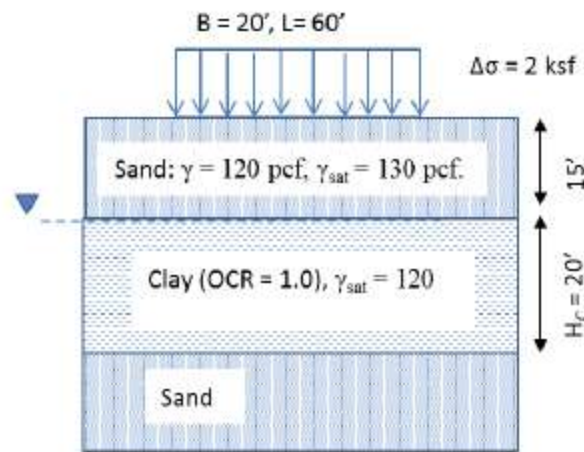


GIVEN



Clay consolidation test data: $e_0 = 0.9$, $C_c = .35$, $C_v = .003 \text{ ft}^2/\text{min}$, $C_\alpha = .01$

$$\begin{aligned} B &:= 20 \text{ ft} & L &:= 60 \text{ ft} \\ q_0 &:= 2 \cdot 10^3 \text{ psf} & & \\ H_s &:= 15 \text{ ft} & H_c &:= 20 \text{ ft} \\ \gamma_1 &:= 120 & \gamma_{\text{sat}2} &:= 120 \\ \gamma_{\text{sat}1} &:= 130 & \text{OCR} &:= 1 \\ C_c &:= .35 & e_0 &:= 0.9 \\ C_v &:= .003 \frac{\text{ft}^2}{\text{min}} & C_\alpha &:= 0.01 \\ D_f &:= 0 & \gamma_w &:= 62.4 \text{ pcf} \end{aligned}$$

FIND

1. Total Settlement at the center of the surcharge after primary consolidation
2. Time to reach end of primary consolidation $U=99\%$ in days.
3. Additional settlement (in.) at the center due to secondary consolidation $t=25$ years later

METHOD

- 1.1 Determine the effective stress in the middle of clay layer
- 1.2 Use Griffiths EQ 5.19 Determine the average vertical stress increase in the given layer
- 1.3 In order to determine Primary Consolidation Use EQ 1.61 (since normally consolidated clay)
- 1.4 Determine Elastic Settlement Using Janbu
- 1.5 Total Settlement = 1.1 + 1.4 = $S_e + S_c$

2.

SOLUTION

Height	Stress	Pore Pressure	Effective Stress
$H_s = 15$	$\sigma_1 := H_s \cdot \gamma_1 = 1.8 \times 10^3$	$u_1 := 0$	$\sigma'_1 := \sigma_1 - u_1 = 1.8 \times 10^3 \text{ psf}$
$\underline{H_c} := \frac{H_c}{2} = 10$	$\sigma_2 := H \cdot \gamma_{\text{sat}2} + \sigma_1 = 3 \times 10^3$	$u_2 := \gamma_w \cdot H = 624$	$\sigma'_2 := \sigma_2 - u_2 = 2.376 \times 10^3 \text{ psf}$
	$\sigma'_o := \sigma'_2 = 2.376 \times 10^3 \text{ psf}$		

1.2 Since EQN 5.19 determines the average vertical stress within a corner of a rectangular area, we'll divide the surcharge area into 4 pieces (Thus $B/2$ and $L/2$). We also have that:

$$\underline{H_2} := H_s + H_c = 35 \quad \underline{H_1} := H_s = 15 \text{ ft}$$

To determine $I_a(h_2)$ and $I_a(h_1)$: (Determine factors below and use Figure 5.7 for I_a 's)

$$\begin{aligned} m_2 &:= \frac{\frac{B}{2}}{H_2} = 0.286 & n_2 &:= \frac{\frac{L}{2}}{H_2} = 0.857 & I_{a2} &:= .155 & m_1 &:= \frac{\frac{B}{2}}{H_1} = 0.667 & n_1 &:= \frac{\frac{L}{2}}{H_1} = 2 & I_{a1} &:= 0.22 \end{aligned}$$

$$\Delta\sigma_1 := q_0 \cdot \frac{(H_2 \cdot I_{a2} - H_1 \cdot I_{a1})}{(H_2 - H_1)} = 212.5$$

The total stress is equal to 4 times the value obtained (since 4 areas):

$$\Delta\sigma_{avg} := 4 \cdot \Delta\sigma_1 = 850$$

Primary Consolidation: $S_p := \left(\frac{C_c \cdot H_c}{1 + e_0} \right) \cdot \log \left(\frac{\sigma'_0 + \Delta\sigma_{avg}}{\sigma'_0} \right) = 0.489$ ft $S_{p'} := S_p \cdot 12 = 5.872$ in

PART 2: TIME TO REACH END OF PRIMARY CONSOLIDATION:

For U=99% $T_v := 1.781$ Since both sand layers are freely draining $\underline{H_v} := \frac{H_c}{2} = 10$ $t := T_v \cdot \frac{H_v^2}{C_v} = 5.937 \times 10^4$ min

$$t' := \frac{t}{60 \cdot 24} = 41.227 \text{ days}$$

PART 3 - SETTLEMENT DUE TO SECONDARY CONSOLIDATION $t_2' := 25$ years $t_2 := t_2' \cdot 365 = 9.125 \times 10^3$ days

$t_1 := t' = 41.227$ days From EQ 5.91 $\Delta e := e_p - e_0$ $e_p := C_\alpha \cdot \log \left(\frac{t_2}{t_1} \right) + e_0 = 0.923$

$C_\alpha' := \frac{C_\alpha}{(1 + e_p)} = 5.199 \times 10^{-3}$ $S_s := C_\alpha' \cdot H_c \cdot \log \left(\frac{t_2}{t_1} \right) = 0.244$ ft $S_{s'} := S_s \cdot 12 = 2.926$ in