO OUTSIDE. DUCTS TO BE OF METAL WITH SMOOTH INTERIOR SURFACES AND EQUIPPED WITH BACK–DRAFT DAMPERS. TERMINATE DUCT AT THE EXTERIOR OF THE BUILDING, NO SCREWS TO BE USED. MAXIMUM LENGTH OF DRYER DUCT IS TO BE 25'. LENGTH TO BE REDUCED 5' PER BEND. DIAMETER OF DUCT SHALL BE 4 IN. UNLESS OTHERWISE SPECIFIED BY MANUFACTURER'S RECOMMENDATIONS.
7. PROVIDE MINIMUM OF 30 IN. CLEARANCE ABOVE RANGE TOP TO COMBUSTIBLES.
8. MICROWAVE OR OTHER SIMILAR OVENS INSTALLED ABOVE COOKTOPS SHALL CONFORM TO THE CONDITIONS OF THEIR LISTINGS.
9. ALL BATHROOMS AND LAUNDRY ROOMS WITHOUT AN OPERABLE WINDOW ARE REQUIRED TO HAVE EXHAUST FANS PROVIDING (5) AIR CHANGES PER HOUR.
10. VERIFY AIR CONDITIONING REQUIREMENTS WITH OWNER.
11. LINE FLOOR JOIST CAVITIES WITH 5/8" TYPE "X" GYPSUM BOARD WHERE HVAC OCCURS AS REQUIRED.
12. MECHANICAL SUPPLY VENTS IN THE DWELLING UNIT CEILINGS SHALL HAVE FIRE DAMPERS INSTALLED, UNLESS THE VENTS ARE INSTALLED IN DROPS BELOW THE FIRE RATED FLOOR/CEILING ASSEMBLY.
13. DUCTS INSTALLED WITHIN A SINGLE DWELLING UNIT SHALL BE SIZED IN ACCORDANCE WITH ACCA MANUAL D OR OTHER APPROVED METHODS. DUCTS INSTALLED WITHIN ALL OTHER BUILDINGS SHALL BE SIZED IN ACCORDANCE WITH THE ASHRAE HANDBOOK OF FUNDAMENTALS OR OTHER EQUIVALENT COMPUTATION PROCEDURE. 603.2

ELECTRICAL NOTES:

1. SMOKE DETECTORS ARE REQUIRED AT EACH BEDROOM AND AT HALLWAYS LEADING TO BEDROOMS. SMOKE DETECTORS IN EACH UNIT SHALL BE HARD WIRED TOGETHER IN SERIES WITH BATTERY BACKUP.
2. PROVIDE WIRING FOR DISHWASHER, GARBAGE DISPOSAL, AND RANGE HOOD.
3. GFCI OUTLETS ARE REQUIRED IN BATHROOMS, GARAGES (UP MIN. 18 IN OFF FLOOR), OUTDOORS WITH DIRECT GRADE LEVEL ACCESS, INCLUDING DECKS AND BALCONIES, WITHIN 6 FT. OF ANY SINK, AND IN FRONT AND REAR OF DWELLING.
4. HALLWAYS 10 FT. OR MORE IN LENGTH SHALL HAVE AN OUTLET RECEPTACLE.
5. SPACING FOR ALL ELECTRICAL OUTLETS AT KITCHEN COUNTER AND BAR/ISLAND SHOULD NOT EXCEED 48 IN. O.C.
6. LIGHTS IN CLOSETS SHALL CONFORM TO CLEARANCES SPECIFIED IN NEC 410–8
7. PROVIDE CARBON MONOXIDE DETECTORS IN EACH DWELLING UNIT AS PER THE UTAH STATE AMENDMENT.
8. A LADDER ACCESS AND GFCI OUTLETS WILL BE REQUIRED TO SERVICE THE MECHANICAL UNITS LOCATED ON THE TOP OF THE ROOF.
9. DWELLING UNITS. ALL 120–VOLT, SINGLE PHASE, 15– AND 20–AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS, SUNROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC–FAULT CIRCUIT INTERRUPTER,COMBINATION–TYPE, INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT. NEC 210.12 (A) (NOTE THE UTAH STATE AMENDMENT ONLY APPLIES TO BUILDINGS BUILT UNDER THE IRC)
10. FIRE ALARM MUST MEET NFPA STANDARDS OR JURISDICTION HAVING AUTHORITY.

PLUMBING NOTES:

1. PROVIDE FLOOR DRAIN PAN UNDER WATER HEATERS.
2. PROVIDE SEISMIC STRAPS FOR WATER HEATERS. THESE STRAPS MUST BE LOCATED IN THE UPPER 1/3 AND IN THE LOWER 1/3 OF THE WATER HEATER.
3. SHOWER SIZE MIN. 1024 SQ. IN. WITH A 30 IN. DIA. CIRCLE WITHIN AREA – FINISH TO HEIGHT OF 70 IN. ABOVE DRAIN.
4. WATER CLOSET CLEARANCES MUST BE 30 IN. AT WIDTH, AND 24 IN. IN FRONT OF CLOSET.
5. LOW FLUSH WATER CLOSETS MUST HAVE MIN. OF 1.6 GALLONS PER FLUSH.
6. FLOW RATE OF SHOWER HEAD MUST NOT EXCEED 2.5 GALLONS PER MINUTE.
7. PLUMBING VENTS SHALL BE AT LEAST 36 IN. ABOVE OR 10 FT. BEYOND AIR–INTAKE OPENINGS.
8. FLAGPOLING OF PLUMBING VENTS IS PROHIBITED EXCEPT WHERE ROOF IS OCCUPIED.
9. PROVIDE EXPANSION TANK FOR WATER HEATER.

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SYMBOLS:

Symbol	Meaning
(hatch)	EARTH
(hatch)	GRAVEL
(hatch)	MASONRY
(hatch)	BATT INSULATION
(hatch)	CONCRETE
(symbol)	FLOOR ELEVATION
1 / X–X.X 2 / 3 / 4	INTERIOR ELEVATION
SHT	WALL SECTION FLAG
SHT	BUILDING SECTION FLAG
SHT	DETAIL SYMBOL
(arrow)	NORTH ARROW

APPLICABLE CODES:

2015 IBC
2015 IMC
2015 IPC
2015 IFC
2017 NEC
2015 IFGC
2015 IECC
2009 ANSI 117.1

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
COVER SHEET

PLOT SCALE: NONE
DATE: 6/7/19
JOB: JOB# 19-006
CS_HR40_CS.DWG
JEB

ISSUES/REV:	DATE:
CITY COMMENTS (1)	7/25/19
VALUE ENGINEERING (2)	9/24/19

CS

MINIMUM FASTENER SCHEDULE

1.	JOIST TO SILL PLATE OR GIRDER, TOENAIL	(3) 8d
2.	BRIDGING TO JOIST, TOENAIL EACH END	(2) 8d
3.	SILL SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d @ 16" O.C.
4.	TOP PLATE TO STUD, END NAIL	(2) 16d
5.	STUD TO SOLE PLATE	(4) 8d TOENAIL, OR 16d END NAIL
6.	DOUBLE STUDS, FACE NAIL	16d @ 24" O.C.
7.	DOUBLE TOP PLATES, FACE NAIL	16d @ 12" O.C.
8.	TOP PLATES, LAPS AND INTERSECTION, FACE NAIL	(2) 16d
9.	CONTINUOUS HEADER, TWO PIECES, ALONG EACH FACE	16d @ 16" O.C.
10.	CEILING JOIST TO PLATE, TOE NAIL	(3) 8d
11.	CONTINUOUS HEADER TO STUD, TOE NAIL	(4) 8d
12.	CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	(3) 16d
13.	CEILING JOIST TO PARALLEL RAFTERS, FACE NAIL	(3) 16d
14.	RAFTERS TO PLATE, TOE NAIL	(3) 8d
15.	BUILT-UP GIRDERS AND BEAMS	16d @ 32" O.C. T & B STAGGERED (2) 20d EACH END AND EACH SPLICE

LEGEND OF MARKS AND ABREVIATIONS

AB	ANCHOR BOLT(S)	JT	JOINT
ABV	ABOVE	JST	JOIST
ALT	ALTERNATE		
APPROX	APPROXIMATE	k	KIP(S)=1000 POUNDS
ARCH	ARCHITECT(URAL)	KLF	KIPS PER LINEAL FOOT
		KSF	KIPS PER SQUARE FOOT
BLDG	BUILDING		
BLW	BELOW	LBS	POUNDS
BM	BEAM	LLH	LONG LEG HORIZONTAL
BOT	BOTTOM	LLV	LONG LEG VERTICAL
BRG	BEARING		
BTWN	BETWEEN	MAX	MAXIMUM
		MECH	MECHANICAL
CC.	CENTER-TO CENTER	MFR	MANUFACTURER
C.J.	CONST/CONTROL JOINT	MIN	MINIMUM
COL	COLUMN	MISC	MISCELLANEOUS
CONC	CONCRETE		
CONST	CONSTRUCTION	NTS	NOT TO SCALE
CRW-x	CONCRETE RETAINING WALL		
CTR	CENTER	O.C.	ON CENTER
CW-x	CONCRETE WALL	OF	OUTSIDE FACE
		OPNG	OPENING
DBA	DEFORMED BAR ANCHOR	OPP	OPPOSITE
DBL	DOUBLE		
DET	DETAIL	PCF	POUNDS PER CUBIC FOOT
DIA	DIAMETER	PL	PLATE
DIA	DIAMETER	PLF	POUNDS PER LINEAL FOOT
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DN	DOWN	PSI	POUNDS PER SQUARE INCH
DWG	DRAWING		
DWL	DOWEL	REINF	REINFORCING
		REQD	REQUIRED
EA	EACH	RD	ROOF DRAIN
EF	EACH FACE	RTU	ROOF TOP UNITS
EJ	EXPANSION JOINT		
ELEC	ELECTRICAL	SBP-x	STEEL BASE PLATE MARK
ELEV	ELEVATION	SC-x	STEEL COLUMN MARK
EQUIP	EQUIPMENT	SCP-x	STEEL CAP PLATE MARK
EQ	EQUAL	SHT	SHEET
EW	EACH WAY	SIM	SIMILAR
EXST	EXISTING	SMU	SUSPENDED MECHANICAL UNITS
EXP	EXPANSION	SOG	SLAB-ON-GRADE
EXT	EXTERIOR	SQ	SQUARE
		STAG	STAGGERED
FC-x	CONTINUOUS FOOTING MARK	STD	STANDARD
FD	FLOOR DRAIN	STD	STANDARD
FDN	FOUNDATION	STL	STEEL
FF	FINISHED FLOOR	STR	STRUCTURAL
FR-x	RECTANGULAR FOOTING MARK	STS	SELF TAPPING SCREWS
FS-x	SQUARE FOOTING MARK		
FT	FOOT	T&B	TOP AND BOTTOM
FTG	FOOTING	TEMP	TEMPERATURE
FTS-x	THICKEN SLAB MARK	TOC	TOP OF CONCRETE
		TOF	TOP OF FOOTING
GA	GAUGE	TOS	TOP OF STEEL
GALV	GALVANIZED	TOW	TOP OF WALL
GSN	GENERAL STRUCTURAL NOTES	TYP	TYPICAL
HB	HORIZONTAL BRIDGING	UNO	UNLESS NOTED OTHERWISE
HORIZ	HORIZONTAL		
HSA	HEADED STUD ANCHOR	VERT	VERTICAL
HT	HEIGHT		
		W/	WITH
IBC	INTERNATIONAL BUILDING CODE	WWF	WELDED WIRE FABRIC
IF	INSIDE FACE	WWM	WELDED WIRE MESH
IN	INCH		
INT	INTERIOR		

GENERAL

1. THE STRUCTURAL NOTES ARE INTENDED TO COMPLIMENT THE PROJECT SPECIFICATIONS. SPECIFIC NOTES AND DETAILS IN THE DRAWINGS SHALL GOVERN OVER THE STRUCTURAL NOTES AND TYPICAL DETAILS.

2. TYPICAL DETAILS AND SECTIONS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN.

3. THE CONTRACTOR SHALL VERIFY ALL SITE CONDITIONS AND DIMENSIONS. IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN IN THE CONTRACT DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE FABRICATION OR CONSTRUCTION OF ANY EFFECTED ELEMENTS.

4. OMISSIONS OR CONFLICTS BETWEEN THE CONTRACT DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH ANY WORK INVOLVED. IN CASE OF CONFLICT, FOLLOW THE MOST STRINGENT REQUIREMENT AS DIRECTED BY THE ARCHITECT/ENGINEER AT NO ADDITIONAL COST TO THE OWNER.

5. THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH ANY CHANGES, SUBSTITUTIONS OR MODIFICATIONS. ANY WORK DONE BY THE CONTRACTOR BEFORE RECEIVING WRITTEN APPROVAL WILL BE AT THE CONTRACTORS RISK.

6. THE CONTRACTOR SHALL COORDINATE WITH ALL TRADES ANY ITEMS THAT ARE TO BE INTEGRATED INTO THE STRUCTURAL SYSTEM SUCH AS OPENINGS, PENETRATIONS, MECHANICAL AND ELECTRICAL EQUIPMENT, ETC. SIZES AND LOCATIONS OF MECHANICAL AND OTHER EQUIPMENT THAT DIFFERS FROM THOSE SHOWN ON THE CONTRACT DRAWINGS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER.

7. THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING AND BRACING AS REQUIRED FOR HIS METHOD OF ERECTION. SHORING AND BRACING SHALL REMAIN IN PLACE UNTIL FINAL CONNECTIONS FOR THE PERMANENT MEMBERS ARE COMPLETED. THE BUILDING SHALL NOT BE CONSIDERED STABLE UNTIL ALL CONNECTIONS ARE COMPLETED. WALLS SHALL NOT BE CONSIDERED SELF-SUPPORTING AND SHALL BE BRACED UNTIL THE FLOOR/ROOF SYSTEM IS COMPLETED.

8. SITE OBSERVATIONS BY THE STRUCTURAL ENGINEER SHALL NOT BE CONSTRUED AS APPROVAL OF CONSTRUCTION PROCEDURES NOR SPECIAL INSPECTION.

9. REVIEW OF SHOP DRAWING SUBMITTALS BY STRUCTURAL ENGINEER IS FOR GENERAL COMPLIANCE ONLY AND IS NOT INTENDED FOR APPROVAL. THE SHOP DRAWING REVIEW SHALL NOT RELIEVE THE CONTRACTOR FROM THE RESPONSIBILITY OF COMPLETING THE PROJECT ACCORDING TO THE CONTRACT DOCUMENTS.

BASIS OF DESIGN

1. GOVERNING BUILDING CODE — INTERNATIONAL BUILDING CODE 2015
   a. RISK CATEGORY — II

2. ROOF SNOW LOAD
   a. GROUND SNOW LOAD — P_g = 42.9 PSF
   b. SNOW IMPORTANCE FACTO — I_s = 1.0
   c. SNOW EXPOSURE COEFFICIENT — C_e = 1.0
   d. THERMAL EXPOSURE COEFFICIENT — C_t = 1.0
   e. ROOF SNOW LOAD — $P_f = 0.7 * C_e * C_t * I_s * P_g$ = 30 PSF PLUS SNOW DRIFT

3. FLOOR LIVE LOADS
   a. RESIDENTIAL — 40 PSF

4. SEISMIC LOADS
   a. SHORT PERIOD MAPPED SPECTRAL ACCELERATION — S_s = 1.386
   b. SOIL SITE CLASS — D
   c. SHORT PERIOD SITE COEFFICIENT — F_a = 1.00
   d. 5% DAMPED DESIGN SPECTRAL RESPONSE ACCELERATION — S_{ds} = 2/3*F*S = 0.831
   e. SEISMIC IMPORTANCE FACTOR — I_e = 1.00
   f. RESPONSE MODIFICATION COEFFICIENT (LIGHT FRAMED WOOD SHEAR WALLS) — R = 6.5
   g. SEISMIC RESPONSE COEFFICIENT — $C_s = S_{ds} * I_e / R$
   h. WEIGHT — DEAD LOADS OF STRUCTURE
   i. BUILDING SEISMIC DESIGN CATEGORY — D
   j. SYSTEM OVERSTRENGTH FACTOR — 3
   k. DEFLECTION AMPLIFICATION FACTOR — 4
   l. BASE SHEAR — V= C*W= 0.128W (STRENGTH)

5. WIND LOADS
   a. WIND VELOCITY (3 SECOND GUST) — 115 MPH (ENCLOSED)
   b. EXPOSURE TYPE — B
   c. WIND IMPORTANCE FACTOR — 1.00
   d. WIND DIRECTIONALITY FACTOR — 0.85
   e. GUST EFFECT FACTOR — 0.85

POST INSTALLED ANCHORS

1. EPOXY/ADHESIVE ANCHORS
   a. EPOXY/ADHESIVE FOR CONCRETE SHALL BE "HIT-HY 200 SAFE SET" (ICC-ESR-3187) OR "HIT-RE 500-SD" (ICC-ESR-2322) BY HILTI CORP, "SET-XP" (ICC-ESR2508) OR "AT-XP" (IAPMO-UESER-263) BY SIMPSON STRONG-TIE.
2. MECHANICAL ANCHORS
   a. MECHANICAL ANCHORS FOR CONCRETE SHALL BE KWIK BOLT TZ-CS (ICC-ESR-1917) BY HILTI CORP OR STRONG-BOLT 2 (ICC-ESR-3037) BY SIMPSON STRONG-TIE.
3. SCREW ANCHORS
   a. SCREW ANCHORS FOR CONCRETE SHALL BE KWIK HUS-EZ (ICC-ESR-3027) BY HILTI CORP OR TITEN HD (ICC-ESR-2713) BY SIMPSON STRONG-TIE.
4. POWDER ACTUATED FASTENERS
   a. POWDER ACTUATED FASTENERS FOR CONCRETE SHALL BE X-U UNIVERSAL KNURLED SHANK FASTENERS (ICC-ESR-2269) BY HILTI CORP, PDP OR PHN (ICC-ESR-2138) BY SIMPSON STRONG-TIE.
   b. POWDER ACTUATED FASTENERS FOR STEEL SHALL BE X-U P8 TH UNIVERSAL KNURLED SHANK FASTENERS (ICC-ESR-2269) BY HILTI CORP OR PDPA (ICC-ESR-2138) BY SIMPSON STRONG-TIE.
5. ADDITIONAL ANCHORS MAY BE USED WITH ICC-ESR OR IAPMO-UES EVALUATION REPORTS PROVIDED.
6. ALL HOLES THAT MAY BE REQUIRED SHALL BE DRILLED AND PROPERLY PREPARED ACCORDING TO THE MANUFACTURES RECOMMENDATIONS FOR THE SPECIFIC ANCHORS AND EPOXY/ADHESIVE SYSTEM USED.
7. IN ALL CASES FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION.

CONCRETE

1. MATERIALS, UNLESS NOTED OTHERWISE:
   a. NORMAL WEIGHT AGGREGATES — ASTM C 33
   b. REINFORCING STEEL — ASTM 615 GRADE 60 (Fy=60 ksi) USE GRADE 40 (Fy=40 ksi) FOR FIELD BENT DOWEL WITH SPACING INDICATED REDUCED BY 1/3.
   c. ANCHOR RODS
      GRAVITY COLUMNS — ASTM F1554, GRADE 36, WITH ASTM A563 HEAVY HEX NUTS AND HARDENED WASHERS GRADE A
   d. ADMIXTURES
      i. AIR-ENTRAINING ADMIXTURES COMPLY WITH ASTM C 260 (WHEN USED).
      ii. CALCIUM CHLORIDE SHALL NOT BE ADDED TO THE CONCRETE MIX.
   e. TYPE I/II CEMENT COMPLYING WITH ASTM C-150 SHALL BE USED FOR ALL CONCRETE.
   f. THE WATER/CEMENT RATIOS SHALL MEET THE REQUIREMENTS OF ACI 318.
   g. PROVIDE AIR ENTRAINING AS RECOMMENDED BY ACI 318.
   h. NO ALUMINUM CONDUIT OR PRODUCT CONTAINING ALUMINUM OR ANY OTHER MATERIAL INJURIOUS TO CONCRETE SHALL BE EMBEDDED IN CONCRETE.
2. COMPRESSIVE STRENGTHS OF CONCRETE AT 28 DAYS SHALL BE AS FOLLOWS:
   a. EXTERIOR FOOTINGS AND FOUNDATION WALLS..........3,000 psi (F1,S0,P0,C1)
   b. INTERIOR FOOTINGS AND FOUNDATIONS WALLS..........3,000 psi (F0,S0,P0,C0)
   c. INTERIOR SLABS ON GRADE..........4,000 psi (F0,S0,P0,C0)
   d. SITE CONCRETE (AREAS EXPOSED TO DEICING CHEMICALS)..........5,000 psi (F3,S0,P1,C2)
   e. SITE CONCRETE (AREAS NOT EXPOSED TO DEICING CHEMICALS)..........4,500 psi (F3,S0,P1)
3. ONLY ONE GRADE OR TYPE OF CONCRETE SHALL BE POURED ON THE SITE AT ANY GIVEN TIME.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, CARE, PLACEMENT AND REMOVAL OF ALL FORMWORK AND SHORES.
   a. SUPPORTING FORMS AND SHORING SHALL NOT BE REMOVED UNTIL STRUCTURAL MEMBERS HAVE ACQUIRED SUFFICIENT STRENGTH TO SAFELY SUPPORT THEIR OWN WEIGHT AND ANY CONSTRUCTION LOAD TO WHICH THEY MAY BE SUBJECTED.
5. REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVER:
   CAST-IN-PLACE CONCRETE:.......... CLEAR COVER
   k. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.......... 3"
   l. FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:
      #6 THRU #18 BARS.......... 2"
      #5 AND SMALLER BARS.......... 1-1/2"
   c. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTRACT WITH GROUND:
      SLABS, WALLS: #11 BARS AND SMALLER.......... 3/4"
      TIES, STIRRUPS, SPIRALS.......... 3/8"
7. CONSTRUCTION JOINTS AND CONTROL JOINTS:
   a. PROVIDE A FORMED AND BEVELED 2x4xCONTINUOUS KEYWAY IN ALL HORIZONTAL AND VERTICAL CONSTRUCTION JOINTS INCLUDING BETWEEN TOP OF FOOTING AND FOUNDATION WALLS, UNLESS NOTED OTHERWISE. IN ADDITION, ALL JOINTS SHALL BE INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF APPROXIMATELY 1/4 INCH.
   b. CONTROL JOINTS SHALL BE INSTALLED IN SLABS ON GRADE SO THE LENGTH TO WIDTH RATIO OF THE SLAB IS NO MORE THAN 1.25:1. CONTROL JOINTS SHALL BE COMPLETED WITHIN 12 HOURS OF CONCRETE PLACEMENT. CONTROL JOINTS MAY BE INSTALLED BY:
      i. SAW CUT A DEPTH OF 1/4 THE THICKNESS OF THE SLAB
      ii. TOOLED JOINTS A DEPTH OF 1/4 THE THICKNESS OF THE SLAB
   c. INSTALL CONSTRUCTION OR CONTROL JOINTS IN SLABS ON GRADE AT A SPACING NOT TO EXCEED 30 TIMES THE SLAB THICKNESS IN ANY DIRECTION FOR UNREINFORCED SLABS AND 75 TIMES THE SLAB THICKNESS IN ANY DIRECTION FOR REINFORCED SLABS, UNLESS NOTED OTHERWISE. CONSTRUCTION JOINTS SHALL NOT EXCEED A DISTANCE OF 30'-0" O.C. IN ANY DIRECTION.
8. CONSTRUCTION
   a. USE CHAIRS OR OTHER SUPPORT DEVICES RECOMMENDED BY THE CRSI TO SUPPORT AND TIE REINFORCEMENT BARS AND WWF PRIOR TO PLACING CONCRETE. WWF SHALL BE CONTINUOUSLY SUPPORTED AT 36" O.C. MAXIMUM. REINFORCING STEEL FOR SLABS ON GRADE SHALL BE ADEQUATELY SUPPORTED ON PRECAST CONCRETE UNITS. LIFTING THE REINFORCING OFF THE GRADE DURING PLACEMENT OF CONCRETE IS NOT PERMITTED.
   b. CONCRETE TO BE MECHANICALLY CONSOLIDATED DURING PLACEMENT PER ACI STANDARDS.
   c. CONTRACTOR SHALL COORDINATE PLACEMENT OF ALL OPENINGS, CURBS, DOWELS, SLEEVES, CONDUITS, BOLTS, INSERTS AND OTHER EMBEDDED ITEMS PRIOR TO CONCRETE PLACEMENT.
   d. ALL EMBEDS AND DOWELS SHALL BE SECURELY TIED TO FORMWORK OR TO ADJACENT REINFORCING PRIOR TO THE PLACEMENT OF CONCRETE.
   e. NO PIPES, DUCTS, SLEEVES, ETC SHALL BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. PENETRATIONS THROUGH WALLS WHEN APPROVED SHALL BE BUILT INTO THE WALL PRIOR TO CONCRETE PLACEMENT. PENETRATIONS WILL NOT BE ALLOWED IN FOOTINGS OR GRADE BEAMS UNLESS DETAILED. PIPING SHALL BE ROUTED AROUND THESE ELEMENTS AND FOOTINGS STEPPED TO AVOID PIPING.
   f. REINFORCING BARS SHALL NOT BE WELDED.
9. DETAILING
   a. LAP SPLICE LENGTHS SHALL BE DETAILED TO COMPLY WITH THE "TYPICAL REINFORCING BAR REQUIREMENTS" BELOW. SPLICES MAY BE MADE WITH MECHANICAL SPLICES CAPABLE OF 125% TENSION CAPACITY OF THE BAR BEING SPLICED. MECHANICAL SPLICES SHALL BE THE POSITIVE CONNECTING TYPE COUPLER AND SHALL MEET ALL INTERNATIONAL BUILDING CODE REQUIREMENTS. USE "CADWELD", "LENTON" STANDARD COUPLERS. "BAR-LOCK" OR EQUAL WITH INTERNAL PROTECTOR. IF MECHANICAL SPLICES ARE USED, SPLICES OR COUPLERS ON ADJACENT BARS SHALL BE STAGGERED A MINIMUM OF 24" APART ALONG THE LONGITUDINAL AXIS OF THE REINF. BARS.
   b. AT JOINTS PROVIDE REINFORCING DOWELS TO MATCH THE MEMBER REINFORCING. UNLESS NOTED OTHERWISE.
   c. AT ALL DISCONTINUOUS CONTROL OR CONSTRUCTION SLAB ON GRADE JOINTS, PROVIDE 2-#4x48 INCHES.
   d. PROVIDE CORNER BARS AT INTERSECTING WALL CORNERS USING THE SAME BAR SIZE AND SPACING AS THE HORIZONTAL WALL REINFORCING.
   e. ALL VERTICAL REINFORCING SHALL BE DOWELED TO FOOTINGS, OR TO THE STRUCTURE BELOW WITH THE SAME SIZE AND SPACING AS THE VERTICAL REINFORCING FOR THE ELEMENT ABOVE. DOWELS EXTENDING INTO FOOTINGS SHALL TERMINATE WITH A 90 DEGREE STANDARD HOOK AND SHALL EXTEND TO WITHIN 4" OF THE BOTTOM OF THE FOOTING.
   f. HORIZONTAL WALL REINFORCING SHALL TERMINATE AT ENDS OF WALLS AND OPENINGS INTO THE FAR END OF THE JAMB COLUMN WITH A 90-DEGREE STANDARD HOOK PLUS A 6 BAR DIAMETER EXTENSION. HORIZONTAL WALL REINFORCING SHALL BE CONTINUOUS THROUGH CONSTRUCTION AND CONTROL JOINTS.

FOUNDATION

1. SOILS REPORT BY — CIVIL SOLUTIONS GROUP
   PROJECT # — 18-317
   DATE — 11-5-2018
2. SOIL BEARING PRESSURE: — 1,750 PSF (OVER 18" MIN STRUCTURAL FILL)
3. FROST PROTECTION: — 30" MINIMUM
4. SOIL SITE CLASS — D
5. LATERAL SOIL PRESSURE FLUID EQUIVALENT DENSITY.
   a. ACTIVE..........43 PCF
   b. PASSIVE..........332 PCF
   C. AT-REST..........55 PCF
6. COEFFICIENT OF FRICTION..........0.35

EARTHWORK

1. ALL FOOTINGS SHALL BEAR ON 18" MIN STRUCTURAL FILL EXTENDING DOWN TO UNDISTURBED NATIVE SOIL.

DEFERRED SUBMITTALS

FOR THE PURPOSE OF THIS SECTION, DEFERRED SUBMITTALS ARE DEFINED AS PER SECTION 106.3.4.2 OF THE IBC SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ENGINEER/ARCHITECT FOR THEIR REVIEW FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. DEFERRED STRUCTURAL SUBMITTALS FOR THIS PROJECT ARE:
   1. PRE-ENGINEERED WOOD TRUSSES AND GIRDERS
      (SEE ROOF TRUSSES NOTES FOR LOADING)

WOOD

1. MEMBER GRADES SHALL BE AS FOLLOWS
   a. GLU-LAM BEAM, SIMPLE SPAN — 24F-V4 DF/DF
   b. GLU-LAM BEAM, CANTILEVERED — 24F-V8 DF/DF
   c. JOISTS AND RAFTERS — DOUG FIR #2 OR BETTER
   d. HEADERS — DOUG FIR #2 OR BETTER
   e. POSTS — DOUG FIR #1 OR BETTER
   f. STUDS, EXTERIOR WALL — DOUG FIR #2 OR BETTER
   g. STUDS, INTERIOR NON BEARING — DOUG FIR STUD GRADE OR BETTER
   h. PRE-ENGINEERED TRUSSES/JOISTS — AS PER MANUFACTURER
   i. SILL PLATES IN CONTACT WITH CONCRETE — AS PER MANUFACTURER
2. CONNECT ALL WOOD TO CONCRETE, WOOD TO STEEL, AND WOOD TO WOOD WITH SIMPSON METAL CONNECTORS OR EQUIVALENT UNLESS NOTED OTHERWISE. FILL ALL HOLES UNLESS NOTED OTHERWISE IN DRAWINGS OR MANUFACTURERS LITERATURE.
3. USE REDWOOD OR PRESSURE TREATED LUMBER FOR ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY IN CONTACT WITH EARTH. ALL FASTENERS IN CONTACT WITH PRESSURE TREATED LUMBER OR FIRE TREATED WOOD SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED.
4. ALL SILL PLATE ANCHOR BOLTS SHALL HAVE A 3x3x¼" GALVANIZED PLATE WASHER. SEE SHEAR WALL SCHEDULE FOR SIZE AND SPACING OF ANCHOR BOLTS.
5. BLOCK ALL HORIZONTAL EDGES OF WALL SHEATHING WITH 2" NOMINAL BLOCKING. BLOCK EDGES ON FLOORS AND ROOFS AS DIRECTED ON THE DRAWINGS AND SCHEDULES. 2" NOMINAL BLOCKING SHALL BE PROVIDED AT ALL BEARING POINTS OF WOOD TRUSSES AND JOISTS. BRIDGING/BLOCKING SHALL BE DESIGNED AND PROVIDED BY THE JOIST/TRUSS MANUFACTURER FOR ALL APPLICABLE LOADS.
6. MINIMUM NAILING SHALL BE AS PER THE REQUIREMENTS OF THE ADOPTED BUILDING CODE AND LOCAL BUILDING OFFICIAL. ALSO SEE THE MINIMUM FASTENER SCHEDULE.
7. FASTENERS SUCH AS STAPLES CAN ONLY BE SUBSTITUTED FOR NAILS AT A RATE EQUAL TO THE LOAD VALUES PROVIDED BY THE I.C.B.O.

ROOF TRUSSES

1. FRAMING SHALL BE DESIGNED FOR THE FOLLOWING UNIFORM LOADS (ASD).
   ROOF
      TOP CHORD DEAD LOAD — 12PSF
      TOP CHORD LIVE LOAD — 30PSF
      BOTTOM CHORD DEAD LOAD — 6PSF
2. PRE-ENGINEERED FRAMING SHALL BE DESIGNED FOR ALL UNIFORM AND POINT LOADING SHOWN ON THE PLANS AND ALL LOAD COMBINATIONS SPECIFIED IN THE IBC INCLUDING UNBALANCED AND DRIFTING SNOW LOADS. TRUSS MANUFACTURER IS RESPONSIBLE TO PROVIDE WEB AND CHORD MEMBERS TO SATISFY ALL LOADING REQUIREMENTS.
3. LIMIT DEFLECTION TO L/480 FOR LIVE LOADS AND L/300 FOR TOTAL LOADS.
4. VERIFY ALL DIMENSION WITH ARCHITECTURAL DRAWINGS.
5. THE ENGINEER OF RECORD IS NOT RESPONSIBLE FOR THE DESIGN OR INSTALLATION OF THE PRE-ENGINEERED TRUSSES. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW BY THE ENGINEER/ARCHITECT PRIOR TO FABRICATION AND SHALL BE STAMPED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE STRUCTURE IS.
6. ALL TRUSS TO TRUSS AND TRUSS TO BEAM CONNECTORS SHALL BE SPECIFIED BY THE TRUSS MANUFACTURER.
7. THE CONTRACTOR SHALL PROVIDE AND INSTALL ALL TRUSSES/JOISTS AND BRIDGING, BRACING, AND BLOCKING SHOWN ON THE TRUSS/JOIST SHOP DRAWINGS.
8. NO WEB OR CHORD MEMBERS SHALL BE MODIFIED IN THE FIELD. NO PANEL POINT SHALL HAVE MORE THAN ONE PLATE PER TRUSS SIDE. PLATES SHALL HAVE A MINIMUM BITE OF 2½" ON EACH MEMBER SIDE.
9. EACH CHORD SHALL BE INVOLVED IN TWO PANEL POINTS BEFORE BEING SPLICED. PROVIDED ⅛" CAMBER FOR EACH 6FT OF TRUSS, UNLESS NOTED OTHERWISE.

SHEATHING

1. SHEATHING SHALL BE APA RATED CDX OR OSB SHEATHING. SEE SHEATHING SCHEDULE AND SHEAR WALL SCHEDULE FOR SIZE AND NAILING REQUIREMENTS.
2. INSTALL ROOF AND FLOOR SHEATHING WITH THE LONG DIMENSION PERPENDICULAR TO THE SUPPORTS AND STAGGERED THE END JOISTS.
3. BLOCK ALL FRAMING AT ALL BEARING POINTS WITH 2" NOMINAL OR WIDER FRAMING.

STATEMENT OF SPECIAL INSPECTION AND QUALITY ASSURANCE

1. SPECIAL INSPECTION AND QUALITY ASSURANCE, AS REQUIRED BY CHAPTER 17 OF THE IBC, SHALL BE PROVIDED BY AN INDEPENDENT AGENCY EMPLOYED BY THE OWNER. THE CONTRACTOR SHALL COORDINATE AND COOPERATE WITH THE REQUIRED INSPECTIONS. ALL TESTING AND INSPECTION REPORTS SHALL BE SENT WITHIN 24 HOURS OF THE TEST TO THE ARCHITECT, ENGINEER, CONTRACTOR, AND BUILDING DEPARTMENT FOR REVIEW.
2. THE NAMES AND CREDENTIALS OF THE SPECIAL INSPECTORS TO BE USED SHALL BE SUBMITTED TO THE BUILDING OFFICIAL FOR APPROVAL.
3. DUTIES OF THE SPECIAL INSPECTOR:
   a. THE SPECIAL INSPECTOR SHALL REVIEW ALL WORK LISTED BELOW FOR CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THE CURRENT IBC.
   b. THE SPECIAL INSPECTOR SHALL FURNISH SPECIAL INSPECTION REPORTS TO THE CONTRACTOR, OWNER, BUILDING OFFICIAL, ARCHITECT, AND ENGINEER OF RECORD ON A WEEKLY BASIS, OR MORE FREQUENTLY AS REQUIRED BY THE BUILDING OFFICIAL. ALL ITEMS NOT IN COMPLIANCE SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION.
   c. ONCE CORRECTIONS HAVE BEEN MADE BY THE CONTRACTOR, THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT TO THE BUILDING OFFICIAL STATING THAT WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF HIS KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AS WELL AS THE APPLICABLE WORKMANSHIP PROVISIONS IN THE CURRENT IBC.
4. DUTIES AND RESPONSIBILITIES OF THE CONTRACTOR
   a. THE CONTRACTOR SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE OWNER AND THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF WORK. IN ACCORDANCE WITH IBC 1709.1, THE STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGEMENT OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED WITHIN THE "STATEMENT OF SPECIAL INSPECTION".
   b. THE CONTRACTOR SHALL NOTIFY THE RESPONSIBLE SPECIAL INSPECTOR THAT THE WORK IS READY FOR INSPECTION AT LEAST ONE WORKING (24 HOURS MINIMUM) BEFORE SUCH INSPECTION IS REQUIRED.
   c. ALL WORK REQUIRING SPECIAL INSPECTION SHALL REMAIN ACCESSIBLE AND EXPOSED UNTIL IT HAS BEEN OBSERVED BY THE SPECIAL INSPECTOR.
5. PLEASE SEE THE "SPECIAL INSPECTION SCHEDULE" FOR TYPES, EXTENTS, AND FREQUENCY OF SPECIFIC ITEMS REQUIRING SPECIAL INSPECTION AND STRUCTURAL TESTS AS PART OF THIS PROJECT.

SPECIAL INSPECTION SCHEDULE

AREAS REQUIRING SPECIAL INSPECTION:	FREQUENCY CONT.	FREQUENCY PERIODIC	COMMENTS AND/OR REFERENCES
CONCRETE CONSTRUCTION (IBC 1705.3)			
REINFORCING STEEL PLACEMENT		◆	ACI 318: 3.5, 7.1-7.7, IBC 1910.4
ANCHORS CAST INTO CONCRETE		◆	ACI 318: 8.1.3, 21.2.8, IBC 1908.5, 1909.1
ANCHORS POST INSTALLED IN HARDENED CONCRETE		◆	ACI 318: 3.8.6, 8.1.3, 21.2.8, IBC 1909.1
VERIFY REQUIRED DESIGN MIX		◆	ACI 318: Ch4, 5.2-5.4, IBC 1904.2, 1910.2, 1910.3
CONCRETE SAMPLING TO FABRICATE SPECIMANS FOR STRENGTH, SLUMP, AIR CONTENT, AND TEMPERATURE	◆		ASTM C172, ASTM C31, ACI 318: 5.6, 5.8 IBC 1910.10
CONCRETE PLACEMENT	◆		ACI 318: 5.9, 5.10, IBC 1910.6, 1910.7, 1910.8
CURING TEMPERATURE AND TECHNIQUES		◆	ACI 318: 5.11-5.13, IBC 1910.9
FORMWORK		◆	ACI 318: 6.1.1
WOOD CONSTRUCTION (IBC 1705.5, 1705.10.1. 1705.11.2)			
HIGH LOAD DIAPHRAGMS		◆	VERIFY THICKNESS AND GRADE OF SHEATHING, SIZE OF FRAMING MEMBERS AT PANEL EDGES, NAIL/ STAPLE SIZE AND SPACING PER APPROVED PLANS
STRUCTURAL WOOD		◆	IF FASTENERS SPACING < 4" O.C. VERIFY PROPER NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF SHEAR WALLS, DIAPHRAGMS, DRAG STRUTS, BRACES, AND HODOWNS
POST INSTALLED ANCHORS (IBC 1704.15)	◆		IN ACCORDANCE WITH APPROVED ICC ES REPORTS AND MANUFACTURER'S RECOMMENDATIONS. PERIODIC INSPECTIONS MAY BE ALLOWED PER ICC ES REPORT
STRUCTURAL OBSERVATIONS (IBC 1704.6)		◆	PRIOR TO INSTALLING BUILDING WRAP AND ENCLOSING BUILDING EXTERIOR

1

PROFESSIONAL ENGINEER
375048-2202
JOHN E. CHARCHENKO
9-18-19
STATE OF UTAH

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scholz ARCHITECTS

CHLOE'S POINTE APARTMENTS
OGDEN, UTAH

GENERAL STRUCTURAL NOTES

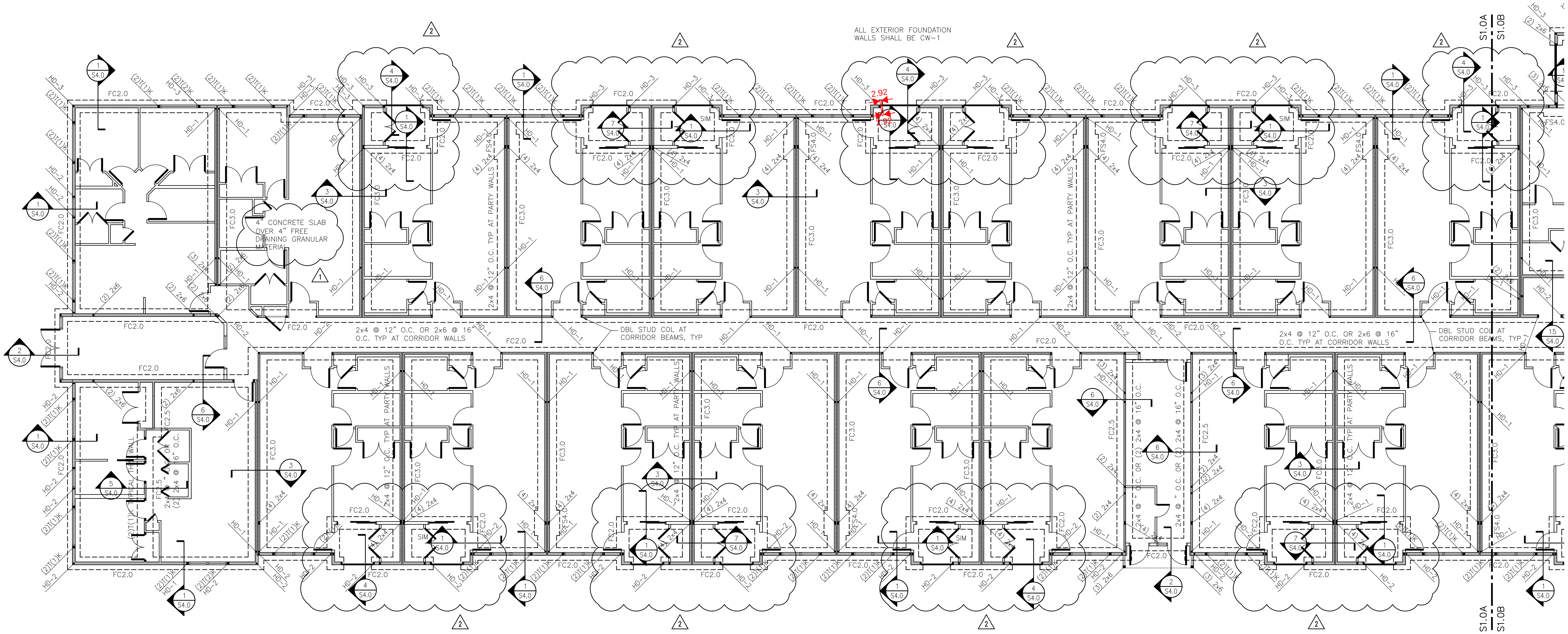
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

1

S0.1

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES



A FOOTING & FOUNDATION PLAN - A
S1.0A 1/8" = 1'-0"

PROFESSIONAL ENGINEER 375048-2202 JOHN E. CHARCHENKO 9-18-19 STATE OF UTAH

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH

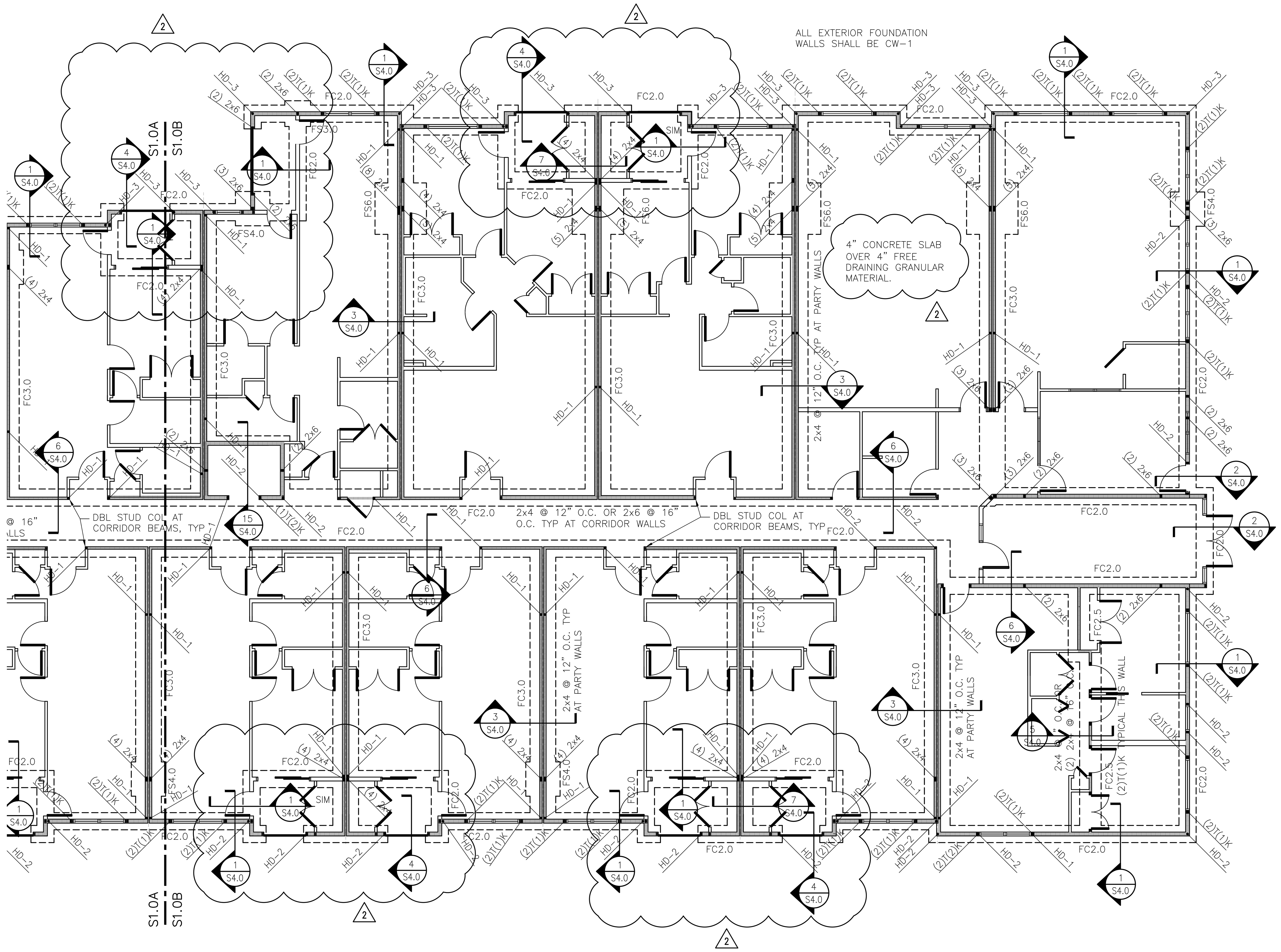
FOOTING & FOUNDATION PLAN - A

PLOT SCALE:	AS NOTED
DATE:	9/18/19
JOB:	JOB# 19-006
	JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
2 STRUCT ADDENDUM 1	9-18-19

S1.0A

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES



B FOOTING & FOUNDATION PLAN - B
S1.0B 1/8" = 1'-0"

PROFESSIONAL ENGINEER
375048-2202
JOHN E. CHARCHENKO
9-18-19
STATE OF UTAH

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CHLOE'S POINTE APARTMENTS

OGDEN, UTAH

FOOTING & FOUNDATION PLAN - B

PLOT SCALE: AS NOTED

DATE: 9/18/19

JOB: JOB# 19-006

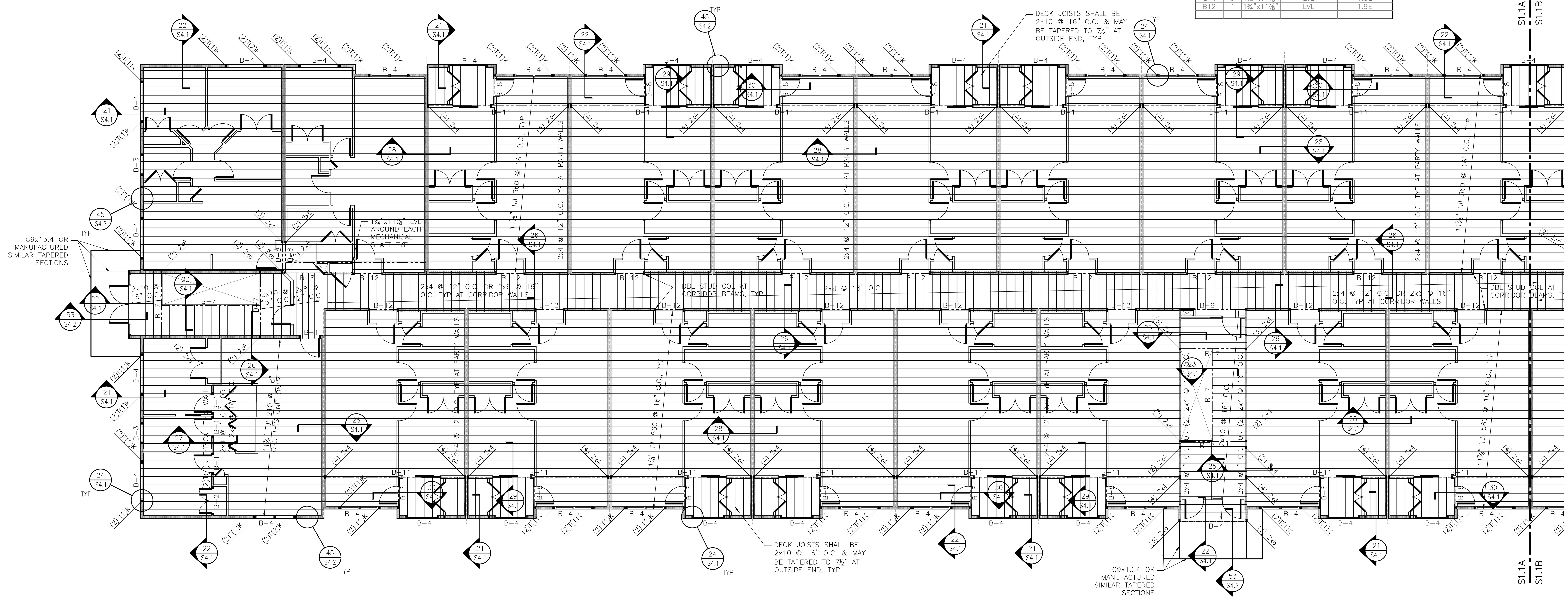
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
2 STRUCT ADDENDUM 1	9-18-19

S1.0B

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF-L#2
B2	2	2x8	DIM. LUMBER	DF-L#2
B3	3	2x6	DIM. LUMBER	DF-L#2
B4	3	2x8	DIM. LUMBER	DF-L#2
B5	3	2x10	DIM. LUMBER	DF-L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E



ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

A
S1.1A
2ND FLOOR FRAMING PLAN - A
1/8" = 1'-0"

PROFESSIONAL ENGINEER 375048-2202 JOHN E. CHARCHENKO 9-18-19 STATE OF UTAH

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH

2ND FLOOR FRAMING PLAN - A

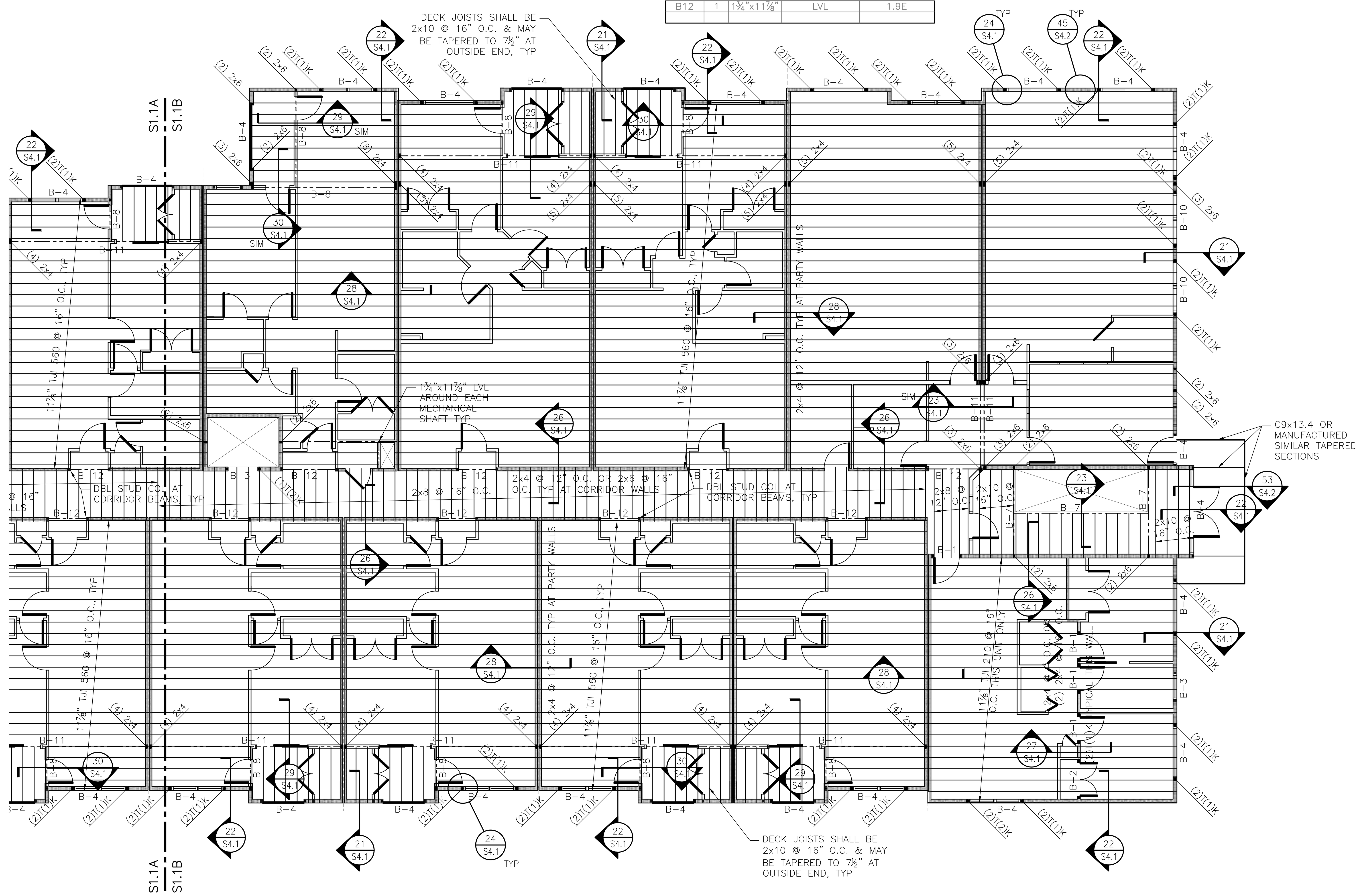
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S1.1A

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF-L#2
B2	2	2x8	DIM. LUMBER	DF-L#2
B3	3	2x6	DIM. LUMBER	DF-L#2
B4	3	2x8	DIM. LUMBER	DF-L#2
B5	3	2x10	DIM. LUMBER	DF-L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E



B 2ND FLOOR FRAMING PLAN - B
S1.1B 1/8" = 1'-0"

ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

PROFESSIONAL ENGINEER 375048-2202 JOHN E. CHARCHENKO 9-18-19 STATE OF UTAH

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CHLOE'S POINTE APARTMENTS

OGDEN, UTAH

2ND FLOOR FRAMING PLAN - B

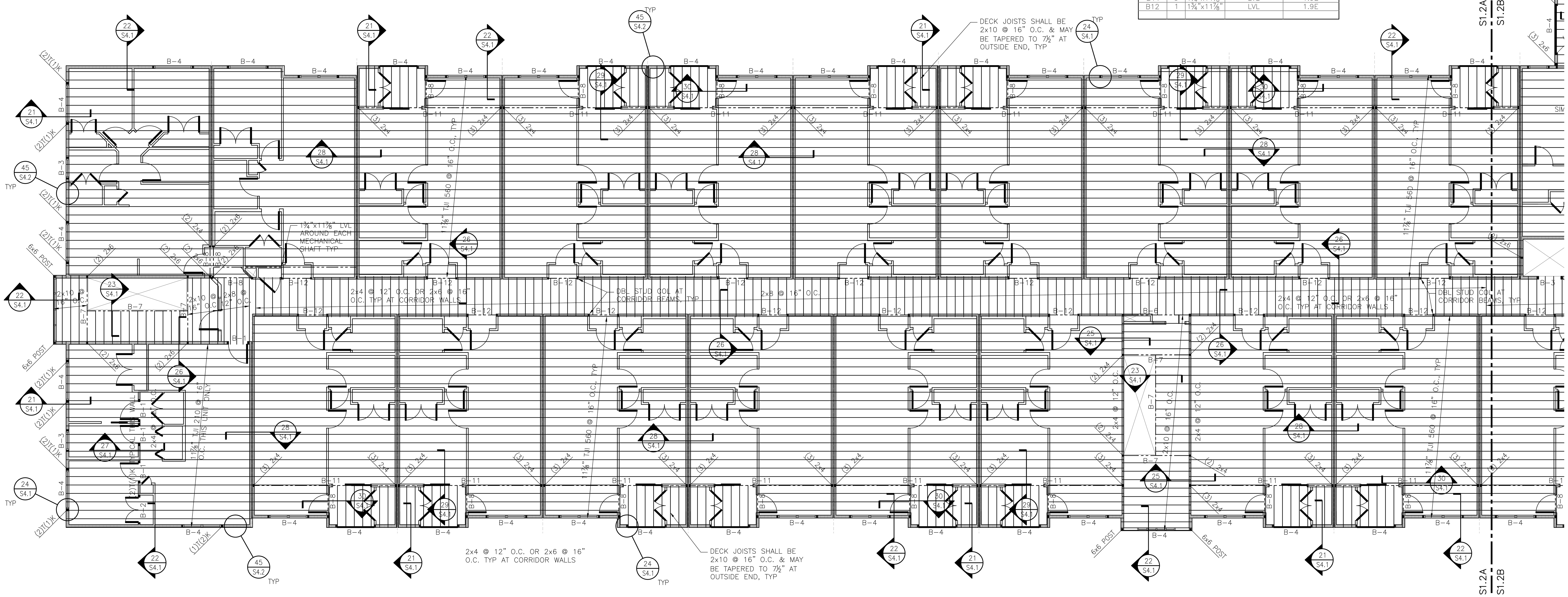
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT. ADDENDUM 1	9-18-19

S1.1B

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF–L#2
B2	2	2x8	DIM. LUMBER	DF–L#2
B3	3	2x6	DIM. LUMBER	DF–L#2
B4	3	2x8	DIM. LUMBER	DF–L#2
B5	3	2x10	DIM. LUMBER	DF–L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E



ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

A / S1.2A — 3RD FLOOR FRAMING PLAN - A
1/8" = 1'-0"

PROFESSIONAL ENGINEER 375048-2202 JOHN E. CHARCHENKO 9-18-19 STATE OF UTAH

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH

2ND FLOOR FRAMING PLAN – A

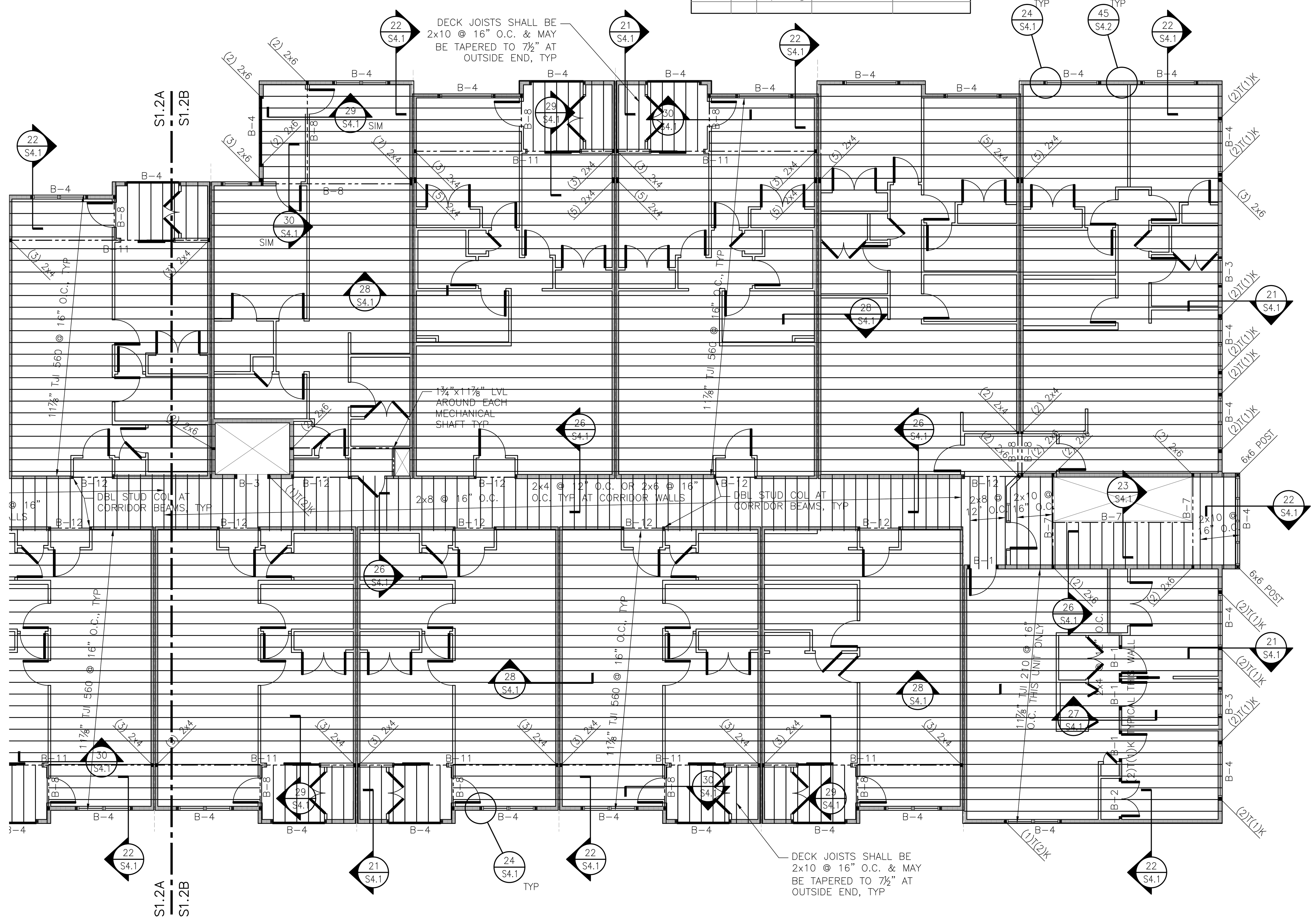
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S1.2A

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF–L#2
B2	2	2x8	DIM. LUMBER	DF–L#2
B3	3	2x6	DIM. LUMBER	DF–L#2
B4	3	2x8	DIM. LUMBER	DF–L#2
B5	3	2x10	DIM. LUMBER	DF–L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E



B 3RD FLOOR FRAMING PLAN - B
S1.2B 1/8" = 1'-0"

ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH

3RD FLOOR FRAMING PLAN – B

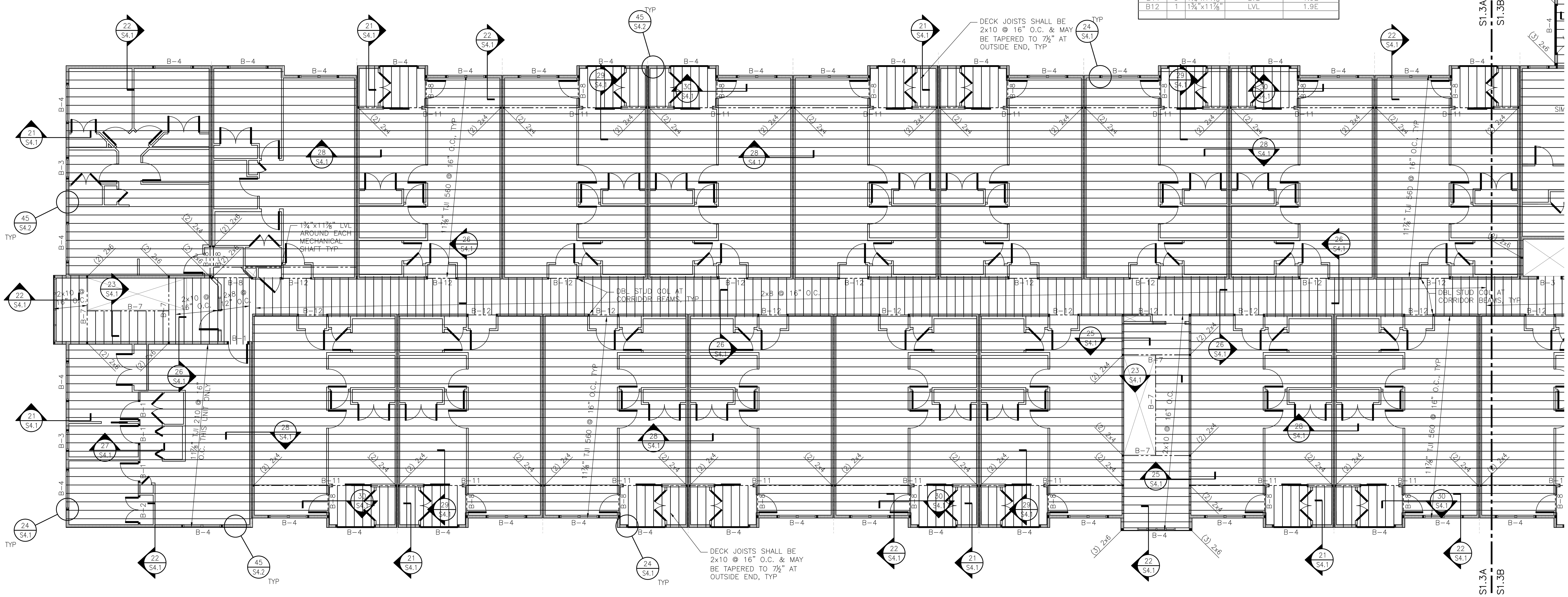
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S1.2B

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF-L#2
B2	2	2x8	DIM. LUMBER	DF-L#2
B3	3	2x6	DIM. LUMBER	DF-L#2
B4	3	2x8	DIM. LUMBER	DF-L#2
B5	3	2x10	DIM. LUMBER	DF-L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E



ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

A / S1.3A 4TH FLOOR FRAMING PLAN - A
1/8" = 1'-0"

PROFESSIONAL ENGINEER
375048-2202
JOHN E. CHARCHENKO
9-18-19
STATE OF UTAH

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
4TH FLOOR FRAMING PLAN - A

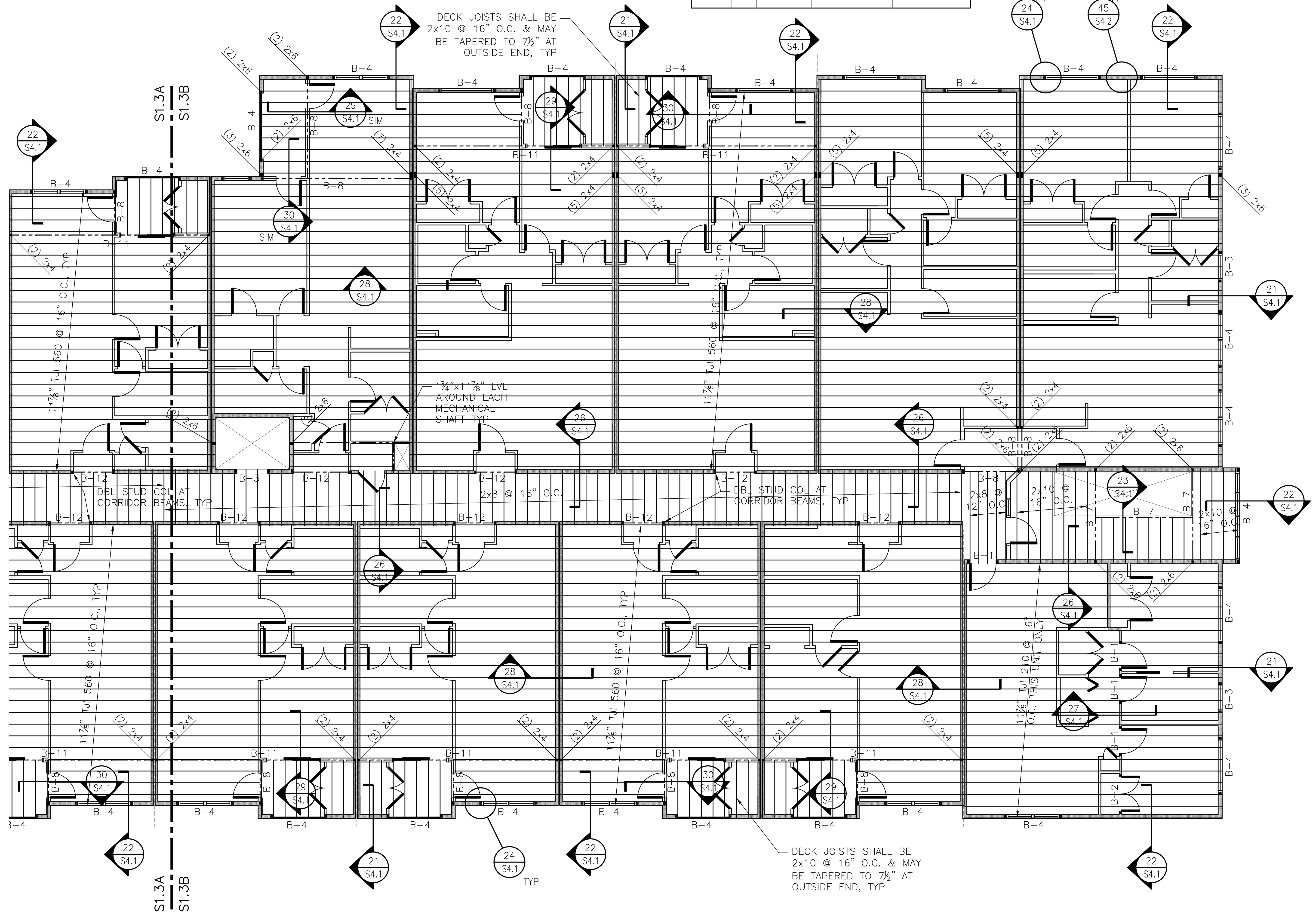
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S1.3A

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF-L#2
B2	2	2x8	DIM. LUMBER	DF-L#2
B3	3	2x6	DIM. LUMBER	DF-L#2
B4	3	2x8	DIM. LUMBER	DF-L#2
B5	3	2x10	DIM. LUMBER	DF-L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E



B 4TH FLOOR FRAMING PLAN - B
S1.3B 1/8" = 1'-0"

ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
3RD FLOOR FRAMING PLAN - B

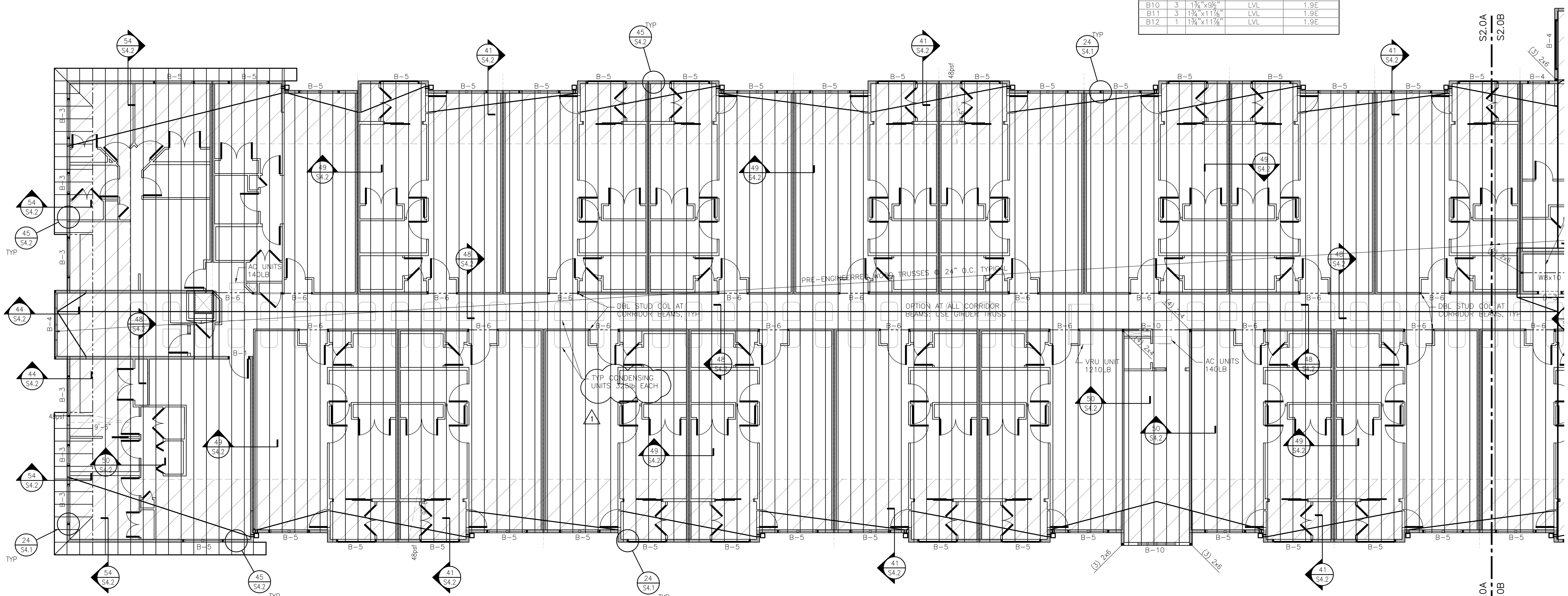
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S1.3B

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF-L#2
B2	2	2x8	DIM. LUMBER	DF-L#2
B3	3	2x6	DIM. LUMBER	DF-L#2
B4	3	2x8	DIM. LUMBER	DF-L#2
B5	3	2x10	DIM. LUMBER	DF-L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E



A ROOF FRAMING PLAN - A
S2.0A 1/8" = 1'-0"

ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
ROOF FRAMING PLAN - A

PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

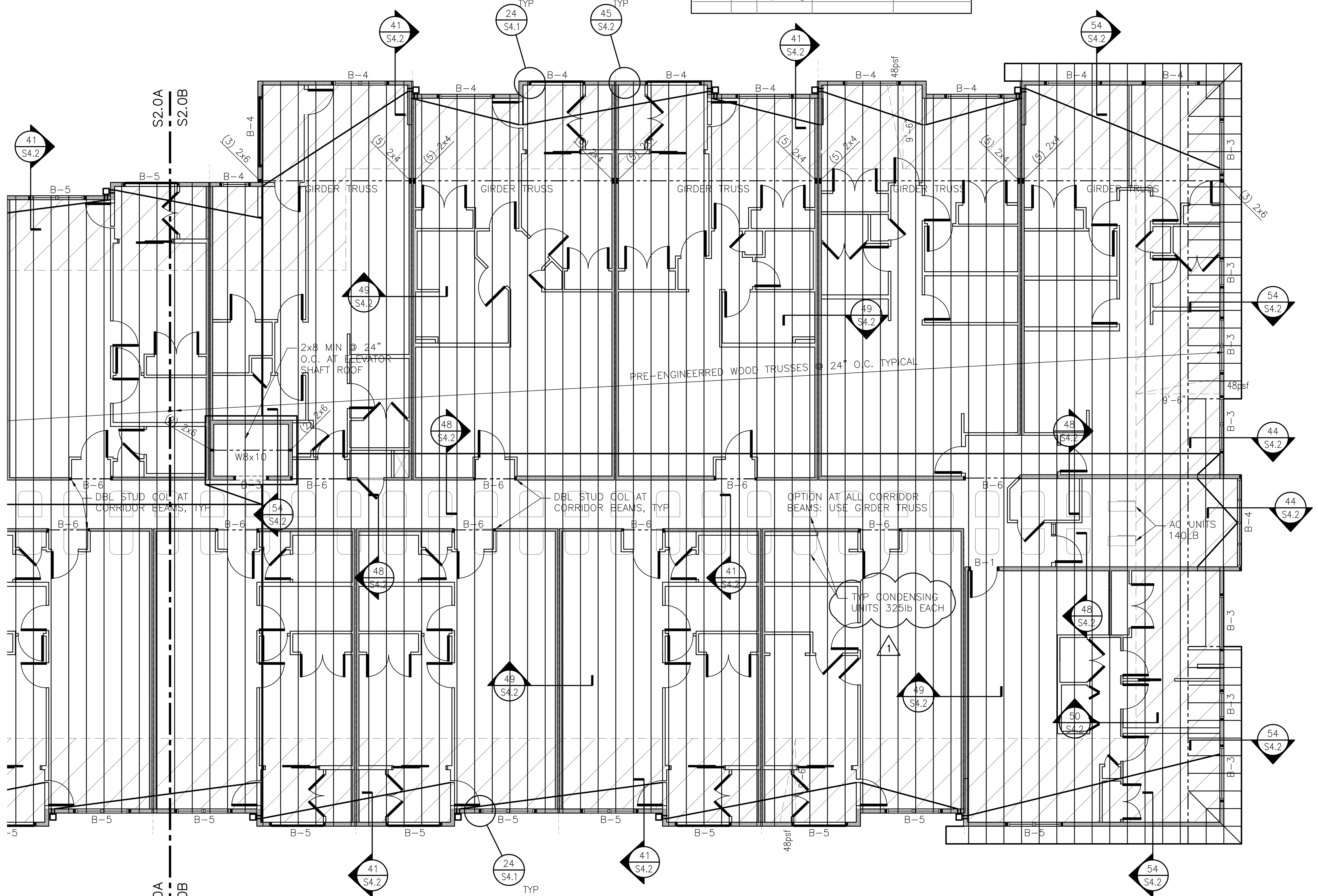
ISSUES/REV:	DATE:
PERMIT SET	6-7-19
1 CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S2.0A

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

BEAM SCHEDULE				
MARK	QTY.	SIZE	TYPE	GRADE
B1	2	2x6	DIM. LUMBER	DF-L#2
B2	2	2x8	DIM. LUMBER	DF-L#2
B3	3	2x6	DIM. LUMBER	DF-L#2
B4	3	2x8	DIM. LUMBER	DF-L#2
B5	3	2x10	DIM. LUMBER	DF-L#2
B6	2	1¾"x7¼"	LVL	1.9E
B7	2	1¾"x9½"	LVL	1.9E
B8	2	1¾"x11⅞"	LVL	1.9E
B9	3	1¾"x7¼"	LVL	1.9E
B10	3	1¾"x9½"	LVL	1.9E
B11	3	1¾"x11⅞"	LVL	1.9E
B12	1	1¾"x11⅞"	LVL	1.9E

1 VERIFY ROOF TOP MECHANICAL LOCATIONS AND WEIGHTS WITH MECHANICAL PLANS



B ROOF FRAMING PLAN - B
S2.0B 1/8" = 1'-0"

ALL JOISTS/TRUSS TO BEAM, BEAM TO BEAM, AND BEAM TO COLUMN CONNECTIONS SHALL BE SIMPSON, USP, OR EQUIVALENT

EXTERIOR WALLS SHALL BE 2x6 @ 16" O.C. INTERIOR WALLS SHALL BE 2x4 @ 16" O.C. U.N.O.

U.N.O. USE (1) TRIMMER & (1) KING STUD AT OPENING

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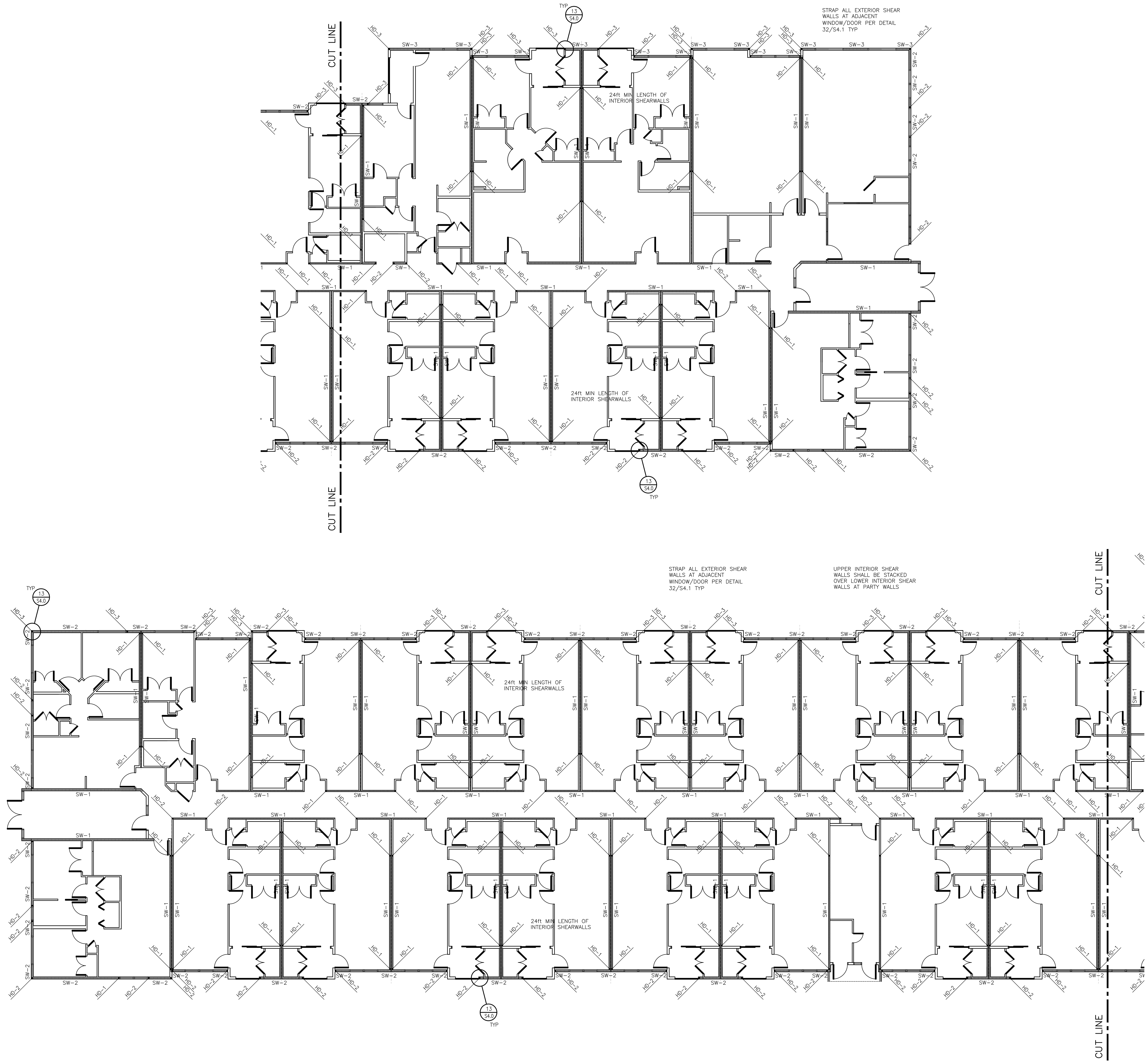
CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
ROOF FRAMING PLAN - B

PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
1 CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S2.0B

SEE SHEET S5.0
FOR NOTES,
LEGENDS, AND
SCHEDULES



A 1ST FLOOR SHEAR WALL PLAN
S3.0 3/32" = 1'-0"

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9-18-19
STATE OF UTAH

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH

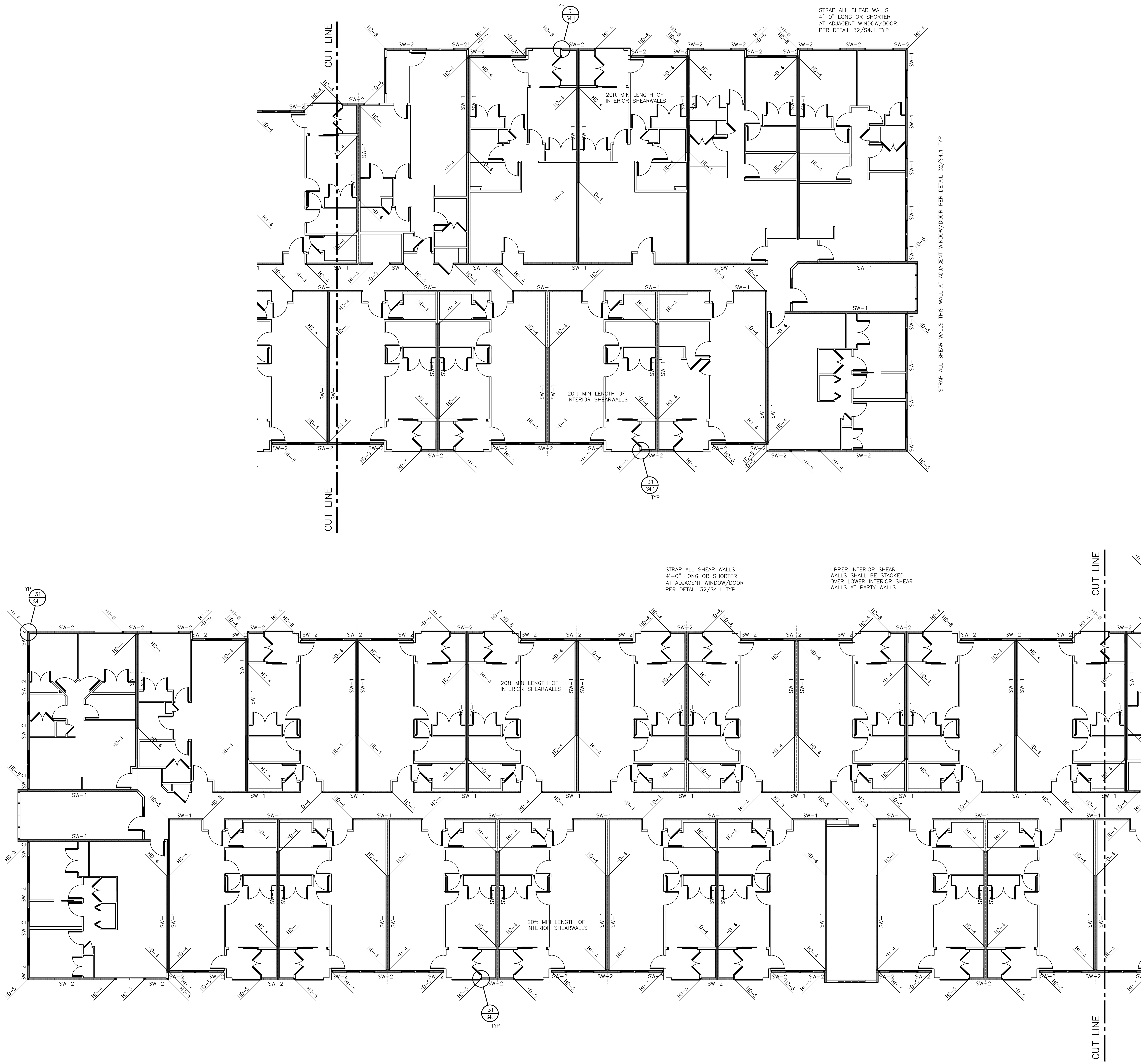
1ST FLOOR SHEAR WALL PLAN

PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S3.0

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES



A / S3.1 — 2ND FLOOR SHEAR WALL PLAN
3/32" = 1'-0"

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9-18-19
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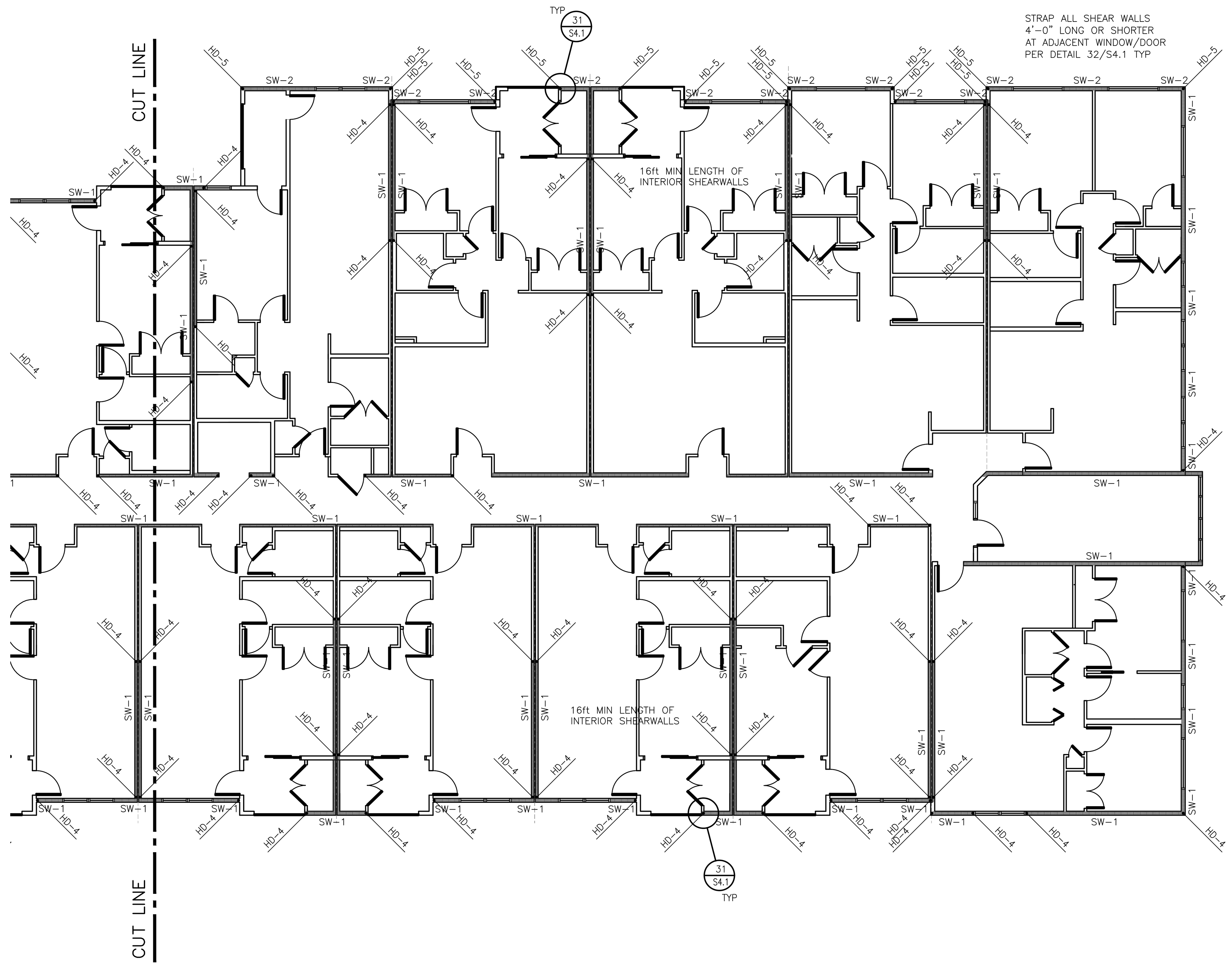
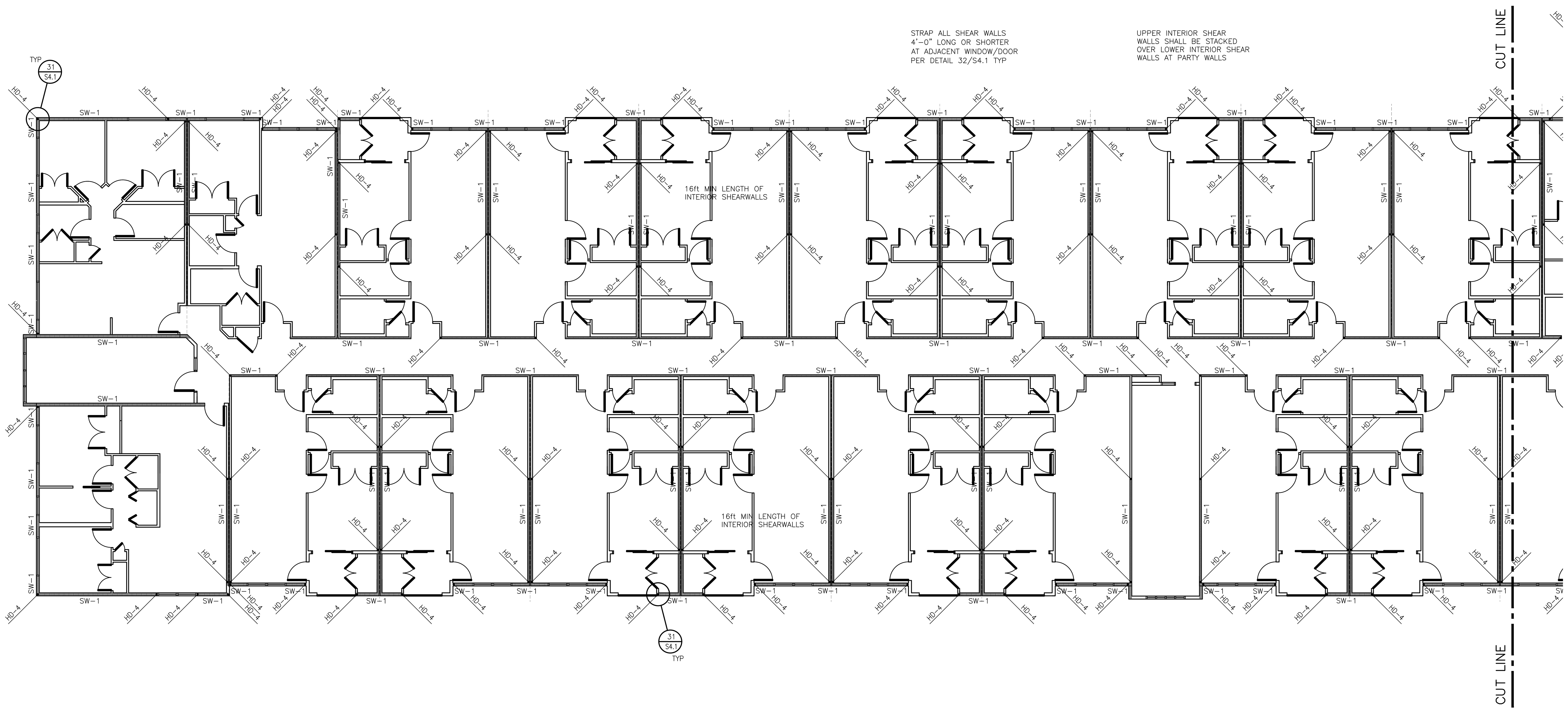
CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
2ND FLOOR SHEAR WALL PLAN

PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S3.1





A / S3.2 — 3RD FLOOR SHEAR WALL PLAN
3/32" = 1'-0"

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES

PROFESSIONAL ENGINEER 375048-2202 JOHN E. CHARCHENKO 9-18-19 STATE OF UTAH

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
3RD FLOOR SHEAR WALL PLAN

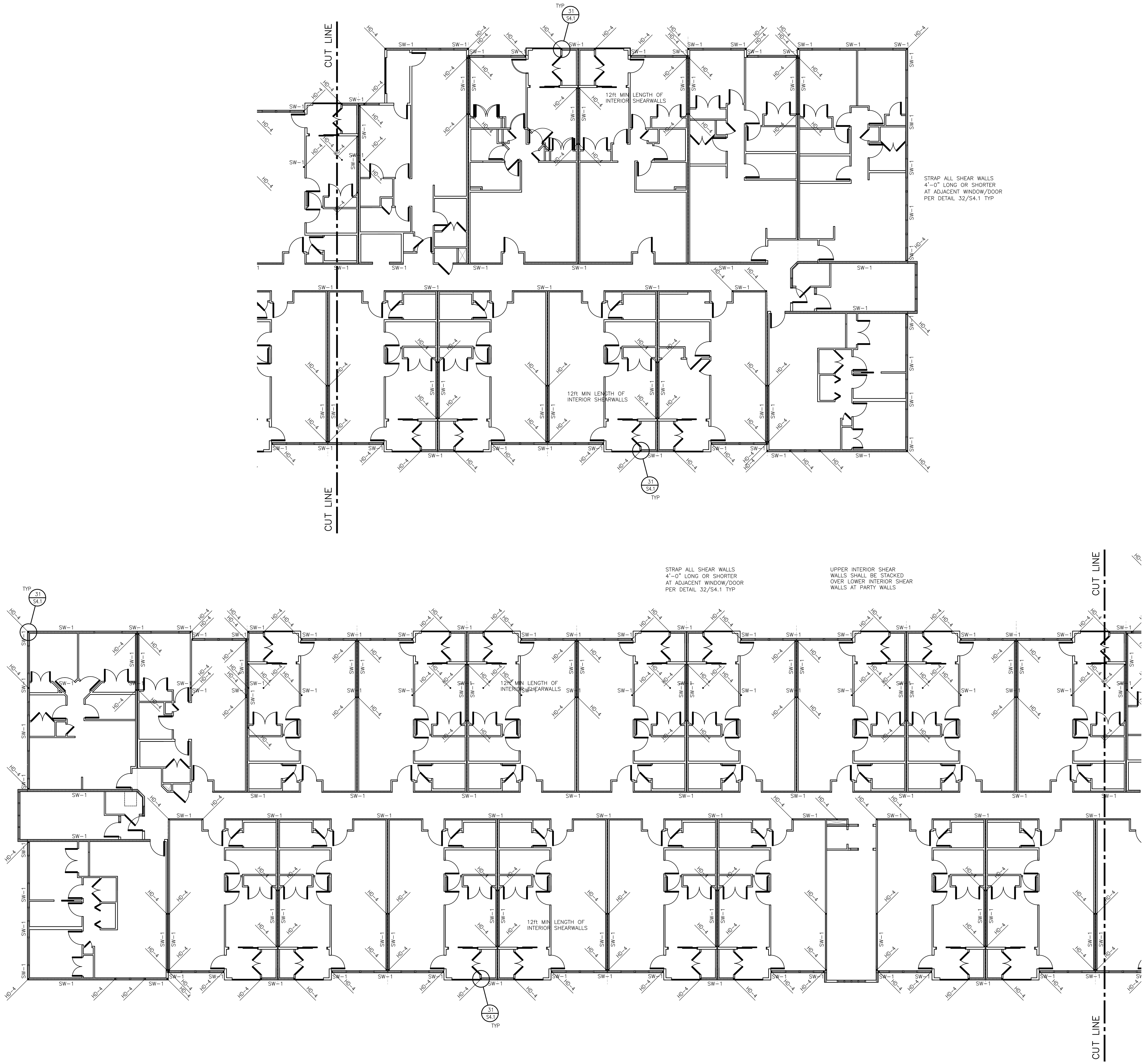
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S3.2

PLANS ARE TO BE CONSIDERED NOT FOR CONSTRUCTION UNLESS SEAL AND SIGNATURE OF ARCHITECT ARE AFFIXED.

SEE SHEET S5.0 FOR NOTES, LEGENDS, AND SCHEDULES



A
S3.2
4TH FLOOR SHEAR WALL PLAN
3/32" = 1'-0"

PROFESSIONAL ENGINEER
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JOHN E. CHARCHENKO
9-18-19
STATE OF UTAH

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scholz ARCHITECTS

CHLOE'S POINTE APARTMENTS

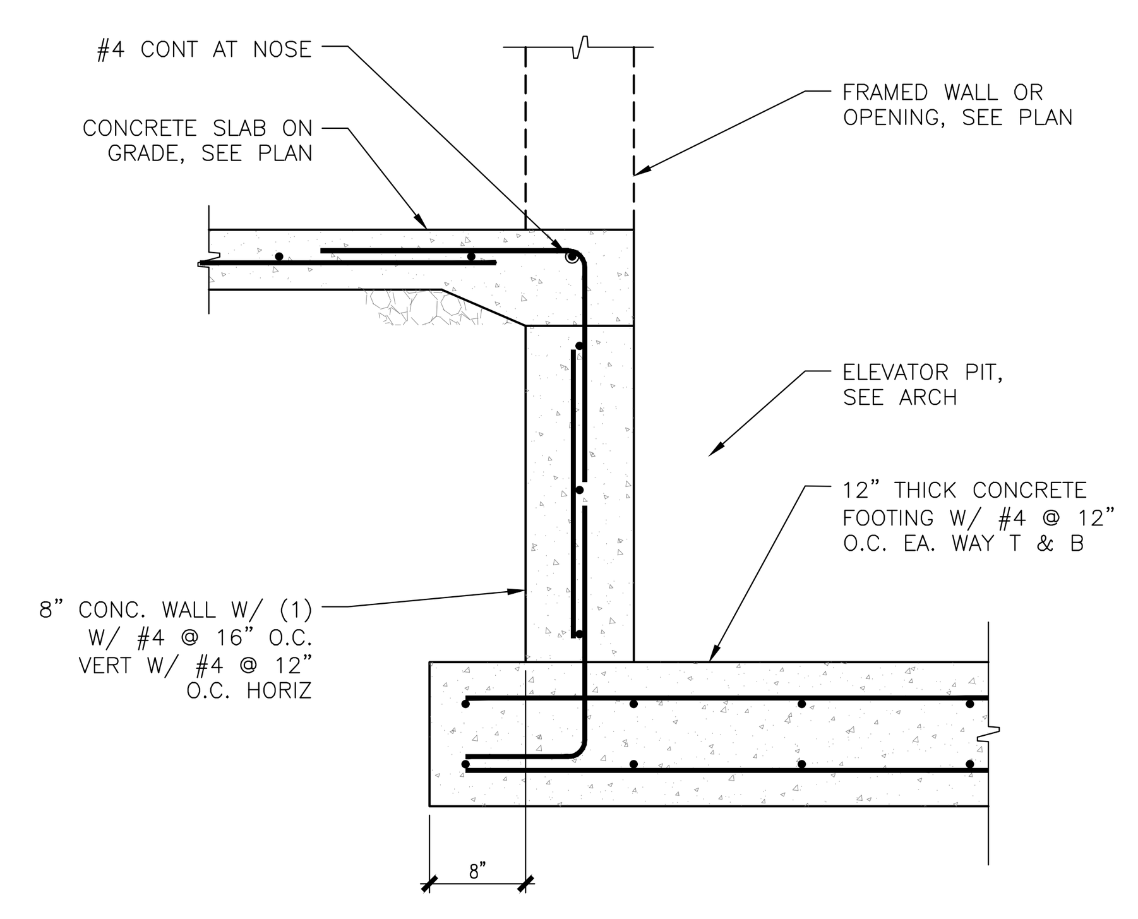
OGDEN, UTAH

4TH FLOOR SHEAR WALL PLAN

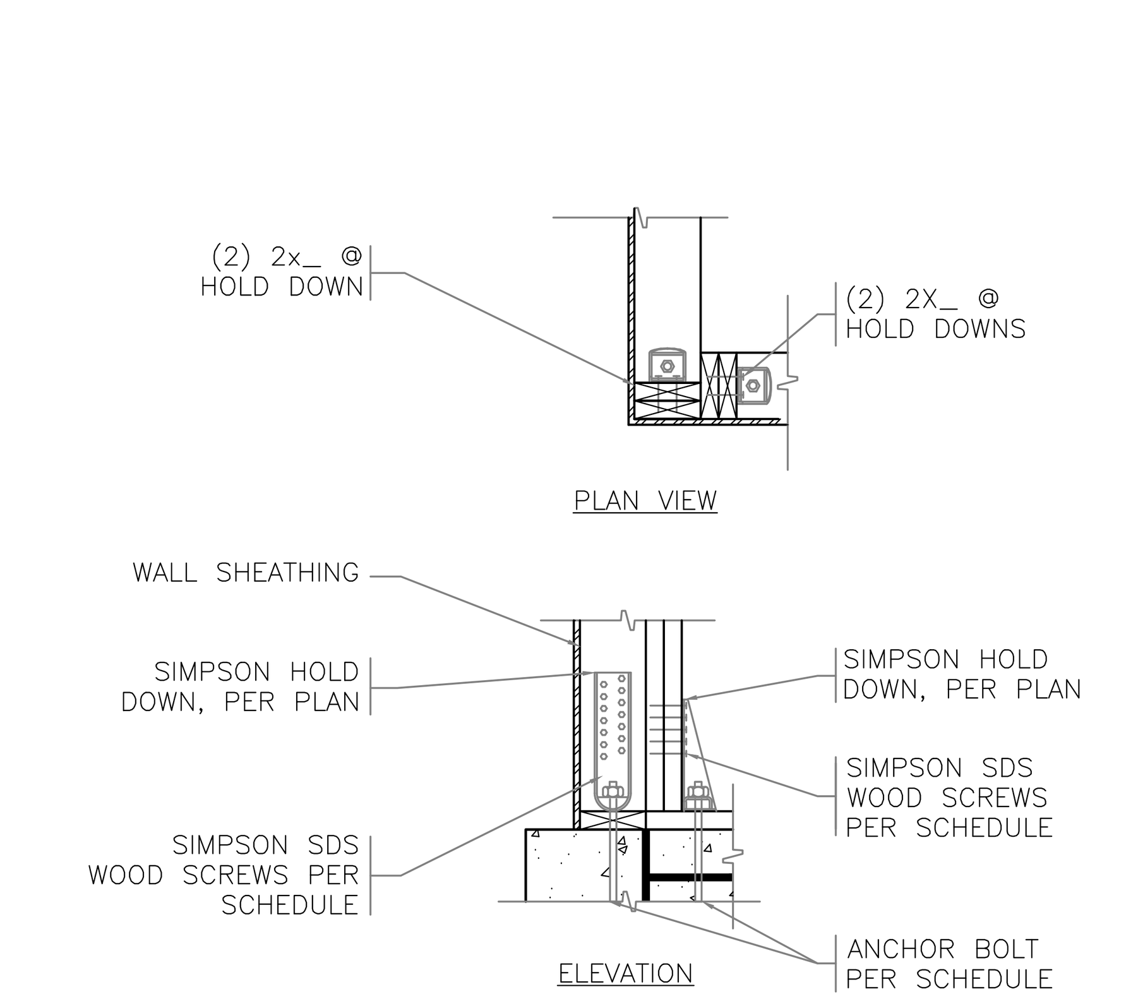
PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

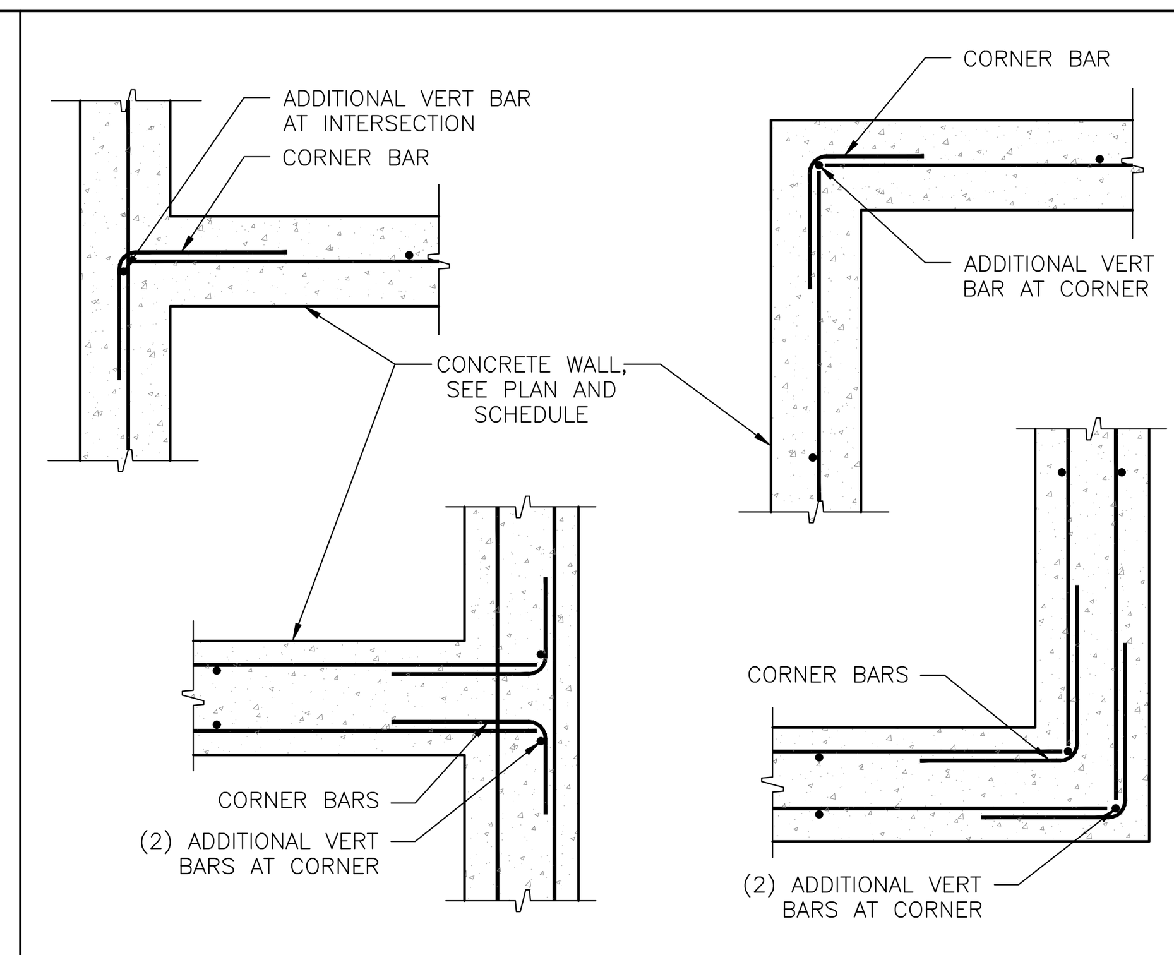
ISSUES/REV:	DATE:
PERMIT SET	6-7-19
CITY RESPONSE	7-22-19
STRUCT ADDENDUM 1	9-18-19

S3.3

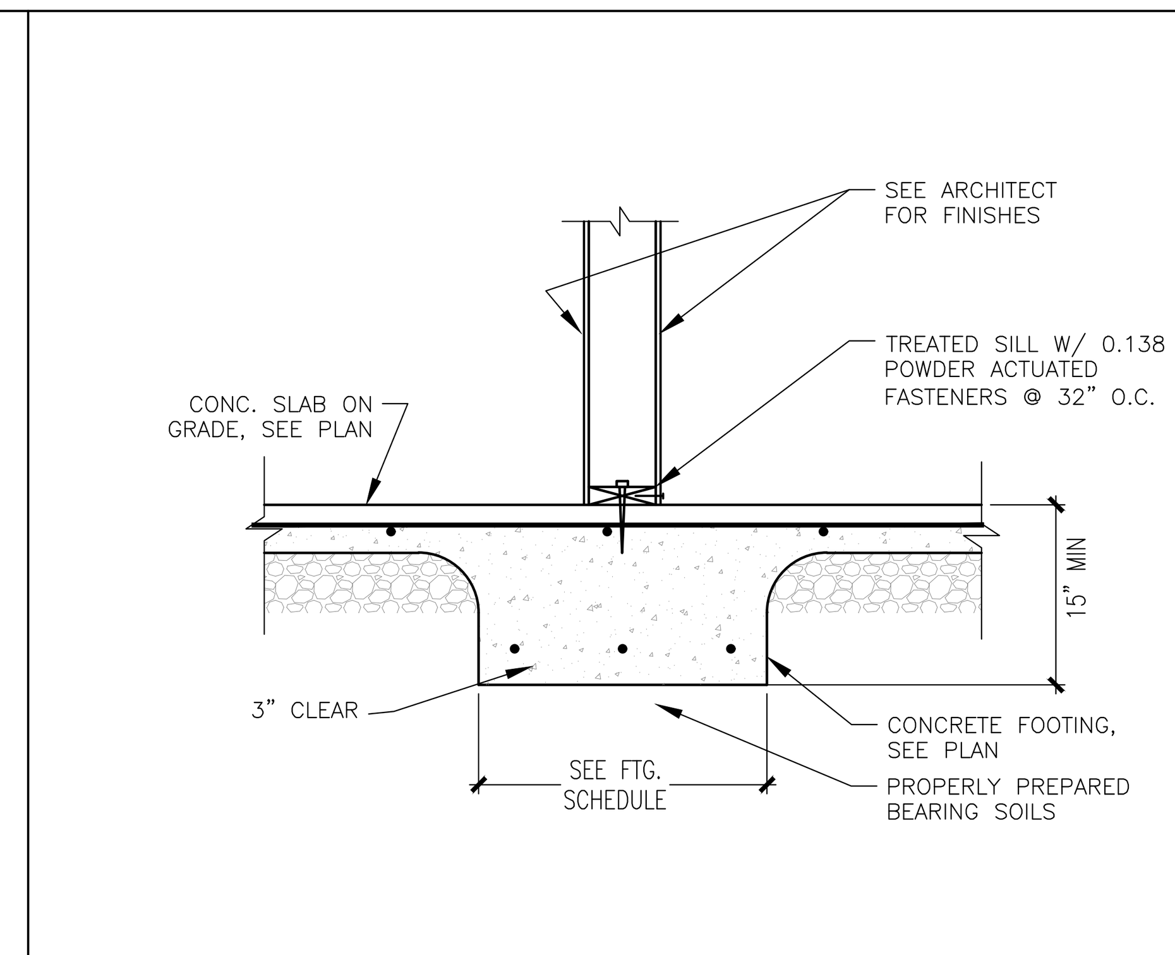


15 ELEVATOR PIT @ OPENING
N.T.S.

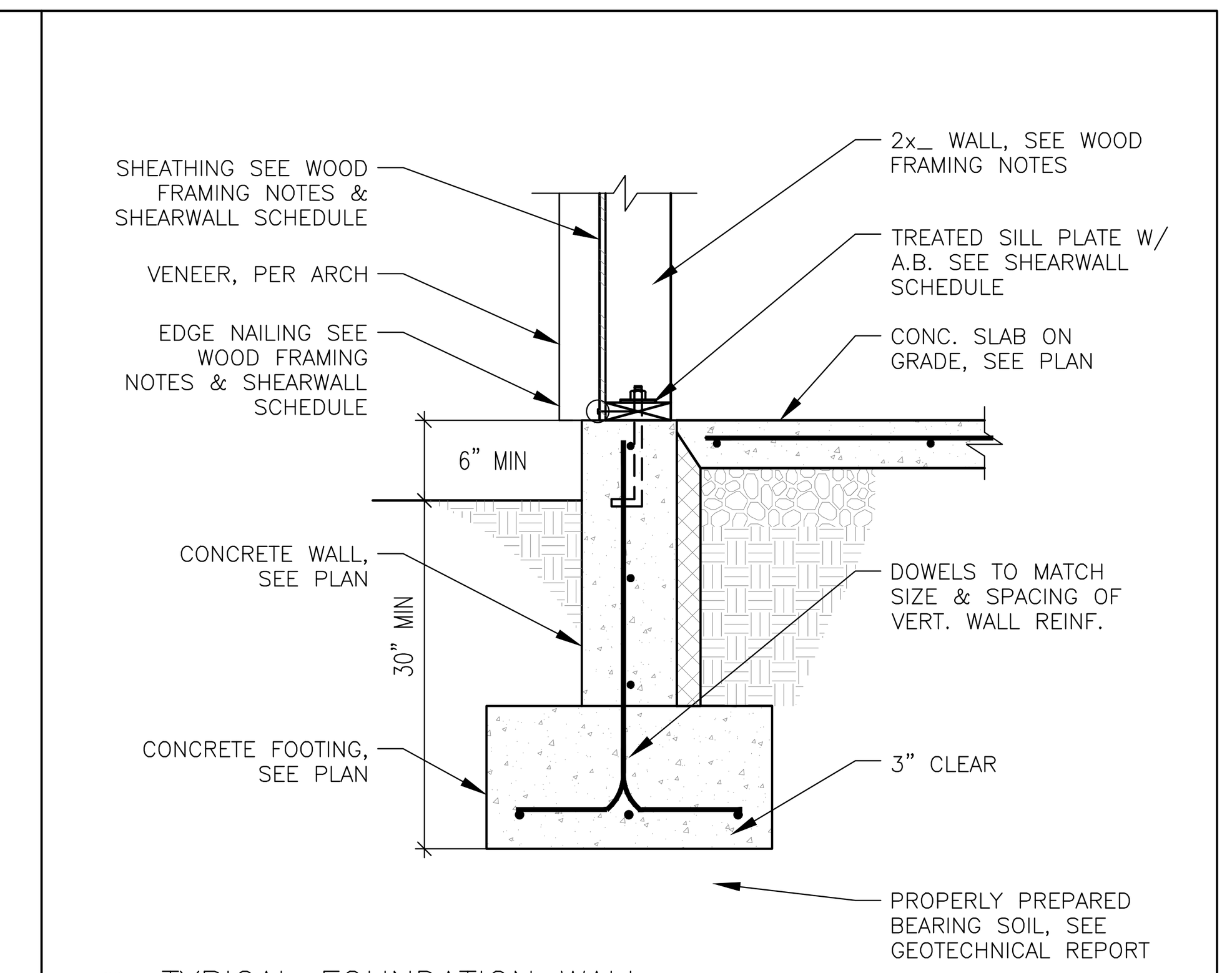




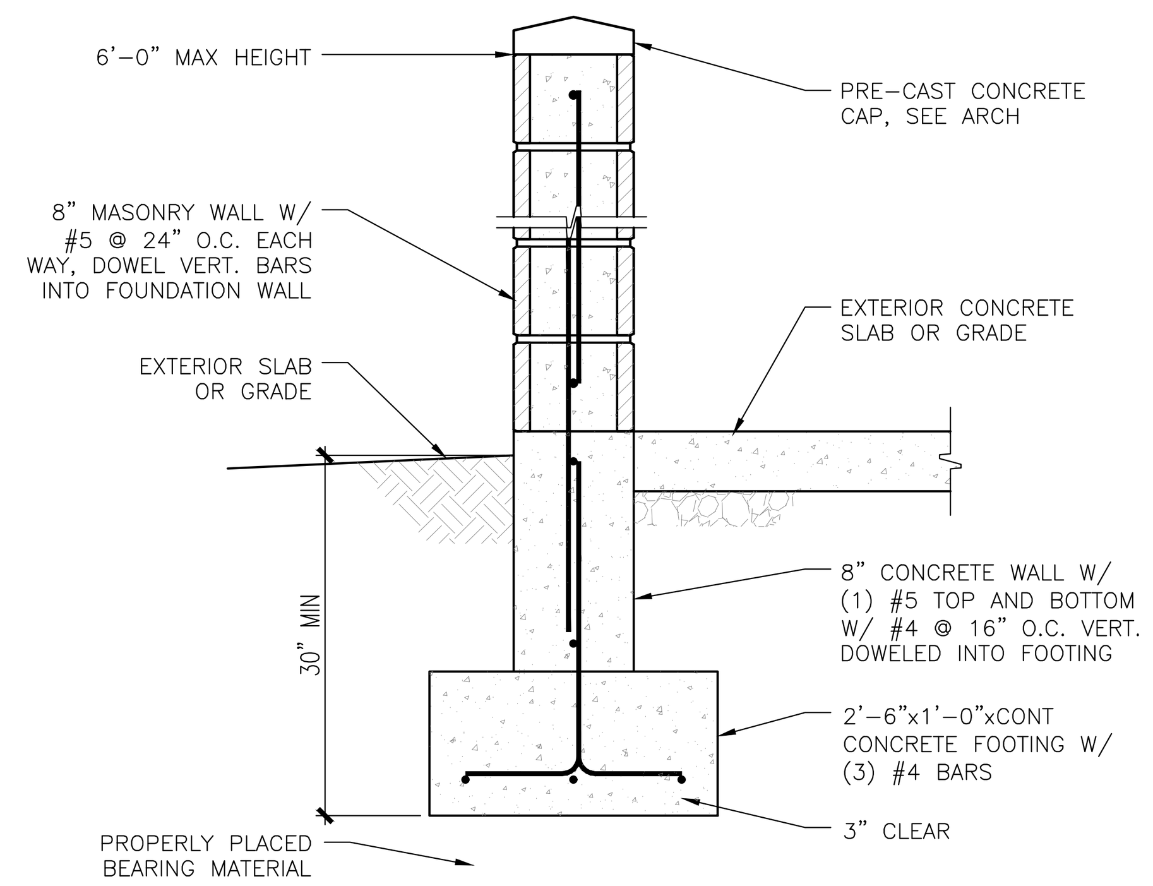
9 WALL REINFORCING AT CORNER & INTERSECTIONS
N.T.S.



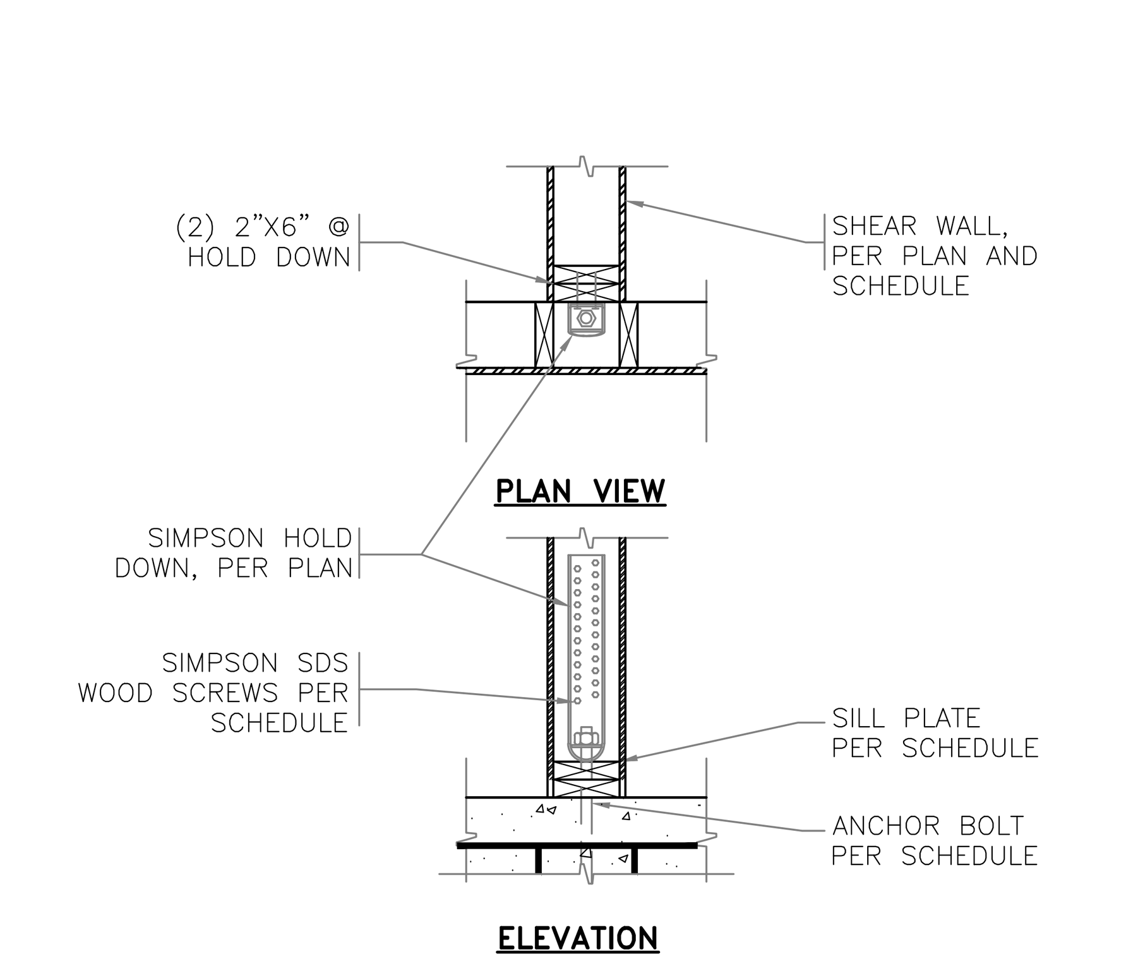
5 CONT. INTERIOR FOOTING AT BEARING WALL
N.T.S.

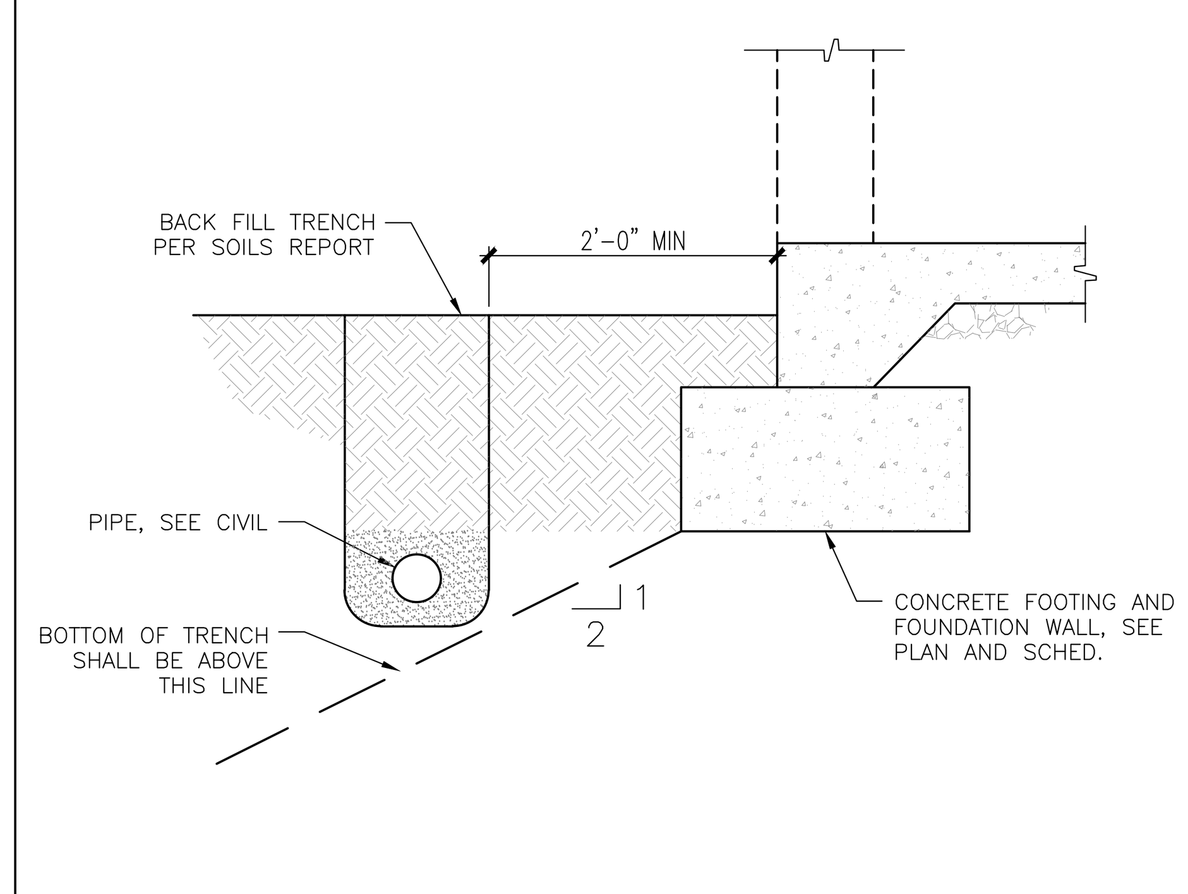


1 TYPICAL FOUNDATION WALL
N.T.S.

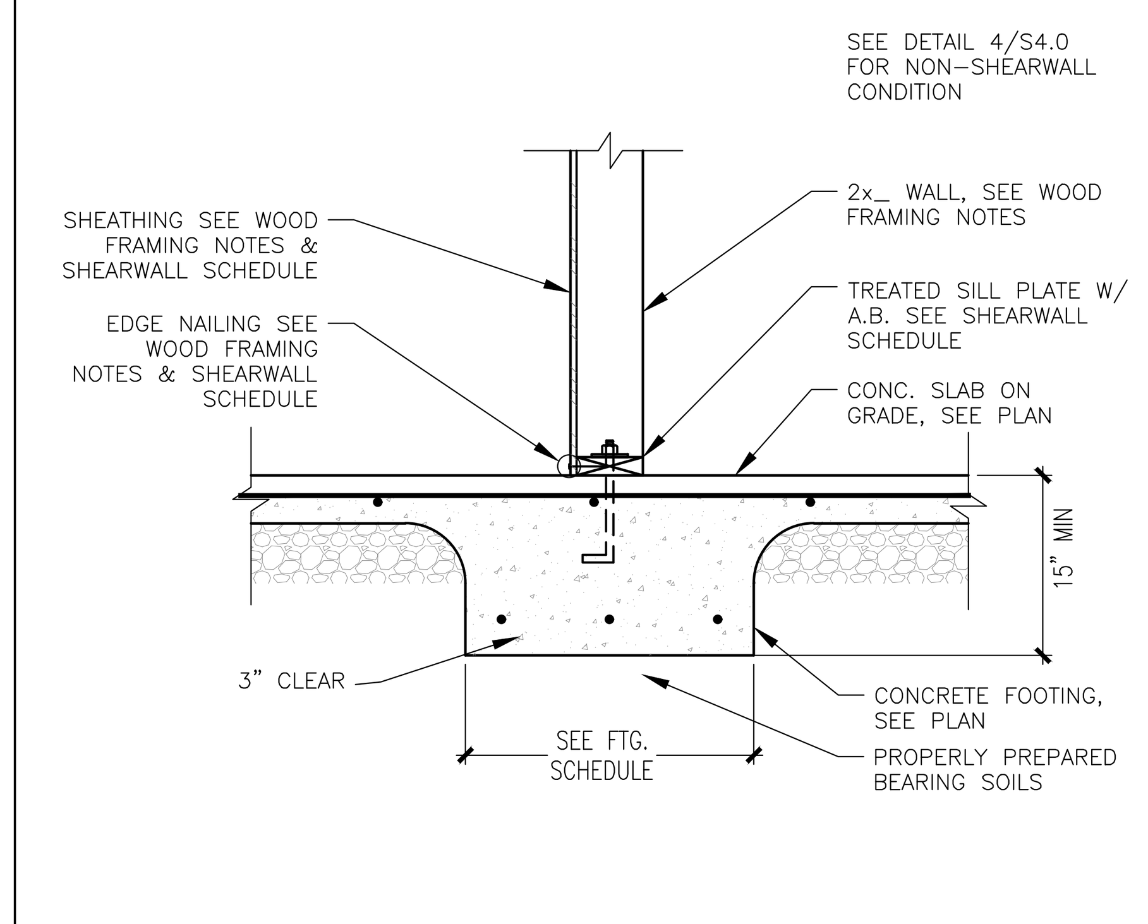


16 TYPICAL TRASH ENCLOSURE WALL
N.T.S.

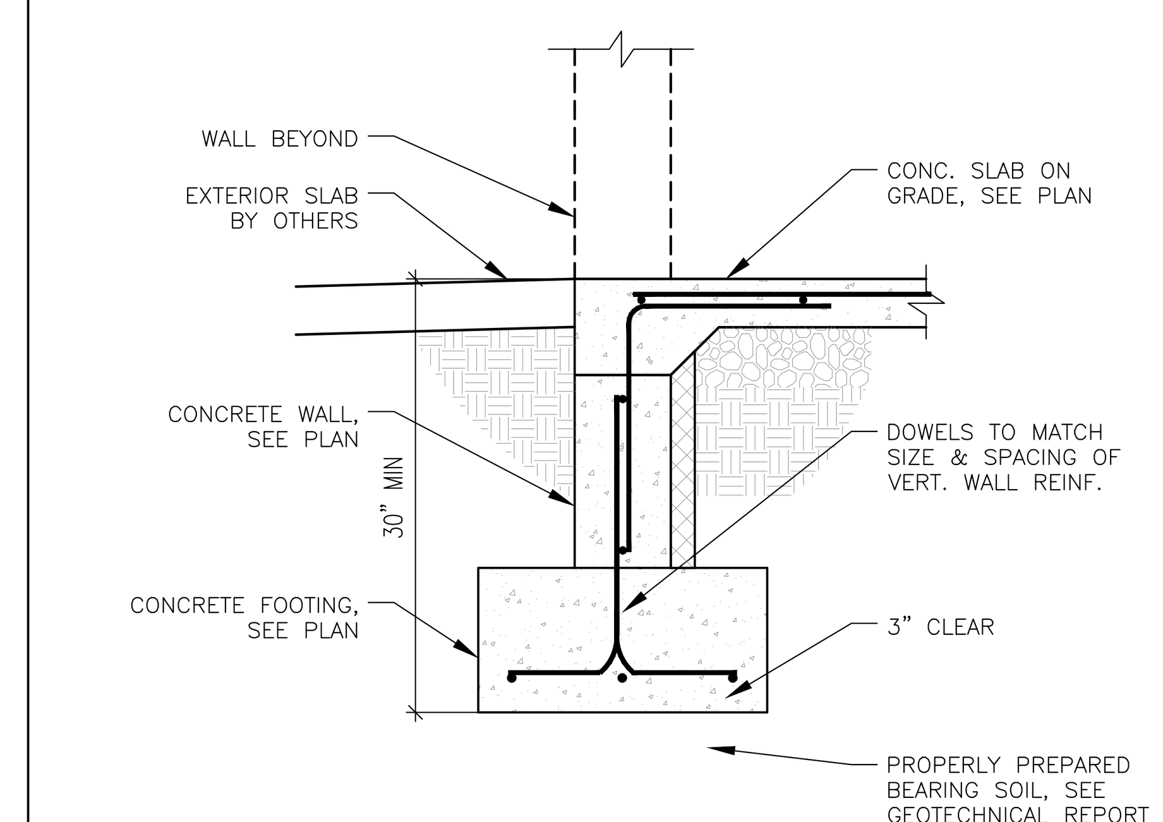




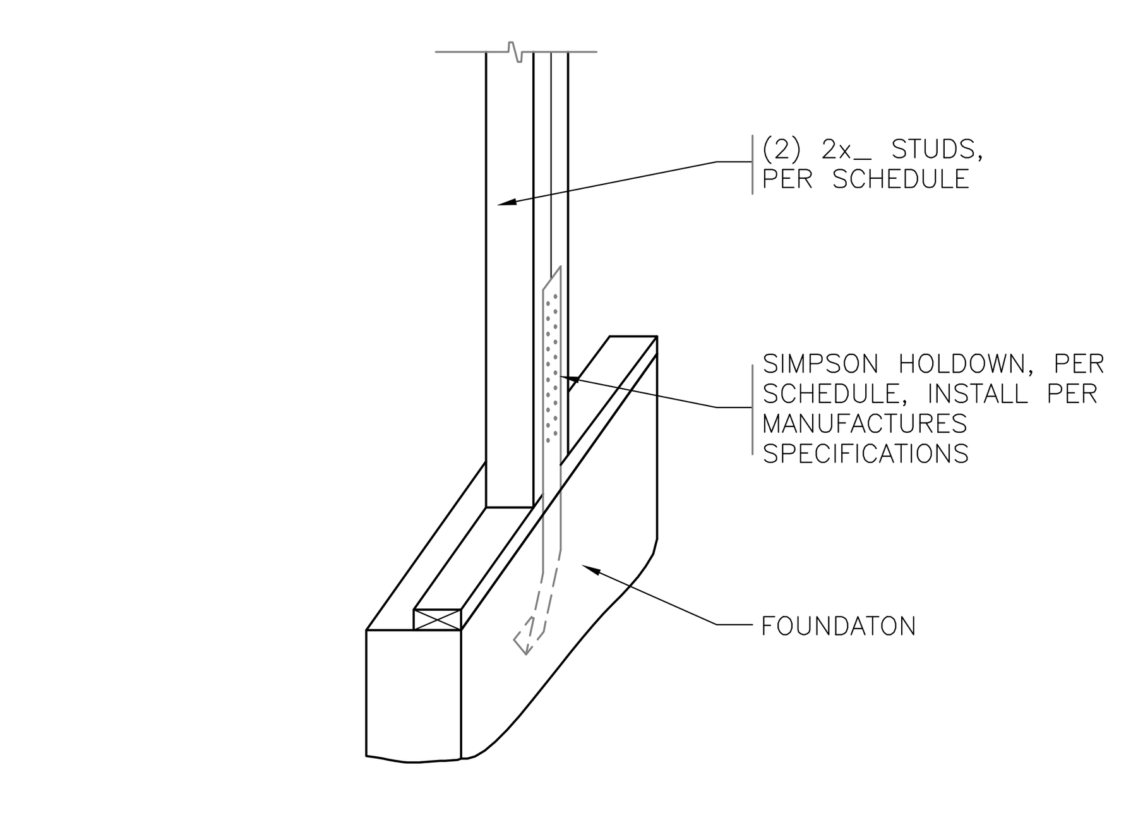
10 PIPES PARALLEL TO FOOTING
N.T.S.



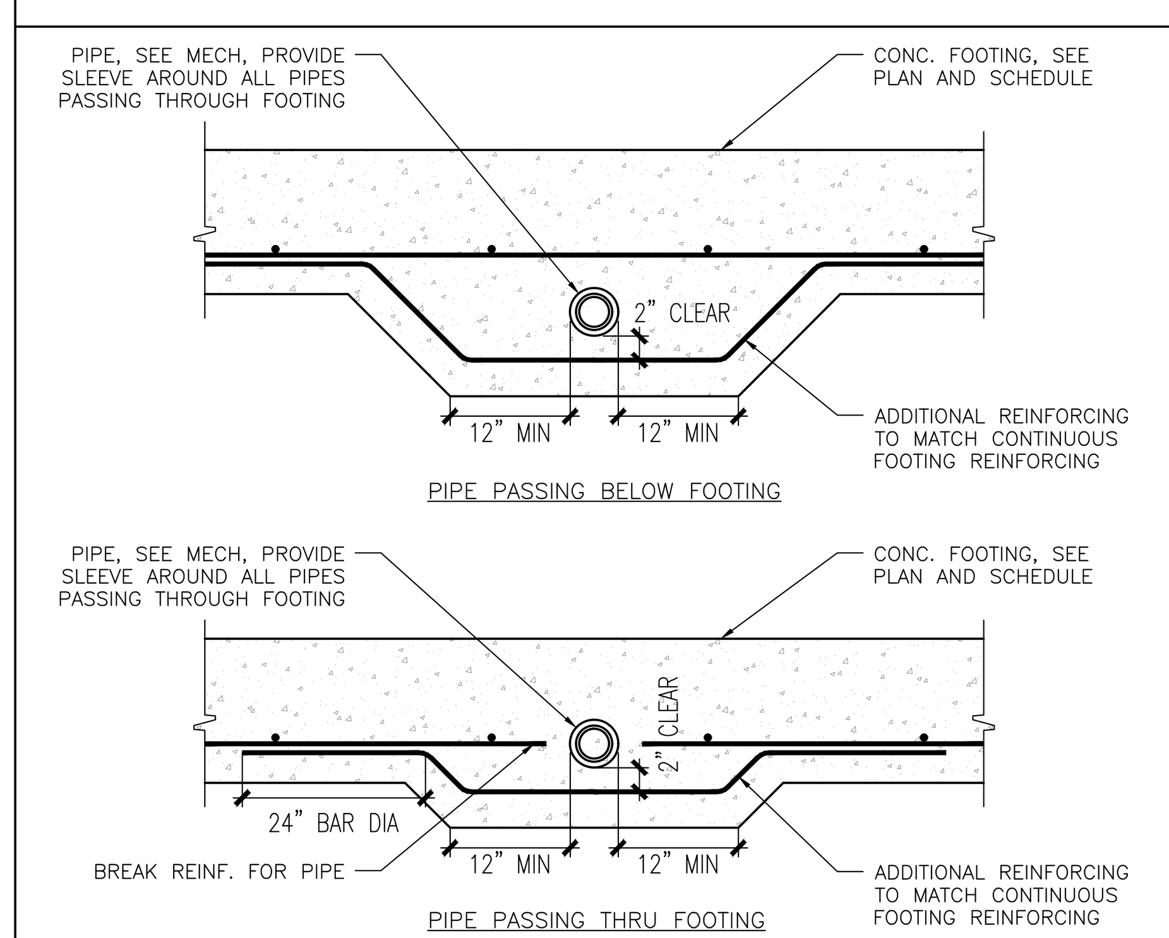
6 CONT. INTERIOR FOOTING AT SHEAR WALL
N.T.S.



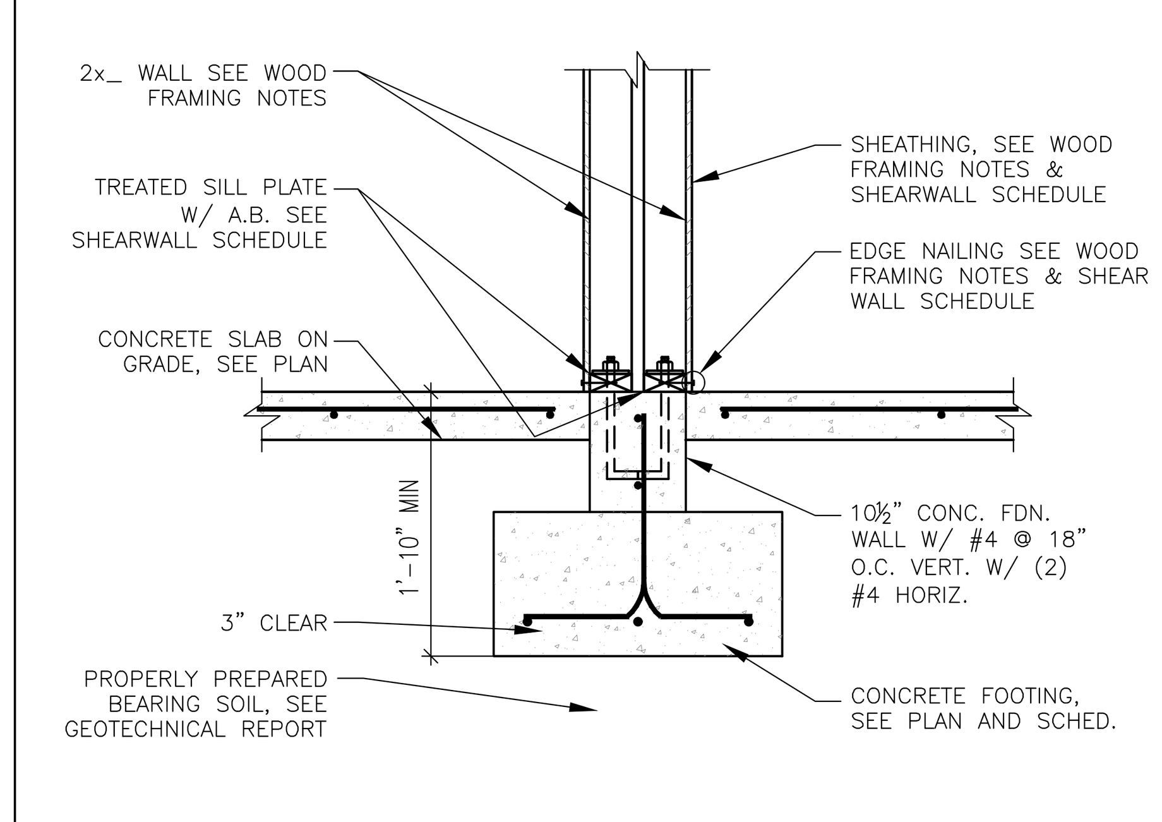
2 OPENING @ FOUNDATION WALL
N.T.S.



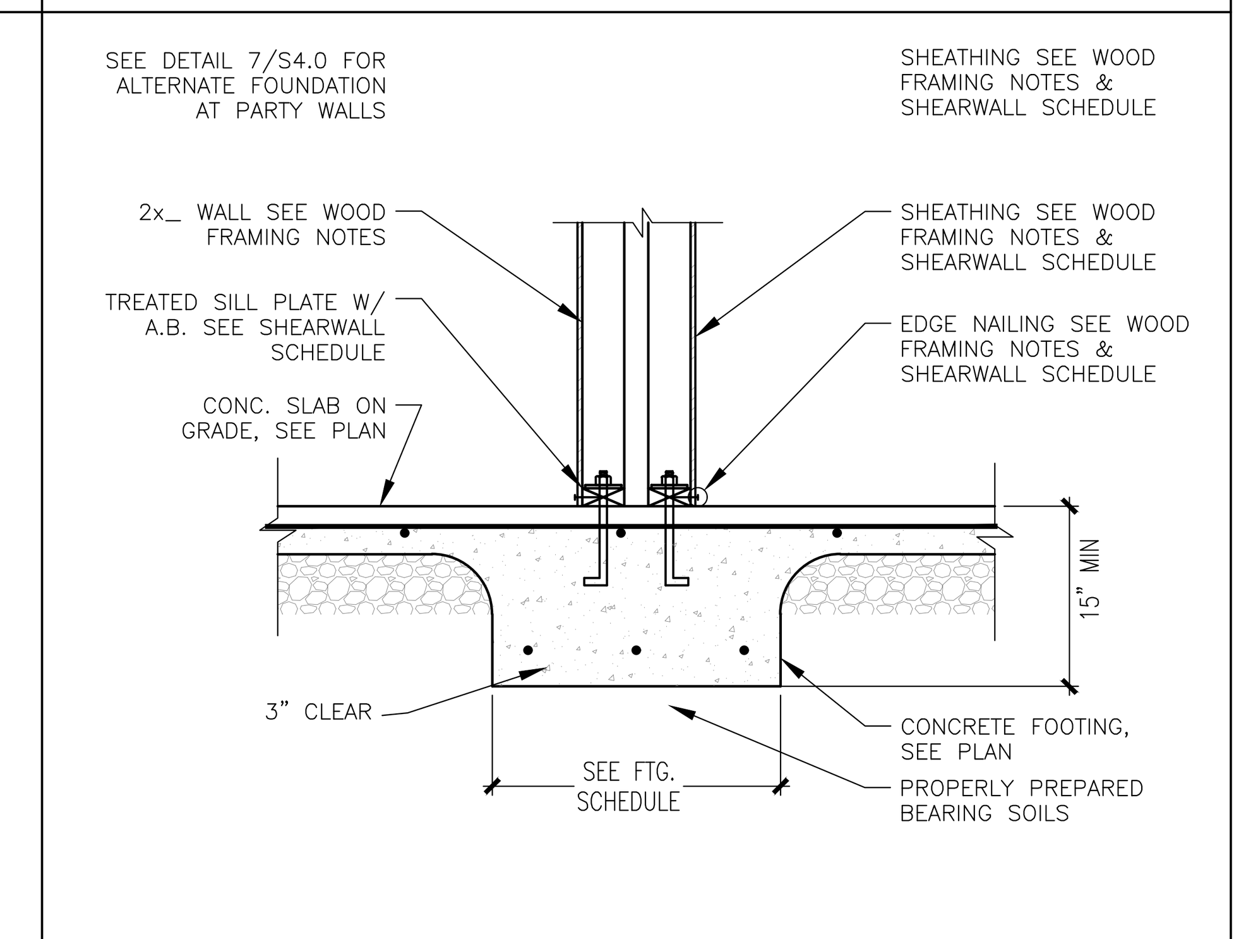
13 SHEAR WALL TO FOUNDATION HOLD DOWN
N.T.S.



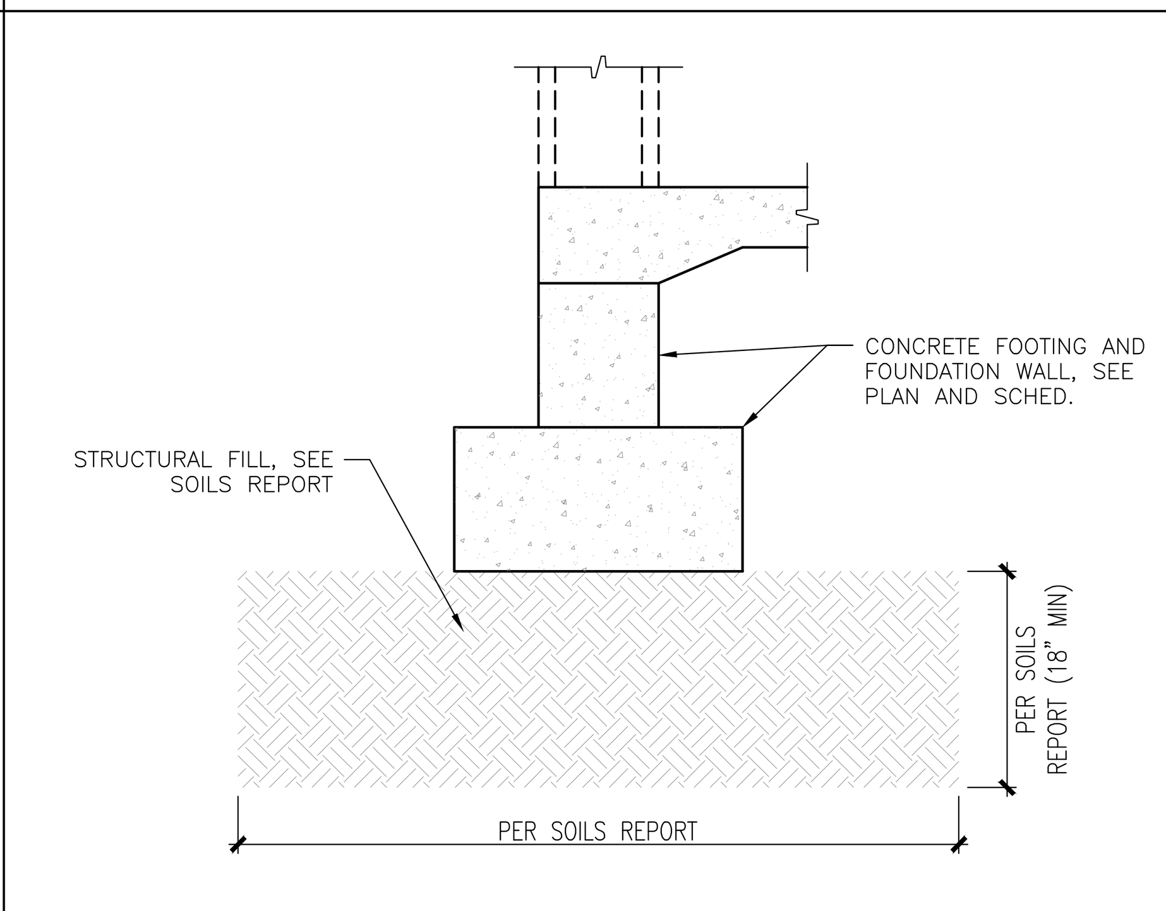
11 PIPES PERPENDICULAR TO FOOTING
N.T.S.



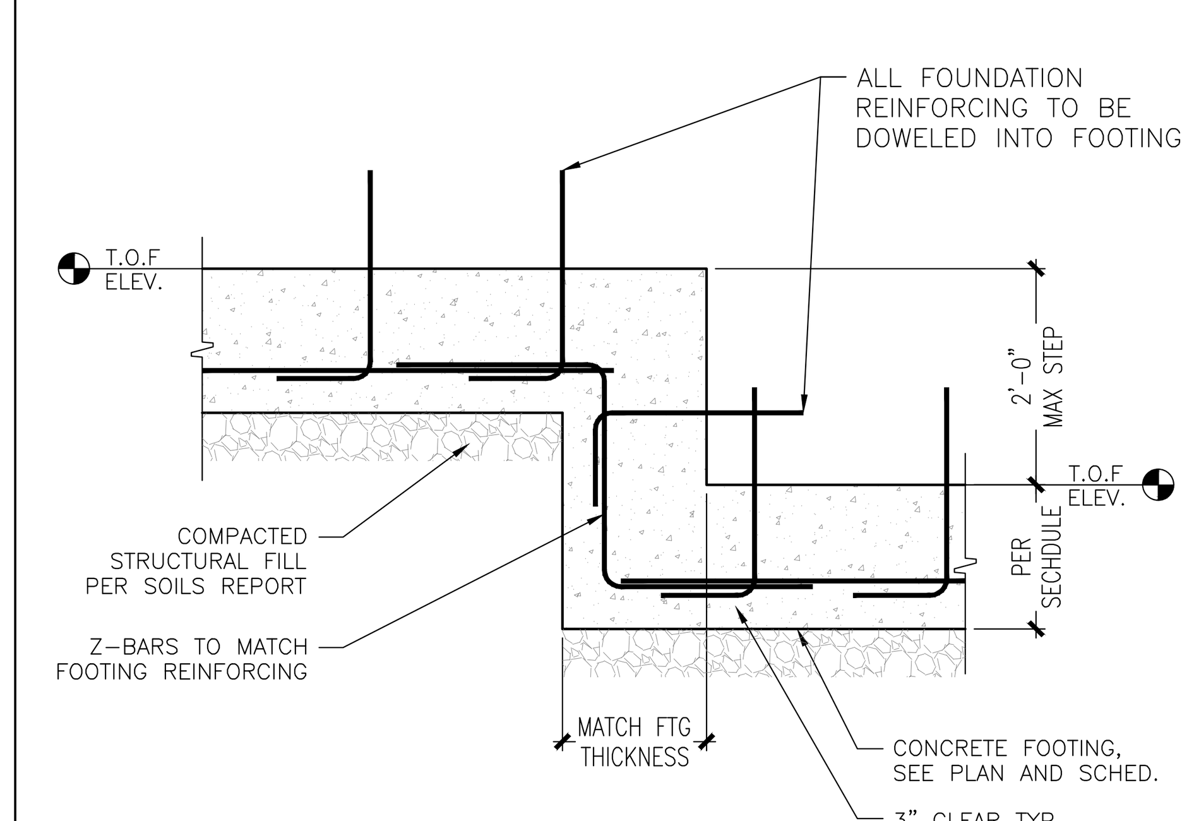
7 ALTERNATE FOOTING AT PARTY WALL
N.T.S.



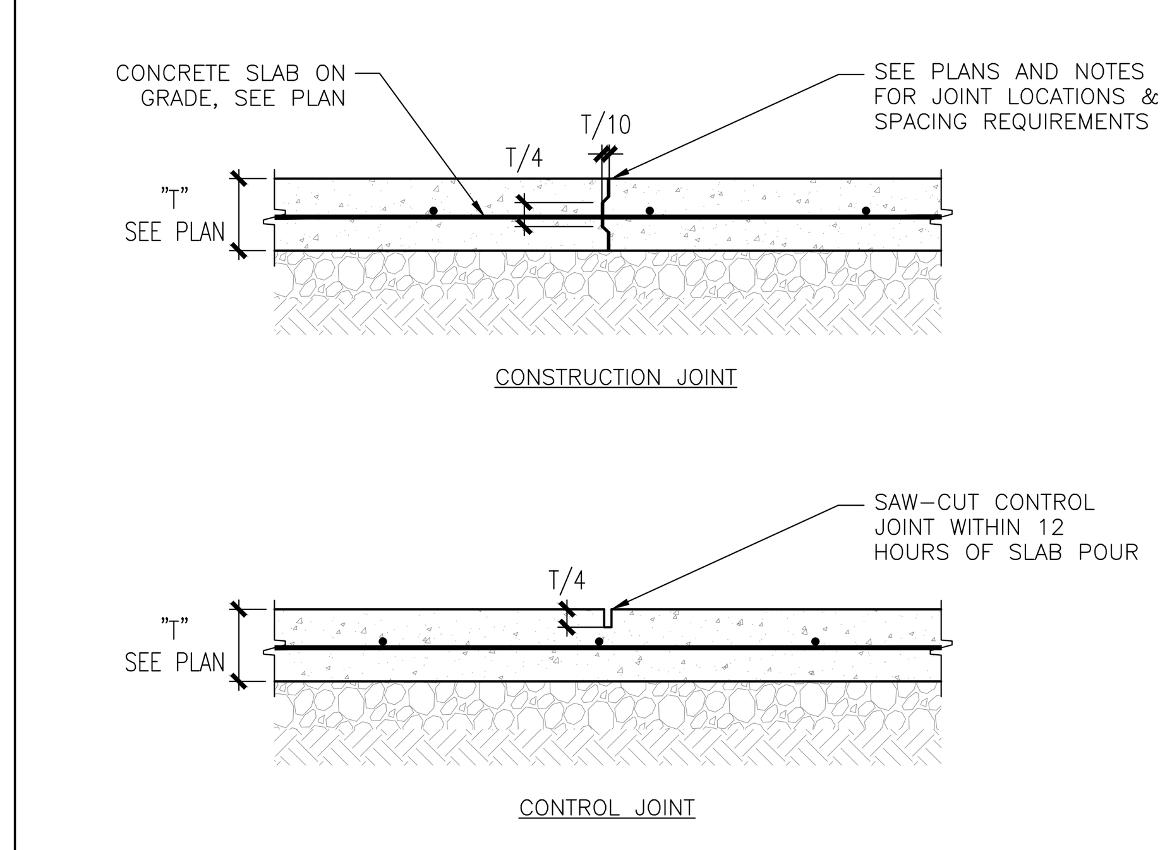
3 CONT. INTERIOR FOOTING AT PARTY WALL
N.T.S.



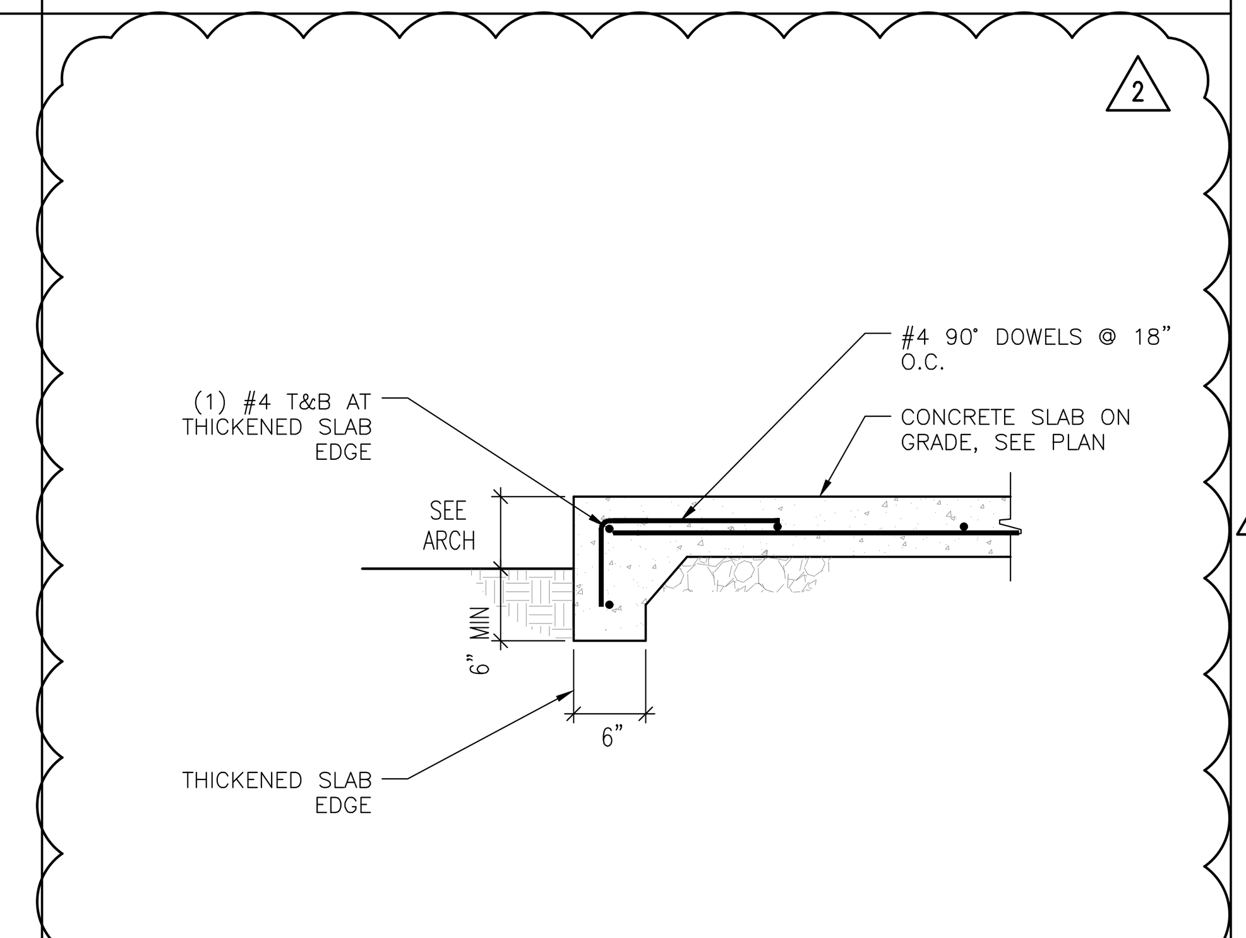
14 TYPICAL STRUCTURAL FILL
N.T.S.



12 STEP FOOTING DETAIL
N.T.S.



8 TYPICAL SLAB ON GRADE JOINTS
N.T.S.



4 CONCRETE SLAB EDGE
N.T.S.

PROFESSIONAL ENGINEER
375048-2202
JOHN E. CHARCHENKO
9-18-19
STATE OF UTAH

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CHLOE'S POINTE APARTMENTS

OGDEN, UTAH

FOUNDATION DETAILS

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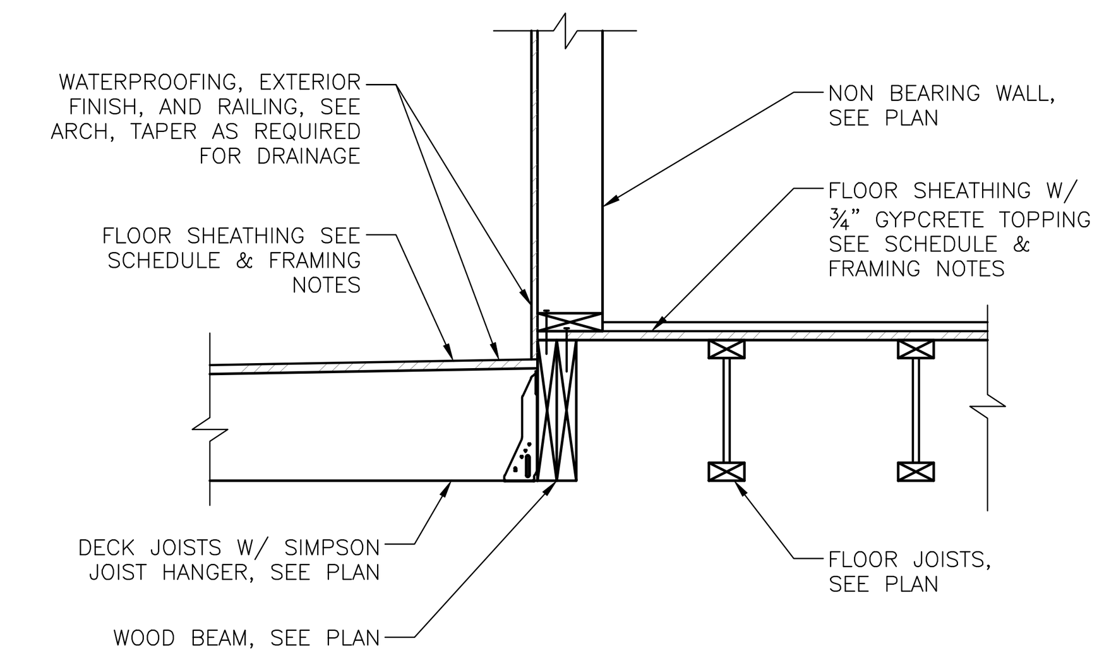
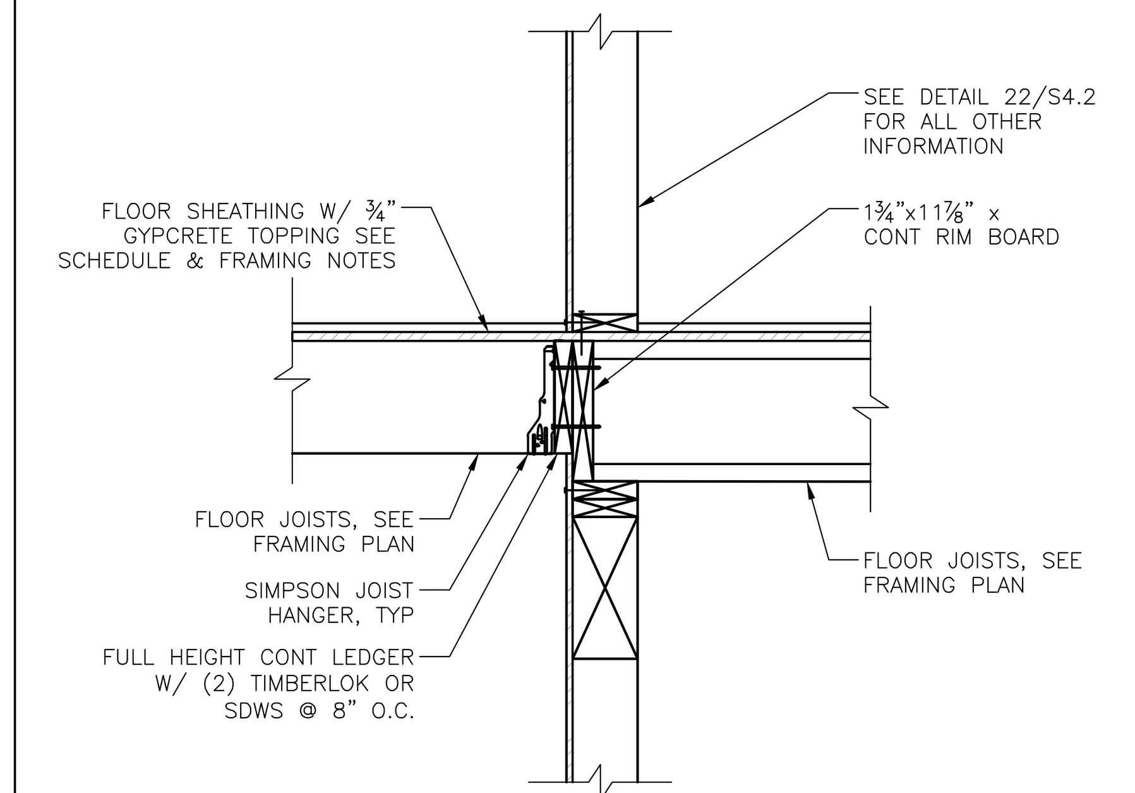
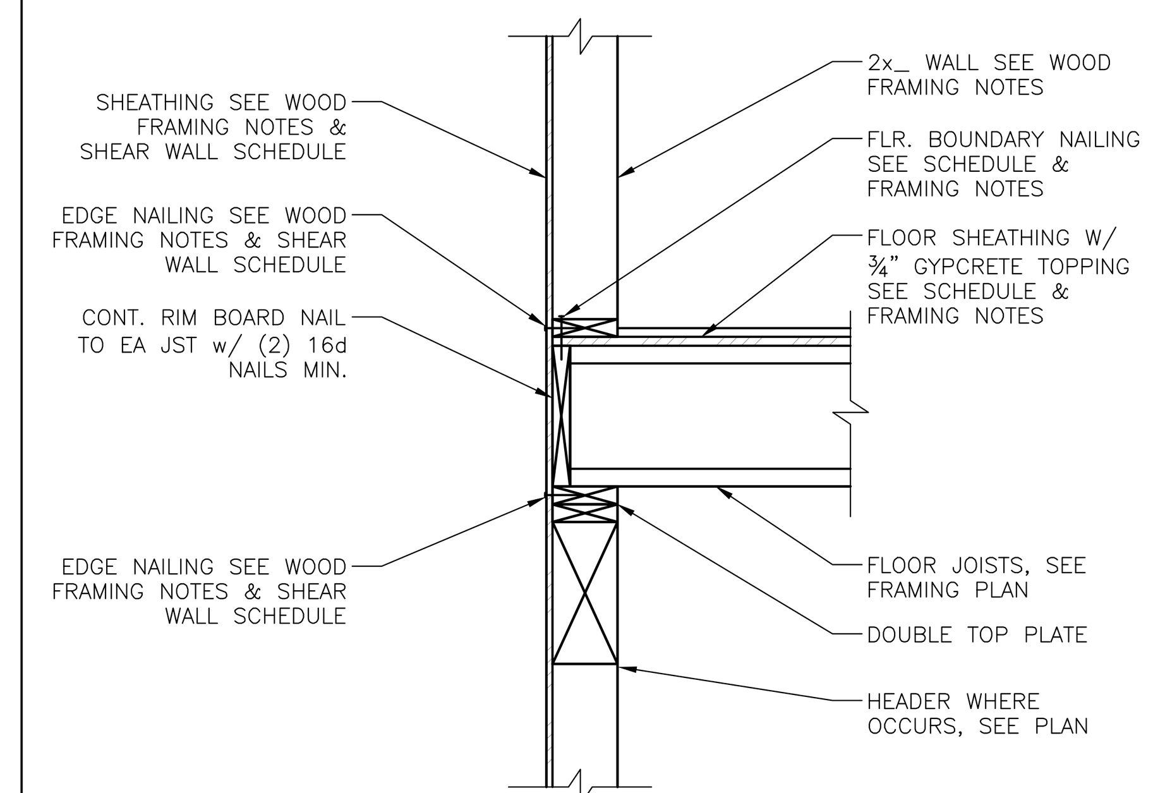
S4.0



29 DECK JOISTS TO BEAM
N.T.S.



25 CORRIDOR JOISTS TO BEARING WALL
N.T.S.



21 PERP. FLOOR JOISTS TO BEARING WALL
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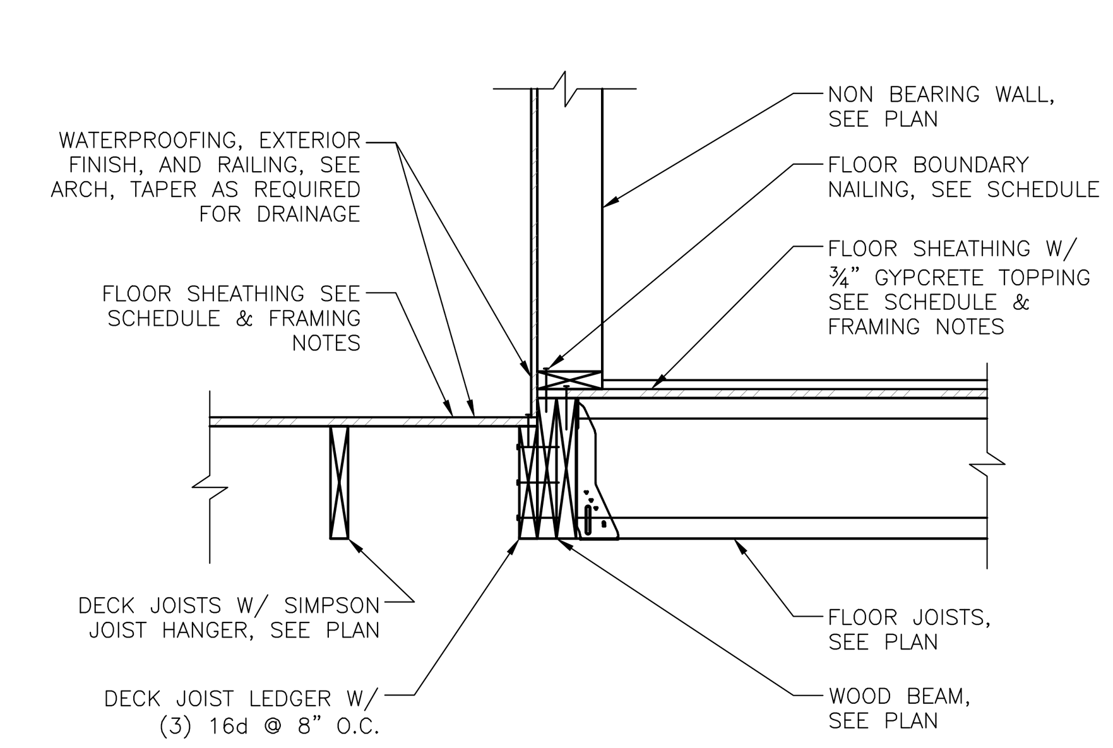
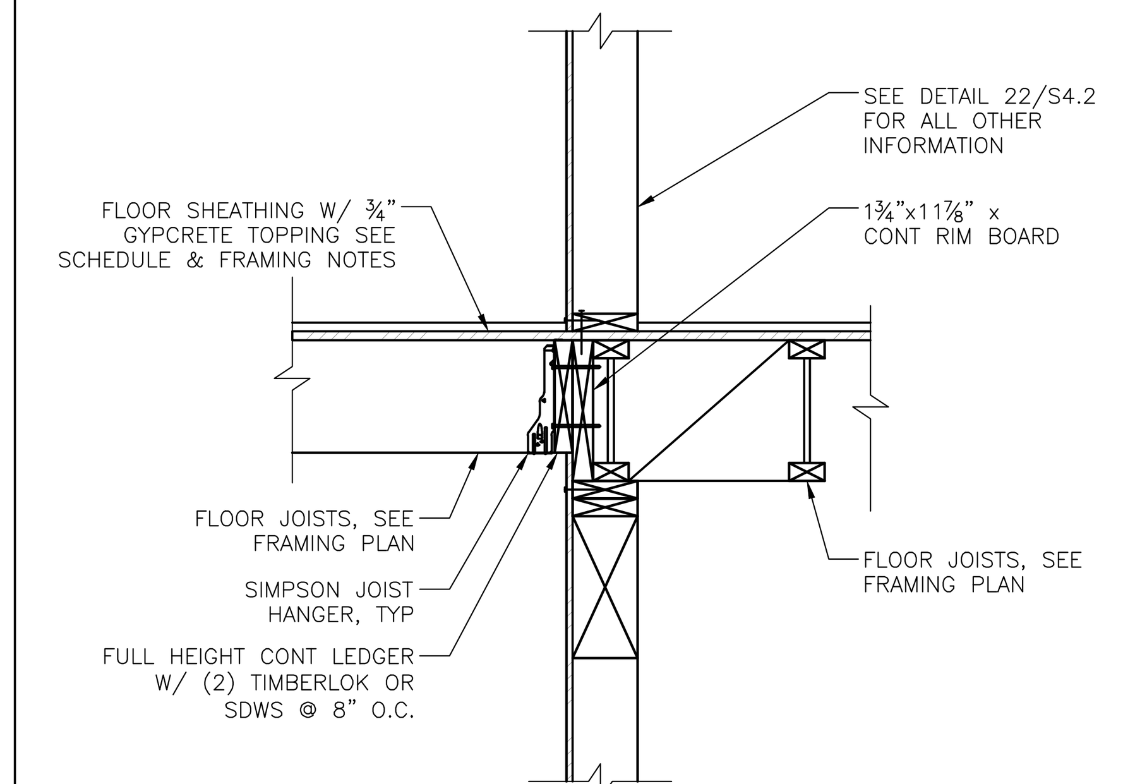
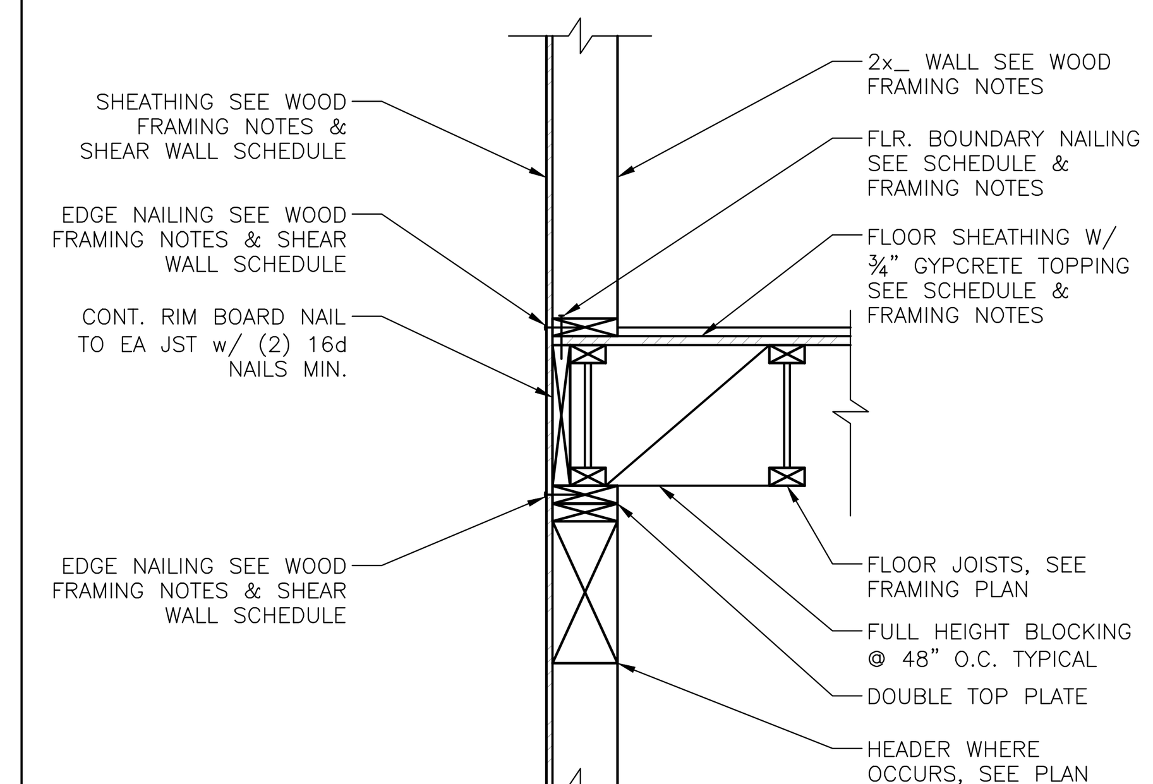


30 DECK JOISTS TO BEAM
N.T.S.



26 CORRIDOR JOISTS TO BEARING WALL
N.T.S.



22 PAR. FLOOR JOISTS TO BEARING WALL
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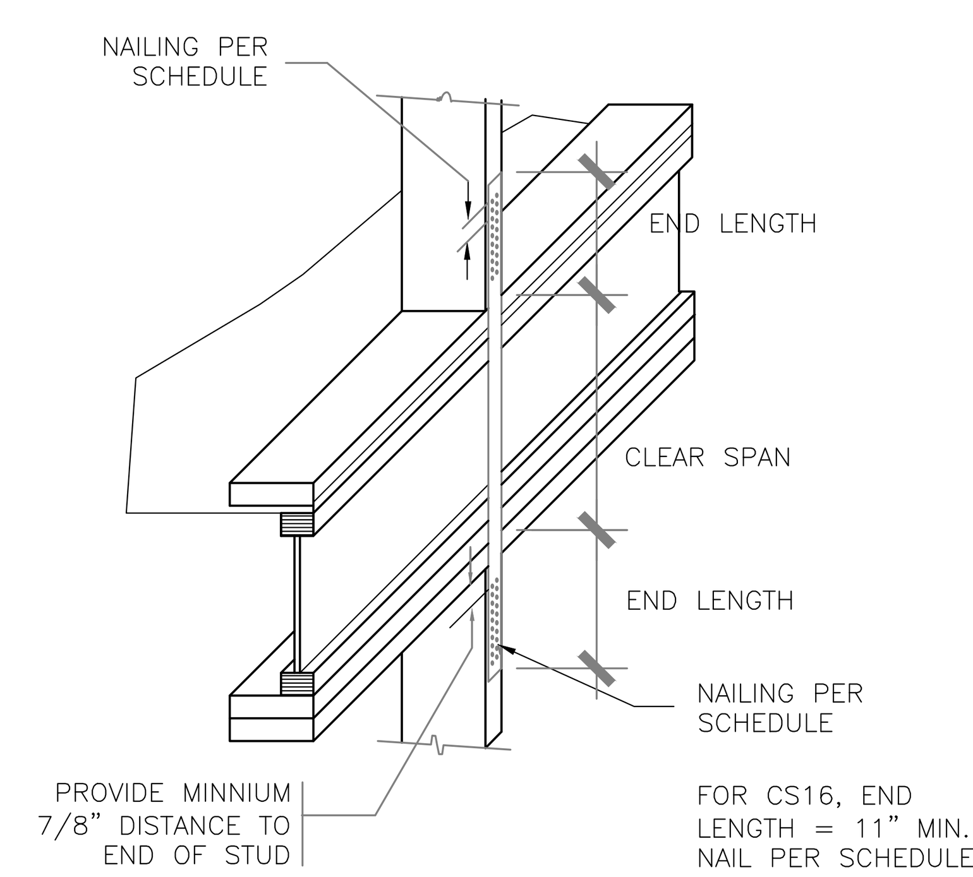
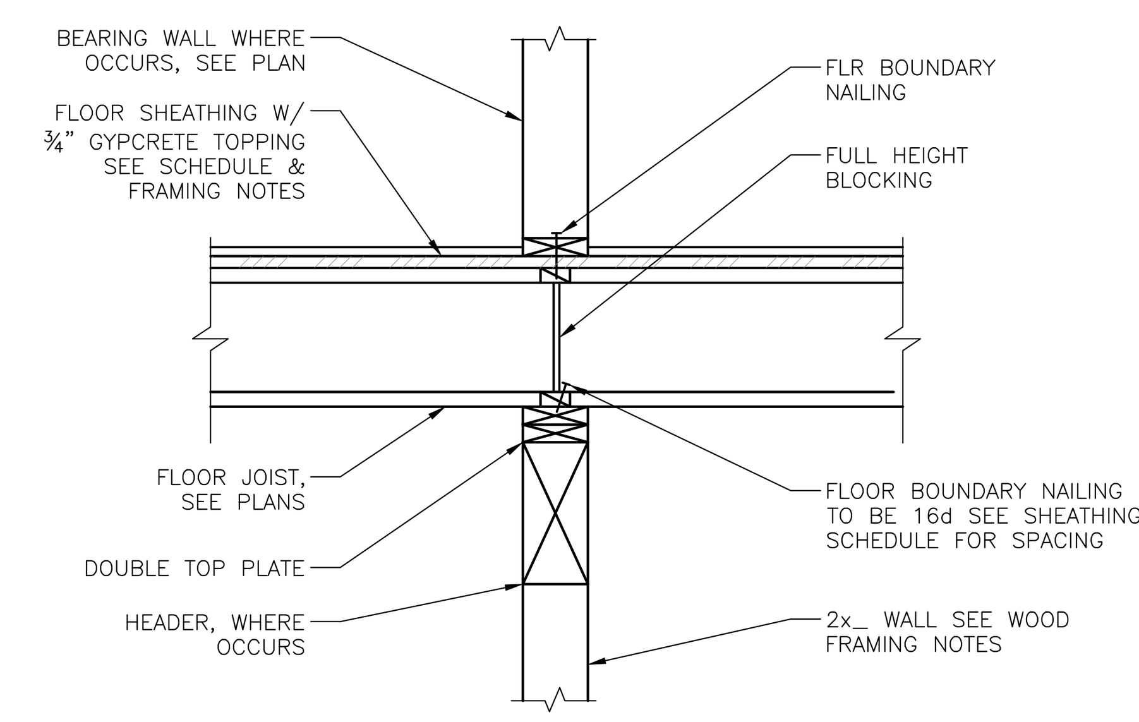


31 FLOOR TO FLOOR HOLD DOWN STRAP
N.T.S.



27 PERP. FLOOR JOISTS TO BEARING WALL
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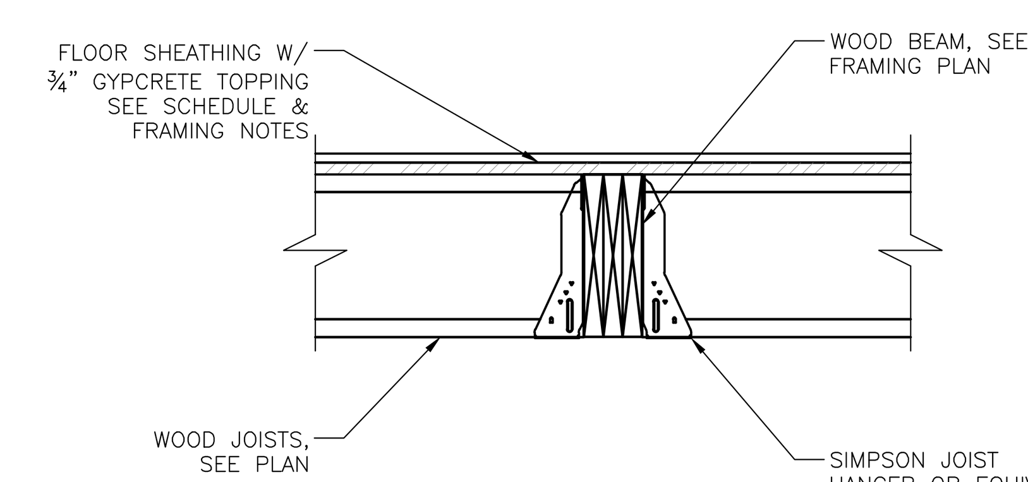

23 PERP. FLOOR JOISTS TO BEAM
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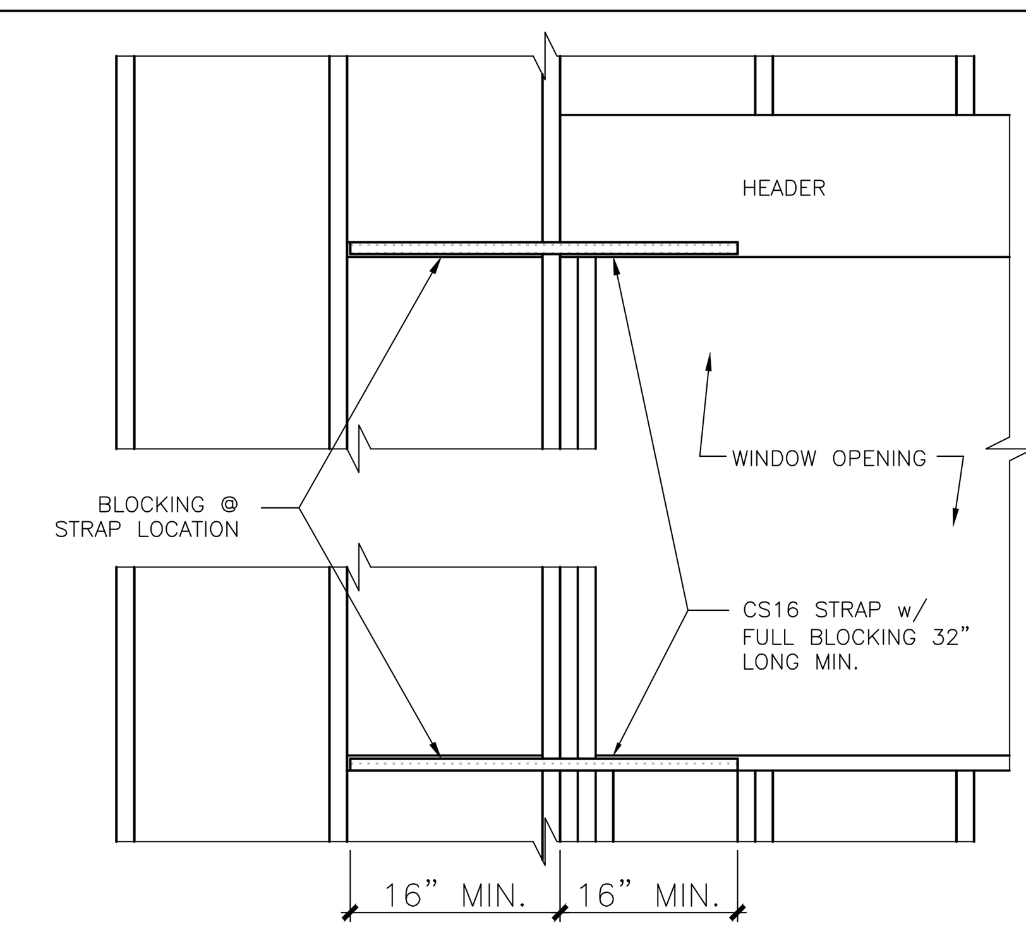
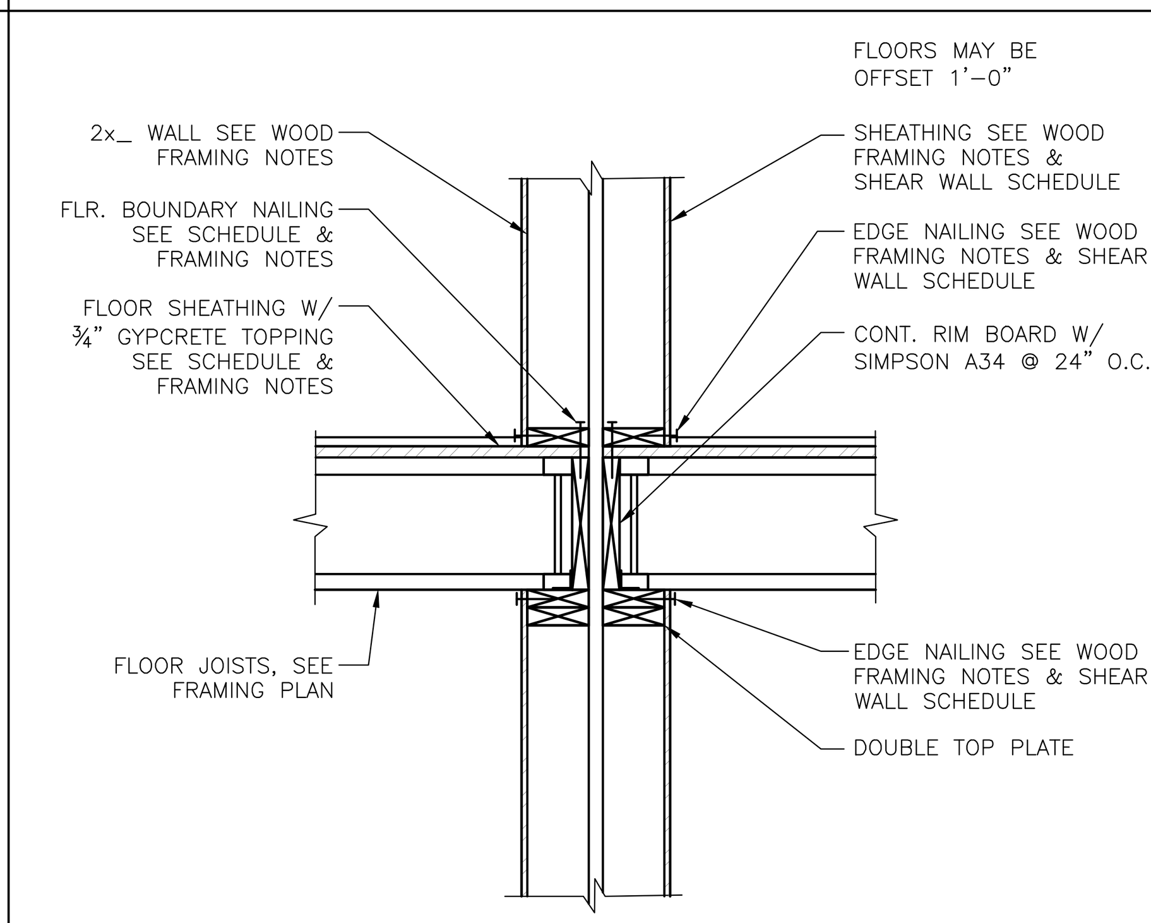
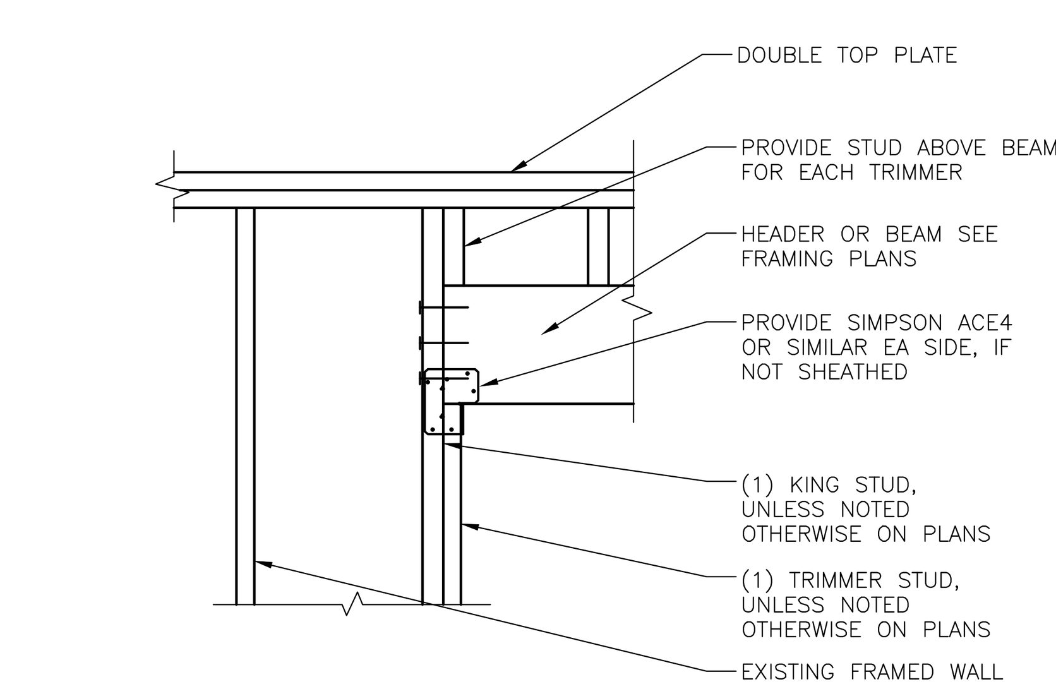


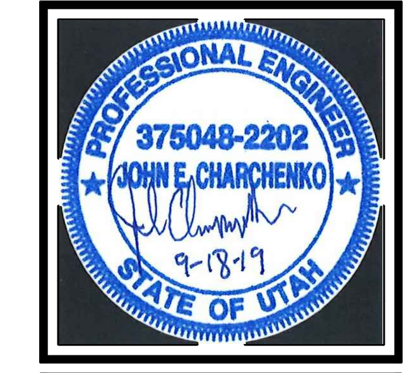
32 SHEAR WALL STRAPING AT WINDOW
N.T.S.



28 PERPENDICULAR FLOOR JOISTS @ PARTY WALL
N.T.S.



24 TYPICAL HEADER
N.T.S.



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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
FLOOR FRAMING DETAILS

PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
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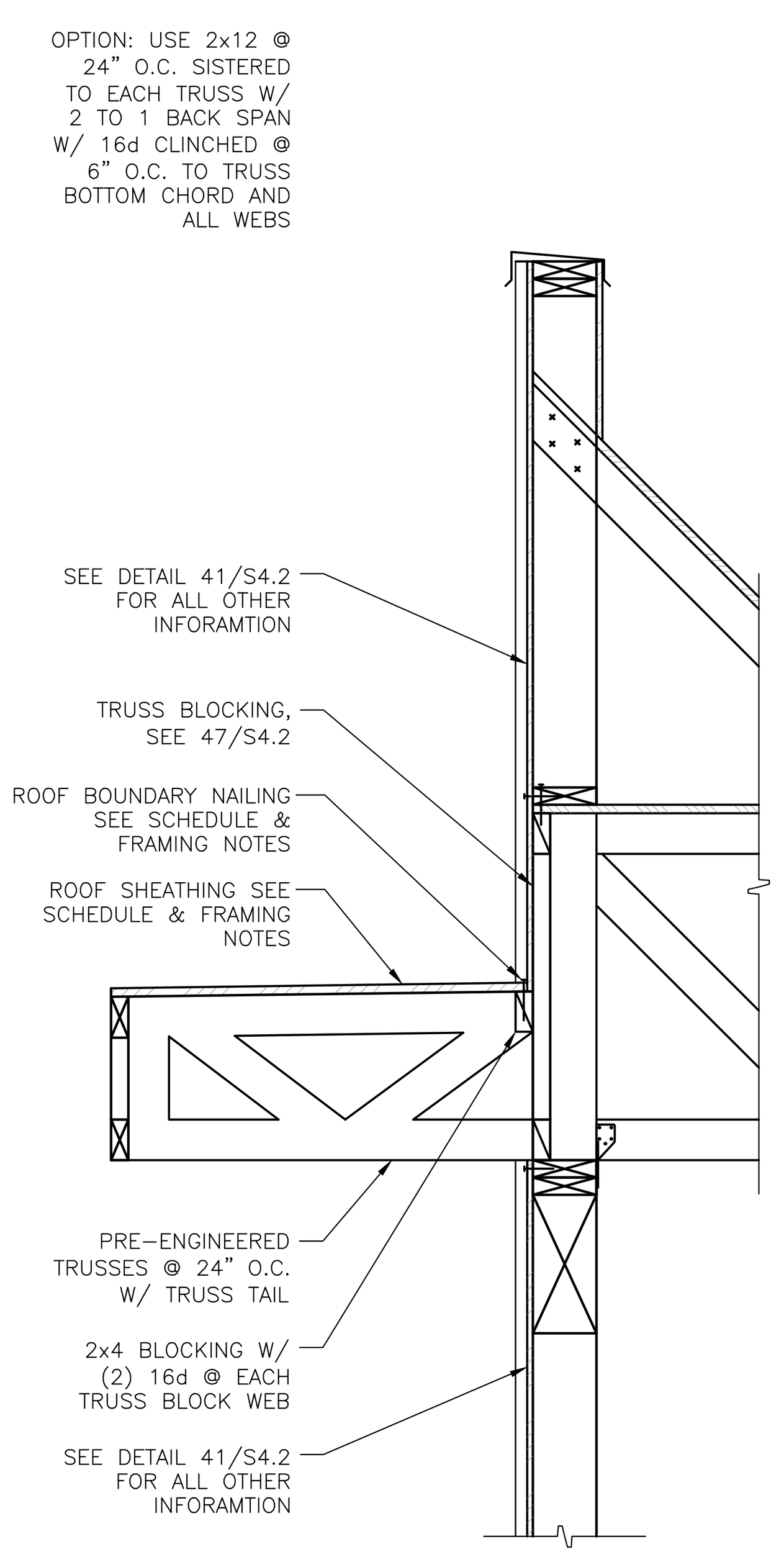
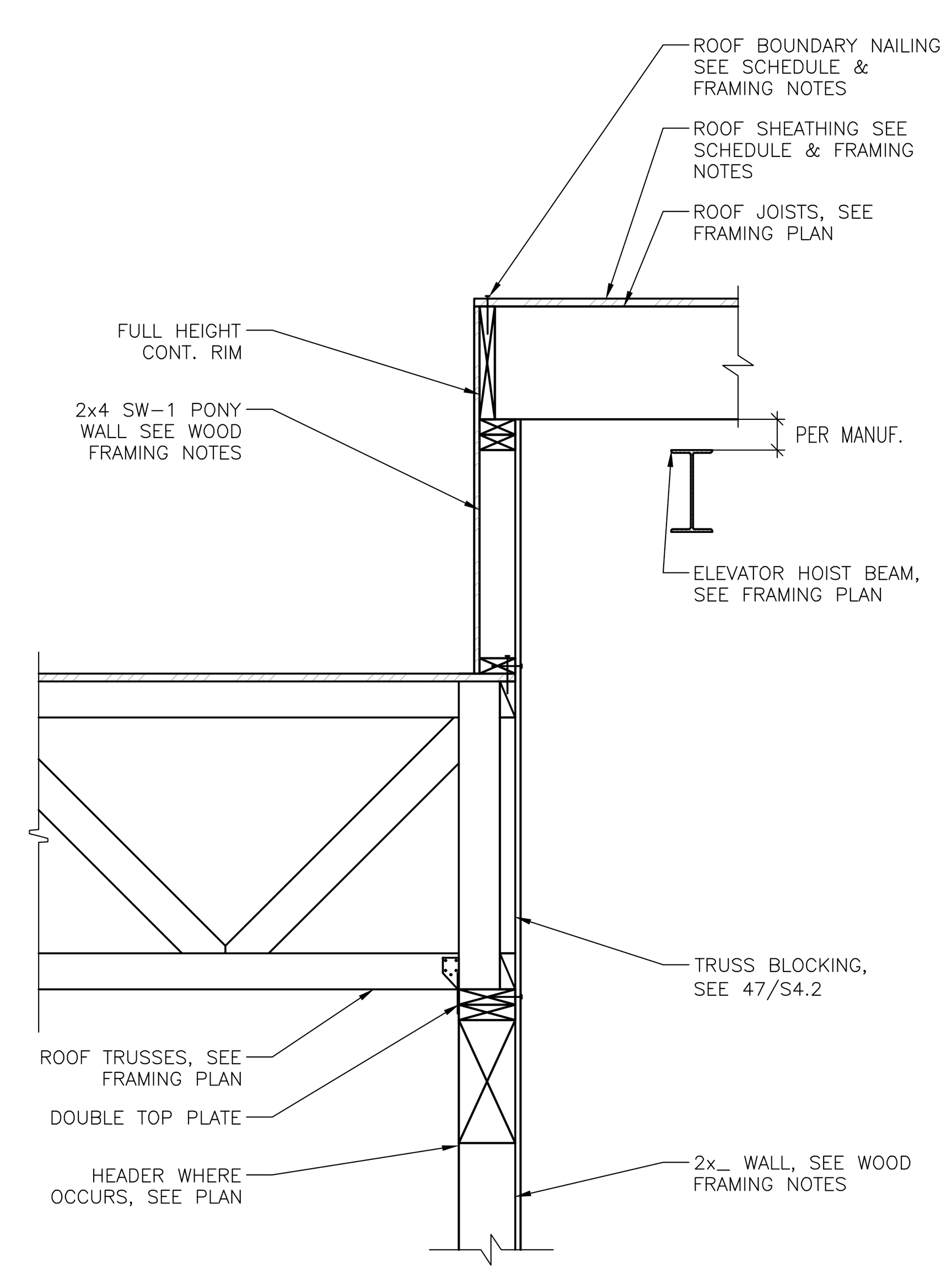
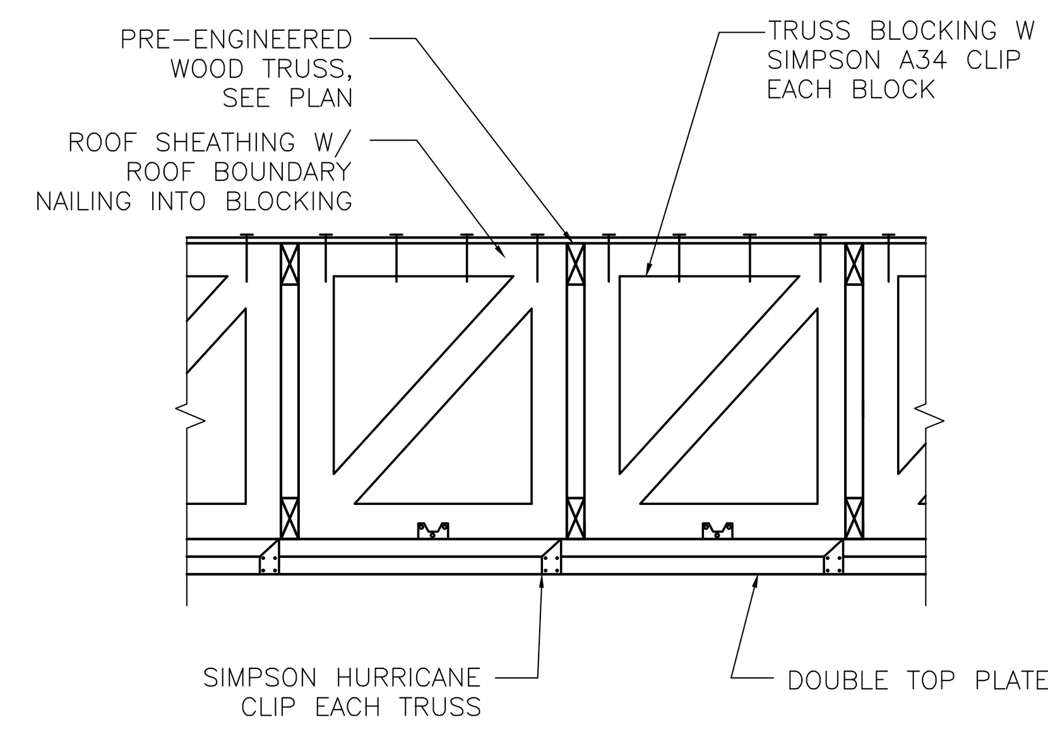
S4.1



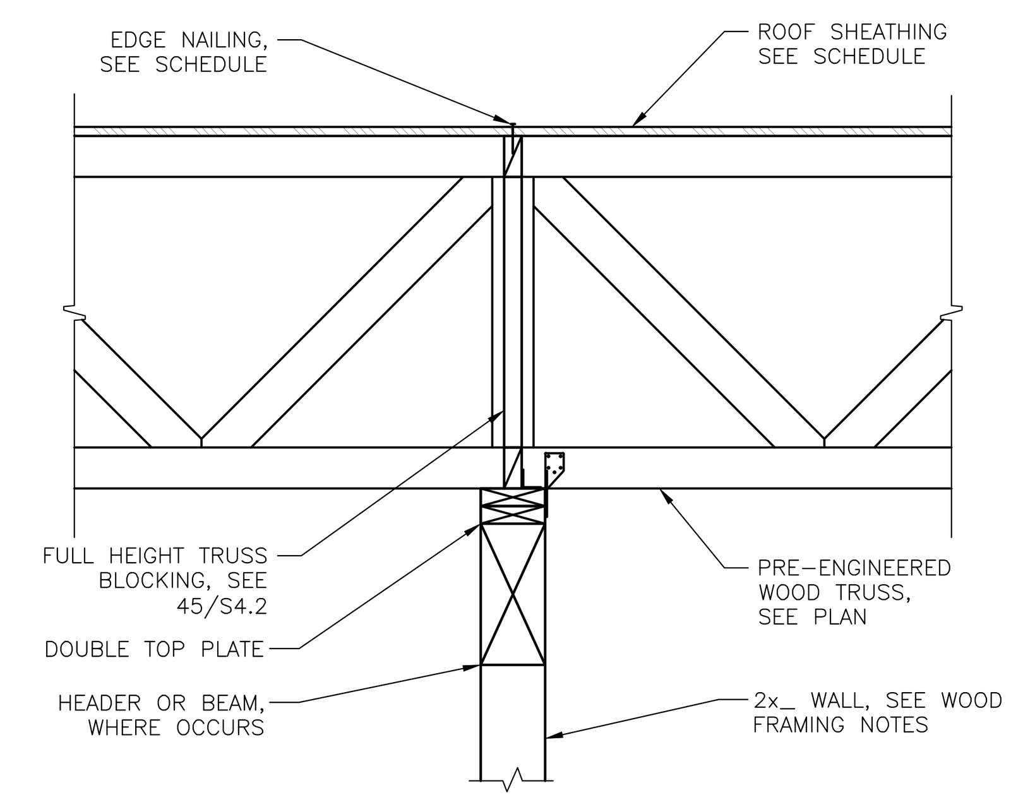
54 TYPICAL ROOF EYEBROW
N.T.S.



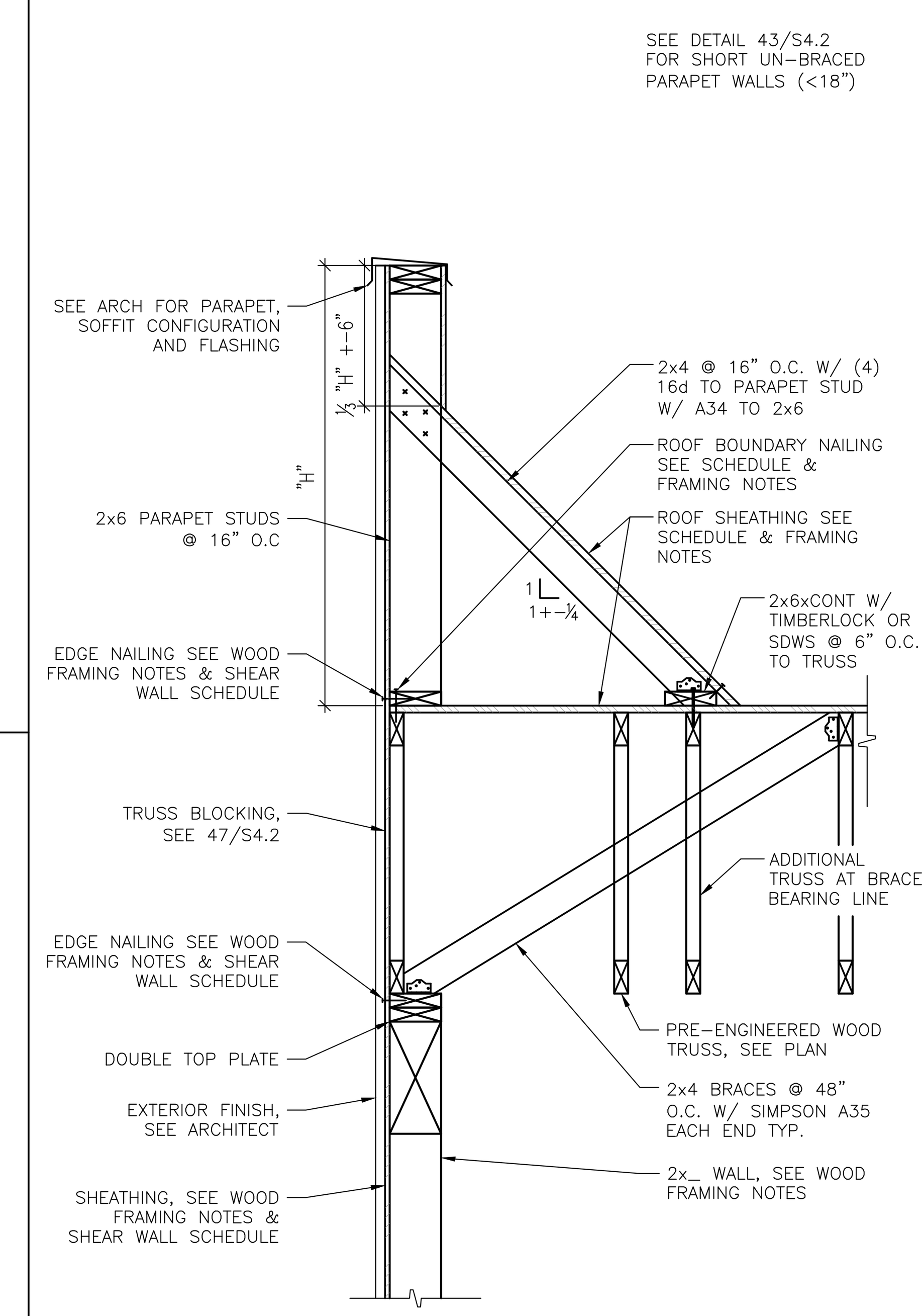
51 PERP. JOISTS @ ELEVATOR SHAFT
N.T.S.



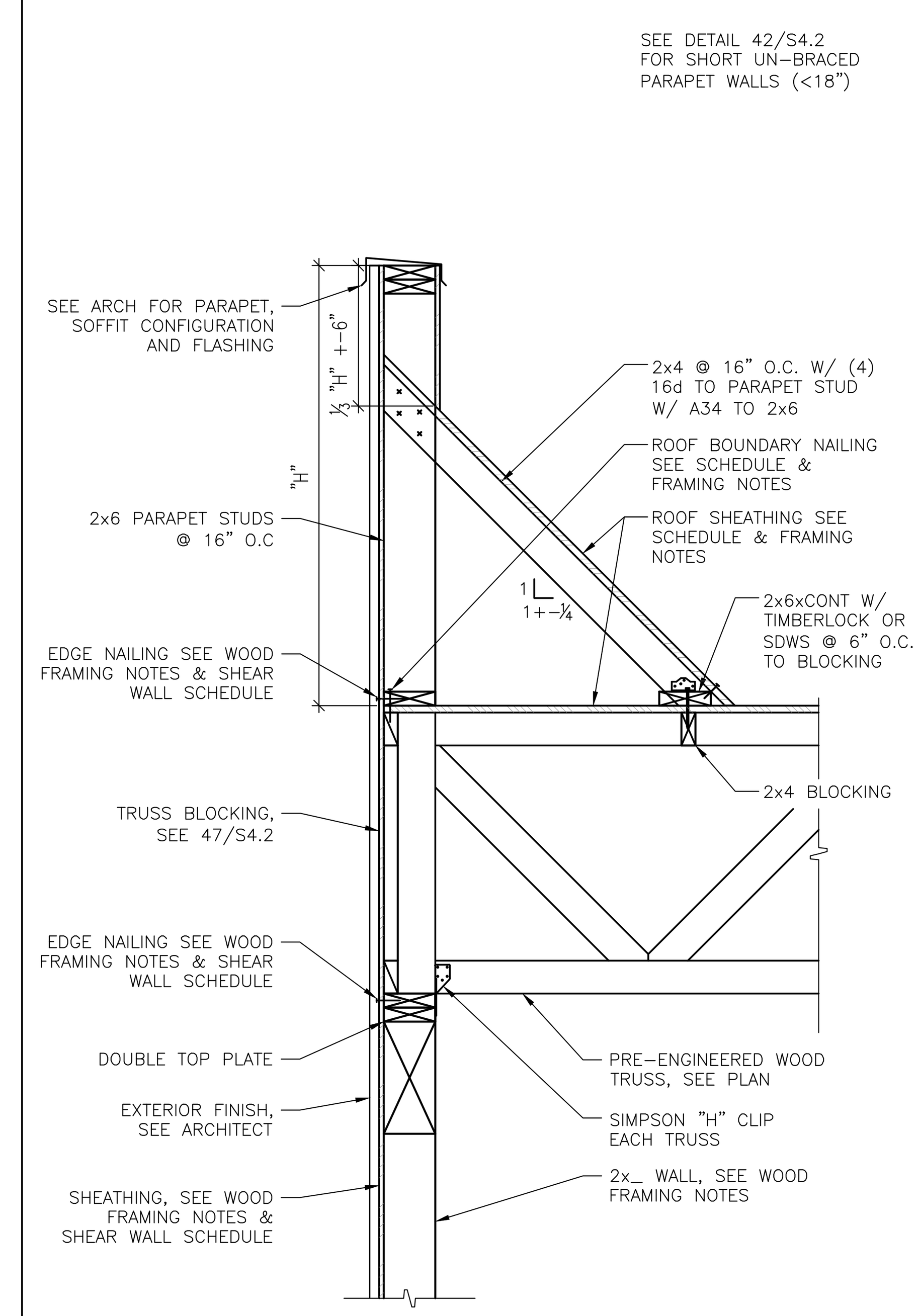
47 TYPICAL TRUSS BLOCKING
N.T.S.



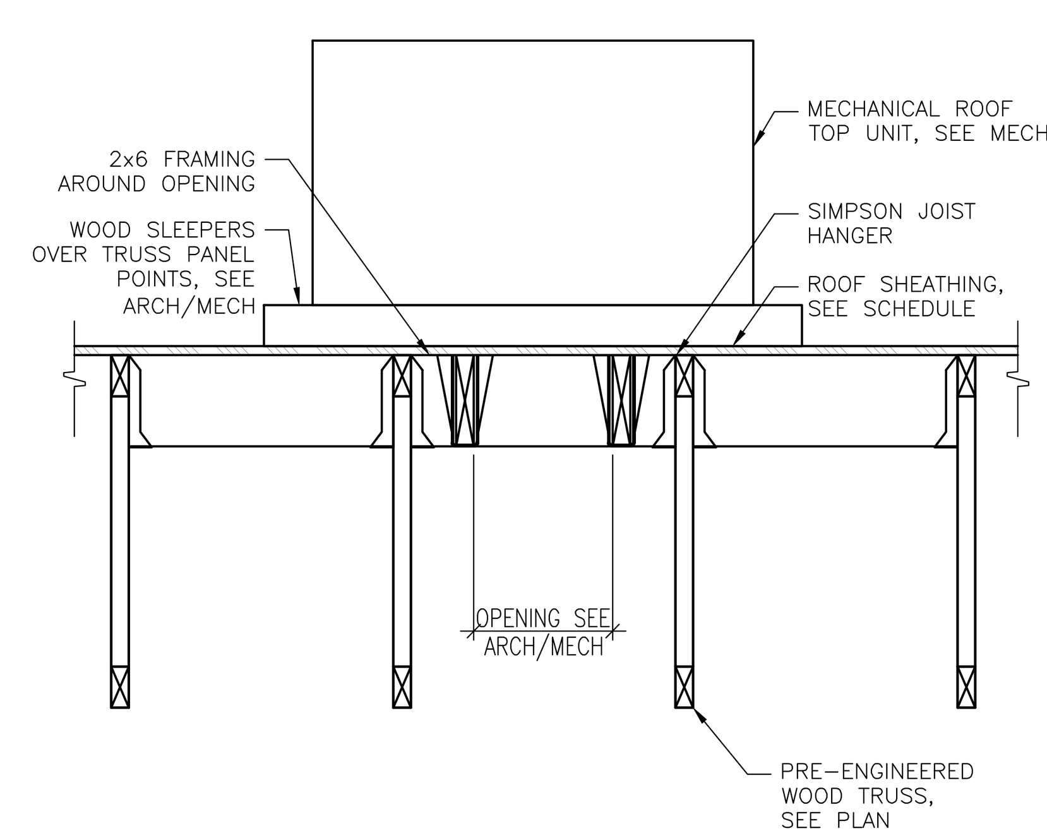
48 TYPICAL TRUSS TO BEARING WALL
N.T.S.



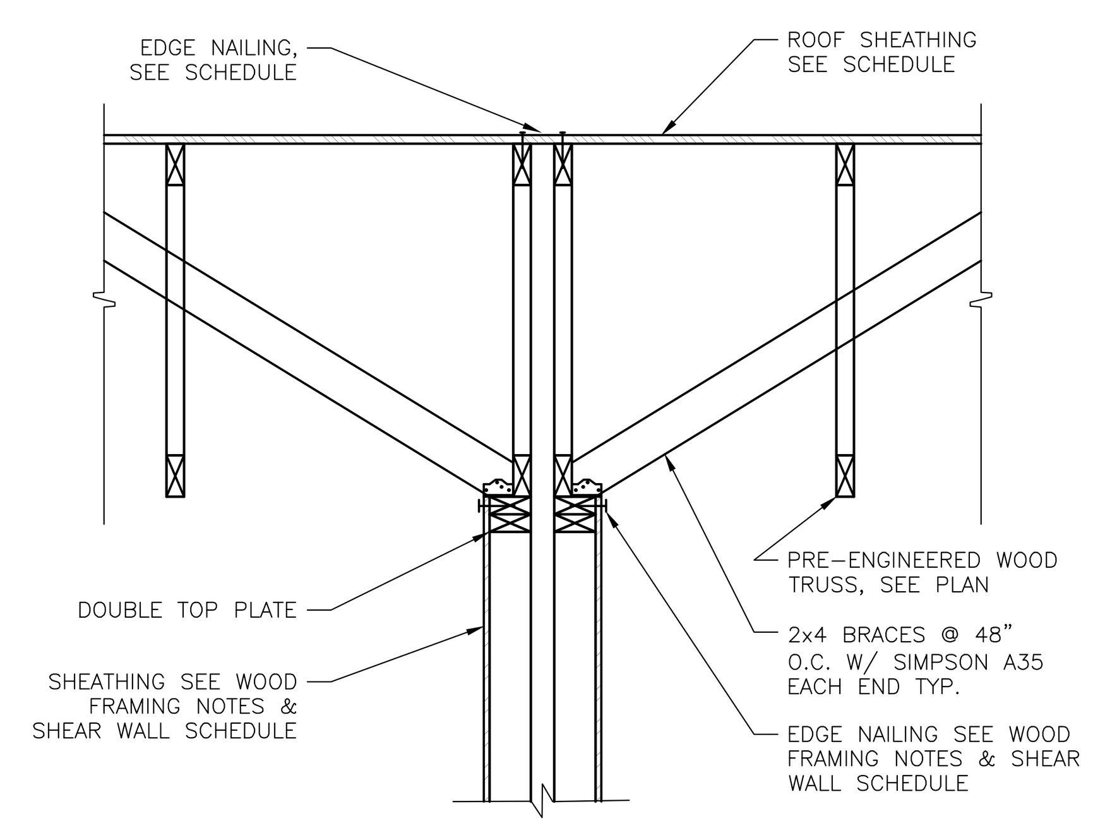
44 TRUSS BEARING @ EXTERIOR WALL
N.T.S.



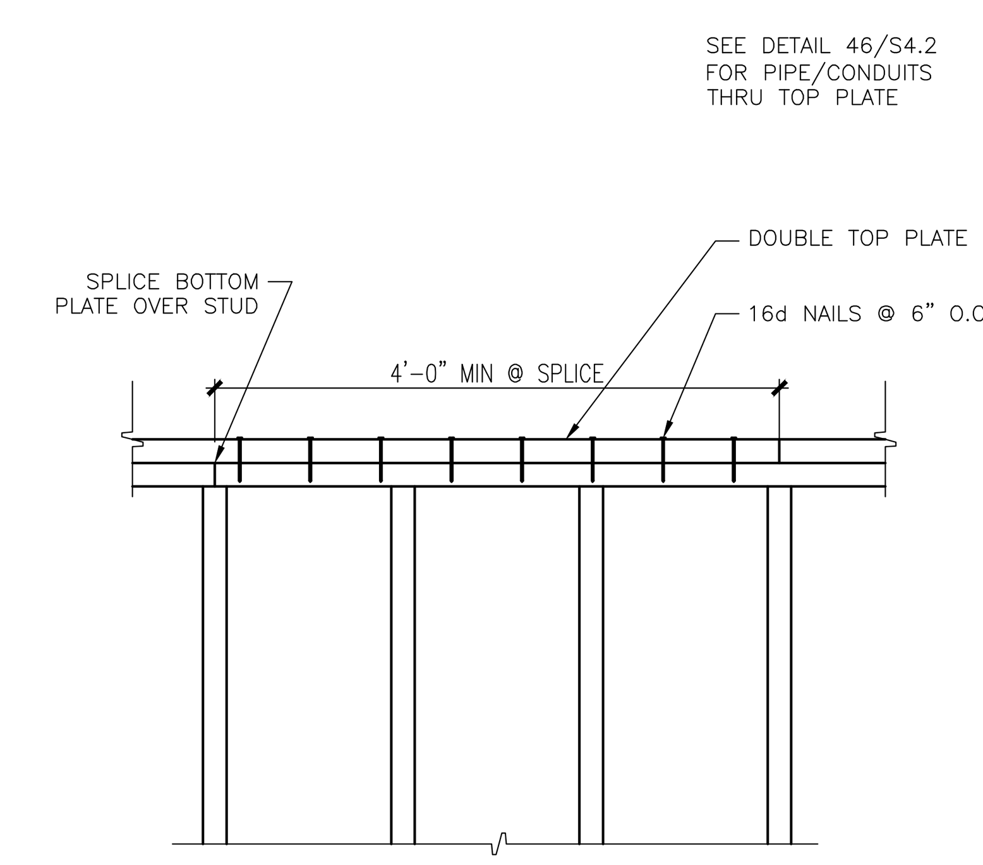
41 TRUSS BEARING @ EXTERIOR WALL
N.T.S.



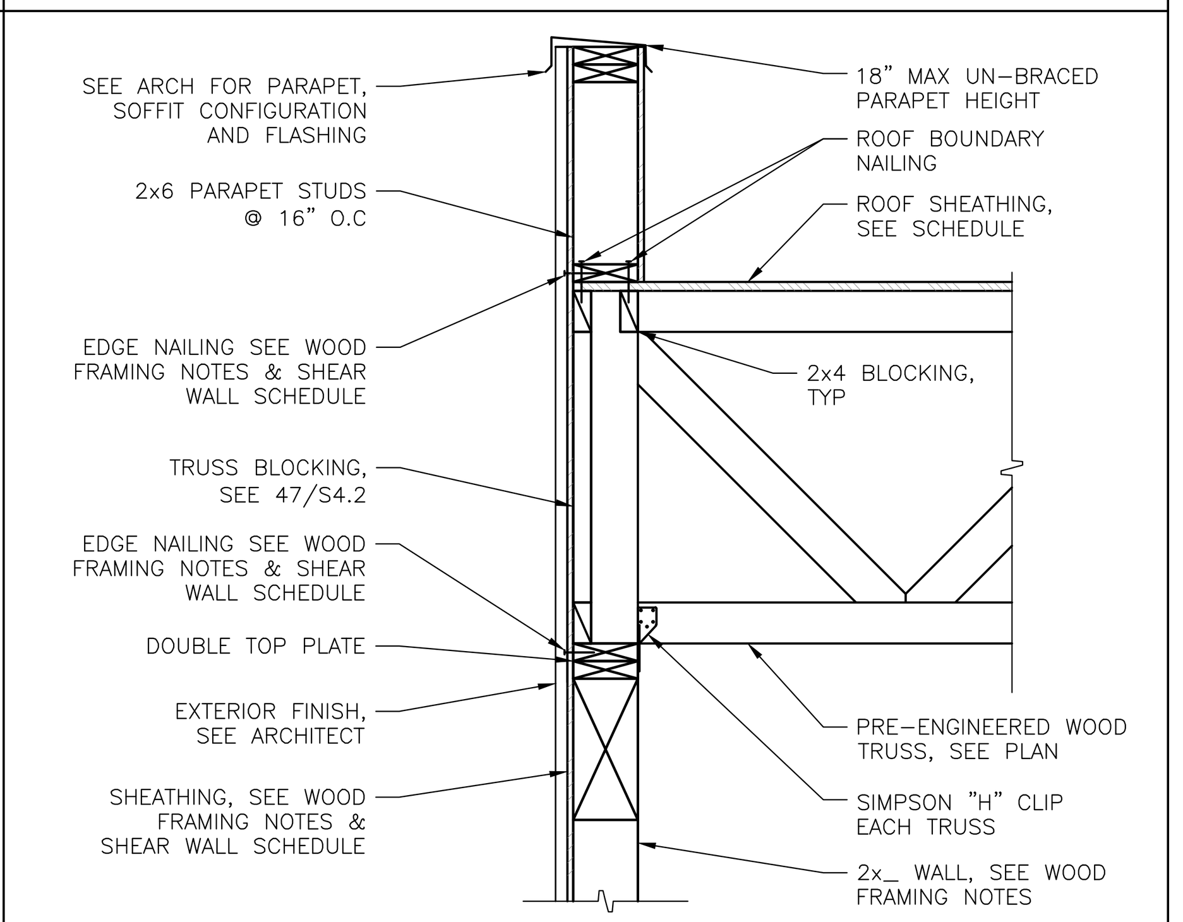
52 TYPICAL MECHANICAL UNIT SUPPORT
N.T.S.



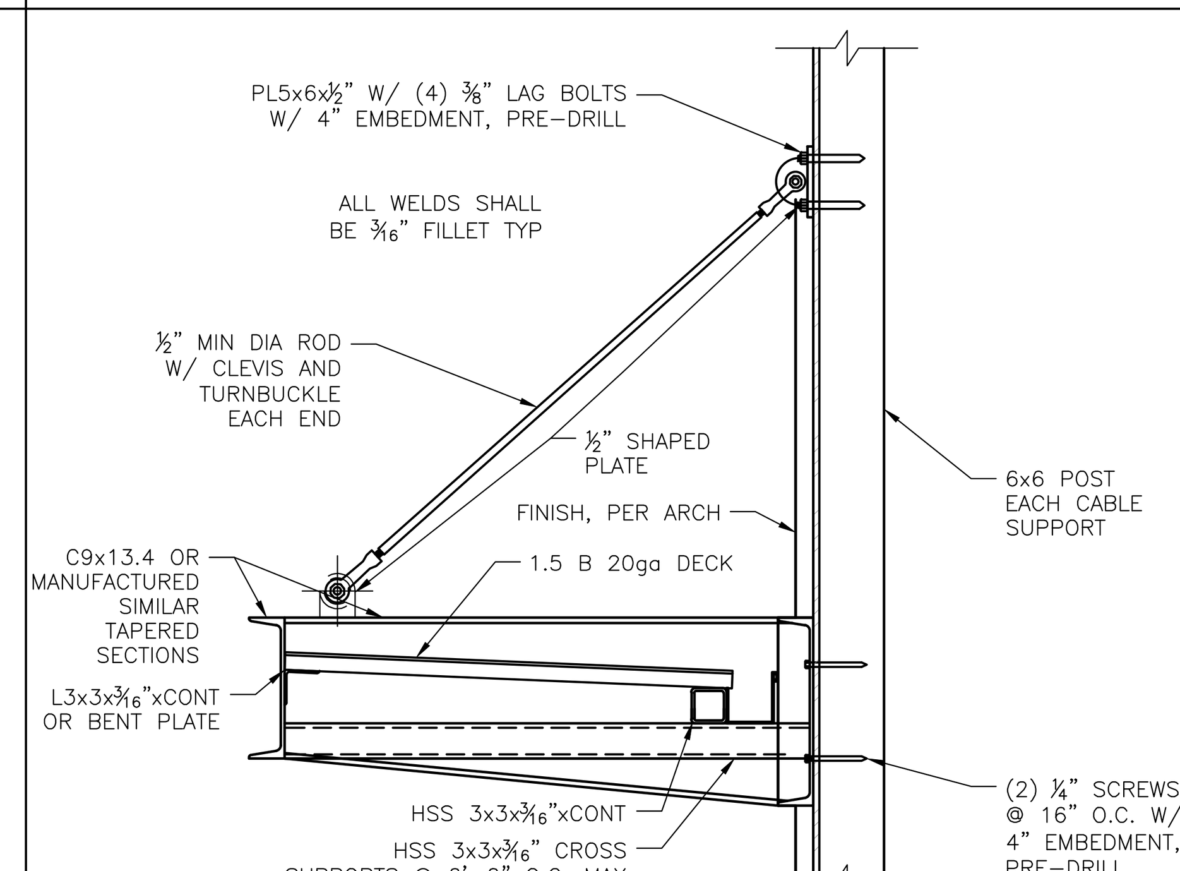
49 TYPICAL TRUSS AT PARTY WALL
N.T.S.



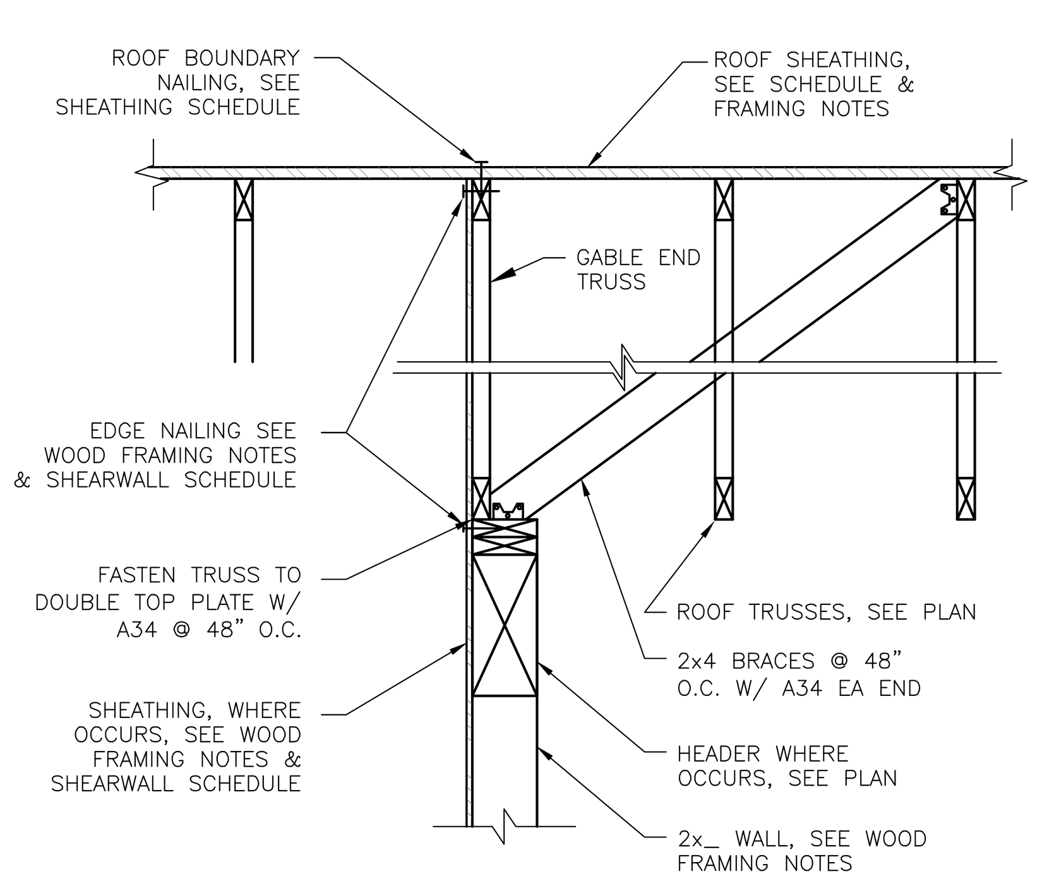
45 TOP PLATE SPLICE
N.T.S.



42 TRUSS BEARING @ EXTERIOR WALL
N.T.S.

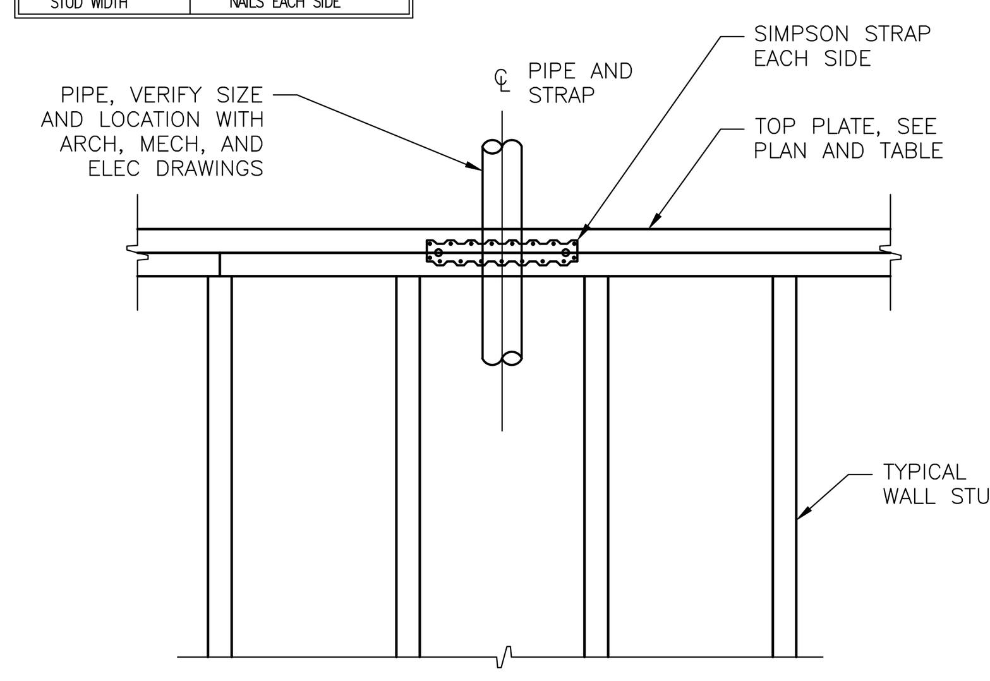


53 TYPICAL CANOPY DETAIL
N.T.S.

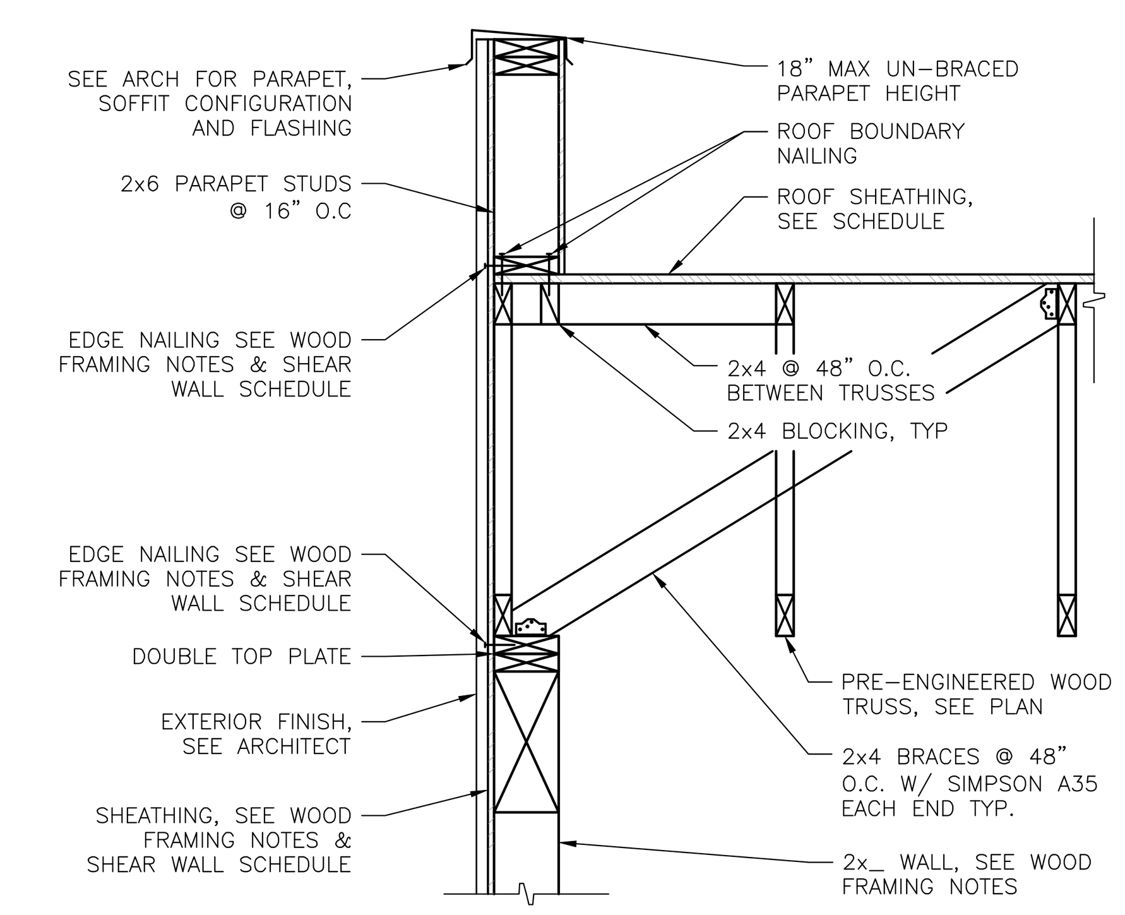


50 INTERIOR SHEAR/BEARING WALL TO ROOF
N.T.S.

TOP PLATE SPLICE	
HOLE SIZE	STRAP
LESS THAN ½ STUD WIDTH	ST2122 W/ (6) 16d NAILS EACH SIDE
MORE THAN ½ STUD WIDTH	ST2215 W/ (8) 16d NAILS EACH SIDE



46 TOP PLATE SPLICE AT PIPE
N.T.S.



43 TRUSS BEARING @ EXTERIOR WALL
N.T.S.

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH

ROOF FRAMING DETAILS

PLOT SCALE: AS NOTED
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JEC

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S4.2

LEGEND

Symbol	Description
DETAIL MARK / SHEET NUMBER	
FCXX	INDICATES CONTINUOUS FOOTING, SEE SCHEDULE ON S5.0
FSXX	INDICATES SPOT FOOTING, SEE SCHEDULE ON S5.0
(dashed)	DEPRESSED FOUNDATION AND POUR SLAB OVER, SEE DETAIL 2/S4.0
C.J.	INDICATES CONTROL/ CONSTRUCTION JOINT, SEE DETAIL ON S0.1
CW-X	CONCRETE WALL, SEE SCHEDULE ON S5.0
SW-X	SHEARWALL, SEE SCHEDULE ON S5.0
HD-X	HOLDOWN, SEE SCHEDULE ON S5.0

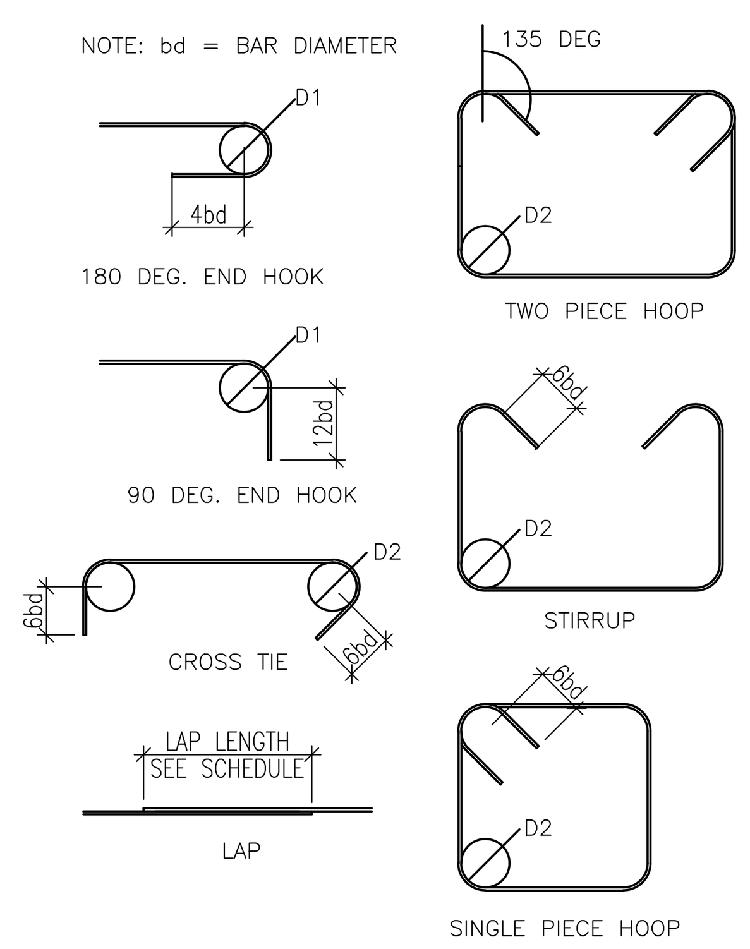
NOTES

1. COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL DRAWINGS.
2. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
3. ALL SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS (U.N.O.)
4. SEE DETAIL 8/S4.0 FOR TYPICAL SLAB ON GRADE JOINTS.
5. SEE DETAIL 9/S4.0 FOR TYPICAL WALL REINFORCING AT CONCRETE WALL CORNERS AND INTERSECTIONS.
6. SEE DETAIL 10,11/S4.0 FOR BURIED PIPES PARALLEL AND PERPENDICULAR TO FOOTING.
7. SEE DETAIL 12/S4.0 FOR TYPICAL STEP FOOTING, WHERE OCCURS, SEE PLANS.
8. SEE DETAIL 14/S4.0 FOR TYPICAL STRUCTURAL FILL REQUIREMENTS.
9. SEE DETAIL 16 FOR TYPICAL DUMPSTER ENCLOSURE WALL AND FOUNDATION.
10. VERIFY ALL ROOF AND FLOOR OPENINGS FOR MECHANICAL SHAFTS, STAIRS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
11. TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT ALL APPLICABLE SNOW, MECHANICAL. AND LATERAL LOADS PER THE IBC.
12. TRUSS MANUFACTURER SHALL DESIGN TRUSSES AND SUPPLY ADDITIONAL BLOCKING/BRIDGING AS REQUIRED FOR UPLIFT DUE TO WIND.
    ASSUME: DEAD LOAD = 12psf
    WIND UPLIFT = 8psf (NET)
    NO 1/3 STRESS INCREASE ALLOWED.
13. ALL GIRDER TRUSSES SHALL HAVE FULL WIDTH COLUMNS BELOW IN WALL.

REINFORCING BAR REQUIREMENTS

BAR SIZE	D1	D2	LAP LENGTH
#3 REBAR	2 1/4"	1 1/2"	16"
#4 REBAR	3"	2"	19"
#5 REBAR	3 3/4"	NA	24"
#6 REBAR	4 1/2"	NA	32"
#7 REBAR	5 1/4"	NA	42"
#8 REBAR	6"	NA	55"

NOTE: bd = BAR DIAMETER



7 REINFORCING BAR SCHEDULE
SCALE: NTS

CONCRETE FOOTING SCHEDULE

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE NO	SIZE	LENGTH	SPACING	REINFORCING LENGTHWISE NO	SIZE	LENGTH	SPACING	COMMENTS
FC2.0	2'–0"	CONT.	1'–0"					3	#5	CONT.	EQUAL	
FC2.5	2'–6"	CONT.	1'–0"					3	#5	CONT.	EQUAL	
FC3.0	3'–0"	CONT.	1'–0"		#5	2'–6"	14" O.C.	4	#5	CONT.	EQUAL	
FS3.0	3'–0"	3'–0"	1'–0"	3	#5	2'–6"	EQUAL	3	#5	2'–6"	EQUAL	
FS4.0	4'–0"	4'–0"	1'–0"	4	#5	4'–6"	EQUAL	4	#5	4'–6"	EQUAL	
FS6.0	6'–0"	6'–0"	1'–0"	6	#5	5'–6"	EQUAL	6	#5	5'–6"	EQUAL	

NOTES:

1. ALLOWABLE SOIL BEARING PRESSURE = 1,750 PSF (PER GEOTECHNICAL REPORT).
2. MINIMUM COMPRESSIVE CONCRETE STRENGTH f'c = 3000 PSI.
3. ALL REINFORCING STEEL SHALL BE GRADE 60 AND BE PROPERLY TIED INTO PLACE PRIOR TO POUR.
4. ALL CONCRETE WORK MUST MEET THE REQUIREMENTS OF THE 2015 IBC, ACI 318 AND LOCAL ORDINANCES.
5. PLACE FOOTING REINFORCING IN THE BOTTOM OF THE FOOTING WITH 3" CLEAR CONCRETE COVER (UNO).

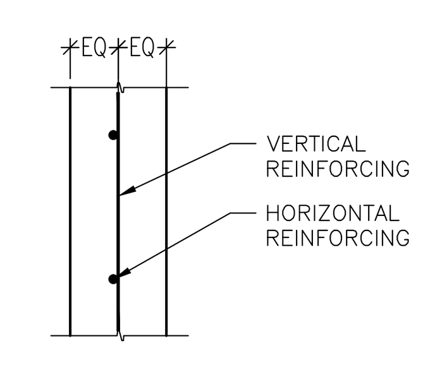
4 CONCRETE FOOTING SCHEDULE
SCALE: NTS

CONCRETE WALL SCHEDULE

WALL MARK	THICKNESS	REINFORCING VERTICAL	HORZONTAL	TOP AND BOTTOM	WALL TYPE	COMMENTS
CW–1	8"	#4 @ 18" O.C.	#4 @ 12" O.C.	(2) #4	"A"	

NOTES:

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. CONCRETE WALLS NOT DESIGNATED ON PLANS SHALL BE CW–1.

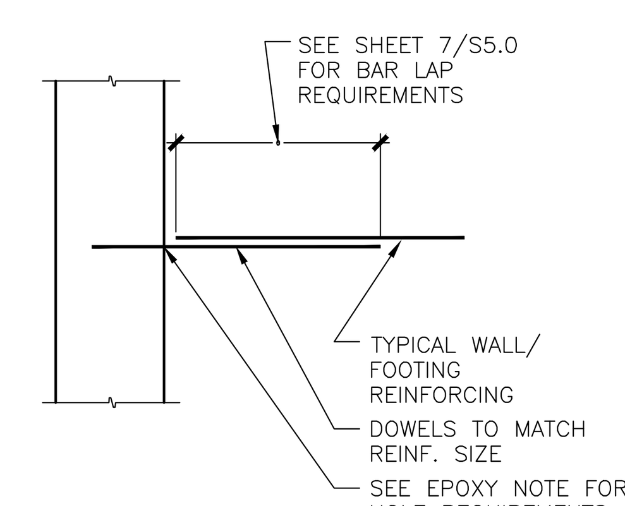


WALL TYPE "A"

5 CONCRETE WALL SCHEDULE
SCALE: NTS

EPOXY DOWEL EMBED SCHEDULE

DOWEL SIZE	MINIMUM EMBEDMENT INTO EXISTING CONCRETE
#3 REBAR	4 1/2"
#4 REBAR	6"
#5 REBAR	7 1/2"
#6 REBAR	9"
#7 REBAR	10 1/2"
#8 REBAR	12"



NOTES:

1. USE EMBEDMENT DEPTH ON SCHEDULE WHERE NO EMBEDMENT IS CALLED OUT ON PLANS.
2. SEE EPOXY NOTES AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

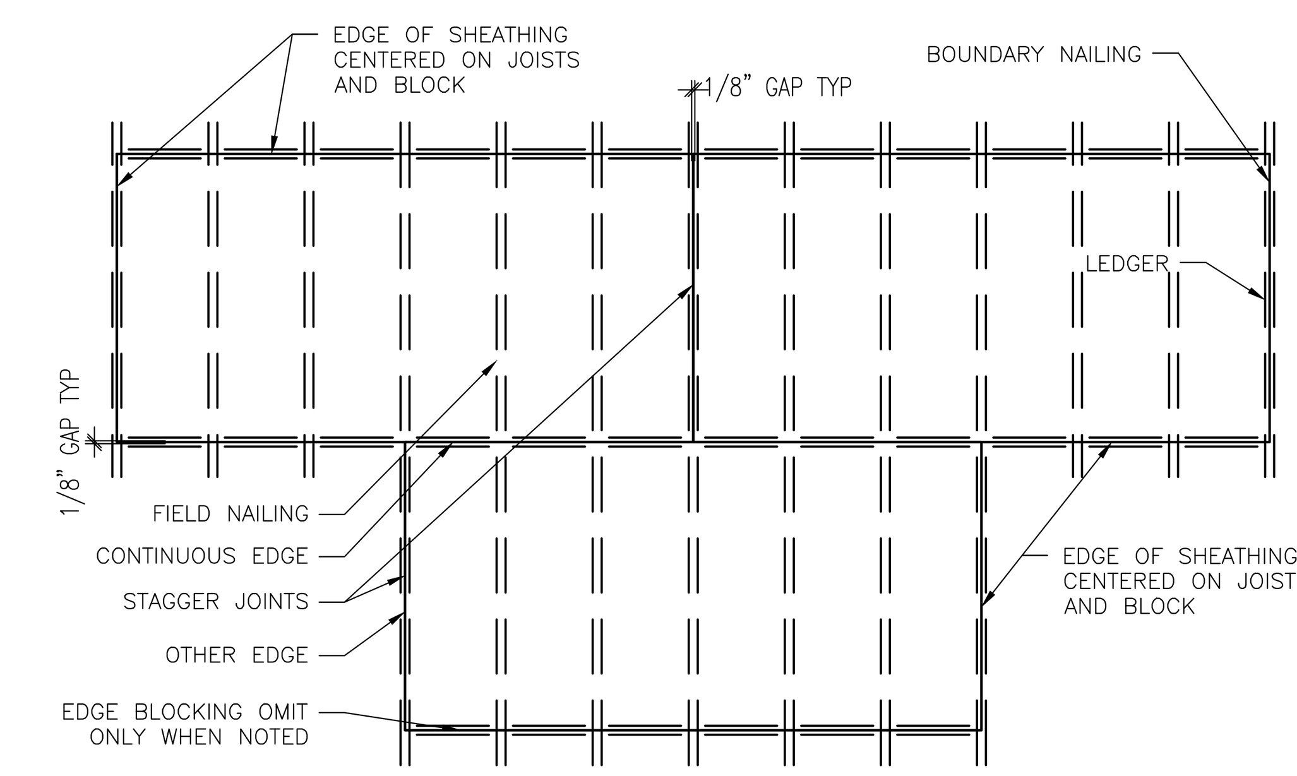
6 EPOXY DOWEL SCHEDULE
SCALE: NTS

SHEATHING SCHEDULE

LOCATION	SHEATHING	NAIL SIZE	EDGE NAILING	FIELD NAILING	BOUNDARY NAILING	EDGE BLOCKING	COMMENTS
ROOF	$\frac{19}{32}$"	10d	6" O.C.	12" O.C.	6" O.C.	NO	
FLOOR	$\frac{23}{32}$"	10d	6" O.C.	12" O.C.	6" O.C.	NO	

NOTES:

1. MIN NAIL PENETRATION INTO FRAMING, 8d – $1\frac{1}{2}$", 10d – $1\frac{5}{8}$".
2. USE COMMON NAILS.
3. NAILING SHALL BE SPACED AT $\frac{3}{8}$" MINIMUM FROM EDGE OF PANEL.
4. CONTINUOUS EDGE NAILING IN ONLY REQUIRED WHEN BLOCKING IS REQUIRED.



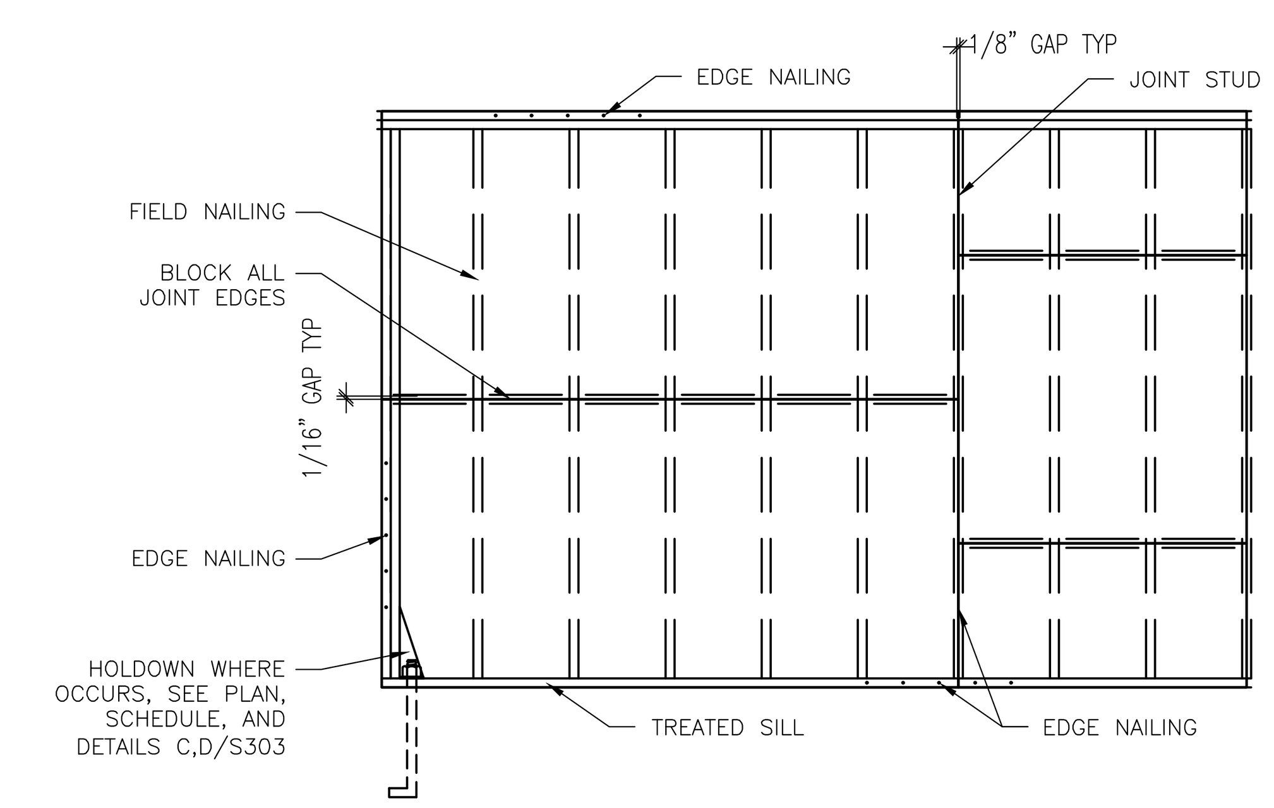
1 SHEATHING SCHEDULE
SCALE: NTS

SHEARWALL SCHEDULE

MARK	SHEATHING	NAIL SIZE	EDGE NAILING	FIELD NAILING	PANEL EDGE MEMBER	SILL BOLTING TO CONCRETE	COMMENTS
SW–1	$\frac{7}{16}$"	8d	6" O.C.	12" O.C.	2x	$\frac{5}{8}$"x12" @ 32" O.C.	
SW–2	$\frac{7}{16}$"	8d	4" O.C.	12" O.C.	2x	$\frac{5}{8}$"x12" @ 32" O.C.	
SW–3	$\frac{7}{16}$"	8d	3" O.C.	12" O.C.	**3x**	$\frac{5}{8}$"x12" @ 16" O.C.	

NOTES:

1. MIN NAIL PENETRATION INTO FRAMING, 8d – $1\frac{1}{2}$", 10d – $1\frac{5}{8}$". USE COMMON NAILS.
2. (2) 2x MEMBERS SHALL NOT BE USED TO REPLACE 3x MEMBERS. 4x MEMBERS MAY BE USED TO REPLACE 3x MEMBERS.
3. PLYWOOD, ORIENTED STRAND BOARD AND COMPOSITE BOARD (BUT NOT STRUCTURAL PARTICLE BOARD) ARE ACCEPTED AS EQUIVALENT.
4. ALL ANCHOR BOLTS SHALL HAVE A 3x3x$\frac{3}{16}$" PLATE WASHER.
5. SHEAR WALL PANELS INDICATED ON SCHEDULE ARE TO BE SHEATHED FOR THE FULL HEIGHT OF THE WALL.
6. ALL FRAMING SHALL BE SPACED AT 16" O.C.
7. NAILING SHALL BE SPACED AT $\frac{3}{8}$" MINIMUM FROM EDGE OF PANEL.
8. SEE HOLDOWN SCHEDULE AND PLAN FOR LOCATION AND SIZE OF HOLDOWNS REQUIRED.
9. ALL EXTERIOR WALL NOT SPECIFIED AS A SHEAR WALL SHALL BE SW–1



2 SHEARWALL SCHEDULE
SCALE: NTS

HOLDOWN SCHEDULE

MARK	HOLDOWN	EDGE MEMBER	FOUNDATION ANCHOR	EMBEDMENT	EDGE MEMBER ATTACHMENT	COMMENTS
HD–1	HDU2–SDS2.5	(2) 2x_	$\frac{5}{8}$" DIA.	12"	(6) SDS $\frac{1}{4}$"x$2\frac{1}{2}$"	
HD–2	HDU4–SDS2.5	(2) 2x_	$\frac{5}{8}$" DIA.	14"	(10) SDS $\frac{1}{4}$"x$2\frac{1}{2}$"	
HD–3	HDU8–SDS2.5	(2) 2x_	$\frac{7}{8}$" DIA.	21"	(20) SDS $\frac{1}{4}$"x$2\frac{1}{2}$"	
HD–4	CS16	(1) 2x_	N/A	N/A	(13) 8d EACH END	
HD–5	MST37	(2) 2x_	N/A	N/A	(11) 16d EACH END	
HD–6	MST48	(2) 2x_	N/A	N/A	(17) 16d EACH END	

NOTES:

1. ALL ANCHORS SHALL BE SIMPSON OR EQUIVALENT.
2. INSTALLATION OF HOLDOWN ANCHORS AND STRAPS SHALL BE PER MANUFACTURERS RECOMMENDATIONS AND SPECIFICATIONS.
3. WHEN 3x MEMBERS ARE USED FOR PANEL EDGES IN SHEAR WALL. 4x MEMBERS SHALL BE USED FOR HOLDOWNS.
4. SSTB ANCHOR BOLTS MAY BE SUBSTITUTED FOR THE SCHEDULED ANCHOR BOLTS.
5. INCREASE FOOTING DEPTH WHERE EMBEDMENT LENGTH PLUS 3" IS GREATER THAN FOOTING DEPTH SPECIFIED.
6. PROVIDED EDGE NAILING ALONG ALL STUDS CONNECTED TO HOLDOWN ANCHORS AND STRAPS.

3 HOLDOWN SCHEDULE
SCALE: NTS

PROFESSIONAL ENGINEER 375048-2202 JOHN E. CHARCHENKO 9-18-19 STATE OF UTAH

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CHLOE'S POINTE APARTMENTS
OGDEN, UTAH
NOTES AND SCHEDULES

PLOT SCALE: AS NOTED
DATE: 9/18/19
JOB: JOB# 19-006
JEC

ISSUES/REV:	DATE:
PERMIT SET	6–7–19
CITY RESPONSE	7–22–19
STRUCT ADDENDUM 1	9–18–19

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