

HOMEWORK # 4

CHAPTER 4

PROBLEM 21

GIVEN: Service tensile loads: PD= 35k, PL=115 k, and Ps=65 k. The connection is through the flange with two lines of three 3/4-in diameter bolts 4 in on center. Use A572 Grade 50 steel. Neglect block shear.

Load Info:

D =	35	kips	L =	115	kips	R =	0	kips	S =	65	kips
I =	0	kips	Lr =	0	kips	W =	0	kips	E =	0	kips
Pu =	0	kips	Pa =	0	kips						

REFERENCE:

<u>Structural</u>	Yield stress	Fy =	50 ksi	table 2-4	page 2-48
<u>A572 Gr50</u>	Ultimate stress	Fu =	65 ksi	table 2-4	page 2-48

<u>Shape</u>	Shape:		page 1-72
<u>Values:</u>	Gross area	Ag =	in ² table 1- page 1-72
	xbar	x =	in table 1- p 1-42
	bolt diameter	=	0.75 in Statement
	nominal hole dia	=	0.813 in table J3.3 p 16.1-121
	effective hole dia	h =	0.875 in B4.3b p 16.1-18
	length	l =	6 in
	thickness	tf =	0.285 in Statement
	Member Length	L =	20 ft Statement
	Number of bolts in A, N =		Statement
	Gage	g =	1 in
	Pitch	s =	in
	Number of sections, Ns =		
	Diagonal Factor =	df =	0

Figure

FIND: Select an ST shape to be used as a 20ft long tension member that will safely support the service tensile loads.

METHOD: 1. Find loads according to AISC p2-10

2. Determine min Ag

2.1) YIELDING STRENGTH

- 1 $P_n = F_y \cdot A_g$
- 2 LRFD yield strength = $\Phi \cdot P_n$ $\Phi = 0.9$
- 3 ASD yield strength = P_n / Ω $\Omega = 1.67$

D2-1 page 16.1-26

2.2) RUPTURE STRENGTH:

- 1 $P_n = F_u \cdot A_e$
- 2 LRFD rupture strength $= \Phi \cdot P_n$ $\Phi = 0.75$
- 3 ASD rupture strength $= P_n / \Omega$ $\Omega = 2.00$

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WHERE: Effective net area: $A_e = A_n \cdot U$
 A_n = net area determined
 U = shear lag factor

D3-1 p 16.1-27
 B4.3
 Table D3.1

3. Determine min r
4. Check limit states

SOLUTION: **1. Finding loads:**

1.1) LRFD:

			Uniform Load Cases						
	Load Combinations	D	L+I	Lr	S	R	W	E	Sum
1	1.4D	35							35
2	1.2D+1.6L+0.5(Lr or S or R)	42	184	0	32.5	0			259
3	1.2D+1.6(Lr or S or R)+(L* or 0.5W)	42	57.5	0	104	0	0		204
4	1.2D+1.0W+L*+0.5(Lr or S or R)	42	57.5	0	32.5	0	0		132
5	1.2D+1.0E+L* + 0.2S	42	57.5		13			0	113
6	0.9D+1.0W	31.5					0		32
7	0.9D+1.0E	31.5						0	32

***Note:** Change Load Factor for 1 for public assembly, live loads in excess of 100 psf and for parking

The governing factored load for design is equal to: **258.5** kips

1.2.) ASD:

			Uniform Load Cases						
	Load Combinations	D	L+I	Lr	S	R	W	E	Sum
1	D	35							35
2	D+L	35	115						150
3	D+(Lr or S or R)	35		0	65	0			100
4	D+0.75L + 0.75(Lr or S or R)	35	86.25	0	48.75	0			170
5	D+(0.6W OR 0.7E)	35					0	0	35
6 (a)	D+0.75L+0.75(0.6W)+0.75(Lr/S/R)	35	86.25	0	48.75	0	0		170
6 (b)	D+0.75L+0.75(0.7E)+0.75S	35	86.25		48.75			0	170
7	0.6D+0.6W	21					96		117
8	0.6D+0.7E	21						0	21

The governing factored load for design is equal to: **170** kips

Pu = 258.5 kips

Pa = 170 kips

2. Determine min Ag

2.1) YIELDING STRENGTH

LRFD:

$$A_g \geq P_u / \phi * F_y = 5.744 \text{ in}^2$$

ASD:

$$A_g \geq P_a * \Omega / F_y = 5.678 \text{ in}^2$$

2.2) RUPTURE STRENGTH:

*This assumption should start with smaller number for a

Assuming $U = 0.85$ larger value of area.

We have that, $P_u = F_u * A_e$ or $A_g \geq P_u / \phi * F_u * U + 0.15 A_g$

LRFD:

$$A_g \geq P_u / \phi * F_u * U * 0.85 = 7.339 \text{ in}^2$$

ASD:

$$A_g \geq P_a * \Omega / F_u * U * 0.85 = 5.68 \text{ in}^2$$

LRFD:	ASD:
Thus the governing minimum $A_g \geq 7.339$	5.678 in^2

3. Determining Slenderness Ratio Limitation

$$r \geq L / 300 \text{ in} \geq 0.8 \text{ in}$$

$$r \geq 0.8 \text{ in}$$

*Make other used cases sheet.

4. Trials

TRIAL SHAPE			Ag>	r>	SHEAR LAG: CASE 2		SHEAR LAG: CASE 7 (a)			Ag,r	Sat*	Umax
TYPE		WGT			x-	U	bf	d	U			
ST	7.5	25	7.34	2.35	2.25	0.625	5.64	7.5	0.9	8.1547	0	0.9
ST	9	27.35	8.02	2.79	2.51	0.582	6	9	0.9	8.1547	0	0.9
ST	10	33	9.7	3.1	2.81	0.532	6.26	10	0.85	8.6343	1	0.9
						1		0	0.9	7.3392	0	1
						1		0	0.9	7.3392	0	1
						1		0	0.9	7.3392	0	1
						1		0	0.9	7.3392	0	1
						1		0	0.9	7.3392	0	1

Sat*: 1- Mean if the area gross used given the shape used still satisfy the possibly new maximum minimum area.

0- New area does not satisfied whichever new maximum minimum area.

Min Area that satisfy Smallest section to start check=

5. Checking Limit States

SECTION STRENGTH

5.1. YIELD

TRIAL SHAPE			Ag> 0	LRFD		ASD	
				Pn*φ= Fy*Ag*φ		Pn/Ω= Fy*Ag*Ω	
TYPE	WGT			Pn*φ	Pn*φ> Pu	Pn/Ω	Pn/Ω> Pa
ST	7.5	25	7.34	330.3	1	219.8	1
ST	9	27.35	8.02	360.9	1	240.1	1
ST	10	33	9.7	436.5	1	290.4	1
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

<- Given the sort this is the best option.
The are organized accordingly till the last row.

5.1. RUPTURE

IF STAGGERED
(use diagonal
factor):

$$**A_n = A_g - N \cdot h \cdot t_f + N_s \cdot (s^2 / 4g)$$

$$A_e = A_n \cdot U$$

TRIAL SHAPE				Ag> 0	An**	U	Ae	Pn	LRFD		ASD	
									Pn*φ= Fu*Ae*φ		Pn/Ω= Fu*Ae/Ω	
TYPE	WGT								Pn*φ	Pn*φ> >Pu	Pn/Ω	Pn/Ω> >Pa
ST	7.5	25	7.34	7.34	0.9	6.606	429.4	322.04	1		214.7	1
ST	9	27.35	8.02	8.02	0.9	7.218	469.2	351.88	1		234.59	1
ST	10	33	9.7	9.7	0.85	8.245	535.9	401.94	1		267.96	1
0	0	0	0	0	1	0	0	0	0		0	0
0	0	0	0	0	1	0	0	0	0		0	0
0	0	0	0	0	1	0	0	0	0		0	0
0	0	0	0	0	1	0	0	0	0		0	0
0	0	0	0	0	1	0	0	0	0		0	0

<-

Sat*: 1- Mean if the area gross used given the shape used still satisfy the possibly new maximum minimum area.

0- New area does not satisfied whichever new maximum minimum area.

≤- Given the sort filter applied the top rows represents the best options

Given these two values the choice of section is: ST 10X 33