AERO-S Manual

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1 INTRODUCTION

This section compiles basic information on how to prepare an input data for a Finite Element analysis using the FEM code. The input data consists of command macros, clustered numerical data, and comment lines. Each data cluster is preceded by a command statement.

The following rules of thumb are suggested:

1 The users are allowed to have as many comment lines as they wish, as long as the first column of each line starts with

* (see **COMMENTS**).

- The subsequent data clusters may appear in any order. The command statement preceding each cluster may be in lower, upper, or mixed characters.
- 3 The data in each cluster are free format.
- 4 Only the first four letters of a command need to be specified unless otherwise noted.

Any segment of a FEM input file — for example, the data corresponding to any command macro, or a command macro and its data, or any number of input lines — can be replaced by a statement of the form

INCLUDE filename (or "filename")

where filename is the name of the file containing the information. "filename" can contain a path or can be replaced by <filename> in which case the path is that of the environment variable \\$FEM_INCLUDE. Furthermore, the included file filename can be compressed by gzip, bzip2, or zip.

The following subsections discuss in detail the syntax of each data cluster command.

Next: ATDARB, Previous: INTRODUCTION

2 INSTALLATION

The installation of **AERO-S** on a given computing system requires the availability on that system of the following tools:

C++ compiler g++ Version 4.1.2 or higher. Fortran compiler gfortran Version 4.1.2 or higher.

Flex utility Version 2.5 or higher. Flex is a lexical analyser required for building the parser of **AERO-S**'s input

command data file.

Bison utility Version 2.3 or higher. Bison is a parser generator required for building the parser of AERO-S's input

command data file.

CMake utility Version 2.6 or higher. CMake is a cross-platform open-source build system. It is comparable to the Unix

Make program in that the build process is ultimately controlled by configuration files (CMakeLists.txt). However unlike Make, it does not directly build the final software but instead generates standard build files such as makefiles for Unix and projects/workspaces for Windows Visual C++. The CMake version 2.6

utility can be obtained from http://www.cmake.org. (Note: a "README.cmake" file discussing details on cmake options for code configuration and installation is available in the directory containing the source code of **AERO-S**).

and following libraries:

BLAS library BLAS is a set of Basic Linear Algebra Subprograms required by various operations performed in

AERO-S.

LAPACK library LAPACK is a high-performance Linear Algebra PACKage with advanced solvers.

MPI library openmpi Version 1.2.6 or higher. Open MPI is a high-performance implementation of the Message Passing Interface

(MPI) required for performing interprocessor communication, among others. More specifically, **AERO-S** requires an MPI-2 implementation such as the one provided by the Open MPI project.

OpenMP API Open Multi-Processing is an Application Programming Interface (API) that supports multi-platform

shared memory multiprocessing programming in C, C++ and Fortran on many architectures, including

Unix. As an option, AERO-F can be compiled with OpenMP to enable multi-threaded execution.

In addition, the following optional libraries extend the capabilities of **AERO-S**:

SPOOLES library SPOOLES is a library for solving sparse real and complex linear systems of equations with a sparse direct

solver, written in the C language using object oriented design.

MUMPS library MUMPS is a library for solving sparse real and complex linear systems of equations with a multifrontal

massively parallel sparse direct solver.

ARPACK library ARPACK is the Arnoldi PACKage for the solution of large-scale symmetric, nonsymmetric, and generalized

eigenproblems.

Scalapack library Scalapack is also known as the Scalable LAPACK. This library includes a subset of LAPACK routines

redesigned for distributed memory MIMD parallel computers.

BLACS library BLACS (Basic Linear Algebra Communication Subprograms) is a linear algebra oriented message passing

interface designed for linear algebra.

PARPACK library PARPACK is the parallel version of ARPACK used by **AERO-S**'s parallel eigensolver.

METIS library METIS is a library of graph manipulation routines that can be used by AERO-S for reordering of a sparse

matrix to reduce the number of fill-in entries created during factorization.

ACME library ACME is a C++ based library for performing general finite element proximity searches and transient

dynamic contact enforcement. It is fully parallel and currently used in AERO-S for contact and

(frequency domain) fluid-structure interaction problems.

Zoltan library The Zoltan library includes among other things a suite of dynamic load-balancing and parallel partitioning

tools that are used by the ACME library for parallel proximity searches.

Eigen2 library Eigen2 is a versatile C++ template library for linear algebra (vectors, matrices, and related algorithms (see

http://eigen.tuxfamily.org)).

To install **AERO-S**, follow the procedure specified below:

• From the directory containing the source code of **AERO-S**, type cmake -DAERO=1 . Note the space and the "." after the command cmake. The "." specifies the current directory.

• Watch the computer screen and verify that all invoked components were found and all build options were correct. A sample computer screen output of the cmake command is:

```
-- The C compiler identification is GNU
-- The CXX compiler identification is GNU
-- Check for working C compiler: /usr/bin/gcc
-- Check for working C compiler: /usr/bin/gcc -- works
-- Detecting C compiler ABI info
-- Detecting C compiler ABI info - done
-- Check for working CXX compiler: /usr/bin/c++
-- Check for working CXX compiler: /usr/bin/c++ -- works
-- Detecting CXX compiler ABI info
-- Detecting CXX compiler ABI info - done
  The Fortran compiler identification is GNU
-- Check for working Fortran compiler: /usr/bin/gfortran
-- Check for working Fortran compiler: /usr/bin/gfortran -- works
-- Detecting Fortran compiler ABI info
-- Detecting Fortran compiler ABI info - done
-- Checking whether /usr/bin/gfortran supports Fortran 90
-- Checking whether /usr/bin/gfortran supports Fortran 90 -- yes
 -- Found MPI: /usr/lib/openmpi/lib/libmpi_cxx.so
-- Try OpenMP C flag = [-fopenmp]
-- Performing Test OpenMP_FLAG_DETECTED
-- Performing Test OpenMP_FLAG_DETECTED - Success
-- Try OpenMP CXX flag = [-fopenmp]
-- Performing Test OpenMP_FLAG_DETECTED
-- Performing Test OpenMP_FLAG_DETECTED - Success
-- Found OpenMP: -fopenmp
-- Looking for Fortran sgemm
-- Looking for Fortran sgemm - found
-- A library with BLAS API found.
-- Looking for Fortran cheev
-- Looking for Fortran cheev - found
-- A library with LAPACK API found.
-- A library with ACME API found.
 - A library with ARPACK API found
-- A library with SPOOLES API found.
-- A library with EIGEN2 API found.
 -- A library with METIS API found.
 - A library with MUMPS API found.
-- A library with BLACS API found.
-- A library with SCALAPACK API found.
-- A library with METIS API found.
           Summary of build options
Distributed FETI:
                           YES
Aeroelastic:
                           YES
Mumps:
                           YES
Arpack:
                           YES
Spooles:
                           YES
Acme:
                           YES
Metis:
                           YES
OpenMP:
                           YES
Build type:
                           Release
Extra libraries:
-- Configuring done
```

-- Generating done

-- Build files have been written to: /home/pavery/Codes/FEM

- If necessary, edit the CMakeCache.txt file to include the file paths to all required and desired optional components that were not automatically found by cmake. Typically, the compilers, the utilities Flex and Bison, the libraries MPI, BLAS and LAPACK, and the API OpenMP will be automatically found. However, it may be necessary to specify the paths for the libraries SPOOLES, MUMPS, Scalapack, Blacs, Arpack, Parpack, Metis, Acme, Zoltan and Eigen2.
- Then, also from the directory containing the source code of AERO-S, type make.

The successful completion of the procedure described above leads to the creation in the bin/directory of **AERO-S**'s executable aeros.

Next: ATDDIR, Previous: INSTALLATION

3 ACOUSTIC TIME-DOMAIN ARTIFICIAL BOUNDARY *S*

Command Statement:	ATDARB
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The ATDARB command statement is used to specify the artificial boundary on which an Antoine absorbing condition is to be applied in the time domain, and the order of this absorbing boundary condition. The input format is given below.

ATDARB ORDER

FACE	FACE_TYPE	CONNECTIVITY_NODES
	ORDER	Order of Antoine's absorbing boundary condition in the time domain (real). Currently supported values are 0 and 1.
	FACE	Face (or edge in two dimensions) identification number whose type and connectivity are to be specified (integer). In practice, this identification number is ignored by AERO-S .
	FACE_TYPE	
	1	2-node line segment. To be used with two-dimensional linear elements.
	2	3-node line segment. To be used with two-dimensional quadratic elements.
	3	3-node triangular face. To be used with three-dimensional linear tetrahedral element.
	4	4-node quad face. To be used with three-dimensional linear hexahedral element.
	6	6-node triangular face. To be used with three-dimensional quadratic tetrahedral element.
	10	n^2 -node arbitrarily higher-order quad face where n is the number of nodes on an edge of this face. To be used with element type 95.
	11	arbitrarily higher-order triangular face.
	12	arbitrarily higher-order line segment.
	13	edge of a full isoparametric triangular element where the nodes are numbered linearly along it.
	14	n^2 -node arbitrarily higher-order (spectral) quad face where n is the number of nodes on an edge of this face. To be used with spectral element type 105.
CONNEC	TIVITY_NODES	These should be listed in a stacked fashion on a single line, and numbered clockwise (when looking from infinity in three dimensions).

Next: <u>ATDDNB</u>, Previous: <u>ATDARB</u>

4 ACOUSTIC TIME-DOMAIN DIRICHLET BOUNDARY CONDITIONS *S*

Command Statement:	ATDDIR
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The ATDDIR command statement is used to specify *nodal* Dirichlet boundary conditions for a time-domain acoustic scattering problem. The input format is given below.

ATDDIR

NODE# VALUE

NODE# Node number where the Dirichlet boundary condition is specified (integer).

VALUE Value of the specified boundary condition (real).

Next: ATDNEU, Previous: ATDDIR

5 ACOUSTIC TIME-DOMAIN DISTRIBUTED NEUMANN BOUNDARY CONDITION *S*

Command Statement: ATDDNB

The ATDDNB command statement is used to specify the surface of a scatterer on which a *distributed* Neumann boundary condition of the form $\left| \frac{\partial p^s}{\partial n} = c \right|$ is applied in a time-domain acoustic computation, and the value of the constant c.

The input format of this command is given below.

ATDDNB CONSTANT

CONSTANT Value of the constant c (real).

FACE Face (or edge in two dimensions) identification number whose type and connectivity are to be specified (integer). In practice, this identification number is ignored by AERO-S.

FACE_TYPE

1 2-node line segment. To be used with two-dimensional linear elements.

2	3-node line segment. To be used with two-dimensional quadratic elements.
3	3-node triangular face. To be used with three-dimensional linear tetrahedral element.
4	4-node quad face. To be used with three-dimensional linear hexahedral element.
6	6-node triangular face. To be used with three-dimensional quadratic tetrahedral element.
10	n^2 -node arbitrarily higher-order quad face where n is the number of nodes on an edge of this face. To be used with element type 95.^M
11	arbitrarily higher-order triangular face.
12	arbitrarily higher-order line segment.
13	edge of a full isoparametric triangular element where the nodes are numbered linearly along it.
14	n^2 -node arbitrarily higher-order (spectral) quad face where n is the number of nodes on an edge of this face. To be used with spectral element type 105.
CONNECTIVITY_NODES	These should be listed in a stacked fashion on a single line, and numbered clockwise (when looking from infinity in three dimensions).

Next: ATDROB, Previous: ATDDNB

6 ACOUSTIC TIME-DOMAIN NEUMANN BOUNDARY CONDITIONS *S*

Command Statement:	ATDNEU
Command Statement:	AIDNEU

The ATDNEU command statement is used to specify the *nodal* Neumann boundary conditions for a time-domain acoustic scattering problem. The input format is given below.

ATDNEU



NODE# Node number where the Neumann boundary condition is specified (integer).

VALUE Value of the specified boundary condition (real).

Next: ACTUATORS, Previous: ATDNEU

7 ACOUSTIC TIME-DOMAIN ROBIN DISTRIBUTED BOUNDARY

CONDITION *S*

Command Statement:	ATDROB
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The ATDROB command statement can be used to specify the surface of a scatterer on which a *distributed* Robin boundary condition of the form $\alpha \frac{\partial p^s}{\partial n} + \beta p^s = \gamma$ is applied in a time-domain acoustic computation, and the three constants α , β , and γ .

The input format of this command is given below.



FACE	FACE_TYPE	CONNECTIVITY_NODES
11102		
α		Non-zero constant premultiplying the normal derivative of the Robin boundary condition (real).
β		Constant premultiplying the unknown of the Robin boundary condition (float).
γ		Constant right hand-side of the Robin boundary condition (float).
FACE		Face (or edge in two dimensions) identification number whose type and connectivity are to be specified (integer). In practice, this identification number is ignored by AERO-S .
FACE_	_TYPE	
1		2-node line segment. To be used with two-dimensional linear elements.
2		3-node line segment. To be used with two-dimensional quadratic elements.
3		3-node triangular face. To be used with three-dimensional linear tetrahedral element.
4		4-node quad face. To be used with three-dimensional linear hexahedral element.
6		6-node triangular face. To be used with three-dimensional quadratic tetrahedral element.
10		n^2 -node arbitrarily higher-order quad face where n is the number of nodes on an edge of this face. To be used with element type 95.^M
11		arbitrarily higher-order triangular face.
12		arbitrarily higher-order line segment.
13		edge of a full isoparametric triangular element where the nodes are numbered linearly along it.

 n^2 -node arbitrarily higher-order (spectral) quad face where n is the number of nodes on an edge of this 14 face. To be used with spectral element type 105.

CONNECTIVITY NODES

These should be listed in a stacked fashion on a single line, and numbered clockwise (when looking from infinity in three dimensions).

Next: AERO, Previous: ATDROB

8 ACTUATORS *S*

ACTUATORS Command Statement:

The ACTUATORS command statement is used to specify time variant prescribed forces at any node and its associated dofs using a user defined subroutine, and the structural state at that node (displacement, velocity, and acceleration). The user has to write his/her own algorithm for specifying the prescribed forces within a subroutine named "control.C". The SENSORS command statement must be used to pass to "control.C" the structural state at nodes of interest (see SENSORS). The user should also grab the special makefile for this command which is located within the FEM.d/Control.d directory, and use LOAD to activate this command (see LOAD). An example input file using this command can be found in FEM.d/fem_examples/Control.d/

Note 1: For nonlinear analyses, sensor information on the velocity and acceleration of a nodal degree of freedom is not currently available for use in the "control.C" file. These are currently passed as zero.

The syntax for invoking this option is given below.

ACTUATORS

NODE# DOF#

> NODE# Node number where an actuating force is specified (integer).

Degree of freedom local number where an actuating force is specified (integer). DOF#

Next: AEROH, Previous: ACTUATORS

9 AEROELASTICITY



The AERO command statement can be used to perform any or all of the following instructions:

- indicate that AERO-S is to interact with AERO-F to compute a flow-induced load and perform the corresponding static structural

analysis, or perform a coupled static or dynamic aeroelastic simulation

- choose a staggered time-integration algorithm (ALGORITHM subcommand) and, if needed, a displacement predictor (ALGORITHM subcommand) defined by two coefficients α_0 and α_1 and the formula

$$u^{n+1^{p}} = u^{n} + \alpha_0 \Delta t_S \dot{u}^{n} + \alpha_1 \Delta t_S (\dot{u}^{n} - \dot{u}^{n-1})$$

when the structural time-integrator is implicit, or

$$u^{n+\frac{1}{2}^{p}} = u^{n} + \alpha_{0} \Delta t_{S} \dot{u}^{n-\frac{1}{2}} + \alpha_{1} \Delta t_{S} (\dot{u}^{n-\frac{1}{2}} - \dot{u}^{n-\frac{3}{2}})$$

when the structural time-integrator is explicit, to perform a static or dynamic aeroelastic simulation.

Here, u^{n+1^p} ($u^{n+\frac{1}{2}^p}$) is the predicted displacement field at time-step t^{n+1} ($t^{n+\frac{1}{2}}$).

- select an algorithm for computing the corrected pressure field P_c^{n+1} to be used in computing the aerodynamic forces acting on the structure at time-step t^{n+1} (PRESSURE subcommand) of a static or dynamic aeroelastic simulation

The purpose of the displacement predictor and that of the force corrector are to compensate for the effects of time-lagging. More specifically, they improve the time-accuracy and numerical stability of the chosen staggered procedure by minimizing the lack of energy conservation at the fluid-structure interface. The theoretical, algorithmic, and practical aspects of both of these "compensators" are described in S. Piperno and C. Farhat, "Partitioned Procedures for the Transient Solution of Coupled Aeroelastic Problems - Part II: Energy Transfer Analysis and Three-Dimensional Applications," Computer Methods in Applied Mechanics and Engineering, Vol. 190, pp. 3147-3170 (2001).

Before setting the values of α_0 and α_1 that complete the definition of the displacement predictor and choosing the force corrector, the user should note that:

- Currently, the fluid code **AERO-F** can send to **AERO-S** at t^{n+1} either the pressure field at t^{n+1} , t^{n+1} , or the averaged pressure field $t^{n+1} = \frac{1}{\Delta t_s} \int_{t^n}^{t^{n+1}} P(t) dt$. Hence, $t^{n+1} = \frac{1}{\Delta t_s} \int_{t^n}^{t^{n+1}} P(t) dt$. Hence, $t^{n+1} = \frac{1}{\Delta t_s} \int_{t^n}^{t^{n+1}} P(t) dt$. Hence, $t^{n+1} = \frac{1}{\Delta t_s} \int_{t^n}^{t^{n+1}} P(t) dt$. Therefore, the purpose of the PRESSURE subcommand is to propose additional choices for the corrected pressure field $t^{n+1} = \frac{1}{\Delta t_s} \int_{t^n}^{t^{n+1}} P(t) dt$. These can affect the order of time-accuracy of the chosen staggered solution procedure (see C. Farhat, G. van der Zee and P. Geuzaine, "Provably Second-Order Time-Accurate Loosely-Coupled Solution Algorithms for Transient Nonlinear Computational Aeroelasticity," Computer Methods in Applied Mechanics and Engineering, Vol. 195, pp. 1973-2001 (2006)).
- The values of α_0 and α_1 that minimize the variation of energy exchanged at the fluid-structure interface and therefore improve time-accuracy and numerical stability of the chosen staggered solution procedure depend on the chosen fluid and structure time-integrators, the chosen force corrector, and the selected staggered solution procedure itself.
- When the Generalized method is used for time-integrating linear or nonlinear structural dynamics problems (see <u>DYNAMICS</u>), **AERO-S** solves the following equilibrium problem

$$M\ddot{u}^{n+1-\alpha_m} + C\dot{u}^{n+1-\alpha_f} + F_{internal}(u^{n+1-\alpha_f}) = F_{external}^{n+1-\alpha_f}$$

• When the central difference method is used for time-integrating structural dynamics problems (see <u>DYNAMICS</u>), **AERO-S**

solves the following equilibrium problem

$$\label{eq:munu} M\ddot{u}^{n+1} + C\dot{u}^{n+1} + F_{internal}(u^{n+1}) = F_{external}^{n+1}$$

Note 1: AERO-F offers two different computational frameworks for fluid-structure interaction: an Arbitrary Lagrangian/Eulerian (ALE) framework, and an embedded one. When a functional capability described below is meaningful, applicable, or supported for only one of these two computational frameworks, its designating keyword is followed by [ALE] in the case of the ALE computational framework, and by [EMB] in the case of the embedded computational framework. When this capability is applicable, meaningful, and supported for both computational frameworks, its designating keyword is not followed by any symbol.

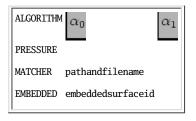
Note 2: The PP, MPP, and CO algorithms require the additional presence of the <u>DYNAMICS</u> command in the input file. The AO, A4, A5, and A6 algorithms require the additional presence in the input file of the <u>QSTATICS</u> command for a static aeroelastic analysis, and that of the <u>DYNAMICS</u> command for a dynamic aeroelastic analysis. The BO algorithm requires the additional presence in the input file of the <u>QSTATICS</u> command for a static aeroelastic analysis.

Note 3: The values of α_0 and α_1 derived in the paper by Piperno and Farhat mentioned above correspond to the case where the structure is linear and time-integrated by the Newmark algorithm with $\beta = \frac{1}{4}$ and $\gamma = \frac{1}{2}$ (see <u>DYNAMICS</u>). For different configurations of the structural time-integrator, that analysis needs to be redone if it is to be used for choosing the parameters of the AERO command.

Note 3: If an aeroelastic analysis is requested with the GEPS and IDISP6 commands present in the input file, and the IDISP1ACEMENTS command is specified to initialize the displacement field, then **AERO-S** sends to the fluid code at each time step the sum of the updated displacement field (interpreted in that case as an increment displacement) and the displacement specified under the IDISP6 command. If in such a case the IDISP1ACEMENTS command is not present in the input file, the displacement field of the structure is then initialized by the IDISP6 command, and **AERO-S** acts as follows: at the first time-step, it sends to the fluid code only the updated displacement field (since initialization accounts in that case for the content of the IDISP6 command), but at each subsequent time-step, it sends the sum of the updated displacement field and the displacement specified under the IDISP6 command.

The syntax for invoking this option is given below.

AERO



ALGORITHM

[ALE]

PP

MPP

This "Ping-Pong" algorithm sends the initial displacement of the structure, specified under either the IDISP6 command or the IDISPLACEMENTS with the MODAL option command (see <u>IDISPLACEMENTS</u>), to the fluid code which receives it and deforms the fluid mesh accordingly. For this purpose, the DYNAMICS command must also be present in the **AERO-S** input file. After the send occurs, the structure code exits gracefully, the fluid code computes the fluid mesh deformation associated with the specified structural displacement and outputs (if requested) the corresponding position and/or displacement of the fluid mesh (characters).

[ALE]

This "Multi-Ping-Pong" algorithm sends several initial displacements of the structure at a time — for example, modal displacements — to the fluid code, which uses them to compute compatible fluid mesh deformations. If a coefficient α_0 is specified after MPP, the initial displacements are amplified by α_0 before they are sent to the fluid code. However, the fluid code scales back by $1/\alpha_0$ the corresponding fluid mesh displacements it

computes before saving them in an output file, in order to preserve the effect of the mass normalization of the modal structural displacements. The deformed fluid mesh configurations can then be used to generate sources of excitations for the construction of fluid POD bases and ROMs, or for linearized flow simulations whose initial conditions involve a (modal) position or velocity of the fluid surface mesh. The READMODE command must be used to input the initial displacements of interest — for example, a set of eigenmodes. As for the "Ping-Pong" case, the DYNAMICS command must also be present in the **AERO-S** input file, **AERO-S** exits gracefully after the send occurs, the fluid code computes the fluid mesh deformation associated with the specified structural displacements and outputs (if requested) the corresponding positions and/or displacements of the fluid mesh. In addition, for each inputted deformed fluid mesh position, the fluid code outputs an identification tag — for example, the frequency of the corresponding input structural mode in the case of modal displacements (characters).

- This is the basic sequential staggered solution procedure (characters). When subcycling is effected, the same algorithm has been referred to in the AIAA Paper 96-1388 by Farhat and co-workers as the A1 algorithm (characters). It should not be used without a displacement predictor as it would reduce the overall order of time-accuracy compared to that intrinsic to the structural time-integrator. For the same reason, it is also not recommended and as a matter of fact not available when the structural time-integrator is explicit.
- This is the basic staggered solution procedure with fluid-structure inter-parallelism (characters). It has been referred to as A2 in the AIAA Paper 96-1388 by Farhat and co-workers. When this algorithm is specified, **AERO-F** sends to **AERO-S** $\boxed{P^n}$ or $\boxed{\overline{P}^n}$ rather than the corresponding pressure fields at $\boxed{t^{n+1}}$. Currently, this algorithm is not available when an explicit structural time-integrator is chosen.
- A5 This staggered solution procedure also features inter-parallelism but offers a better accuracy than A4 when both methods are used without a displacement predictor (characters). Currently, this algorithm is not available when an explicit structural time-integrator is chosen.
- This staggered solution procedure has superior stability and accuracy properties. It is recommended when the fluid is time-advanced by an implicit time-integrator. It has been referred to in most papers by Farhat and co-workers as the ISS (Improved Sequential Staggered algorithm) method (characters). Originally, it was thought that this staggered solution procedure does not require any displacement prediction ($\alpha_0 = \alpha_1 = 0$)

because it incorporates its own first-order prediction. However, it was later discovered that this partitioned procedure is genuinely second-order time-accurate if it is equipped with a second-order displacement prediction. In that case, α_0 can be set to $\alpha_0=0$ in **AERO-S** because it is reset in any case by **AERO-F** to the appropriate value ($\alpha_0=1/2$) and α_1 should be set to $\alpha_1=1/8$. Also, this partitioned solution

procedure prefers a non time-averaged pressure field. Most importantly, it was designed to be used exclusively with the midpoint implementation of the Newmark time-integrator with $\beta = \frac{1}{4}$ and $\gamma = \frac{1}{2}$ (see

<u>DYNAMICS</u>), and with the NONCOLLOCATED scheme of aerodynamic force evaluation (see below) (characters). Currently, this algorithm is not available when an explicit structural time-integrator is chosen.

This algorithm sends the initial displacment of the structure (if any) to the flow solver. After receiving it and updating its mesh, **AERO-F** performs any requested flow simulation and sends to **AERO-S** upon completion the flow-induced load. Then, **AERO-S** receives this load, performs a static analysis if requested, and finally outputs any requested data (such as the computed flow-induced load, and/or resulting displacement, stress, and strain fields).

This staggered solution procedure is designed for the case where the structural subsystem is time-integrated by the explicit central difference method (see <u>DYNAMICS</u>), and the fluid subsystem is time-integrated by either an explicit or an implicit scheme. As shown in C. Farhat, A. Rallu, K. Wang and T. Belytschko, "Robust and Provably Second-Order Explicit-Explicit and Implicit-Explicit Staggered Time-Integrators for Highly Nonlinear Fluid-Structure Interaction Problems," International Journal for Numerical Methods in Engineering, (2010), it is genuinely second-order time-accurate when equipped with the second-order displacement predictor obtained by setting $\alpha_0 = 0.5$ and $\alpha_1 = 0.375$ (see below), the second-order force corrector obtained by setting

PRESSURE to COLLOCATED (see below), and a second (or higher)-order accurate explicit or implicit ALE fluid time-integrator.

When used with the MPP command, this coefficient can be specified to amplify the initial displacements to be sent to the fluid code (however, as noted above, the fluid mesh displacements computed by the fluid code are scaled back by $1/\alpha_0$ before they are saved in an output file). In this case, the default value is 1. Otherwise, this

coefficient is part of the construction of a predictor for the position of the structure, in which case the default value is 0. For $\alpha_0=0$ (and $\alpha_1=0$), no prediction is effected. Setting $\alpha_0=1$ (and $\alpha_1=0$) generates a first-order prediction, and setting $\alpha_0=1$ (and $\alpha_1=\frac{1}{2}$) generates a second-order prediction. However, the optimal values of α_0 and α_1 depend on the specifics of the fluid and structure field time-integrators (see

the aforementioned paper on interface energy conservation by Piperno and Farhat) (real).

 α_1 See above (real).

PRESSURE

COLLOCATED

In this case,
$$P_c^{n+1}$$
 is set to the received pressure field P_c^{n+1} (or P_c^{n+1}), then converted to
$$P_c^{n+1-\alpha_f} = (1-\alpha_f)P_c^{n+1} + \alpha_f P_c^n$$
 before the aerodynamic forces are constructed and fed into the

structural equations of dynamic equilibrium (see the introduction to this command). Therefore, this option covers the cases $P_c^{n+1}=P^{n+1}$ and $P_c^{n+1}=\overline{P}^{n+1}$, which explains the origin of the word "collocated"

(characters).

NONCOLLOCATED

In this case, which is also the default value of PRESSURE, the pressure field
$$P_c^{n+1}$$
 is set to
$$P_c^{n+1} = \frac{1}{\gamma} P^{n+1} - \frac{(1-\gamma)}{\gamma} P_c^n \text{ (or } P_c^{n+1} = \frac{1}{\gamma} \overline{P}^{n+1} - \frac{(1-\gamma)}{\gamma} P_c^n \text{), then converted to }$$

$$P_c^{n+1-\alpha_f}=(1-\alpha_f)P_c^{n+1}+\alpha_fP_c^n \text{ (here, } \gamma \text{ is the } \gamma \text{ parameter of the Generalized } \alpha \text{ method)}. \text{ This specific choice for } P_c^{n+1} \text{ is consistent with the quadrature rule of the Generalized } \alpha \text{ method for evaluating the work done by an external force in the time-interval } [t^n, t^{n+1}] \text{ . In other words, the pressure forces fed into the structural equations of equilibrium are based on } P_c^{n+1-\alpha_f}=\frac{(1-\alpha_f)}{\gamma}P^{n+1}+(1-\frac{(1-\alpha_f)}{\gamma})P_c^n \text{ . For } \gamma = \alpha_f = \frac{1}{2}, P_c^{n+1-\alpha_f}=P^{n+1}, \text{ which explains the origin of the word "non collocated" (characters).}$$

MATCHER

Name (including path, if needed) of the structural matcher file (characters).

pathandfilename

EMBEDDED

embeddedsurfaceid

Integer identification number of a surface defined in <u>SURFACETOPO</u> using 3-noded triangles (integer). This surface, which is to be embedded in an **AERO-F** CFD mesh for fluid-structure computations, is assumed to be identical to the wet surface of the finite element structural model and defined using the same nodes of this model. For this reason, when the EMBEDDED option is used, it is not currently necessary to provide a structure matcher file.

Next: ATTRIBUTES, Previous: AERO

10 AEROHEAT (FEM-HEAT Only)

Command Statement: AEROH

The AEROH command statement is used to indicate that FEM-HEAT is to interact with a fluid code to perform an aerothermal (thermostructure-thermofluid) coupled simulation, to select a staggered time-integration algorithm (ALGORITHM subcommand), and specify the α_0 and α_1 parameters of the following prediction of the temperature of the structure at time-step t^{n+1} (ALGORITHM subcommand)

$$T_S^{n+1^p} = T_S^n + \alpha_0 \Delta t_S^H \dot{T_S}^n + \alpha_1 \Delta t_S^H (\dot{T_S}^n - \dot{T_S}^{n-1})$$

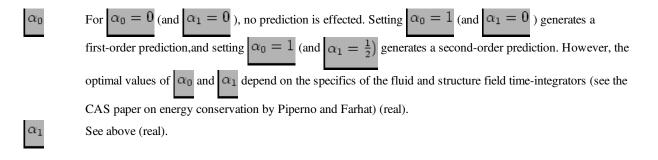
The syntax for invoking this option is given below.

AEROH



ALGORITHM

A0 This is the basic sequential partitioned procedure. It is similar to the A0 algorithm of the AERO command.



Next: <u>BOFFSET</u>, Previous: <u>AEROH</u>

11 ATTRIBUTES *S*

Command Statement: ATTRIBUTES

The ATTRIBUTES command statement is used to label an element with an attribute identification number for linear material properties and/or geometric properties, a composite or orthotropic shell, or anisotropic solid element attribute identification number, and a frame or fiber-angle attribute identification number if this element has been defined as a composite or orthotropic shell or anisotropic solid element.

The attribute identification number for linear material properties and/or geometric properties is used in the MATERIAL command when specifying the linear material properties and/or geometric properties of a group of elements (see <u>MATERIAL</u>).

An "empty" (or "phantom") element — that is, an element with zero generalized stiffness and mass matrices — can be useful in aeroelastic, aerothermal, and aerothermoelastic computations to facilitate the exchange of elastodynamic, thermal, and aerodynamic data between the fluid, thermal, and structural analyzers. Such an element can be specified by assigning it a negative attribute number. In this case, it is not necessary to specify a material for such an element as its properties will be ignored.

An element that is not an "empty" (or "phantom") element and which does not require any material property should be assigned zero (0) as an attribute number and no material property. Examples of such an element are rigid elements and frequency-domain acoustic elements in a homogeneous medium.

On the other hand, an element that is not assigned an attribute number is simply ignored by AERO-S.

AERO-S supports three input formats for this command that are described below. In the case of the first format, there should be as many lines as the number of finite elements. All formats can be mixed.

Note 1: When "empty" (or "phantom" elements) are used in the mesh, the FETI solvers are not guaranteed to work because of issues related to subdomain singularities. Hence, it is not recommended to use FETI solvers in such cases.

Note 2: Phantom elements should be used with care to avoid the generation of degrees of freedom without stiffness (and mass) and therefore avoid introducing artificial singularities in the solution of the problem of interest. In particular, it is strongly recommended to ensure that every node of every phantom element is also a node of a non-phantom element, and that every local degree of freedom of

a phantom element is also a local degree of freedom of a non-phantom element. In other words, it is strongly recommended, for example, to avoid connecting a phantom shell element to a face of a solid (brick element) and use instead a phantom three-dimensional plane stress/plane strain element in this case.

Note 3: Only phantom shell elements and phantom plane stress/plane strain elements can transfer aeroelastic loads to a structure.

ATTRIBUTES						
ELEMENT#	MAT_ATT#	CMP_ATT#	CMP_FRM#			
or						
ELEMENT#	MAT_ATT#	CMP_ATT#	ТНЕТА	$ heta_{Ref}$		
or						
STARTING_ELE	EMENT# END	ING_ELEMENT#	MAT_ATT#	CMP_ATT#	CMP_FRM#	
or						
STARTING_ELE	EMENT# END	ING_ELEMENT#	MAT_ATT#	CMP_ATT#	THETA	θ_{Ref}
or						

STARTING_ELEMENT#	ENDING_ELEMENT# IDENTITY
ELEMENT#	Element number whose attribute numbers are to be specified (integer).
MAT_ATT#	Group identification number (integer); AERO-S supports gaps in numbering. Identifies material properties.
CMP_ATT#	Composite (orthotropic or anisotropic) identification number (integer); AERO-S supports gaps in numbering. Identifies composite (orthotropic or anisotropic) properties. May be left blank if element i not a composite (orthotropic or anisotropic) element.
CMP_FRM#	Composite frame identification number (integer); AERO-S supports gaps in numbering. May be left blank if element is not a composite (orthotropic or anisotropic) element.

 θ_{Ref}

MAT ATT#

THETA	Keyword that must be spelled as THETA (characters). This keyword announces that the next item on the
	same input line is the value $ heta_{Ref}$ of THETA. This unusual $AERO$ - S input is necessary to avoid parsing
	conflicts

Reference angle in degrees between the axis defined by local nodes 1 and 2 of the element, and the material local x-axis defining the direction of the σ_{xx} and ε_{xx} of the constitutive law (real). When also using the LAYN and LAYC commands, the material x-axis corresponds to the direction of $E_1^{(k)}$ when $\theta_F^{(k)}$

is zero. If this angle rather than CMP_FRM# is specified, the composite frame is internally generated by **AERO-S**.

STARTING_ELEMENT#	First element of a sequence of elements that have the same MAT_ATT#, CMP_ATT#, and CMP_FRM#	
	(integer).	

ENDING_ELEMENT# Last element of a sequence of elements that have the same MAT_ATT#, CMP_ATT#, and CMP_FRM# (integer).

Group identification number. Identifies material properties (integer).

CMP_ATT# Composite identification number (integer). Identifies composite (orthotropic or anisotropic) properties.

May be left blank if element is not a composite (orthotropic or anisotropic) element.

CMP_FRM# Composite frame identification number (integer). May be left blank if element is not a composite

(orthotropic or anisotropic) element.

IDENTITY This keyword signals to the AERO-S code that each element in the range delimited by STARTING_ELEMENT#

and ENDING_ELEMENT# has a material attribute identification number equal to its element identification number. For example, the sequence 1 3 IDENTITY means that element#1 has the material attribute identification 1, element#2 has the material attribute identification 2, and element#3 has the material

attribute identification 3.

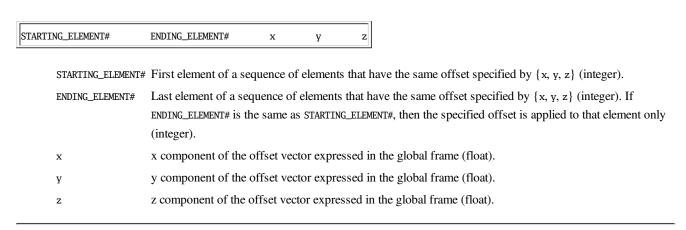
Next: **EFRAMES**, Previous: **ATTRIBUTES**

12 BEAM OFFSET

Command Statement: BOFFSET

The BOFFSET command statement is used to specify a beam's neutral axis offset from the line passing through its two end nodes. This command may be used for both Euler-Bernoulli and Timoshenko beam elements. For beam elements not listed under BOFFSET, a zero offset is used. If the BOFFSET command is absent altogether, then an offset of zero is used for all beam elements. If a listed element is neither an Euler-Bernoulli nor a Timoshenko beam element, then the offset is ignored for that element. The input format is as follows.

BOFFSET



Next: BINARY, Previous: BOFFSET

13 BEAM REFERENCE FRAMES

Command Statement:	EFRAMES
--------------------	---------

The EFRAMES command statement is used to specify a beam's orientation which defines the position of the beam with respect to the global frame. The requirements for each beam frame are that one of the axis, **the local x-axis**, concur with the longitudinal axis of the beam, and the remaining two axes complete an orthogonal triad. If this requirement is violated, FEM regenerates the local x-axis as well as the local z-axis.

FEM also supports a run-time generation of beam frames that is activated either when the target beam element is identified under this command and a third node is specified to generate the frame, or when a third node is found in the definition of a beam element within the TOPOLOGY command. The only requirement for the third node is that it does not be colinear with the other two beam nodes that define the local x-axis (X). Under *this* command, the third node defines the X,Y plane. Under the TOPOLOGY command, it defines the X,Z plane. Using the third node option under the TOPOLOGY command also relieves the user from specifying the EFRAMES command. An example illustrating the third node option in the TOPOLOGY command can be found in FEM.d/fem_examples/Third_Node.d

Otherwise, the input format for this command is given below with the number of lines equal to the number of beams in the problem.

EFRAMES



or

ELEMENT# THIRDNODE	third_node
--------------------	------------

ELEMENT# Element number where the beam frame is specified (integer).

 S_{1x} S_{1y} S_{1z} The first axis, **the local x-axis**, of the beam frame expressed in the global frame. This axis must concur with the longitudinal axis of the beam (floats).

 S_{2x} S_{2y} S_{2z} The second axis of the beam frame expressed in the global frame (floats).

 S_{3x} S_{3y} S_{3z} The third axis of the beam frame expressed in the global frame (floats).

THIRDNODE Keyword that must be spelled as THIRDNODE. This keyword announces that the next item on the same input

line is the value third_node# of THIRDNODE. This unusual FEM input is necessary to avoid parsing conflicts

(characters).

THIRD_NODE# Id number of an existing node to be considered as a third node of the beam element identified by ELEMENT#,

and which must be in the local x-y plane of the beam. The only requirement for the third node is that it does not be colinear with the other two beam nodes. The normalized vector node1 to node2 defines the local x axis. The normalized vector node1 to node3 defines the local y-axis. The local z-axis is

automatically generated as cross product of the other two normalized axes.

Note 1: The above axes have to be normalized. By default, the beam frames concur with the global frame.

Next: CONVECTION, Previous: EFRAMES

14 BINARY INPUT / OUTPUT

Command Statement:	BINARY	[<nothandfilenrefix>]</nothandfilenrefix>
Command Statement:	BINAKY	[<pathandfileprefix>]</pathandfileprefix>

The command BINARY is used to instruct **AERO-S** to read a set of binary files containing distributed intput data associated with a mesh decomposition, and/or generate the specified output results (see <u>OUTPUT</u>) in similar binary distributed output files. Currently, the binary distributed input data files can be created by **SOWER** only. The purpose of **SOWER** is to generate distributed input files for distributed parallel codes, and merge their distributed output files into a single output file. Collectively, the binary distributed input files contain: (a) a distributed geometry and corresponding data associated with the commands of this User's Reference Manual marked by "*S*", (b) a distributed decomposition containing the mesh partition (optional, see **SOWER**'s User's Reference Manual), (c) a global file containing the cluster-to-subdomain and subdomain-to-subdomain connectivities (optional, see **SOWER**'s User's Reference Manual) and (d) a distributed matcher file (optional, see the **MATCHER** and **SOWER** User's Reference Manuals) for aeroelastic computations.

Note 1: This command is active only when **AERO-S** is compiled in the distributed memory execution mode where parallelism is implemented using a combination of the MPI and OpenMP parallel constructs (see <u>SERIAL AND PARALLEL EXECUTIONS</u>).

Note 2: As stated above, any command documented in this User's Reference Manual and marked by "*S*" can be assumed to be contained in the binary file described in (a) above. However, if that command contains both an algorithmic parameter as well as data, only the data will be contained in the appropriate binary file. Therefore, the command itself should be kept in the final **AERO-S** input file together with the algorithmic parameter, but without the data. For example, the command **IDISPLACEMENTS** specifies both an

amplification factor and an initial displacement data. The initial displacement data will be included in the binary file mentioned above, but the amplification factor will not. This is to allow the user to change simple things like the amplification factor without having to re-run **SOWER** to produce the binary distributed input files.

The syntax for this command is as follows.

BINARY <pathandfileprefix>

BINARYINPUT flagBIN BINARYOUTPUT flagBOUT

pathandfileprefix If instructed to read a set of binary input files containing distributed input data associated with a mesh

decomposition, AERO-S expects the names of these files to be by default (decomposition, connectivity,

subdomains, and clusterData. Alternatively, pathandfileprefix can be used to specify a prefix for these four files (and a path, if needed), in which case the expected name of the binary decomposition file is fileprefix.dec, that of the binary connectivity file is fileprefix.con, that of the binary subdomains file is fileprefix.sub, and that of

the binary clusterData file is prefixfile.msh (characthers).

BINARYINPUT Sub-command keyword to specify whether **AERO-S** is to read a specified set of binary input files containing

distributed input data.

flagIN On/Off flag (characters).

on In this case, which is also the default case, AERO-S is to read a specified set of binary input files containing

distributed input data.

off In this case, **AERO-S** is not to read a specified set of binary input files containing distributed input data.

BINARYOUTPUT Sub-command keyword to specify whether **AERO-S** is to generate the specified output results (see OUTPUT)

in binary distributed output files. output files

flagOUT On/Off flag (characters).

on In this case, which is also the default case, **AERO-S** is to generate the specified output results (see OUTPUT)

in binary distributed output files.

off In this case, **AERO-S** is not to generate the specified output results (see OUTPUT) in binary distributed output

files.

Next: DISPLACEMENTS, Previous: BINARY

15 BOUNDARY CONVECTION *S*

Command Statement: CONVECTION

The CONVECTION command statement is used to specify nodal convection type boundary conditions. Each node can have only one degree of freedom. This command statement can be used to solve a prescribed boundary convection problem for both statics and dynamics. The input format is given below.

Note 1: This command contributes to the construction of the right-hand side vector only. For the left-hand side (stiffness matrix) contribution, FEM uses boundary convection elements and information specified in the MATERIAL command.

CONVECTION

NODE#	H-COEFF	AREA TA				
NODE#		Node number where	the convection is specified (integer).			
	H_COEFF	Convection coefficient at the prescribed node (float).				
AREA Value of the node cross-sectional area (float).						
	TA	Ambient temperature	around the node (float).			

Next: FLUX, Previous: CONVECTION

16 BOUNDARY DISPLACEMENTS *S*

Command Statement:	DISPLACEMENTS
Command Statement.	DISTLACEMENTS

The DISPLACEMENTS command is used to prescribe nodal displacements and/or rotations, either directly, or via the definition of a surface using the command <u>SURFACETOPO</u>. In the latter case, the specified displacement or rotation is applied to each specified local degree of freedom of each node of that surface. The user can specify up to three displacements per node if a node can have up to three degrees of freedom, and up to three displacements and three rotations if it can have up to six degrees of freedom.

Note 1: This command can also be used to specify nodal Dirichlet boundary conditions for a time-domain of frequency-domain acoustic simulation by setting DOF# to 8 (see below). Hence, it can also serve as an alternative to the <u>ATDDIR</u> and <u>HDIR</u> commands.

The following two formats are available for this command and can be mixed.

DISPLACEMENTS





NODE# Node number where the displacement or rotation is specified (integer).

DOF# Degree of freedom local number where the displacement or rotation is specified (integer).

VALUE Value of the specified displacement or rotation (real).

SURFACE Keyword indicating that a surface defined in <u>SURFACETOPO</u> is to be identified next by its

integer identification number (characters).

SURFACE# Integer identification of the surface defined in <u>SURFACETOPO</u> where the force VALUE is specified

(integer).

Next: FORCES, Previous: DISPLACEMENTS

17 BOUNDARY FLUXES



The FLUX command statement is used to specify nodal temperature flux type boundary conditions. Each node can have only one degree of freedom. This command statement can be used to solve a prescribed temperature flux boundary problem for both statics and dynamics. The input format is given below.



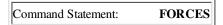


NODE# Node number where the flux is specified (integer).

VALUE Value of the prescribed boundary flux (float).

Next: TEMPERATURES, Previous: FLUX

18 BOUNDARY FORCES *S*

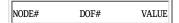


The FORCES command is used to prescribe external forces and/or moments at the node level, either directly, or via the definition of a surface using the command <u>SURFACETOPO</u>. In the latter case, the specified force or moment is applied to each specified local degree of freedom of each node of that surface. The user can specify up to three forces per node if a node can have up to three degrees of freedom, and up to three forces and three moments if it can have up to six degrees of freedom. The user can also time-vary the specified forces and/or moments using the <u>MFTT</u> command.

Note 1: This command can also be used to specify nodal Neumann boundary conditions for a time-domain acoustic simulation by setting DOF# to 8 (see below). Hence, it can also serve as an alternative to the command ATDNEU (see <u>ATDNEU</u>).

The following two formats are available for this command and can be mixed.

FORCES



SURFACE	SURFACE#	DOF#	VALUE				
NODE#		Node n	umber where the force or moment is specified (integer).				
DOF#		Degree	of freedom local number where the force or rotation is specified (integer).				
VALUE			Value of the specified force or moment (real).				
SURFACE		•	Keyword indicating that a surface defined in <u>SURFACETOPO</u> is to be identified next by its integer identification number (characters).				
SURFAC	Œ#	Č	identification of the surface defined in <u>SURFACETOPO</u> where the force or moment VALUE fied (integer).				

Next: **BUCKLE**, Previous: **FORCES**

19 BOUNDARY TEMPERATURES *S*

Command Statement: TEMPER

The TEMPERATURES command statement is used to specify prescribed nodal temperature type boundary conditions. Each node can have only one degree of freedom. For structural analysis using FEM, this command statement causes FEM to construct a thermal load based on the prescribed nodal temperatures and the reference temperatures (see <u>MATLAW</u>, see <u>TOPOLOGY</u>, and use this load as usual in a structural analysis).

The input format is given below.

TEMPERATURES

NODE# VALUE

NODE# Node number where the temperature is specified (integer).

VALUE Value of the prescribed temperature (float).

Next: COMMENTS, Previous: TEMPERATURES

20 BUCKLING

Command Statement: BUCKLE

The BUCKLE command statement is to be used together with the GEPS, IDISP6 (with $\alpha = -1$), and EIGEN command statements to request the buckling analysis of a given structure.

When the EIGEN, GEPS, and IDISP6 (with $\alpha=-1$) command statements are specified in the input file, the BUCKLE command statement signals to FEM that the eigenvalue problem to be solved is $KU=-\lambda K_GU$, where K_G is the geometric stiffness matrix associated with a displacement field specified under the IDISP6 command statement, and whose computation is triggered by the presence of the GEPS command statement in the input file. For each eigenvalue Δ , the corresponding buckling load is λf where f is the load that created the displacement field specified under the IDISP6 command.

An example input file that illustrates a buckling analysis can be found in FEM.d/fem_examples/Buckle.d The syntax for invoking this option is given below.

BUCKLE

Next: COMPOSITE, Previous: BUCKLE

21 COMMENTS

The '*' at the beginning of a line indicates that the subsequent input on the same line is a comment. For FEM, it can be placed anywhere.

Next: CFRAMES, Previous: COMMENTS

22 COMPOSITE (OR ORTHOTROPIC SHELL OR ANISOTROPIC SOLID)

Command Statement: COMPOSITE

The COMPOSITE command statement is used to specify the material properties of a composite shell, or orthotropic shell, or anisotropic solid element. All information concerning the constitutive coefficients or the composite layering is input using this command.

Currently, this command supports only the solid elements type 17, type 23, type 24, type 25, type 72, type 91 and type 92, and the (Kirchhoff) shell elements type 20 and type 2020.

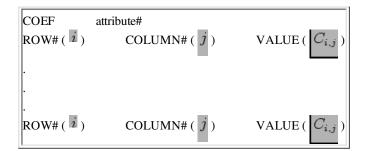
For the above solid elements, the constitutive matrix is inputted using the COEF sub-command. This matrix is defined in the local frame of the element specified in the CFRAMES command. Entries in the matrix that are not specified are determined from symmetry if appropriate, or are set to zero.

For the shell elements, the composite (or orthotropic) constitutive law can be prescribed either by entering the $C_{i,j}$ coefficients of

the 6 by 6 constitutive matrix, or by defining the material properties and geometrical characteristics for each layer of the composite. In the first case, the sub-command COEF should be used. In the second case, either of the sub-commands LAYC or LAYN can be used, depending on whether the coupling between bending and membrane effects is to be enforced or not, respectively. When many layers of the composite are made of the same orthotropic material, the sub-commands LAYD and LAYMAT can be used instead of the sub-command LAYC to simplify the data entry process. Likewise, in similar circumstances, the sub-command LAYO and LAYMAT can be used instead of the sub-command LAYO.

The input format for the COMPOSITE command is given below.

COMPOSITE



Sub-command keyword used to directly input the coefficients of the constitutive law. Since the 6 by 6 constitutive matrix is symmetric, up to 21 independent coefficients may follow.

attribute# Integer value that corresponds to the composite (or orthotropic or anisotropic) attribute of the element.

ROW# Integer value that corresponds to the row i of the coefficient $C_{i,j}$.

A row can be skipped if all its entries are zero.

COLUMN# Integer value that corresponds to the column \vec{j} of the coefficient $C_{i,j}$

VALUE Real value of the coefficient $C_{i,j}$.

For a solid element, the constitutive matrix C relates the stresses to the engineering strains in the element's local frame system $\{x; y; z\}$ as follows:

$$\begin{pmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{xz} \end{pmatrix} = \begin{pmatrix} C_{1,1} & C_{1,2} & C_{1,3} & C_{1,4} & C_{1,5} & C_{1,6} \\ C_{1,2} & C_{2,2} & C_{2,3} & C_{2,4} & C_{2,5} & C_{2,6} \\ C_{1,3} & C_{2,3} & C_{3,3} & C_{3,4} & C_{3,5} & C_{3,6} \\ C_{1,4} & C_{2,4} & C_{3,4} & C_{4,4} & C_{4,5} & C_{4,6} \\ C_{1,5} & C_{2,5} & C_{3,5} & C_{4,5} & C_{5,5} & C_{5,6} \\ C_{1,6} & C_{2,6} & C_{3,6} & C_{4,6} & C_{5,6} & C_{6,6} \end{pmatrix} \begin{pmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{yz} \end{pmatrix}$$

For a (Kirchhoff) shell element, the constitutive matrix C relates the forces and moments to the mid-surface strains and curvatures in the shell element's local frame system $\{x; y; z\}$ as follows:

$$\begin{pmatrix} N_x \\ N_y \\ N_{xy} \\ M_x \\ M_y \\ M_{xy} \end{pmatrix} = \begin{pmatrix} C_{1,1} & C_{1,2} & C_{1,3} & C_{1,4} & C_{1,5} & C_{1,6} \\ C_{1,2} & C_{2,2} & C_{2,3} & C_{2,4} & C_{2,5} & C_{2,6} \\ C_{1,3} & C_{2,3} & C_{3,3} & C_{3,4} & C_{3,5} & C_{3,6} \\ C_{1,4} & C_{2,4} & C_{3,4} & C_{4,4} & C_{4,5} & C_{4,6} \\ C_{1,5} & C_{2,5} & C_{3,5} & C_{4,5} & C_{5,5} & C_{5,6} \\ C_{1,6} & C_{2,6} & C_{3,6} & C_{4,6} & C_{5,6} & C_{6,6} \end{pmatrix} \begin{pmatrix} \epsilon_x^0 \\ \epsilon_y^0 \\ \gamma_{xy}^0 \\ \kappa_x \\ \kappa_y \\ \kappa_{xy} \end{pmatrix}$$

The element's local axes are defined with respect to the global reference frame by the three vectors defined in the CFRAMES command for the corresponding composite (orthotropic, or anisotropic) frame number that is specified in the ATTRIBUTES section. For shell elements, the constitutive matrix, C, can be decomposed into sub-matrices containing the bending and membrane properties of the shell element:

$$C = \left(\begin{array}{cc} C_{mm} & C_{mb} \\ C_{bm} & C_{bb} \end{array}\right)$$

Note the absence of the transverse shear behavior, as can be expected from a Kirchhoff type shell element. In the case of an isotropic material, or a single layer orthotropic or orthotropic material, there is no coupling between the bending and membrane behavior:

$$C_{mb} = C_{bm} = 0_{3\times3}$$

For an isotropic material, the membrane constitutive matrix can be defined in terms of the Young's modulus, E, Poisson's ratio, \mathcal{U} , and shell thickness, h as:

$$C_{mm} = \begin{pmatrix} \frac{Eh}{(1-\nu^2)} & \frac{\nu Eh}{(1-\nu^2)} & 0\\ \frac{\nu Eh^3}{12(1-\nu^2)} & \frac{Eh^3}{12(1-\nu^2)} & 0\\ 0 & 0 & \frac{Eh}{2(1+\nu)} \end{pmatrix}$$

while the bending constitutive matrix is given by:

$$C_{bb} = \begin{pmatrix} \frac{Eh^3}{12(1-\nu^2)} & \frac{\nu Eh^3}{12(1-\nu^2)} & 0\\ \frac{\nu Eh}{(1-\nu^2)} & \frac{Eh}{(1-\nu^2)} & 0\\ 0 & 0 & \frac{Eh^3}{24(1+\nu)} \end{pmatrix}$$

Using the COEF sub-command, different values of E, $\underline{\nu}$, and h can be utilized for building the membrane and bending components of the constitutive law, for example, if one wishes to adjust in some specific manner the bending and membrane behaviors of the shell element.

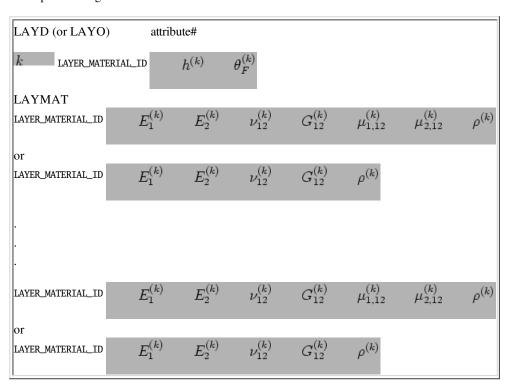
When the sub-command keyword COEF is used, the MATERIAL (see MATERIAL) command must be used as follows: the **density per unit surface** must be input at the fourth position after the attribute number, and for shells, the total thickness must also be defined at the seventh position after the attribute number if a stress analysis is requested.

For shell elements, if the pre-integrated constitutive matrix C is not available, the sub-command keywords LAYC or LAYN (or their counterparts LAYD, LAYO and LAYMAT) should be used to input the material properties and geometrical characteristics of each composite layer. If either LAYN or LAYO is used, no coupling between bending and membrane effects is enforced explicitly during integration through the thickness — that is: $C_{1,4} = C_{1,5} = C_{1,6} = 0$, $C_{2,4} = C_{2,5} = C_{2,6} = 0$ and $C_{3,4} = C_{3,5} = C_{3,6} = 0$.

The input format given below is the same for both sub-commands LAYC and LAYN.

LAYC (or LAYN)		attribute#							
k	$E_1^{(k)}$	$E_2^{(k)}$	$ u_{12}^{(k)}$	$G_{12}^{(k)}$	$\mu_{1,12}^{(k)}$	$\mu_{2,12}^{(k)}$	$ ho^{(k)}$	$h^{(k)}$	$\theta_F^{(k)}$
Г									
k	$E_1^{(k)}$	$E_2^{(k)}$	$ u_{12}^{(k)} $	$G_{12}^{(k)}$	$\mu_{1,12}^{(k)}$	$\mu_{2,12}^{(k)}$	$\rho^{(k)}$	$h^{(k)}$	$\theta_F^{(k)}$

The input format given below is the same for both sub-commands LAYD and LAYO.



LAYC, LAYD, Sub-command keyword used for inputting geometrical and material properties for each layer of a composite LAYN, or LAYO shell element (characters).

attribute# Integer value that corresponds to the composite (orthotropic) attribute of the element.

k Integer value that corresponds to the layer number.

 $E_1^{(k)}$ Young's modulus in the local direction $1^{(k)}$ (that is, in the direction of the fibers for the k^{th} layer).

 $E_2^{(k)}$ Young's modulus in the local direction $2^{(k)}$ (that is, in the direction orthogonal to the fibers for the k^{th} layer).

Poisson's ratio for transverse strain in the local direction $2^{(k)}$ when stressed in the local direction $1^{(k)}$ for $\sigma_1^{(k)}$ = constant and all other stresses zero.

 $G_{12}^{(k)}$ Transverse shear modulus in the plane $1^{(k)}-2^{(k)}$ of the layer.

 $\mu_{1,12}^{(k)}$ Coefficient of mutual influence of the first kind which characterizes stretching in the local direction $1^{(k)}$ caused by shear in the plane $1^{(k)} - 2^{(k)}$ of the layer for $\tau_{12}^{(k)}$ = constant and all other stresses zero.

 $\mu_{2,12}^{(k)}$ Coefficient of mutual influence of the first kind which characterizes stretching in the local direction $2^{(k)}$ caused by shear in the plane $1^{(k)} - 2^{(k)}$ of the layer for $\tau_{12}^{(k)}$ = constant and all other stresses zero.

 $\rho^{(k)}$ Density (mass per unit volume) of the material of the k^{th} layer.

 $h^{(k)}$ Thickness of the k^{th} layer.

Angle between a reference vector and the fibers in the layer that defines the orientation of these fibers. If CMP_FRM# is specified as an attribute for an element containing this layer under the command <u>ATTRIBUTES</u>, the reference vector is the projection onto the plane of that element of the first of the three vectors defining a local frame for this element and specified in the <u>CFRAMES</u> command. On the other hand, if a reference angle θ_{Ref}

is specified as an attribute for an element containing this layer under the command <u>ATTRIBUTES</u>, the reference vector is the vector obtained by rotating the directional edge connecting local nodes 1 and 2 of this element around its normal by an angle equal to θ_{Ref} . In both cases, this angle must be inputed in degrees.

Identifier of a set of material properties (integer).

LAYER_MATERIAL_ID

Sub-command keyword that can be used for inputting the properties of a layer when it is made of a two-dimensional orthotropic material (characters).

The report number CU-CAS-94-16, "The 3-node Composite Shell and Isotropic Timoshenko Beam Elements" by Fran\c cois M. Hemez, provides a detailed description of the theory and implementation for the type-20 composite (orthotropic) shell element.

Next: CONDITION, Previous: COMPOSITE

23 COMPOSITE (ORTHOTROPIC SHELL OR ANISOTROPIC SOLID) ELEMENT FRAMES

Command Statement: CFRAMES

The CFRAMES command statement is used to specify the orientation of the properties of composite laminates and orthotropic or anisotropic elements with respect to the global frame.

The input format is given below with the number of lines equal to the number of different composite (or orthotropic or anisotropic) element frames referenced in the ATTRIBUTES command. The syntax for the user defined composite element frame is:

CFRAMES



CMP_FRM#

Identification of a composite frame (integer).

 S_{1x} S_{1y} S_{1z}

The first axis, **the local x-axis**, expressed in the global frame. For LAYC and LAYN type composites, the orientation of the fibers of the layers should be defined with respect to the projection of this axis onto the plane of the element.

 $S_{2x} S_{2y} S_{2z}$

The second axis of the frame expressed in the global frame (floats).

 S_{3x} S_{3y} S_{3z}

The third axis of the frame expressed in the global frame (floats).

Next: CONSTRAINTS, Previous: CFRAMES

24 CONDITION NUMBER

Command Statement: CONDITION

The CONDITION command statement is used to request the evaluation of the condition number of the system being solved by FEM. The input format is given below.

CONDITION

TOLERANCE MAXITR

TOLERANCE

Error tolerance for computing the lowest and highest eigenvalues of the finite element model using the inverse power and power methods, respectively (float). The default value is 10^{-3} .

MAXITR

Maximum number of iterations for computing the lowest and highest eigenvalues of the finite element model using the inverse power and power methods, respectively (integer). The default value is 100.

Next: CONTACTSURFACES, Previous: CONDITION

25 CONSTRAINTS

Command Statement: CONSTRAINTS

The CONSTRAINTS command statement is used to specify a method for enforcing constraints defined in a problem. Its input format is given below.

Note 1: The direct elimination method for enforcing constraints is currently implemented for linear static and dynamic computations only.

Note 2: The Lagrange multiplier method for enforcing constraints is currently available for all but explicit dynamic analysis. It leads to an indefinite system of equations. Currently, only the spooles and mumps direct solvers with pivoting turned on and the gmres and FETI DP iterative solvers support this approach for solving constrained systems (see <u>STATICS</u>).

Note 3: The penalty method for enforcing constraints is currently available only for nonlinear static and dynamic analyses.

CONSTRAINTS

METHOD

METHOD Specified default method for enforcing constraints (characters). This specified method can be overruled in

the CONTACTSURFACES, MATERIAL, LMPC, NODALCONTACT, and TIEDSURFACES

commands.

multipliers In this case, which is also the default case, **AERO-S** uses the Lagrange multiplier method for enforcing

all constraints associated with the LMPC, NODALCONTACT, CONTACTSURFACES, and

<u>TIEDSURFACES</u> commands, and with joint and rigid elements (see <u>TOPOLOGY</u>).

elimination In this case, **AERO-S** uses an elimination method for enforcing all constraints associated with the <u>LMPC</u>

and <u>TIEDSURFACES</u> commands, and with joint and rigid elements (see <u>TOPOLOGY</u>).

penalty [beta] In this case, AERO-S uses the penalty method for enforcing all constraints associated with the LMPC,

NODALCONTACT, CONTACTSURFACES, and TIEDSURFACES commands, and with joint and rigid elements (see TOPOLOGY). The parameter beta should be a large positive number, typically of the

order of 10⁸ (no default value is provided).

Next: CONTROL, Previous: CONSTRAINTS

26 CONTACT SURFACES

Command Statement: CONTACTSURFACES

The CONTACTSURFACES command statement can be used to enforce contact laws between pairs of surfaces defined using the SURFACETOPO command, for static and dynamic analyses only. Surface interactions are detected using the ACME library's search module.

Note 1: Setting the master and slave contact surfaces to the same surface activates the numerical treatement of self-contact.

Note 2: For the enforcement of contact surface constraints by the Lagrange multiplier method in explicit dynamic analysis, the discrete kinematic constraint equations are defined using a node-to-segment approach, and the contact forces are computed using the enforcement module of ACME. Four enforcement models are supported in this case and this case only: frictionless (default), constant friction, velocity dependent friction, and pressure dependent friction.

Note 3: For the enforcement of contact surface constraints by either the Lagrange multiplier method in static and implicit dynamic analyses, or the penalty method in static, explicit and implicit dynamic analyses, othe discrete kinematic constraint equations are defined using a segment-to-segment (mortar) approach. In these cases, only the frictionless contact model is supported.

Note 4: The enforcement of contact surface constraints by the Lagrange multiplier method in static and implicit dynamic analyses is supported only by the FETI-DP family of solvers.

Note 5: The enforcement of contact surface constraints by the elimination method is not supported.

CONTACTSURFACES

or, for statics or dynamics with an implicit scheme

SURF_PA	[R_ID# M	ASTER S	SLAVE	CONSTRAINT_METHOD	MORTAR_TYPE	NORMAL_TOL	TANGENTIAL_TOL
---------	----------	---------	-------	-------------------	-------------	------------	----------------

or, for dynamics with an explicit scheme and the frictionless enforcement model

SURF_PAIR_ID#	MASTER	SLAVE	CONSTRAINT_METHOD	KPART_TYPE	NORMAL_TOL	TANGENTIAL_TOL	NUM_ITER
CONVERG_TOL							

or, for dynamics with an explicit scheme and the constant friction enforcement model

SURF_PAIR_ID#	MASTER	SLAVE	CONSTRAINT_METHOD	KPART_TYPE	NORMAL_TOL	TANGENTIAL_TOL	NUM_ITER
CONVERG_TOL	FRIC_COEF						

or, for dynamics with an explicit scheme and the velocity dependent friction enforcement model

SURF_PAIR_ID#	MASTER	SLAVE	CONSTRAI	NT_METHOD	KPART_TYPE	NORMAL_TOL	TANGENTIAL_TOL	NUM_ITER
CONVERG_TOL	STATIC_COEF	DYNA	MIC_COEF	VELOCITY_	DECAY			

or, for dynamics with an explicit scheme and the pressure dependent friction enforcement model

SURF_PAIR_ID#	MASTER	SLAVE	CONSTRAINT_METHOD	KPART_TYPE	NORMAL_TOL	TANGENTIAL_TOL	NUM_ITER
CONVERG_TOL	FRIC_COEF	REF_PRE	S OFFSET_PRES	PRES_EXP			

SURF_PAIR_ID# Id number of the surface pair to be described (integer).

MASTER Identification of the master (mortar method) surface (see <u>SURFACETOPO</u>) (integer).

SLAVE Identification of the slave (mortar method) surface (see <u>SURFACETOPO</u>) (integer).

CONSTRAINT_METHOD Method for enforcing the associated constraints (characters). The default method is set in

CONSTRAINTS and used whenever this entry is omitted.

multipliers The Lagrange multiplier method.

elimination The elimination method.

[beta]	penalty	The penalty method. The parameter beta should be a large positive number, typically of the order of 10^8 (no default value is provided).
	MORTAR_TYPE	Mortar type: $0 = \text{standard}$, $1 = \text{dual}$, default value is 0 (integer).
	NORMAL_TOL	Normal search tolerance used by ACME to identify interactions, default value is 0.1 (float) (see Figs. 1.2 and 1.3 in Section 1.3 of ACME's User Reference Manual).
	TANGENTIAL_TOL	Tangential search tolerance used by ACME to identify interactions, default value is 0.001 (float) (see Figs. 1.2 and 1.3 in Section 1.3 of ACME's User Reference Manual).
	KPART_TYPE	Kinematic partitioning type for explicit dynamics: $0 = $ fixed, $1 = $ automatic, default value is $0 = $ (integer).
	NUM_ITER	Maximum number of predictor-corrector iterations to be performed at each time step for explicit dynamics, default value is 5 (integer).
	CONVERG_TOL	Convergence tolerance of the predictor-correction iteration loop for explicit dynamics, default value is $1.0e-10$ (float).
	FRIC_COEF	Friction coefficient (float).
	STATIC_COEF	Static friction coefficient (float).
	DYANAMIC_COEF	Dynamic friction coefficient (float).
	VELOCITY_DECAY	Velocity decay parameter (float).
	REF_PRES	Reference pressure (float).

Next: DECOMPOSE, Previous: CONTACTSURFACES

Offset pressure (float).

Pressure exponent (float).

27 CONTROL STATEMENT

Command Statement: CONTROL

OFFSET_PRES

PRES_EXP

AERO-S always generates a performance file named FNAME.timing which reports on the complexity, memory, and CPU resources associated with the performed computation. By default, the prefix FNAME is set to the name of the **AERO-S** input file (**AERO-S** also outputs some of these performance results on the screen). This optional command statement can be used to reset the value of the prefix FNAME. This prefix is also used by **AERO-S**, when executed with the "-t" (or "-T") option, to name the ASCII output file (FNAME.top) containing the geometry of the performed computation in the **XPost** format. It is also used by **AERO-S** for outputting, when requested, a number of domain decomposition (or mesh partitioning) files (see <u>DECOMPOSE</u>).

CONTROL

FNAME ANATYPE NODESET ELEMSET

FNAME Prefix of the names of the files reporting on the performance, geometry, and mesh decomposition of the

performed computation, when applicable. The default value is the name of the AERO-S input file.

ANATYPE Analysis type identifier (integer). Must be set to 2 for heat conduction analysis, and to 1 otherwise.

NODESET Name of the node set describing the grid points of the geometry of the computation in the file FNAME.top

outputted in the XPost format, when AERO-S is executed with the "-t" (or "-T") option. The default

name is "nodes".

ELEMSET Name of the element set describing the connectivity of the geometry of the computation in the file

FNAME. top outputted in the XPost format, when AERO-S is executed with the "-t" (or "-T") option. The

default name is "elems".

Next: <u>DIMASS</u>, Previous: <u>CONTROL</u>

28 DECOMPOSE

Command Statement: DECOMPOSE	C
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The DECOMPOSE command statement can be used to perform an element-based mesh partitioning (or domain decomposition) of the computational domain, and exit or continue with a finite element analysis. The mesh partitioning algorithm is organized in two steps. The first step, which is based on the Greedy algorithm, generates an initial mesh partition. The second step optimizes the subdomain aspect ratios for the benefit of the FETI iterative solution algorithms.

Note 1: This command requires the presence in the FEM input file of a command specifying a FETI solver. When specified in the input file, this command is governed by the following format.

DECOMPOSE

NSUBS nsubs

DETER

OUTFILE decomposition_filename

OUTLOAD

OUTMEM

EXIT SKIP

NSUBS

nsubs Number of subdomains. Default is 1.

DETER By default, the second step of the decomposition is carried out by a simulated annealing procedure.

This sub-command keyword requests instead a deterministic approach to reduce the CPU time

associated with the second step of the decomposition.

OUTFILE

Requests outputing the decomposition in decomposition_filename. The format of this output is decomposition_filename

essentially the number and list of the elements in each subdomain. Default is ${\tt FNAME.optDec}$ where

FNAME is the prefix specified under **CONTROL** (or its default value).

OUTLOAD Requests outputing the load distribution in file decomposition_filename.load. The load distribution

is based on the weights of the elements in the subdomains (see WEIGHTS). Default is off.

OUTMEM Requests outputing in file decomposition_filename.mem an estimate of the subdomain-based memory

comsumption. This estimate is based on the requirements for factoring the subdomain matrices by a direct skyline method. By default, mid-side nodes are ignored by the overall mesh partitioning algorithm. However, when this option is specified, mid-side nodes are taken into account. Default

is off.

EXIT Requests exiting after the mesh partition is generated. Default is off.

SKIP Requests skipping this command. Default is off.

When specified on the command line, the syntax of this command is as follows.

fem_executable --dec --nsub [number_of_subdomains] --deter --load --mem --exit <FEM_input_filename> where --dec requests a mesh decomposition and --nsub specifies the number of subdomains. In this case, the prefix for the names of the files associated with the mesh decomposition and analysis is the content of the FNAME entry of the CONTROL command or its default value.

Note 1: When this command is executed from the command line and specified in the input file, any parameter specified on the command line overrides its value specified in the input file.

Note 2: When a decomposition file is already available, the syntax for running FEM with this decomposition file on a shared memory is

fem_executable -d [decomposition_filename] -v [verbose_frequency] -n [decomposition_filename] <FEM_input_filename>

The above command can be combined with the decomposition command to obtain

 $\label{lem_executable} \begin{tabular}{l} fem_executable --dec --nsub [number_of_subdomains] --load --mem [decomposition_filename] -v [verbose_frequency] -n [decomposition_filename] < FEM_input_filename > \\ \end{tabular}$

Next: DYNAMICS, Previous: DECOMPOSE

29 DISCRETE NODAL MASS AND INERTIA

Command Statement: DIMASS

The DIMASS command statement is used to lump a discrete mass or inertia on a specified degree of freedom of a specified node. If a node number appears more than once under this command with the same degree of freedom, FEM sums all the lumped masses at the specified degree of freedom of the specified node. If a gravity field is also specified in the input file using the GRAVITY command, FEM generates at each specified degree of freedom of each specified node under this command a weight-force component equal to the product of the specified discrete mass and specified gravity acceleration in the corresponding direction.

The input format is given below.





NODE# Node number where the mass or inertia will be added (integer).

DOF# Degree of freedom where the mass or inertia will be added (integer).

VALUE Value of the prescribed discrete mass or inertia (float).

If more than one line addressing the same degree of freedom of the same node are encountered under this command, the effect is the accumulation of the lumped masses or inertias at this degree of freedom.

Next: EIGEN, Previous: DIMASS

30 DYNAMIC ANALYSIS

Command Statement: DYNAMICS

The DYNAMICS command is used to specify a time-integrator for a desired dynamic (transient, time-domain) structural, acoustic, or heat transfer (thermal) analysis and other relevant parameters. Currently, the following time-integrators are available in **AERO-S**:

- 1. The implicit generalized method and the explicit central difference method for linear and nonlinear (in this case, the NONLINEAR command must also be present in the input file) structural dynamic analyses.
- 2. The implicit generalized method, the explicit central difference method, and the explicit modified wave equation algorithm for linear, time-domain acoustic analysis.
- 3. The generalized midpoint family of methods for linear heat transfer analysis.
- 4. The implicit midpoint rule for nonlinear heat transfer analysis.

Linear and Nonlinear Structural Dynamic Analyses and Linear Time-Domain Acoustic Analyses

• The implicit generalized a method for linear and nonlinear structural dynamic and linear time-domain acoustic analyses enforces the following equation of equilibrium

$$M\ddot{u}^{n+1-\alpha_m} + C\dot{u}^{n+1-\alpha_f} + F_{internal}(u^{n+1-\alpha_f}) = F_{external}^{n+1-\alpha_f}$$

where

$$u^{n+1-\alpha_f} = (1-\alpha_f)u^{n+1} + \alpha_f u^n$$

$$\dot{u}^{n+1-\alpha_f} = (1-\alpha_f)\dot{u}^{n+1} + \alpha_f\dot{u}^n$$

$$\ddot{u}^{n+1-\alpha_m} = (1-\alpha_m)\ddot{u}^{n+1} + \alpha_m \ddot{u}^n$$

$$F_{external}^{n+1-\alpha_f} = F_{external}(t^{n+1-\alpha_f})$$

$$t^{n+1-\alpha_f} = (1-\alpha_f)t^{n+1} + \alpha_f t^n$$

and for linear problems

$$F_{internal}(u^{n+1-\alpha_f}) = Ku^{n+1-\alpha_f}$$

Hence, the above equation of equilibrium can also be written as

$$M\left((1-\alpha_m)\ddot{u}^{n+1}+\alpha_m\ddot{u}^n\right)+C\left((1-\alpha_f)\dot{u}^{n+1}+\alpha_f\dot{u}^n\right)+F_{internal}\left((1-\alpha_f)u^{n+1}+\alpha_fu^n\right)=F_{external}^{n+1-\alpha_f}$$

Given $F_{external}(t)$, u^0 and \dot{u}^0 , the implicit generalized α method solves the above equation as follows

$$\ddot{u}^{0} = M^{-1} \left(F_{external}^{0} - F_{internal} (u^{0}) - C \dot{u}^{0} \right)$$
 $u^{n+1} = u^{n} + \Delta t \dot{u}^{n} + \Delta t^{2} \left(\left(\frac{1}{2} - \beta \right) \ddot{u}^{n} + \beta \ddot{u}^{n+1} \right)$
 $\dot{u}^{n+1} = \dot{u}^{n} + \Delta t \left((1 - \gamma) \ddot{u}^{n} + \gamma \ddot{u}^{n+1} \right)$

where $\beta \neq 0$. The Newmark method is obtained by setting $\alpha_m = \alpha_f = 0$. The midpoint rule is obtained by setting $\alpha_m = \alpha_f = \frac{1}{2}$, $\beta = \frac{1}{4}$ and $\gamma = \frac{1}{2}$. The classical central difference method is obtained by setting $\alpha_m = \alpha_f = 0$, $\beta = 0$ and $\gamma = \frac{1}{2}$.

The Newmark method ($\alpha_m=\alpha_f=0$) is second-order time-accurate if and only if $\gamma=\frac{1}{2}$. For undamped linear systems, it is unconditionally stable if and only if $\gamma\geq\frac{1}{2}$ and $\beta\geq\frac{1}{4}(\gamma+\frac{1}{2})^2$.

The generalized α method is second-order time-accurate if and only if $\gamma=\frac{1}{2}-\alpha_m+\alpha_f$. For undamped systems, it is unconditionally stable for linear problems if and only if $\alpha_m\leq\alpha_f\leq\frac{1}{2}$ and $\beta\geq\frac{1}{4}+\frac{1}{2}(\alpha_f-\alpha_m)$. It maximizes high-frequency dissipation when

$$\beta = \frac{1}{4}(1 - \alpha_m + \alpha_f)^2$$
, $\alpha_f = \frac{\rho_{00}}{\rho_{00} + 1}$, $\alpha_m = \frac{2\rho_{00} - 1}{\rho_{00} + 1}$

where ρ_{∞} is the user-specified high-frequency dissipation ($0 \le \rho_{\infty} \le 1$).

 The central difference method for linear and nonlinear structural dynamic and time-domain acoustic analyses implemented in AERO-S enforces the following equation of equilibrium

$$M\ddot{u}^{n+1} + C\dot{u}^{n+\frac{1}{2}} + F_{internal}(u^{n+1}) = F_{external}^{n+1}$$

where

$$\dot{u}^{n+\frac{1}{2}} = \dot{u}^n + \frac{\Delta t}{2} \ddot{u}^n$$

Given $F_{external}(t)$, u^0 and \dot{u}^0 , it solves the above equation as follows

$$\begin{split} \ddot{u}^0 &= M^{-1} \big(F^0_{external} - F_{internal} \big(u^0 \big) - C \dot{u}^0 \big) \\ \\ u^{n+1} &= u^n + \Delta t \dot{u}^n + \frac{\Delta t^2}{2} \ddot{u}^n \\ \\ \dot{u}^{n+1} &= \dot{u}^n + \frac{\Delta t}{2} \big(\ddot{u}^n + \ddot{u}^{n+1} \big) \end{split}$$

This central difference method for linear and nonlinear structural dynamic and linear time-domain acoustic analyses computes the acceleration as follows

$$\ddot{u}^{n+1} = M^{-1} \left(F_{external}^{n+1} - F_{internal}(u^{n+1}) - C(\dot{u}^n + \frac{\Delta t}{2}\ddot{u}^n) \right)$$

For this reason, AERO-S automatically lumps the mass matrix M when this method is selected so that it becomes explicit.

• The explicit modified wave equation method for linear time-domain acoustic analysis implemented in **AERO-S** enforces the following equation of equilibrium

$$M\ddot{u}^{n+1} + C\dot{u}^{n+1} + (K - \frac{\Delta t^2}{q}KM^{-1}K)u^{n+1} = F^{n+1}$$

where C is the damping matrix associated with a non-absorbing boundary condition when such a condition is imposed, and Q is a real-valued parameter.

Given F(t) , u^0 and \dot{u}^0 , it solves the above equation as follows

$$\begin{split} \ddot{u}^0 &= M^{-1} \left(F^0 - C \dot{u}^0 - K \big(u^0 - \frac{\Delta t^2}{q} M^{-1} K u^0 \big) \right) \\ & \dot{u}^{\frac{1}{2}} = \dot{u}^0 + \frac{\Delta t}{2} \ddot{u}^0 \\ & u^{n+1} = u^n + \Delta t \dot{u}^{n+\frac{1}{2}} \\ & G^{n+1} = K \left(I - \frac{\Delta t^2}{q} \big(M + \frac{\Delta t}{2} C \big)^{-1} K \right) u^{n+1} \\ & \dot{u}^{n+\frac{3}{2}} = \big(M + \frac{\Delta t}{2} C \big)^{-1} \left(M \dot{u}^{n+\frac{1}{2}} + \frac{\Delta t}{2} \big(2 F^{n+1} - 2 G^{n+1} - C \dot{u}^{n+\frac{1}{2}} \big) \right) \end{split}$$

For C=0 and q=12, the explicit modified wave equation method is fourth-order time-accurate. For C=0 and $12 \le q \le 12.5$, experience reveals that for many applications, this method delivers an almost sixth-order time-accuracy.

For $\,C
eq 0\,$, the explicit modified wave equation method is second-order time-accurate.

Linear Heat Transfer Analysis

• The generalized midpoint family of methods for linear heat transfer analysis enforces the equation of equilibrium

$$Q\dot{u}^{n+\frac{1}{2}} + Hu^{n+\frac{1}{2}} = F^{n+\frac{1}{2}}$$

where

$$u^{n+\frac{1}{2}} = \frac{1}{2}(u^{n+1} + u^n)$$

$$\dot{u}^{n+\frac{1}{2}} = \frac{1}{2}(\dot{u}^{n+1} + \dot{u}^n)$$

and solves it as follows

$$\dot{u}^{0} = Q^{-1}(F^{0} - Hu^{0})$$

$$\dot{u}^{n+\alpha} = \alpha \dot{u}^{n+1} + (1-\alpha)\dot{u}^{n}$$

$$u^{n+1} = u^{n} + \Delta t \left(\alpha \dot{u}^{n+1} + (1-\alpha)\dot{u}^{n}\right)$$

The generalized midpoint family of methods outlined above is unconditionally unstable for $\alpha>\frac{1}{2}$. Hence, only the choices satistying $\alpha\leq\frac{1}{2}$ are allowed. The forward Euler method is obtained for $\alpha=0$. The backward Euler implicit method is obtained for $\alpha=1$. The implicit trapezoidal rule is obtained for $\alpha=\frac{1}{2}$.

Nonlinear Heat Transfer Analysis

• The implicit midpoint rule for nonlinear heat transfer analysis enforces the following equation of equilibrium

$$Q\dot{u}^{n+\frac{1}{2}} + F_{internal}(u^{n+\frac{1}{2}}) = F_{external}^{n+\frac{1}{2}}$$

where

$$u^{n+\frac{1}{2}} = \frac{1}{2}(u^{n+1} + u^n)$$

$$\dot{u}^{n+\frac{1}{2}} = \frac{1}{2}(\dot{u}^{n+1} + \dot{u}^n)$$

Given $F_{external}(t)$ and u^0 , it solves the above equation as follows

$$\dot{u}^0 = Q^{-1} \left(F_{external}^0 - F_{internal}(u^0) \right)$$

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$$u^{n+1} = u^n + \frac{\Delta t}{2} (\dot{u}^{n+1} + \dot{u}^n)$$

Note 1: Explicit time-integration algorithms are currently not supported for finite element models with rigid elements, massless (or moment-of-interia-less) degrees of freedom, or linear multipoint constraints.

Note 2: Whenever an implicit time-integration algorithm is selected, an equation solver must be specified under the <u>STATICS</u> command.

Note 3: For nonlinear heat transfer analysis (see <u>NONLINEAR</u>), the appropriate midpoint rule algorithm is automatically selected by this command.

The input format of this command is given below.

DYNAMICS

IACC					
	flagIACC				
STABLE	flagSTABLE	E TOLERANCE	E MAXITR		
MODAL	(optional)				
MECH	β	γ			(midpoint implementation of Newmark — that is, $lpha_m=lpha_f=rac{1}{2}$)
месн	β	γ	α_f	α_m	(generalized α)
MECH	ρ_{∞}				(generalized \(\omega\))
ACOU	β	γ			(midpoint implementation of Newmark — that is, $lpha_m=lpha_f=rac{1}{2}$)
ACOU	β	γ	α_f	α_m	(generalized \alpha)
ACOU	ρ_{∞}				(generalized \(\omega\))
ACOU	β	γ	q		(modified wave equation method)
HEAT	α				
TIME	TH	TM	TT		
DAMP	a	b			
DAMP	MODAL				
MODE#	MDV				

MODE#	MDV	

IACC

For linear and nonlinear explicit dynamic computations, and for nonlinear implicit dynamic computations, the initial acceleration is always computed to satisfy equilibrium at t=0. For linear implicit dynamic computations, the initial acceleration is always set to zero if a FETI solver is specified under the <u>STATICS</u> command; otherwise, the user can request to have it either set to zero or computed to satisfy equilibrium at t=0. IACC is the sub-command keyword that enables the user to make this choice in the latter case (characters).

flagIACC

On/Off flag (characters). The default value is on.

on

In this case, for linear implicit dynamic computations, as long as a FETI method is not chosen as the equation solver under the <u>STATICS</u> command, the acceleration is initialized to satisfy equilibrium at t=0. This initialization requires the factorization of the mass matrix. The sparse direct solver is always used for this purpose, no matter which direct equation solver is specified under the <u>STATICS</u> command. If for some reason the finite element model has one or several massless or moment-of-inertia-less degrees of freedom, the sparse solver will typically encounter zero pivots and set the initial acceleration to zero at the corresponding degrees of freedom.

off

In this case, for linear implicit dynamic computations, the initial acceleration is set to zero.

STABLE

Sub-command keyword for managing the time-step of a linear or nonlinear, structural dynamic or acoustic time-integration using the central difference method (characters).

flagSTABLE

On/Off flag (characters).

on

In this case, which is also the default case, **AERO-S** computes automatically the critical time-step — that is, the maximum stability time-step — for the linear and nonlinear explicit central difference methods. For this purpose, **AERO-S** computes the largest eigenvalue of the finite element model using the power method. If the specified time-step TM (see below) is larger than the critical time-step, then the critical time-step is used instead for the computation.

off

In this case, no check is performed for the maximum stability time-step and the time-step TM (see below) is used as specified during the computation.

TOLERANCE

Error tolerance for computing the highest eigenvalue of the finite element model using the power method (float). The default value is 10^{-3} .

MAXITR

Maximum number of iterations for computing the highest eigenvalue of the finite element model using the power method (integer). The default value is 100.

MODAL

Sub-command keyword that requests a modal superposition dynamic analysis (characters). In this case, the READMODE command must be present in the input file to provide the eigenmodes to be used (characters). As in the case of a direct time-integration rather than modal analysis, the initial displacement (see IDISPLACEMENTS) and velocity (see IVELOCITIES) conditions can be input in either the global or generalized coordinates (or reduced system), or in both.

MECH or ACOU or HEAT Sub-command keyword MECH signals a structural dynamic analysis (characters). Sub-command keyword ACOU signals a time-domain acoustic analysis. Sub-command keyword HEAT signals a thermal analysis.

β

Newmark coefficient β for a second-order system (real). Set $\beta=0$ ($\alpha_m=\alpha_f=0$) and $\gamma=\frac{1}{2}$ to

specify the explicit central difference method.

 γ

Newmark coefficient γ for a second-order system (real).

$lpha_f$	Generalized α coefficient α_f for a second-order system (real). To obtain the standard Newmark algorithm, set
	$lpha_f=0$. The default is $lpha_f=rac{1}{2}$.

$$lpha_m$$
 Generalized $lpha$ coefficient $lpha_m$ for a second-order system (real). To obtain the standard Newmark algorithm, set $lpha_m=0$. The default is $lpha_m=rac{1}{2}$.

Generalized
$$\alpha$$
 infinite frequency spectral radius for a second-order system (real). When this option is used and this coefficient is specified, α_f , α_m , β and γ are automatically set to
$$\alpha_f = \frac{\rho_{\infty}}{\rho_{\infty}+1}, \quad \alpha_m = \frac{2\rho_{\infty}-1}{\rho_{\infty}+1}, \quad \beta = \frac{1}{4}(1-\alpha_m+\alpha_f)^2, \quad \gamma = \frac{1}{2}-\alpha_m+\alpha_f$$

, which results in a second-order unconditionally stable scheme that minimizes low frequency dissipation.

For q=12 , the explicit modified wave equation method is fourth-order time-accurate. Furthermore, for many qapplications, experience shows that for $12 \le q \le 12.5$, this method behaves almost like a sixth-order time-accurate scheme.

HEAT Sub-command keyword that preceeds the parameter of the Newmark algorithm for a first-order time-dependent system (characters).

 α Main parameter of the generalized trapezoidal family of methods for first-order systems (real). This parameter must be chosen within $\alpha \leq \frac{1}{2}$.

TIME Sub-command keyword to specify time-interval parameters (characters).

TH Integration time-step for heat transfer analysis (real). This time-step should be put to zero if a structural dynamic or time-domain acoustic analysis is to be performed.

TM Integeration time-step for a structural dynamic or time-domain acoustic analysis (real).

TT End of time-interval for time-integration (real); beginning of time-interval for time-integration is zero for all types of time-dependent analyses.

DAMP Sub-command keyword to specify a Rayleigh damping (characters). This option is active only when MECH is

specified.

Rayleigh damping stiffness coefficient (real). This option is active only when MECH is specified. In the nonlinear case, this coefficient is assigned at each iteration to the linearized stiffness matrix and the product of these two quantities, which varies with the nonlinear iterations, constitutes the contribution to the damping term.

Ь Rayleigh damping mass coefficient **b** (real). This option is active only when MECH is specified.

> Sub-command keywords to signal that the following lines specify modal damping ratios (characters). This option is active only when the MECH and MODAL sub-commands are also specified — that is, for a dynamic analysis of a mechanical system using mode superposition. If a mode is repeated, the modal damping ratio values are added for that mode. If both DAMP and DAMP MODAL commands are specified, and the keyword MODAL is also

specified, then modal damping takes precedence over Rayleigh damping.

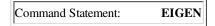
MODE# Mode id number (integer).

MDV Modal damping value (ξ) for mode MODE# (real).

Next: END, Previous: DYNAMICS

DAMP MODAL

31 EIGENVALUE PROBLEMS



The EIGEN command statement is used to request an eigenvalue analysis and specify its parameters. Currently, only the subspace iteration method (the default choice) and the ARPACK package are implemented and hooked in **AERO-S**.

The input format of this command is given below. An example input file can be found in FEM.d/fem_examples/Eigen.d.

Note 1: In principle, the usage of the EIGEN command requires the simultaneous usage of the <u>STATICS</u> command to specify an equation solver. By default, EIGEN selects the default of the <u>STATICS</u> command as an equation solver.



VERSION			
SHIFT	σ		
NSBSPV	nsbspv		
NEIGPA	neigpa		
TOLEIG	toleig		
TOLJAC	toljac		
ARPACK	which		
ARPACK	lbound	nshifts	
ARPACK	lbound	ubound	neigps
MAXITR	maxitr		

VERSION

explicit

In this case, the "stiffness" matrix is used explicity when constructing the reduced eigen problem in the subspace iteration method. Otherwise (default), the reduced eigen problem is constructed by exploiting some mathematical identities to avoid using explicitly the "stiffness" matrix which has been factored by then. Note that specifying the explicit version of the subspace iteration algorithm incurs additional memory storage.

SHIFT

 σ

Value of a specified shift for the "stiffness" matrix (real).

NSBSPV

nsbspv

In the case of the subspace iteration method (default eigensolver), nsbspv denotes the number of subspace iteration vectors and should be set to the minimum between twice the number of requested eigen pairs, $2\times$ neigpa, and neigpa + 8. If rigid body modes are expected, the previous formula should be increased

by the total number of rigid body modes. In the case of ARPACK, nsbspv denotes the number of Krylov vectors (per shift, if a shifting strategy is used).

NEIGPA

neigpa

Number of requested eigen pairs. If rigid body modes are found, their number is included in the specified value of neigpa.

TOLEIG

toleig Tolerance for the convergence of the subspace iteration method.

TOLJAC

BE

toljac Tolerance for the Jacobi Determinant algorithm used in the subspace iteration method.

ARPACK The presence of this keyword under the EIGEN command specifies the usage of the ARPACK eigensolver

> in the shift-invert mode. Otherwise, the default choice is the subspace iteration method. The ARPACK choice is required when one or both of the two matrices governing the eigenvalue problem is/are indefinite. This is the case, for example, when the problem contains linear multipoint constraints, mortar elements, rigid elements, etc. In this case, it is also recommended to use the SPOOLES or MUMPS

sparse direct solver.

which Keyword (characters) to specify which eigenpairs to be computed by ARPACK using the shift-invert

solution method. This keyword can take one of the following values.

In this case, the neigpa eigenpairs whose eigenvalues are just to the right of the shift \square are computed. LA

This is the default value of which when the shift is zero ($\sigma = 0$).

In this case, the neigpa eigenpairs whose eigenvalues are just to the left of the shift are computed. SA

In this case, neigpa eigenpairs with eigenvalues on either side of the shift σ are computed. This is the

default value of which when the shift is non zero ($\sigma \neq 0$).

Lower bound of a set or range of eigenvalues to be computed by ARPACK (real). 1hound

Number of shifts to be used by ARPACK when computing the neigpa eigenvalues that are greater than nshifts

1bound (integer). The specific values of the shifts are automatically selected by ARPACK, and

neigpa/nshifts eigenpairs are computed per shift.

Upper bound of a range of eigenvalues to be computed by ARPACK (real). ubound

neigps When a range of eigenvalues [1bound, ubound] is specified, ARPACK computes all eigenpairs whose

> eigenvalues lie within this range. In this case, a first shift is set to 1bound and neigps consecutive eigenvalues within the range [1bound, ubound] are computed. Then, a recursive procedure in which the largest previously computed eigenvalue is chosen as a new shift and neigps new eigenvalues within the range [1bound, ubound] are computed using this shift is applied until all eigenvalues within the specified

range are captured. The default value for neigns is 50.

MAXITR

Maximum number of iterations for the eigensolver. The default value is $10 \times$ nsbspv. maxitr

Next: FSINTERFACE, Previous: EIGEN

32 END

The END statement is used to indicate the end-of-file. It should always be the last statement.

Next: HFTT, Previous: END

33 FLUID/STRUCTURE INTERFACE

Command Statement: **FSINTERFACE**

The FSINTERFACE command statement is used to specify a fluid/structure interface between two surfaces defined using the SURFACETOPO command (see <u>SURFACETOPO</u>). In this case, the treatment of this interface relies on the capabilities of the ACME library.

The input format of this command is given below.



SURF_PAIR_ID#	FLUID_SURF	STR_SURF
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or

SURF_PAIR_ID#	FLUID_SURF	STR_SURF	NORMAL_TOL	TANGENTIAL_TOL		
SURF_PAIR_I	D#	Id number of	the surface pair to l	oe described (integer	·).	
FLUID_SURF				(see SURFACETO		
STR_SURF		Identification	of the structure sur	face (see SURFACE	ETOPO) (integer).	
NORMAL_TOL		Normal search	n tolerance used by	ACME to identify i	nteractions, default value is 0.1 (float)	
		(see Figs. 1.2	and 1.3 in Section	1.3 of ACME's Use	er Reference Manual).	
TANGENTIAL_	TOL	C		•	y interactions, default value is 0.001 E's User Reference Manual).	

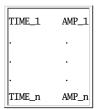
Next: MFTT, Previous: FSINTERFACE

34 FORCE TIME TABLE-HEAT CONDUCTION

Command Statement:	HFTT
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The HFTT command statement must be used to specify the time-dependent amplification of the boundary fluxes. Skipping this statement for dynamics problems results in specifying unit boundary flux amplitudes. For FEM, pairs of time and amplification values are input. Linear interpolation is also used for "in between" points.

HFTT



TIME_1
AMP_1

A specified time point (float).

A specified amplification value at time point TIME_1 (float). This amplification factor is automatically set to zero for all times prior to the earliest specified time point and all times later than the latest specified time point.

Next: GRBM, Previous: HFTT

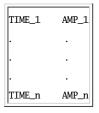
35 FORCE TIME TABLE-MECHANICS AND ACOUSTICS



The MFTT command statement can be used to implement time-dependent tensor-product forms of the boundary conditions enforced by the commands FORCE, ATDDNB, ATDROB, and PRESSURE. In the absence of this statement, these commands imply unit amplitudes. Pairs of time and amplification values are input. Linear interpolation is also used for "in between" points.

When applied with the FORCE command, the result is the amplification of the force value. When applied with the ATDROB command, the result is the amplification of the resulting distributed Neumann boundary condition. When applied with the ATDROB command, the result is the amplification of the right hand-side of the distributed Robin boundary condition. A FEM example input file using this command can be found in FEM.d/fem_examples/MFTT.d

MFTT



TIME_1

AMP 1

A specified time point (float).

A specified amplification value at time point TIME_1 (float). This amplification factor is automatically set to zero for all times prior to the earliest specified time point and all times later than the latest specified time point.

Next: GEPS, Previous: MFTT

36 GEOMETRIC RIGID BODY MODES



The GRBM command is useful for a number of analyses of unrestrained or partially restrained finite element models. It has two separate effects.

- 1. The GRBM command requests the computation of the rigid body modes (or zero energy modes in general) of a given (unrestrained, partially restrained, or even restrained) finite element model using the hybrid geometric-algebraic method published in "C. Farhat and M. Geradin, On the General Solution by a Direct Method of a Large-Scale Singular System of Linear Equations: Application to the Analysis of Floating Structures, International Journal for Numerical Methods in Engineering, Vol. 41, pp. 675-696 (1998). This hybrid method combines a geometry-based algorithm and the SVD factorization of a matrix of constraints corresponding to the given boundary conditions. The SVD factorization relies on the first tolerance specified under this command for identifying the deemed zero singular values, which are associated with the sought-after singular modes. Whenever AERO-S finds the GRBM in its input file, it computes the rigid body modes (or zero energy modes in general) of the given finite element model and prints on the screen the found number of rigid body or zero energy modes. The computed rigid body or zero energy modes themselves are used, for example, by the RBMFILTER command. They are also automatically included in the output of an eigen computation (see OUTPUT).
- 2. When the requested analysis incurs the solution of a singular system of finite element equations, the GRBM command requests the computation of the generalized inverse of the representative singular finite element matrix in factored form using the algorithm published also in "C. Farhat and M. Geradin, On the General Solution by a Direct Method of a Large-Scale Singular System of Linear Equations: Application to the Analysis of Floating Structures, International Journal for Numerical Methods in Engineering, Vol. 41, pp. 675-696 (1998). This algorithm relies on the information computed by the hybrid geometric-algebraic method outlined above (number of rigid body or zero energy modes of the finite element model, and in some cases, the rigid body or zero energy modes themselves). Singular finite element systems arise in general in static problems, whether they are linear or nonlinear. However, tangent finite element matrices associated with unrestrained or partially restrained nonlinear finite element models can have a number of rigid body or zero energy modes that is different from that which can be deduced from the analysis of the matrix of constraints associated with the specified boundary conditions. For this reason, this second part of the GRBM command which selects the aforementioned algorithm for solving a singular system of equations takes effect only for linear static, quasistatic, eigen (structural), aeroelastic, and thermoaeroelastic analyses. When the STATICS command is specified in the input file, then: (a) if a direct solver is specified under this command, the generalized inverse of the stiffness matrix is computed in factored form using the aforementioned algorithm associated with the first part of the GRBM command, and its pivots are monitored for small values (deemed to be zero) using the second tolerance specified under the GRBM command — such deemed zero pivots are associated with zero energy modes associated with mechanisms or model oddities that cannot be detected by the hybrid geometric-algebraic method outlined above; (b) if a FETI-1 or FETI-2 solver is specified under the STATICS command, the rigid body modes of the floating subdomains are computed using the same hybrid geometric-algebraic method and their singular matrices are factored using the same aforementioned algorithm (see section on FETI parameters in STATICS); (c) if a FETI-DP solver is specified under the STATICS command, the tolerance for detecting and computing the rigid body modes of the global structure is set to the first tolerance specified under the GRBM command.

Note 1: See TRBM for an alternative option for analyzing singular systems.

Note 2: If both the <u>GRBM</u> and <u>TRBM</u> commands are specified in the same **AERO-S** input file, the one specified last is chosen. However, if that one is <u>TRBM</u>, it does not de-activate the <u>GRBM</u> command but simply overwrites the value of its second tolerance, VALUE_2, by the value of the tolerance specified under <u>TRBM</u>.

GRBM

VALUE_1	/ALUE_2
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VALUE 1

Tolerance for identifying the zero singular values of the constraint matrix analyzed by the hybrid geometricalgebraic method for finding the rigid body or zero energy modes of a finite element model (real). Its default

value is 1.0e-6.

VALUE_2

Tolerance for capturing algebraically the mechanisms and other oddities that cause a finite element matrix to be singular and which are not visible by the geometric-algebraic method for finding the rigid body or zero energy modes of a finite element model (real). Its default value is 1.0e-16.

Next: GRAVITY, Previous: GRBM

37 GEOMETRIC STIFFENING DUE TO PRESTRESS



The GEPS command statement is essentially a switch that turns on the accounting of pre-stress effects in the form of a geometric stiffness matrix K_G . This geometric stiffness matrix is computed around a displacement field specified under the IDISP6 command

which must be present in the input file. A sample input file illustrating the use of GEPS can be found in FEM.d/fem_examples /Buckle.d/

The effect of this switch on the EIGEN command statement is to replace the stiffness matrix by the tangent stiffness matrix and therefore compute the modes of the structure around the deformed position implied by the displacement field specified under the IDISP6 command.

Its effect on the DYNAMICS and STATICS commands is to replace the stiffness matrix by the tangent stiffness matrix around the displacement field specified under the IDISP6 command, including in aeroelastic applications.

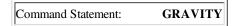
Note 1: The GEPS command can be used only in a linear analysis.

Note 2: If an aeroelastic analysis is requested with the GEPS and IDISP6 commands present in the input file, and the IDISPLACEMENTS command is specified to initialize the displacement field, then FEM sends to the fluid code at each time step the sum of the updated displacement field (interpreted in that case as an increment displacement) and the displacement specified under the IDISP6 command. If in such a case the IDISPLACEMENTS command is not present in the input file, the displacement field of the structure is then initialized by the IDISP6 command, and FEM acts as follows: at the first time-step, it sends to the fluid code only the updated displacement field (since initialization accounts in that case for the content of the IDISP6 command), but at each subsequent time-step, it sends the sum of the updated displacement field and the displacement specified under the IDISP6 command.



Next: **GROUPS**, Previous: **GEPS**

38 GRAVITATIONAL ACCELERATION



The GRAVITY command is used to specify directional gravitational acceleration constants. The input format is given below.

Note 1: By default, AERO-S computes all element gravity loads by a consistent approach, except for explicit dynamic computations (see DYNAMICS), in which case AERO-S always uses a lumped approach for this purpose. If a consistent mass matrix is not available for a particular element (see TOPOLOGY), then AERO-S uses in all cases a lumped gravity load for that element. Alternatively, the LUMPED command can be used to instruct AERO-S to compute all element mass matrices and gravity loads by a lumped approach.



COEFF_X	COEFF_Y	COEFF_Z
COEFF	_X	Gravitational Acceleration Constant in the X direction.
COEFF	_Y	Gravitational Acceleration Constant in the Y direction.
COEFF	_Z	Gravitational Acceleration Constant in the Z direction.

Next: <u>HZEM</u>, Previous: <u>GRAVITY</u>

39 GROUPS



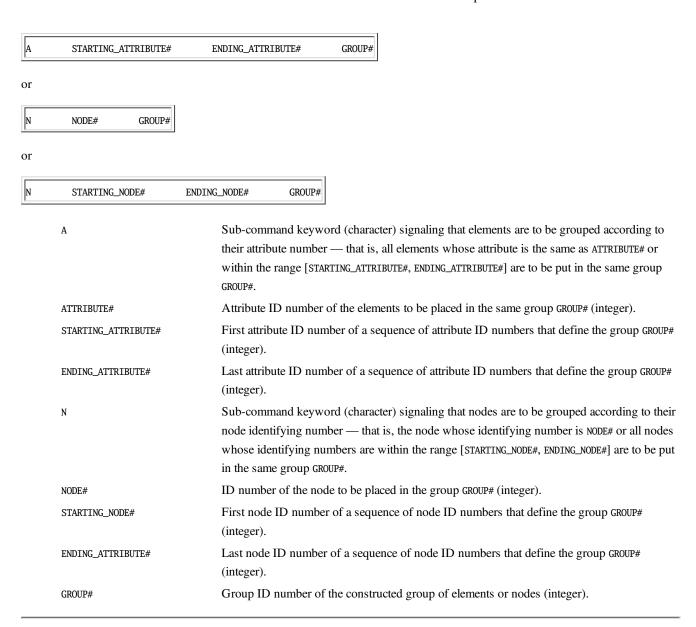
The GROUPS command can be used to organize elements into groups according to their attribute number (see <u>ATTRIBUTES</u>), and nodes into groups according to their integer node identifying number. For example, the notion of a group of elements is used by the <u>RANDOM</u> command to attribute a defined random material property to a group of elements. That of a group of nodes is used by the <u>OUTPUT</u> command to output computed results for groups of nodes.

Note 1: Group identifiers must be contiguous integers starting from 1.

GROUPS



or



Next: <u>HZEMFILTER</u>, Previous: <u>GROUPS</u>

40 HEAT ZERO ENERGY MODE

Command Statement:	HZEM
--------------------	------

This command is effective only for quasistatic and transient thermal, thermoelastic, aerothermal, and aerothermoelastic analyses involving thermal zero energy modes. It is used to request the computation of the constant temperature mode and generalized inverse of the conductivity matrix using a physics-based algorithm (rather than the tolerance-based algorithm associated with the TRBM command). More specifically, if the STATICS command is also present in the input file and a direct solver is specified under this command, the generalized inverse of the conductivity matrix is computed in this direct solver by the physics-based method associated with the HZEM command.



Next: HEFSB, Previous: HZEM

41 HEAT ZERO ENERGY MODES FILTER

Command Statement: HZEMFILTER

This command is effective for thermal simulations involving thermal zero energy modes. When this command is specified in the input file and the thermal problem contains zero energy modes, a non-trivial projector is constructed. In a static or quasistatic analysis, the transpose of this projector is applied to the (possibly variable) right hand-side to make sure that the external load is self-equilibrated and therefore admissible. In a dynamic analysis: a) this projector is applied to the initial solution (initial displacement *and* velocity fields), and b) at each time-step, the transpose of this projector is applied to the (time-dependent) right-hand side of the equation being solved. In all cases, the net effect is to work with (and output) a solution where the thermal zero energy modes have been filtered out.

The constructed projector can be written as $P = I - Z(Z^TQZ)^{-1}Z^TQ$ where I is the identity matrix, Z is the matrix of the

zero energy modes of the thermal system (always computed with the HZEM method), and Q is either the identity matrix for static analysis, or the capacity matrix for quasistatic and dynamic analyses. Hence, in a quasistatic analysis, this command requires the specific heat to be present in the material properties specified under the MATERIAL command.

HZEMFILTER

Next: HEFRS, Previous: HZEMFILTER

42 HYDROELASTIC FLUID/STRUCTURE BOUNDARY

Command Statement: HEFSB

The HEFSB command is used to specify the fluid-structure interface boundary in a hydroelastic eigenvalue problem. The underlying discrete model assumes that each node on this interface is shared by fluid and structural elements. The input format of this command is given below.

Note 1: AERO-S automatically determines that a hydroelastic eigenvalue problem is to be solved once it finds in the input file the HEFSB command, or the <u>HEFRS</u> command which specifies its free-surface boundary. This problem is solved using the added mass approach. The computed frequencies and structural mode shapes can be output as in a regular structural eigen computation.

HEFSB

FACE# FACE_TYPE CONNECTIVITY_NODES

FACE#	Face (or edge in two dimensions) identification number whose type and connectivity are to be specified
FACE_TYPE	(integer).
1	2-node line segment. To be used with the appropriate two-dimensional linear elements.
2	3-node line segment. To be used with the appropriate two-dimensional quadratic elements.
3	3-node triangular face. To be used with the appropriate three-dimensional linear tetrahedral elements.
4	4-node quadrilateral face. To be used with the appropriate three-dimensional linear hexahedral elements.
6	6-node triangular face. To be used with the appropriate three-dimensional quadratic tetrahedral elements.
10	Quadrilateral face. To be used with the appropriate higher-order iso-parametric hexahedral elements.
11	Triangular face. To be used with the appropriate higher-order iso-parametric tetrahedral elements.
12	Line segment. To be used with the appropriate higher-order iso-parametric quadrilateral elements.
13	Line segment. To be used with the appropriate higher-order iso-parametric triangular elements.
CONNECTIVITY_NODES	These should be listed in a stacked fashion on a single line. The nodes of low-order faces should be ordered counter-clockwise (when looking from infinity in three dimensions). Those of higher-order faces (type 10–13) should be ordered from left to right and bottom to top using any convenient axis system (when viewed from infinity in three dimensions).
	\$10 to 1 to 10 to

Next: HELMHOLTZ, Previous: HEFSB

43 HYDROELASTIC FREE SURFACE BOUNDARY

Command Statement:	HEFRS
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The HEFRS command can be used to specify the free surface boundary either in a hydroelastic eigenvalue problem, or in an acoustic or elastoacoustic (fluid-structure interaction) problem where the fluid is assumed to be weightless. At each node located on this free surface, **AERO-S** automatically applies a zero Dirichlet boundary condition for the fluid potential equation in the first case, and for the scattered pressure in the second case.

The input format of this command is given below.

Note 1: AERO-S automatically determines that a hydroelastic eigenvalue problem is to be solved once it finds in the input file the HEFRS command, or the <u>HEFSB</u> command which specifies its hydroelastic fluid/structure interface boundary. This problem is solved using the added mass approach. The computed frequencies and structural mode shapes can be output as in a regular structural eigen computation.



FACE# FACE_TYPE	PE CONNECTIVITY_NODES
-----------------	-----------------------

FACE#

Face (or edge in two dimensions) identification number whose type and connectivity are to be specified

	(integer).
FACE_TYPE	
1	2-node line segment. To be used with the appropriate two-dimensional linear elements.
2	3-node line segment. To be used with the appropriate two-dimensional quadratic elements.
3	3-node triangular face. To be used with the appropriate three-dimensional linear tetrahedral elements.
4	4-node quadrilateral face. To be used with the appropriate three-dimensional linear hexahedral elements.
6	6-node triangular face. To be used with the appropriate three-dimensional quadratic tetrahedral elements.
CONNECTIVITY_NODES	These should be listed in a stacked fashion on a single line, and ordered clockwise (when looking from infinity in three dimensions).

Next: <u>HARB</u>, Previous: <u>HEFRS</u>

44 HELMHOLTZ

ommand Statement:	HELM
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The HELM command statement is used to specify a medium-homogeneous, frequency-domain, acoustic (Helmholtz) or elastoacoustic problem and define some of its parameters. Its input format is given below.

Note 1: The time-harmonic form of the solution of the frequency-domain problem considered in the HELM command is $p=Pe^{-i\omega t}$, where P denotes the amplitude of the field p and ω its frequency.

Note 2: The HELM command solves always for the *scattered* pressure p^s . However, one can output the scattered pressure as well as the total pressure.

Note 3: The boundary conditions of the acoustic (Helmholtz) or elastoacoustic problem defined in this command can be set either by the keyword Planewave or the keyword Pointsource (see below). When either is specified, the Real_Part_value and IMAGINARY_Part_value of any specified Helmholtz dirichlet boundary condition (see HDIR) are overwritten by $p^s = -p^i$, where p^i is the incident time-harmonic plane wave or spherical wave implied by Planewave or Pointsource, respectively.

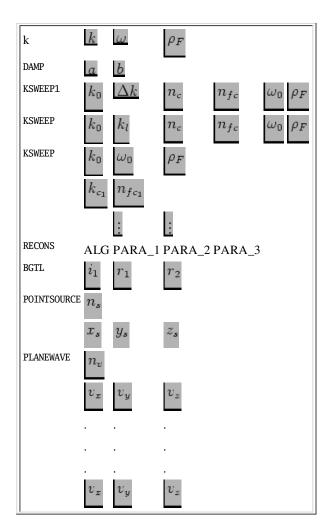
Note 4: The first- and second-order generalized Bayliss-Gunburger-Turkel non reflecting conditions are not implemented for all three-dimensional brick elements.

Note 5: For Helmholtz problems defined in a homogenous (fluid) medium, the user may set MAT_ATT# = 1 in <u>ATTRIBUTES</u> for all of the acoustic elements and not define any material property for this attribute in <u>MATERIAL</u>.

Note 6: For Helmholtz problems defined in a heterogeneous (fluid) medium — and this includes the case where a Perfect Matching

Layer is present in the computational domain — the user may define in <u>MATERIAL</u> material properties for the acoustic elements, in which case the wavenumber \boxed{k} and fluid density $\boxed{\rho_F}$ specified under this command are ignored and the wavenumber(s) and fluid densities specified using the FLUMAT sub-command of <u>MATERIAL</u> are used instead.





k Wavenumber for the fluid system or subsystem (float).
 Pulsation for a coupled elastoacoustic problem. Skipping the specification of ω and ρ_F signals a pure acoustic (Helmholtz) problem (float).
 P_F Fluid density for a coupled elastoacoustic problem. Skipping the specification of ω and ρ_F signals a pure acoustic (Helmholtz) problem (float).
 DAMP Structured Rayleigh damping for a coupled electroelastic problem.
 a Rayleigh damping stiffness coefficient a (float).
 b Rayleigh damping stiffness coefficient b (float).

KSWEEP1

This option requests a sweeping on the wavenumber using a one-point scheme to be defined in RECONS.

The swept wavenumbers are given by

kers are given by
$$k_0 - n\Delta_k \le k \le k_0 + n\Delta_k, \quad -n_{\Delta_k} \le n \le n_{\Delta_k}$$

Wavenumber defining the center of a band (float).

Increment for the wavenumber (float).



Half the number of increments to be performed (integer).

KSWEEP

This option requests sweeping on the wavenumber using a numerical reconstruction scheme to be defined in RECONS. The swept wavenumbers are defined via a regular or irregular "coarse-grid" wavenumber axis (characters). In the regular case, the coarse-grid wavenumber points are specified via a wavenumber band $[k_0, k_l]$ and a number of equally spaced apart frequencies, n_c . Within this wavenumber band, and

within each pair of consecutive coarse-grid wavenumber axis points, $n_{fc}-1$ "fine-grid" wavenumber

axis points are inserted and the solution of the forced wavenumber response problem is reconstructed at each of thes fine-grid wavenumber axix points. In the irregular case, the first wavenumber point k_0 is

specified on the first line after the keyword KSWEEP. Next, on a separate line, a coarse-grid wavenumber point k_{c_1} can be specified together with an integer number n_{fc_1} specifying that $n_{fc}-1$ equally spaced "fine-grid" wavenumber axis points are to be generated in the interval $[k_0, k_{c_1}]$. The solution

of the forced wavenumber response problem will be reconstructed at each of these fine-grid wavenumber axis points. This separate line can be repeated as many times as desired, with the understanding that the

implied wavenumber intervals are $[k_{c_1}, k_{c_2}]$, then $[k_{c_2}, k_{c_3}]$ etc. Clearly, this command is best exploited if the k_{c_i} values are input in a monotonic sequence of increasing or decreasing numbers.

Defines the reconstruction strategy in the case of a wavenumber sweep and sets its parameters. RECONS

ALG Reconstruction algorithm (characters). Three such algorithms are available and listed below.

Taylor series algorithm (characters). Taylor

Conventional Pade algorithm (characters). Pade

PadeLanczos Pade algorithm based on a Lanczos procedure (characters). This version of the Pade reconstruction

algorithm is less prone to ill-conditioning and therefore is more performing.

PARA_1 First parameter of the reconstruction algorithm. For a Taylor series, it specifies the order of the series. For

a Pade series, it specifies the number of points on the coarse-grid wavenumber axis to be used at-one-time

to reconstruct the solution.

Second parameter of the reconstruction algorithm that is relevant only for the case of a Pade-based PARA_2

reconstruction algorithm. It specifies the order of the numerator of the rational function (L of [L/M])

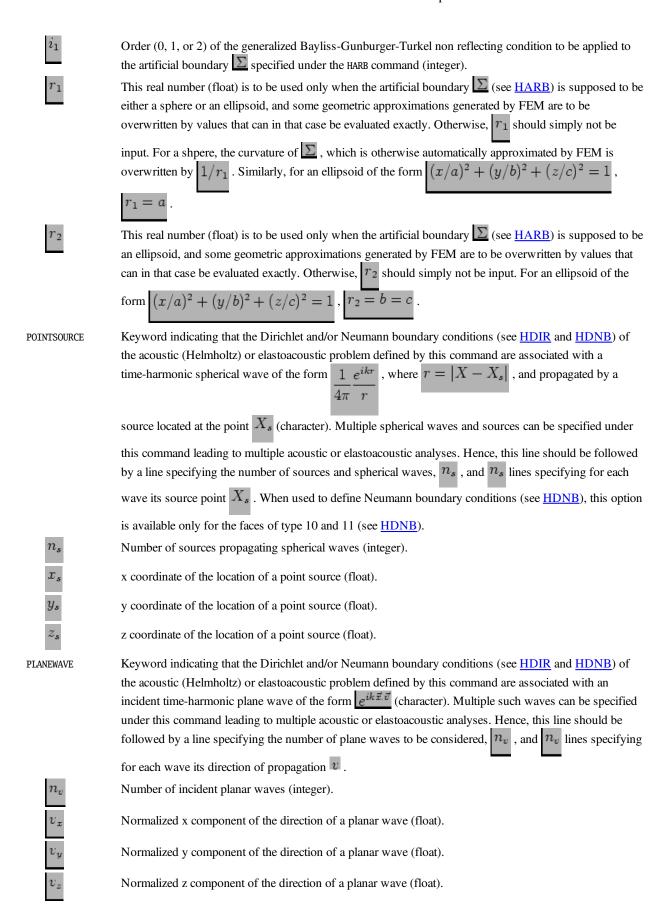
(integer).

PARA_3 Third parameter of the reconstruction algorithm that is relevant only for the case of a Pade-based

reconstruction algorithm. It specifies the order of the denominator of the rational function (M of [L/M])

(integer).

BGTL



Next: HDIR, Previous: HELMHOLTZ

45 HELMHOLTZ ARTIFICIAL BOUNDARY *S*

Command Statement:	HARB
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The HARB command statement is used to specify the artificial boundary \sum on which the absorbing condition specified in the HELMHOLTZ command (see <u>HELMHOLTZ</u>) is to be applied. The input format is given below.

Note 1: In general, the absorbing condition in frequency domain computations is applied to the solution variable. Since in frequency domain computation it does not make sense to absorb the incident field, the solution variable in FEM is in this case the scattered field.



FACE#	FACETYPE	CONNECTIVITY_NODES
F	FACE#	Face (or edge in two dimensions) id number whose type and connectivity are to be specified (integer). In practice, this id number is ignored by FEM.
F	FACE_TYPE	
	1	2-node line segment. To be used with two-dimensional linear elements.
	2	3-node line segment. To be used with two-dimensional quadratic elements.
	3	3-node triangular face. To be used with three-dimensional linear tetrahedral element.
	4	4-node quad face. To be used with three-dimensional linear hexahedral element.
	6	6-node triangular face. To be used with three-dimensional quadratic tetrahedral element.
	10	4, 9, 16 or 25-node quadrilaterial face. To be used with three-dimensional full isoparametric hexahedral elements.
	11	3, 6, 10 or 15-node triangular face. To be used with three-dimensional full isoparametric tetrahedral elements.
	12	2, 3, 4 or 5-node line segment. To be used with two-dimensional full isoparametric quadrilateral elements.
	13	2, 3 or 4-node line segment. To be used with two-dimensional full isoparametric triangular elements.
CONNECTI	TVITY_NODES	These should be listed in a stacked fashion on a single line, and numbered clockwise (when looking from outside in three dimensions).

Next: <u>HDNB</u>, Previous: <u>HARB</u>

46 HELMHOLTZ DIRICHLET BOUNDARY CONDITIONS *S*

	Command Statement:	HDIR
Ш	Communa Statement.	III

The HDIR command statement is used to specify *nodal* Dirichlet boundary conditions for a frequency-domain acoustic scattering (Helmholtz) problem. The input format is given below.

Note 1: When the keyword Planewave or Pointsource is specified under $\frac{\text{HELMHOLTZ}}{\text{HELMHOLTZ}}$, it overwrites the Real_part_value and IMAGINARY_PART_value (see below) by $p^s = -p^i$, where p^i is an incident time-harmonic plane wave specified by Planewave, or an

incident time-harmonic spherical wave propagating from a source located at a point specified by POINTSOURCE.

HDIR

NODE#	DOF#	REAL_PART_VALUE IMAGINARY_PART_VALUE
N	IODE#	Node number where the Dirichlet boundary condition is specified (integer).
D	OF#	Degree of freedom local number where the boundary condition is specified (integer). This numb should be set to 8.
R	EAL_PART_VALUE	Real part of the value of the specified boundary condition (float).
I	MAGINARY_PART_VA	Imaginary part of the value of the specified boundary condition (float).

Next: HNEU, Previous: HDIR

47 HELMHOLTZ DISTRIBUTED NEUMANN BOUNDARY CONDITION *S*

Command Statement: HDNB

The HDNB command statement can be used to specify on all or segments of the surface of a scatterer a *distributed* Neumann boundary condition of the form $\left[\frac{\partial p^s}{\partial n} = -\frac{\partial p^i}{\partial n}\right]$, where the superscripts and designate scattered and incident quantities, respectively. The

incident time-harmonic wave can be a time-harmonic plane wave specified by PLANEWAVE, or a time-harmonic spherical wave propagating from a source located at a point specified by POINTSOURCE in <a href="https://hem.ncbi.nlm.ncbi.

The input format of this command is given below.

HDNB

FACE# FACE_TYPE CONNECTIVITY_NODES

FACE#	Face (or edge in two dimensions) id number whose type and connectivity are to be specified (integer). In practice, this id number is ignored by FEM.
FACE_TYPE	
1	2-node line segment. To be used with two-dimensional linear elements.
2	3-node line segment. To be used with two-dimensional quadratic elements.
3	3-node triangular face. To be used with three-dimensional linear tetrahedral elements.
4	4-node quad face. To be used with three-dimensional linear hexahedral elements.
6	6-node triangular face. To be used with three-dimensional quadratic tetrahedral elements.
10	4, 9, 16 or 25-node quadrilaterial face. To be used with three-dimensional full isoparametric hexahedral elements.
11	3, 6, 10 or 15-node triangular face. To be used with three-dimensional full isoparametric tetrahedral elements.
12	2, 3, 4 or 5-node line segment. To be used with two-dimensional full isoparametric quadrilateral elements.
13	2, 3 or 4-node line segment. To be used with two-dimensional full isoparametric triangular elements.
CONNECTIVITY_NODES	These should be listed in a stacked fashion on a single line, and numbered counter clockwise (when looking from infinity in three dimensions).

Next: <u>HSCB</u>, Previous: <u>HDNB</u>

48 HELMHOLTZ NEUMANN BOUNDARY CONDITIONS *S*

Command Statement:	HNEU
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The HNEU command statement is used to specify the *nodal* Neumann boundary conditions for a frequency-domain acoustic scattering (Helmholtz) problem. The input format is given below.



HNEU	REAL_PART_VALUE	IMAGINARY_PART_VALUE
NODE#		Node number where the Neumann boundary condition is specified (integer).
REAL_PART_VALUE		Real part of the value of the specified boundary condition (float).
IMAGINARY_PART_VALUE		Imaginary part of the value of the specified boundary condition (float).

Next: **HWIB**, Previous: **HNEU**

49 HELMHOLTZ SCATTERER BOUNDARY *S*

Command Statement:	HSCB
Command Statement:	HSCB

The HSCB command statement is used to specify the surface of a scatterer. It is used to compute the farfield pattern of a frequency-domain acoustic solution, and therefore is necessary for the output of that farfield pattern (see OUTPUT).

The input format of this command is given below.



FACE#	FACE_TYPE	CONNECTIVITY_NODES		
FACE#		Face (or edge in two dimensions) id number whose type and connectivity are to be specified (integer). It practice, this id number is ignored by FEM.		
F	ACE_TYPE			
	1	2-node line segment. To be used with two-dimensional linear elements.		
	2	3-node line segment. To be used with two-dimensional quadratic elements.		
	3	3-node triangular face. To be used with three-dimensional linear tetrahedral elements.		
4 6		4-node quad face. To be used with three-dimensional linear hexahedral elements.		
		6-node triangular face. To be used with three-dimensional quadratic tetrahedral elements.		
	10	4, 9, 16 or 25-node quadrilaterial face. To be used with three-dimensional full isoparametric hexahedral elements.		
	11	3, 6, 10 or 15-node triangular. To be used with three-dimensional full isoparametric tetrahedral elements		
	12	2, 3, 4 or 5-node line segment. To be used with two-dimensional full isoparametric quadrilateral element		
2, 3 or 4-node line segment. To be used with two-dimensional full isoparametric triangu				
CONNECTI	VITY_NODES	These should be listed in a stacked fashion on a single line, and numbered counter clockwise (when looking from outside in three dimensions).		

Next: HLMPC, Previous: HSCB

50 HELMHOLTZ WET INTERFACE BOUNDARY *S*



The HWIB command statement is used to describe the wet interface boundary for coupled elasto-acoutic frequency response analysis using fluid and structure meshes with matching discrete interfaces. The input format is given below.

Note 1: This is an alternative way to specify a **matching** fluid/structure interface using the same input format as that for the HARB and HDNB commands (see <u>HARB</u> and see <u>HDNB</u>) but is supported only for face types 10, 11, 12 and 13.

Note 2: In this case, the capabilities of the ACME library are not used to generate the fluid-structure coupling coefficients.



FACE# FACETYPE	CONNECTIVITY_NODES	
FACE#	Face (or edge in two dimensions) id number whose type and connectivity are to be specified (integer). In practice, this id number is ignored by FEM.	
FACETYPE		
4, 9, 16 or 25-node quadrilaterial face. To be used with three-dimensional full isoparametric elements.		
11	3, 6, 10 or 15-node triangular. To be used with three-dimensional full isoparametric tetrahedral elements.	
2, 3, 4 or 5-node line segment. To be used with two-dimensional full isoparametric quadrilate 2, 3 or 4-node line segment. To be used with two-dimensional full isoparametric triangular el		
	9-noded quad:	
	7 8 9	
	4 5 6	
	1 2 3	
	6-noded triangle:	
	6	
	4 5	
	1 2 3	
	etc.	

Next: KIRLOC, Previous: HWIB

51 HELMHOLTZ LINEAR MULTIPOINT CONSTRAINTS

Command Statement: HLMPC

The HLMPC command statement is used to specify a set of complex linear multipoint constraint equations of the form

$$\sum_{j=1}^{j=n} c_{ij} u_j = r_i, \quad i = 1, 2, \dots$$

in a frequency-domain acoustic model. There is no limitation on the number of multipoint constraints, or number of degrees of freedom related by the same constraint equation. The format of this command statement is as follows.

Note 1: The Lagrange multiplier method for enforcing the constraints associated with this command is supported only by the FETI-DP family of solvers, the GMRES solver, and the SPOOLES and MUMPS direct sparse solvers (see <u>STATICS</u>).

HLMPC

CONSTRAINT# RHS

	NODE#	DOF#	REAL-PART-COEFF	IMAGINARY- PART-COEFF	
	NODE#	DOF#	REAL-PART-COEFF	IMAGINARY- PART-COEFF	
	CONSTRAINT#		This corresponds	to the constrain	int equation number i (integer).
		RHS	This is the right-l	hand side r_i of	f the i-th constraint equation (float). It can have both a real and an
	CONSTRAINT_METHOI		imaginary part, ir	n which case the	e real part is specified first and followed by the imaginary part.
			This is the method for enforcing the constraint (characters). The default method is set in CONSTRAINTS		
			and used whenev	ver this entry is	omitted.
			The Lagrange mu	altiplier method.	

multipliers

The elimination method.

CONSTRAINT_METHOD

elimination

penalty The penalty method. The parameter beta should be a large pos

The penalty method. The parameter beta should be a large positive number, typically of the order of 10^8

[beta] (no default value is provided).

NODE# This is the number of the node contributing the coefficient C_{ij} of the *i*-th constraint equation (integer).

DOF# This is the local number of the degree of freedom at the node specified above contributing the coefficient

 c_{ij} of the *i*-th constraint equation (integer).

REAL_PART_VALUE This is the real part of the coefficient C_{ij} of the *i*-th constraint equation (float).

This is the imaginary part of the coefficient C_{ij} of the *i*-th constraint equation (float).

IMAGINARY_PART_VALUE

Next: IMPEDANCE, Previous: HLMPC

52 HELMHOLTZ LOCATIONS WHERE TO COMPUTE THE KIRCHHOFF INTEGRAL

Command Statement: KIRLOC

When KIRCHHOFF is selected as an OUTPUT result and the surface of the scatterer is defined using the command HSCB, this command should be used to specify the locations of the points where the (far-field) solution of the HELMHOLTZ problem to be solved is to be evaluated using the Kirchhoff integral and outputted. These points do not need to be nodes of the mesh. Usually, but not necessarily, they are outside the computational domain, in the far-field. The keyword KIRLOC should be followed by as many lines as there are points where it is desired to evaluate the solution of the frequency-domain acoustic or elastoacoustic problem defined in

HELMHOLTZ. Each line contains the coordinates of such a point.

The syntax of this command is given below.

KIRLOC

X-ORDINATE	Y-ORDINATE	Z-ORDINATE
X-ORDINATE		X-ordinate of a point where to evaluate the solution of the <u>HELMHOLTZ</u> problem to be solved using the Kirchhoff integral (real).
Y-ORDINATE		Y-ordinate of a point where to evaluate the solution of the <u>HELMHOLTZ</u> problem to be solved using the Kirchhoff integral (real).
Z-ORDINATE		Z-ordinate of a point where to evaluate the solution of the <u>HELMHOLTZ</u> problem to be solved using the Kirchhoff integral (real).

Next: IACCELERATIONS, Previous: KIRLOC

53 IMPEDANCE

Command Statement:	IMPEDANCE
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The IMPEDANCE command statement can be used for three purposes: 1) to signal that the problem to be solved is a forced frequency response structural dynamics problem of the form

$$(K - \omega^2 M - i\omega D)u = f$$

 $(K-\omega^2M-i\omega D)u=f$ where K is a stiffness matrix, M is a mass matrix, D=aK+bM is a Rayleigh damping matrix, $\omega^2=4\pi^2f^2$ with f

denoting the forced frequency which can be varied in a specified interval, \overline{u} is the amplitude of the time-harmonic solution $v=ue^{-i\omega t}$, and g is the amplitude of the time-harmonic forcing function $h=ge^{-i\omega t}$, 2) to specify a shift σ to be applied to

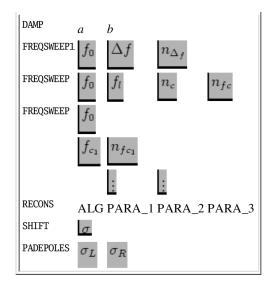
the stiffness matrix so that it is replaced in any dynamic analysis specified by any other command such as **EIGEN** or other by

$$\tilde{K} = K - \sigma M$$

, and 3) to identify within a specified range of interest the eigenvalues missed by a previous eigenvalue computation and their corresponding eigenvectors.

The input format of this command is given below.

IMPEDANCE FREQ f



FREQ

f Value of the forced frequency (float). The corresponding value of ω^2 is $4\pi^2 f^2$.

DAMP

- a Rayleigh damping stiffness coefficient a (float).
- b Rayleigh damping stiffness coefficient b (float).

FREQSWEEP1 This option requests sweeping on the frequency using a one-point scheme to be specified in RECONS. The swept frequencies are given by

$$f_0 - n\Delta_f \le f \le f_0 + n\Delta_f, \quad -n_{\Delta_f} \le n \le n_{\Delta_f}$$

 f_0

Frequency defining the center of a band (float).

 Δ_f

Increment for the frequency (float).

 n_{Δ_f}

Half the number of increments to be performed (integer).

FREQSWEEP

This option requests sweeping on the frequencies defined by refining a regular or irregular "coarse-grid" frequency axis (characters). In the regular case, the coarse-grid frequency points are specified via a frequency band [f_0 , f_l] and a number of equally spaced apart frequencies, n_c . Within this frequency band, and within each pair of consecutive coarse-grid frequency axis points, $n_{fc}-1$ "fine-grid" frequency axis points are inserted and the solution of the forced frequency response problem is reconstructed at each of thes fine-grid frequency axix points. In the irregular case, the first frequency point f_0 is specified on the first line after the keyword FREQSWEEP. Next, on a separate line, a coarse-grid frequency point f_0 can be specified together with an integer number n_{fc_1} specifying that $n_{fc}-1$ equally spaced "fine-grid" frequency axis points are to be generated in the interval f_0 , f_0 . The solution of the forced frequency response problem will be reconstructed at each of these fine-grid frequency axis points. This separate line can be repeated as many times as desired, with the understanding that the implied frequency intervals are f_0 , f_0 , then f_0 then f_0 then f_0 then f_0 then f_0 then f_0 then

	Clearly, this command is best exploited if the f_{c_i} values are input in a monotonic sequence of increasing or	
	decreasing numbers.	
RECONS	Defines reconstruction strategy in the case of a frequency sweep and sets its parameters.	
ALG	Reconstruction algorithm (characters). Three such algorithms are available and listed below.	
Taylor	Taylor series algorithm (characters).	
Pade	Conventional (multipoint) Pade algorithm (characters).	
PadeLanczos	(multipoint) Pade algorithm based on a Lanczos procedure (characters). This version of the Pade reconstruction algorithm is less prone to ill-conditioning and therefore is more performing.	
PARA_1	First parameter of the reconstruction algorithm. For a Taylor series, it specifies the order of the series. For a Pade series, it specifies the number of points on the coarse-grid frequency axis to be used at-one-time to reconstruct the solution.	
PARA_2	Second parameter of the reconstruction algorithm that is relevant only for the case of a Pade-based reconstruction algorithm. It specifies the order of the numerator of the rational function (L of [L/M]) (integer).	
PARA_3	Third parameter of the reconstruction algorithm that is relevant only for the case of a Pade-based reconstruction algorithm. It specifies the order of the denominator of the rational function (M of [L/M]) (integer).	
SHIFT	Specifies a shift σ for the stiffness matrix (real).	
PADEPOLES	PADEPOLES Can be used only with ALG = PadeLanczos and needs input from the FREQSWEEP sub-command (see above). A requires the presence in the input file of the MODE command in order to retrieve the EIGENMODES file files placed in the execution path. In this case, the PadVe-based approximation of a rational transfer function whose poles are exactly the eigenvalues of the symmetric pencil (K, M) is computed and the poles of the	
	approximation are exploited to identify, in a specified range of interest $[\sigma_L, \sigma_R]$, the eigenvalues that may	
	have been missed by a previous eigen computation in which the $\underline{\text{MODE}}$ command was used to store the results in the EIGENMODES file or files (characters). The points $\sigma_i \in [\sigma_L, \sigma_R]$ needed by the multipoint Pade	
	expansion should be generated with the FREQSWEEP sub-command and σ_i playing the role of f_i , $f_0=\sigma_L$,	
	$f_l=\sigma_R$ (and for example, $n_{fc}=0$). The parameters of this expansion should be specified in PARA_1,	
	PARA_2 and PARA_3 (see above). At the end of the computation, AERO-S outputs on the screen the poles of the	
	Pade rational function in the specified range $[\sigma_L, \sigma_R]$, excluding those poles corresponding to the modes read	
	from the EIGENMODE file or files. The specified range $[\sigma_L,\sigma_R]$ can be narrower than that of the eigenvalues	
	read in the EIGENMODE file or files. However, if it is wider, some of the poles of the Pade rational function	

 σ_L Lower bound of an eigenvalue interval of interest (real).

L Upper bound of an eigenvalue interval of interest (real).

Next: <u>IDISPLACEMENTS</u>, Previous: <u>IMPEDANCE</u>

54 INITIAL ACCELERATIONS (Not Supported Yet)

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with the poles or missed eigenvalues is not currently implemented in AERO-S.

may, in some cases, not correspond to missed eigenvalues. In any case, the output of the eigenvectors associated

Command Statement: IACCELERATIONS

The IACCELERATIONS command statement is used to specify a nodal acceleration type of initial conditions. Each node can have up to six degrees of freedom.

IACCELERATIONS

VALUE

NODE# VALUE

NODE# Node number where the initial acceleration is specified (integer).

DOF# Degree of freedom local number where the initial acceleration is specified (integer).

Value of the specified initial acceleration (float).

Next: IDISP6, Previous: IACCELERATIONS

55 INITIAL DISPLACEMENTS *S*

Command Statement:	IDISPLACEMENTS
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For a structural analysis, the IDISPLACEMENTS command statement can be used to specify a nodal (or generalized coordinate) displacement type of initial conditions (in this case, each node can have up to six degrees of freedom). For a time-domain acoustic analysis, this command statement can be used to specify initial nodal values of the solution by setting DOF# to 8 (see below).

Note 1: If the GEPS command is not specified, and both DISP and IDISP6 commands are present in the input file, **AERO-S** selects the IDISP6 command to initialize the displacement field.

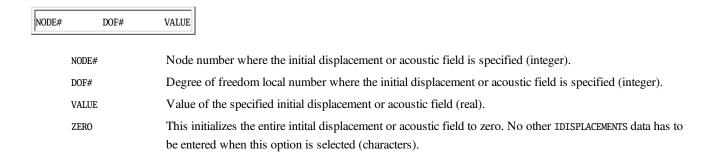
Note 2: If the GEPS command is present in the input file, then AERO-S uses the content of the command IDISP6 to construct the geometric stiffness, and the content of IDISPLACEMENTS to initialize the displacement field. If IDISPLACEMENTS is not specified in the input file, then AERO-S uses the command IDISP6 to perform both tasks of constructing the geometric stiffness and initializing the displacement field.

When working with the usual basis, the format of this command is as follows.

IDISPLACEMENTS

or

IDISPLACEMENTS ZERO



When working in the basis of generalized coordinates (or reduced basis), the format of this command is as follows.

IDISPLACEMENT	S
MODAL	
MODE#	VALUE

MODAT

MODE#

This keyword, completely spelled out, can be used either (1) to input the initial conditions in the generalized coordinates (or reduced) system, whether a modal or direct time-integration analysis is requested in the DYNAMICS command, or (2) to superpose to the initial conditions specified in the global coordinates system additional initial conditions formulated in the generalized coordinates (or reduced) system (characters). In the second case, IDISPLACEMENTS and/or IVELOCITIES should be specified only once and the keyword MODAL can be inserted either before or after the set of data associated with the initial conditions specified in the global coordinates. This option can also be used to perform a "Ping-Pong" analysis (AERO). In both cases, the READMODE command statement must also be present in the input file to provide the eigenmodes to be used. Mode id number for which the initial condition is specified (integer).

VALUE Value of the specified initial condition (real).

Next: IDISP6PITA, Previous: IDISPLACEMENTS

56 INITIAL DISPLACEMENT 6 COLUMNS (IDISP6 completely spelled out) *S*



The IDISP6 command statement is used to specify a nodal displacement type of initial conditions in a different format than that of the IDISPLACEMENTS command. This format is specified below.

Note 1: If the GEPS command is not specified, and both IDISPLACEMENTS and IDISP6 commands are present in the input file, **AERO-S** selects the IDISP6 command to initialize the displacement field.

Note 2: If the GEPS command is present in the input file, then AERO-S uses the content of the command IDISP6 to construct the geometric stiffness, and the content of IDISPLACEMENTS to initialize the displacement field. If IDISPLACEMENTS is not specified in the input file, then AERO-S uses the command IDISP6 to perform both tasks of constructing the geometric stiffness and initializing the displacement field.

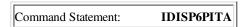
Note 3: Modal initial condition is not supported under IDISP6 because it is not relevant.



NODE#	VAL_DOF1	VAL_DOF2	VAL_DOF3	VAL_DOF4	VAL_DOF5	VAL_DOF6							
	α	Amplification fa	Amplification facteur that multiplies each VAL_DOFi value for each node (real).										
N	NODE#	Node number where the initial displacement is specified (integer).											
7	VAL_DOFi#	Value of the specified initial displacement for the i-th degree of freedom (real).											

Next: IVEL6PITA, Previous: IDISP6

57 INITIAL OR SEED DISPLACEMENT FOR PITA (IDISP6PITA completely spelled out)



The IDISP6PITA command statement can be used to specify the time-slices' initial or seed displacements for the PITA methodology (see <u>PITA</u>), thus overriding the alternative approach consisting of generating this information by applying the basic time-integrator on the coarse time-grid. It is applicable in both linear and nonlinear settings. Its syntax is similar to that of the IDISP6 (see <u>IDISP6</u>) command except that it can introduce up to as many sets of initial displacements as there are time-slices, one set per time-slice. Each set of initial displacements must be input after the previous one, starting with that corresponding to the first time-slice and continuing in consecutive order. All those time-slices for which a set of seed displacement values is not specified are initialized by the aforementioned alternative approach.



NODE#	VAL_DOF1	VAL_DOF2	VAL_DOF3	VAL_DOF4	VAL_DOF5	VAL_DOF6

NUMSLICES Number of consecutive time-slices for which seed displacements are specified (integer).

Node number where the seed displacement is specified (integer).

VAL_DOFi#

Value of the seed displacement for the i-th degree of freedom at the beginning of a time-slice

(float).

Next: ITEMPERATURES, Previous: IDISP6PITA

58 INITIAL OR SEED VELOCITY FOR PITA (IVEL6PITA completely spelled out)

Command Statement: IVEL6PITA

The IVEL6PITA command statement can be used to specify the time-slices' initial or seed velocities for the PITA methodology (see PITA), thus overriding the alternative approach consisting of generating this information by applying the basic time-integrator on the coarse time-grid. It is applicable in both linear and nonlinear settings. Its syntax is similar to that of the IDISP6 (see IDISP6) command except that it can introduce up to as many sets of initial velocities as there are time-slices, one set per time-slice. Each set of initial velocities must be input after the previous one, starting with that corresponding to the first time-slice and continuing in consecutive order. All those time-slices for which a set of seed velocity values is not specified are initialized by the aforementioned alternative approach.

IVEL6PITA NUMSLICES

NODE#	VAL_DOF1	VAL_DOF2	VAL_DOF3	VAL_DOF4	VAL_DOF5	VAL_DOF6
-------	----------	----------	----------	----------	----------	----------

NUMSLICES Number of consecutive time-slices for which seed velocities are specified (integer).

Node number where the seed velocity is specified (integer).

VAL_DOFi# Value of the seed velocity for the i-th degree of freedom at the beginning of a time-slice (float).

Next: IVELOCITIES, Previous: IVEL6PITA

59 INITIAL TEMPERATURES *S*

Command Statement: ITEMPERATURES

The ITEMPERATURES command statement is used to specify a nodal displacement type of initial conditions. Each node can have one degree of freedom.

ITEMPERATURES



NODE# Node number where the initial temperature is specified (integer).

VALUE Value of the specified initial temperature (float).

Next: INPC, Previous: ITEMPERATURES

60 INITIAL VELOCITIES *S*

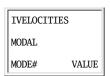
Command Statement:	IVELOCITIES
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The IVELOCITIES command statement is used to specify a nodal velocity type of initial conditions. Each node can have up to six degrees of freedom. For a time-domain acoustic analysis, this command statement can be used to specify nodal values of the initial time-derivative of the solution by setting DOF# to 8 (see below).

IVELOCITIES

N	ODE# I	OOF# VALUE	
	NODE#	Node	number where the initial velocity is specified (integer).
	DOF#	Degree	e of freedom local number where the initial velocity is specified (integer).
	VALUE	Value	of the specified initial temperature (float).

When working in the basis of generalized coordinates (or reduced basis), the format of this command is as follows.



MODAL

This keyword, completely spelled out, can be used either (1) to input the initial solution time-derivatives in the generalized coordinates (or reduced) system, whether a modal or direct time-integration analysis is requested in the DYNAMICS command, or (2) to superpose to the initial velocity conditions specified in the global coordinates system additional initial velocity conditions formulated in the generalized coordinates (or reduced) system (characters). In the second case, IVELOCITIES should be specified only once and the keyword MODAL can be inserted either before or after the set of data associated with the initial velocity conditions specified in the global coordinates system. The READMODE command statement must also be present in the input file to provide the eigenmodes to be used.

MODE# Mode id number for which the initial condition is specified (integer).

VALUE Value of the specified initial condition (real).

Next: MATERIAL, Previous: IVELOCITIES

61 INTRUSIVE POLYNOMIAL CHAOS

Command Statement: INPC

The INPC command signals to the FEM code to perform an intrusive non deterministic analysis using the Polynomial Chaos representation of the solution. It requires that the input file also contains the <u>GROUP</u>, <u>RANDOM</u>, and <u>STATICS</u> commands and their respective data.

Note 1: Currently, this command is limited to linear, static, structural analysis.

Note 2: Currently, this command is supported only by the PCG, BCG, and CR iterative solvers (see STATICS).

The input format of this command is given below.

INPC

DEG_OUT

DEG_OUT Degree of the polynomial chaos representation of the solution (integer).

Next: LMPC, Previous: INPC

62 LINEAR MATERIAL PROPERTIES *S*

Command Statement: MATERIAL

The MATERIAL command statement is used to signal that the following data lines correspond **and/or point to**: (1) definitions of materials and geometric properties for materially linear elements, and/or geometric properties for materially nonlinear elements, and (2) requests for specific methods for enforcing the constraints associated with joint and rigid elements (see <u>CONSTRAINTS</u>). Several input formats are possible for this command and are described below.

Note 1: Even for a given input format, the interpretation of a data entry can change from one element type to another. For example for the first input format, the main interpretation of some data entries changes when the element type is that of a lumped torsional spring, a torsional spring connector, a linear spring connector, a Timoshenko beam, a thermal element, a rigid translational link, or a rigid rotational link.

Note 2: For composite elements, this command is not required unless stress and/or strain output is requested. Even in this case, only the total thickness of the element must be specified under this command. Dummy or zero values can be entered for the other properties as their real values should be specified under the COMPOSITE command.

MATERIAL

MID	A	E	ν	ρ	h	k	t	P	Та	c_p	W	Ixx	Іуу	Izz	ymin	ymax	zmin	zmax
DAMP	ING	а	b															
	MID			The i	mater	ial id	numl	er.										
	Α			Cros	s sec	tional	area	for tr	usses,	beams,	and lii	neal the	mal ele	ments (1	float).			
	E			You	ng's N	Modu	lus fo	or all	elemen	t types (float).	Howe	ver, this	field ca	an also b	e filled by	a nega t	tive
				integ	ger in	whic	h cas	e it m	eans t	nat You	ng's M	Iodulus	is a fun	ction of	f tempera	ature and	is to be	determined
															_		points to	the id of
	_			the Y	oung	g's mo	odulu	s-tem	peratu	re table i	n the	YMTT coi	nmand	(negativ	e integer	r).		
	ν			Poiss	son's	ratio :	for al	l elen	nent ty	pes exce	pt tru	sses (flo	oat).					
	ρ			Mass	s den	sity p	er un	it vol	ume ex	cept for	eleme	ent type	20 (con	nposite	or ortho	tropic she	ell eleme	nt) when
	_			defin	ed us	sing th	ne COI	EF opt	ion, in	which o	ase it	is mass	density	per un	it area (fl	loat).		
	h			Heat	conv	ection	1 coe	fficie	nt (onl	y for bo	undary	y conve	ction ele	ments:	type 47,	48, and 4	49 (float)).
	k			Heat	cond	uction	n coe	fficie	nt (floa	at).								
	t			Thicl	kness	for p	late a	nd sh	nell ele	ments (f	loat).							
	P			Perin	neter	circui	mfere	nce a	rea foi	thermal	elem	ents (flo	at), or t	he deptl	h of the b	ooundary	where c	onvection
				occu	rs wh	nen ele	emen	t type	47 is	used to	nodel	non-lat	eral bou	ndary c	convection	n (float).		
	Ta			Refe	rence	temp	eratu	re (A	bsolute	e) of the	eleme	ent (floa	t). WA l	RNING	G: if this	value is	differen	t from the
				noda	al ten	npera	ture	of th	at eler	nent or	this v	alue is	differe	ıt from	zero an	d no no	dal temp	erature is
				spec	ified,	it cre	eates	a the	ermal l	loading	even	in a pu	re struc	tural a	malysis.			
	C_p			Spec	ific h	eat co	effici	ent ir	a thei	mal ana	lysis (float).						
	W			Coef	ficier	t of tl	herma	al exp	ansior	in eithe	r a the	ermal an	alysis,	or a stru	ıctural ar	alysis if	the speci	ified
				ambi	ent te	mper	ature	Ta is	differe	nt from	the sp	ecified	nodal te	mperatı	ures (see	TEMPE	<u>RATUR</u>	ES and
																		eaning that
									-				-			letermine		
													-	_	_	ints to the		e
								_		_						gative into	_	
	Ixx/	SS						•								s-section		
														•		ne-domain the attribu		
	T									_				-	cipal y-a		uw mumil	.c.i.
	Іуу													•				
	Izz													-	cipal z-a	CIS.		
	ymin			Nega	ative 1	local y	y-coo	rdina	te of th	e bottor	n fibe	r of a be	am cros	s section	on.			

	ymax	Positive local y-coordinate of the top fiber of a beam cross section.
	zmin	Negative local z-coordinate of the top fiber of a beam cross section.
	zmax	Positive local z-coordinate of the top fiber of a beam cross section.
DAMP		Optional sub-command keyword to specify a Rayleigh damping (characters).
a		Rayleigh damping stiffness coefficient (real) for the material identified by MID. In the nonlinear case,
		this coefficient is assigned at each iteration to the linearized stiffness matrix and the product of these two
		quantities, which varies with the nonlinear iterations, constitutes the contribution to the damping term. In a
		structural dynamic analysis, this value overrides for the material identified by MID any value of the
		Rayleigh damping stiffness coefficient specified under DYNAMICS . In any analysis performed using
		the command IMPEDANCE , this value overrides for the material identified by MID any value of the
		Rayleigh damping stiffness coefficient specified under IMPEDANCE .
b		Rayleigh damping mass coefficient b (real). In a structural dynamic analysis, this value overrides for the
		material identified by MID any value of the Rayleigh damping mass coefficient specified under
		<u>DYNAMICS</u> . In any analysis performed using the command <u>IMPEDANCE</u> , this value overrides for the
		material identified by MID any value of the Rayleigh damping mass coefficient specified under
		<u>IMPEDANCE</u> .

If the element is a lumped torsional spring (eltyp = 11), a linear spring connector (eltyp = 21), or a torsional spring connector (eltyp = 22), the material properties are defined as follows.

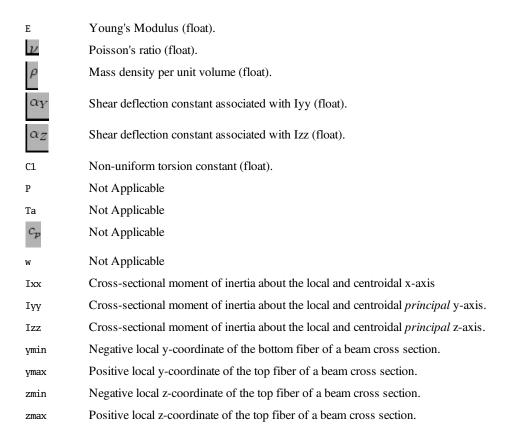
MID	Kx	Ky	Kz	lx1	ly1	lz1	lx2	ly2	lz2	lx3	ly3	lz3
	MID		т	ha mata	riol id n	umbar t	from ele	mant att	ributa t	o bl o		
	MTD		11	ne mate	mai iu ii	umber	HOIH CIC	mem au	moute a	auic.		
	Kx		To	orsional	/Transl	ational	spring c	onstant	along lo	ocal x-a	xis (floa	ıt).
	Ky		To	orsional	/Transl	ational	spring c	onstant	along lo	ocal y-az	xis (floa	ıt).
	Kz		To	orsional	/Transl	ational	spring c	onstant	along lo	ocal z-ax	xis (floa	t).
	lx1 ~	- ly1 ~ l	z1 T	he first	axis of	the loca	1 frame	express	ed in the	e global	frame (floats).
	lx2 ~	- ly2 ~ 1	z2 Tl	he first	axis of	the loca	1 frame	express	ed in the	e global	frame (floats).
	lx3 ~	- ly3 ~ 1	z3 Tl	he first	axis of	the loca	1 frame	express	ed in the	e global	frame (floats).

If the element is a Timoshenko beam (eltyp = 7), the material properties are defined as follows.



MID The material id number from element attribute table.

A Cross sectional area (float).



If the element is a rigid translational or rotational link (eltyp = 68 or 69), the material properties are defined as follows.

MID x y z dum

MID The material id number from element attribute table.

x Any nonzero value implies a rigid motion in this direction (float).

y Any nonzero value implies a rigid motion in this direction (float).

z Any nonzero value implies a rigid motion in this direction (float).

dum Any dummy value (float).

If the element is a joint (eltyp = 120, 122, 123, 124, or 125) or rigid element (eltyp = 65, 66, 67, 68, 69, 70, 71, 73, or 74), the method for enforcing the associated constraints (see <u>CONSTRAINTS</u>) can be specified as follows.

MID CONSTRAINT_METHOD

MID The material id number from element attribute table.

CONSTRAINT_METHOD Specified method for enforcing the constraints associated with the joint or rigid elements sharing the

material id number MID (characters). The default method is set in **CONSTRAINTS** and used whenever

this entry is omitted.

The Lagrange multiplier method.

multipliers

The elimination method.

elimination

penalty The penalty method. The parameter beta should be a large positive number, typically of the order of 10^8

[beta]

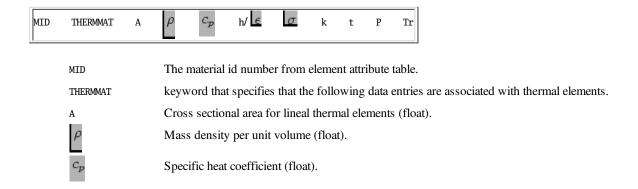
CIRCULAR_FREQUENCY

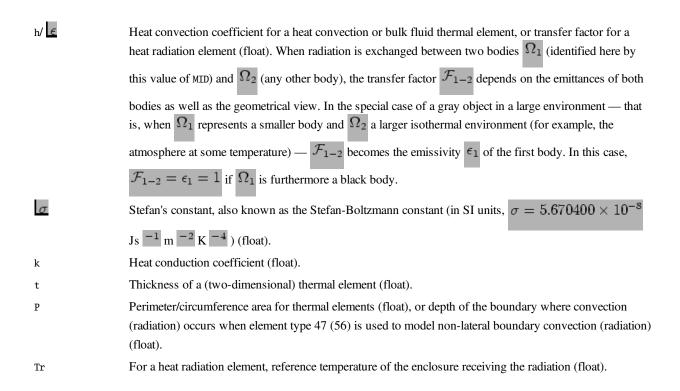
(no default value is provided).

If the element is an actuated joint (eltyp = 126 or 127), the material properties may be defined as follows.

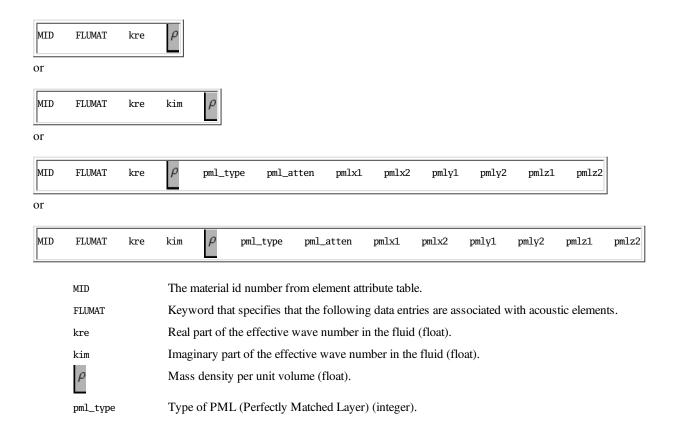
MID	CONSTRAINT_METHOD	AMPLITUDE CIRCULAR_FREQUENCY
	MID	The material id number from element attribute table.
	CONSTRAINT_METHOD	Specified method for enforcing the constraints associated with the actuated joint elements sharing the material id number MID (characters). The default method is set in CONSTRAINTS and used whenever this entry is omitted.
multip	pliers	The Lagrange multiplier method.
elimin	nation	The elimination method.
[beta	penalty a]	The penalty method. The parameter beta should be a large positive number, typically of the order of 10^8 (no default value is provided).
	AMPLITUDE	Amplitude of the harmonic actuation (float).
		Circular frequency of the harmonic actuation in radians/sec (float).

If the element is a thermal element (eltyp = 3, 10, 46–51, 53, 56–58, 81–86, 4646), the material properties are defined as follows.





If the element is an acoustic element (eltyp = 31–45, 63, 90, 93–96, 98–108), the material properties may be defined as follows.



pml_type = 1 designates a box PML. In this case the PML elements are assumed to be in the region defined by pmlx1 < x < pmlx2, -pmlx2 < x < -pmlx1, pmly1 < y < pmly2, -pmly2 < y < -pmly1, pmlz1 < z < pmlz2, and -pmlz2 < z < -pmlz1.

pml_type = 2 designates a spherical PML. In this case the PML elements are assumed to be in the region defined by pmlx1 < r < pmlx2, where r = sqrt(x*x+y*y+z*z).

 $pml_type = 3$ designates a cylindrical PML. In this case the PML elements are assumed to be in the region defined by pmlx1 < r < pmlx2, where r = sqrt(x*x+y*y), pmlx1 < z < pmlx2, and -pmlx2 < z < -pmlx1.

pml_atten PML attenuation parameter (float).

If the element is a fabric truss element (eltyp = 111), the material properties are defined as follows (see Powell, D.A. and Zohdi, T.I. Attachment mode performance of network-modeled ballistic fabric shielding. *Composites: Part B* 2009; 40: 451-460).

MID	FABMAT	type	E	ρ	A	U_c	U_f	λ	h	d	δλ	n_p	N_f	
	MID		The	materia	al id nı	umber f	rom elen	nent attri	bute ta	ıble (i	nteger).			
	FABMAT		Key	word s	pecify	ing that	the follo	wing da	ıta entı	ries ar	e associa	ted with	fabric elen	nents (charac
	type						-							y determined g Gaussian d
	E						g's modu us for the		he fibi	ils tha	ıt make u	ıp the ya	rn (float). I	f type = 2, it
	ρ		Ma	ss dens	ity per	unit vo	lume of	the fibril	/yarn	(float)				
	A		Cross sectional area of the entire yarn element (float).											
	U_c		If $type = 1$, this is the breaking stretch (l/l_0) of the fibrils (float). If $type = 2$, it is the stretch level which the yarn begins to damage.											
	U_f		Stre	etch leve	el at w	hich the	entire y	arn has f	failed ((i.e. th	e stress	response	e is approxi	mately zero)
	λ		valı	ie is po	sitive a	and dete	•	y Newt	on's m	ethod	(float).	If type =	•	f the yarn; it slope of the
	h						igth of ar age parai					-	e truss eler	ment) (float).
	d		fibr		at). If	type =								(misalignme d Gaussian d
	$\delta\lambda$				•		er is set to ian distri		_	•	,		is the stand	dard deviatio
	n_p						aximum r). If type						g the dama	ige paramete
	N_f								-		_		pe = 2, this	s parameter i

Next: LOAD, Previous: MATERIAL

63 LINEAR MULTIPOINT CONSTRAINTS

Command Statement: LMPC

The LMPC command statement is used to specify a set of linear multipoint constraint equations of the form

$$\sum_{j=1}^{j=n} c_{ij} u_j = r_i, \quad i = 1, 2, \dots$$

in the structural model. There is no limitation on the number of multipoint constraints, or number of degrees of freedom related by the same constraint equation.

The format of this command statement is as follows.

Note 1: The Lagrange multiplier method for enforcing the constraints associated with this command is supported only by the FETI-DP family of solvers, the GMRES solver, and the SPOOLES and MUMPS direct sparse solvers with pivoting enabled, and in all but explicit dynamic analyses (see <u>STATICS</u>).

Note 2: For time-dependent problems, the specified initial conditions must verify the specified linear multipoint constraints.

LMPC

CONSTRAINT# RHS CONSTRAINT_METHOD NODE# DOF# COEFF

.
.
NODE# DOF# COEFF

CONSTRAINT# This corresponds to the constraint equation number i (integer).

This is the right-hand side r_i of the *i*-th constraint equation (float).

This is the method for enforcing the constraint (characters). The default method is set in <u>CONSTRAINTS</u> and used whenever this entry is omitted.

The Lagrange multiplier method.

multipliers

The elimination method.

elimination

The penalty method. The parameter beta should be a large positive number, typically of the order of 10^8 (no

penalty [beta] default value is provided).

This is the number of the node contributing the coefficient c_{ij} of the *i*-th constraint equation (integer).

This is the local number of the degree of freedom at the node specified above contributing the coefficient c_{ij} of the *i*-th constraint equation (integer).

This is the coefficient c_{ij} of the *i*-th constraint equation (float).

Next: LUMPED, Previous: LMPC

64 LOAD

Command Statement: LOAD

The LOAD command statement is used to inform FEM where the user defined subroutines for user defined forces and/or displacements and/or control are located. An example input file using the LOAD command can be found in FEM.d/fem_examples/Control.d

LOAD pathandfilename

pathandfilename Specifies between quotes "" the path and filename of the LOAD file. The extension of this file must

be ".so".

Next: MASS, Previous: LOAD

65 LUMPED

Command Statement: LUMPED

By default, **AERO-S** computes all element mass matrices and gravity (see <u>GRAVITY</u>) loads by a consistent approach, **except for explicit dynamic computations** (see <u>DYNAMICS</u>), in which case **AERO-S** always uses a lumped approach for this purpose. If a consistent mass matrix is not available for a particular element (see <u>TOPOLOGY</u>), then **AERO-S** uses in all cases a lumped mass matrix and gravity load for that element.

Alternatively, this command can be used to instruct **AERO-S** to compute all element mass matrices and gravity loads by a lumping method.

LUMPED

Next: MATLAW, Previous: LUMPED

66 MASS EVALUATION



The MASS command statement is used to signal that the user would like that the total mass of the structural system be computed. The result is output on the screen by FEM. In addition to the total mass of the structure, the center of gravity (cg), the center of volume, and the closest node to the cg are also computed and printed on the screen.

Next: MATUSAGE, Previous: MASS

67 MATERIAL LAW

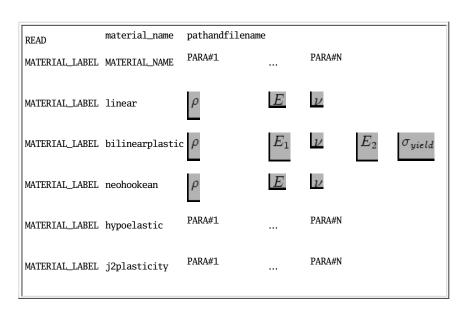
Command Statement: MATLAW

The MATLAW command statement can be used to define one or several general material laws, whether they are linear or nonlinear. Typically, the user will write his/her own software describing a material law and package it in a library with a ".so" extension. The MATLAW command is then used to label this material law and pass to its library the parameters it expects. The user should also grab the special makefile and include files for this command which are located within the AERO-S.d/Matlaw.d directory.

The MATLAW command can also be used to specify two material laws that have been predefined and implemented in **AERO-S**. These are the "linear" material law, and the "bilinplast" material law (bilinear plastic material).

The input format of this command is given below.

MATLAW



READ

Sub-command keyword used to introduce a material name (characters) and associate it with a library to be found at "pathandfilename.so" (characters) and linked with **AERO-S**. The path and filename of the material law library file must be specified between quotes (""). The extension of

this file must be .so.

MATERIAL_LABEL The material law id number (integer).

MATERIAL_NAME Name of the material law (characters).

PARA#1 First parameter expected by the library pathandfilename.so.

PARA#N Last parameter expected by the library pathandfilename.so.

linear Name of the predefined usual linear material law.

 ρ Mass density per unit volume (real).

E Young's Modulus (real).

Poisson's ratio (real).

 E_1 Young's Modulus for the bilinear plastic material law (real).

 E_2 Second slope for the bilinear plastic material law (real).

 σ_{yield} Yield stress for the bilinear plastic material law (real).

PARA#1 Young's modulus (real).
PARA#2 Poisson's ratio (real).
PARA#3 Mass density (real).

PARA#4 Yield stress (j2plasticity) (real).

PARA#5 Hardening modulus (j2plasticity) (real).

PARA#19 Shell shear correction factor (real).

PARA#20 Shell thickness (real).

Next: MODE, Previous: MATLAW

68 MATERIAL LAW USAGE

Command Statement: MATUSAGE

The MATUSAGE command statement can be used to assign a material law specified in the MATLAW command to one or several elements.

The input format of this command is

MATUSAGE

ELEMENT# MATLAW_LABEL#

or

STARTING_ELEMENT#	ENDING_ELEMENT#	MATLAW_LABEL#	
ELEMENT#	Element nur	mber whose materi	l law number is to be specified (integer).
MATLAW_LABEL	Material lav	identification nun	ber (integer).
MATERIAL_LABEL	First elemen	nt of a sequence of	elements that have the same MATLAW_LABEL (integer).
MATERIAL_NAME	Last elemen	t of a sequence of	lements that have the same MATLAW_LABEL (integer).

Next: NODALCONTACT, Previous: MATUSAGE

69 SAVING EIGENMODES OR PROJECTING ONTO EIGENVECTORS

Command Statement: MODE

The MODE command is used to signal that: (a) when the EIGEN command is used, the eigen solutions are to be saved in a binay file named EIGENMODES (or binary files EIGENMODES# when using AERO-S in distributed memory mode with one file per MPI process) and located in the execution path of AERO-S, (b) when the DYNAMICS command is used, the binay file named EIGENMODES (or binary files EIGENMODES# when using AERO-S in distributed memory mode with one file per MPI process) located in the execution path of AERO-S is read, and the projections of the transient solution onto the eigen modes found in that file are computed (and outputted if requested in the OUTPUT command), and (c) when the IMPEDANCE command is used with FREQSWEEP, PADEPOLES and ALG = PadeLanczos, the binay file named EIGENMODES (or binary files EIGENMODES# when using AERO-S in distributed memory mode with one file per MPI process) located in the execution path of AERO-S is read, and the eigen modes stored in this (or these) file(s) are exploited to identify within a range of interest the eigenvalues missed by a previous eigen computation and their corresponding eigenvectors.

Note 1: The EIGENMODES file is written using the "internal" numbering of **AERO-S** for the degrees of freedom. This internal numbering depends on the equation solver and renumbering scheme that were specified under the <u>STATICS</u> and <u>RENUMBERING</u> commands, respectively, when the eigen solutions were computed. Hence, when reusing the EIGENMODES file for computing the projections of the transient solution of a problem onto the vectors contained in this file, the same equation solver and renumbering scheme must be specified under the <u>STATICS</u> and <u>RENUMBERING</u> commands, respectively.

Next: NODES, Previous: MODE

70 NODAL CONTACT

Command Statement: NODALCONTACT

The NODALCONTACT command can be used to specify node-to-node contact of the form

$$(u_2 - u_1) \cdot n_{12} = 0$$

or

$$(u_2 - u_1) \cdot n_{12} \ge gap$$

where u denotes the displacement field, n_{12} denotes the normal to the contact surface oritented from node 1 to node 2, and gap denotes the initial gap. The input format of this command can be as follows.

Note 1: For $mode_v = 1$ and $mode_v = 3$, the enforcement of the nodal contact constraints by the Lagrange multiplier method is supported only by the FETI-DP family of solvers, and for static and implicit dynamic analyses only.

Note 2: For $mode_v = 1$ and $mode_v = 3$, the enforcement of the nodal contact constraints by the elimination method is not supported.

Note 3: For $mode_v = 0$ and $mode_v = 2$, the enforcement of the nodal tied contact constraints by the Lagrange multiplier method is supported only by the FETI-DP family of solvers, the GMRES solver, and the SPOOLES and MUMPS direct sparse solvers with pivoting enabled, and in all but explicit dynamic analyses.

NODALCONTACT

or

NODALCONTACT MODE default_mode_v

followed by one or more lines of the form

NODE_1 NODE_	2 N_X	N_Y	N_Z
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or

NODE_1	NODE_2	N_X	N_Y	N_Z	GAP	gap_v

NODE_1	NODE_2	N_X	N_Y	N_Z	MODE	mode_v
1						

NODE_1	NODE_2	N_X	N_Y	N_Z	MODE	mode_v	GAP	gap_v
1.1022	ozz						0.11	84P-1

NODE_1	NODE_2	N_X	N_Y	N_Z	MODE	mode_v	GAP	gap_v
CONSTRAINT_	_METHOD							

	NODE_1	Id number of the first node (integer).
	NODE_2	Id number of the second node (integer).
	N_X	x-component of the normal to the contact surface oriented from NODE_1 to NODE_2 (float).
	N_Y	y-component of the normal to the contact surface oriented from NODE_1 to NODE_2 (float).
	N_Z	z-component of the normal to the contact surface oriented from NODE_1 to NODE_2 (float).
	GAP	Keyword to be spelled out (characters).
	gap_v	Initial gap between NODE_1 and NODE_2. A negative gap means an initial penetration (float).
	MODE	Keyword to be spelled out (characters).
	mode_v	Specifying $mode_v = 0$ designates a tie-of-the-normal-component-of-the-displacement constraint.
		Specifying mode_v = 1 designates a normal contact constraint. Specifying mode_v = 2 designates a tie-of-
		the-normal-and-tangential-components-of-displacment constraint. Specifying mode_v = 3 desinates both
		normal contact and tie-of-the-tangential-components-of-displacement constraints (integer).
	default_mode_v	Default mode_v for all constraints. If not specified, the default mode is set to 1, i.e. normal contact
		(integer).
	CONSTRAINT_METHOD	This is the method for enforcing the associated constraints (characters). The default method is set in
		<u>CONSTRAINTS</u> and used whenever this entry is omitted.
		The Lagrange multiplier method.
multip.	liers	
elimin	ation	The elimination method.
CIIMII	penalty	The penalty method. The parameter beta should be a large positive number, typically of the order of 10^8
[beta]		(no default value is provided).
	-	

Next: NONINPC, Previous: NODALCONTACT

71 NODES *S*

Command Statement:	NODES
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The NODES command statement is used to signal that the following data lines correspond to the coordinates of each node. The input data format is given below. Note that there should be as many lines as the number of nodes in the mesh. If the Y-ordinate and Z-ordinate are not given for a node, FEM assumes they are zero. Similarly, if the Z-ordinate is not given, FEM assumes it is zero.

NODES

NODE#	X-ORDINATE	Y-ORDINATE	Z-ORDINATE		
N	ODE#	Node number w	hose coordinate	s are to be specified (integer
X	-ORDINATE	X-ordinate of th	e given node nu	mber (float).	
Y	-ORDTNATE	Y-ordinate of th	e given node nu	mber (float).	

Z-ORDINATE

Z-ordinate of the given node number (float).

Next: NONLINEAR, Previous: NODES

72 NON INTRUSIVE POLYNOMIAL CHAOS

Command Statement: NONINPC

The NONINPC command signals to the FEM code to perform a non intrusive uncertainty quantification analysis using the Polynomial Chaos (PC) representation. It requires that the input file also contains the <u>GROUPS</u>, <u>RANDOM</u>, and <u>STATICS</u> commands as well as their respective data. Essentially, FEM computes in this case the mean and standard deviation of a computed primal field v as follows

$$\bar{v} = \int_{-\infty}^{+\infty} vp(v)dv = v_0 = \frac{\sum_{i=1}^{N} v^{(i)}}{N}$$

$$\sigma_v = \sqrt{\int_{-\infty}^{+\infty} (v - \bar{v})^2 p(v)dv} = \sqrt{\sum_{i=1}^{i=P-1} \left(\int_{-\infty}^{+\infty} \psi_i^2 p(\psi_i(\{\xi_j\})) d\psi_i \right) v_i^2}$$

$$= \sqrt{\sum_{i=1}^{i=P-1} \left(\int_{-\infty}^{+\infty} \psi_i^2 p(\psi_i(\{\xi_j\})) d\psi_i \right) \left(\frac{\sum_{j=1}^{N} v^{(j)} \psi_i^{(j)}}{N \int_{-\infty}^{+\infty} \psi_i^2 p(\psi_i) d\psi_i} \right)^2}$$

where v_0 denotes the constant of the PC expansion of the non deterministic v_i field ("the output"), v_i denotes the number of realizations, v_i denotes the deterministic solution of the field v_i associated with the i-th realization of the random system, v_i denotes the probability density function, v_i denotes the j-th "basic" random variable which is assumed to be Gaussian, v_i denotes the deterministic coefficient of the i-th PC shape function v_i , v_i denotes the total number of PC shape functions v_i representing the non deterministic output, and v_i denotes the evaluation of v_i at the j-th realization of the random system.

Note 1: Currently, this command is limited to the post-processing of the results of a linear, static, structural analysis.

The input format of this command is given below.

NONINPC

DEG_OUT NUM_RLZ

DEG_OUT Degree of the PC representation of the output which, together with the data of the <u>RANDOM</u> command,

determines P

NUM_RLZ Number of realizations N of the random system (which implies the number of deterministic solutions)

(integer).

Next: OUTPUT, Previous: NONINPC

73 NONLINEAR

Command Statement: NONLINEAR

The NONLINEAR command signals to the **AERO-S** code that the current statics or dynamics analysis is to be nonlinear. For structural problems, the nonlinear analysis is by default a geometrically nonlinear analysis where the tangent stiffness matrix and/or internal force is built using the Corotational Finite Element Method for some elements and the Lagrangian Method for others. For structural problems, materially and geometrically + materially nonlinear analyses are also possible but require, in addition to this command, the selection under the TOPOLOGY command of element types that support such analyses. Also for structural problems, material nonlinearities can be specified and described using the MATLAW command, among others. For nonlinear static and implicit dynamic analyses, only Newton-like methods are available.

Note 1: Currently, geometric stiffnesses are implemented only for the bar, beam, 3-noded shell and composite shell, 4-noded shell and composite shell, 4-noded tetra, 8-noded brick, spring, and rigid elements (via their conversion to linear multipoint constraints).

Note 2: Currently, the list of materially nonlinear elements includes only an 8-noded brick element.

Note 3: Currently, the list of geometrically + materially nonlinear elements includes only an 8-noded brick element.

The input format is given below.

NONLINEAR

METHOD (keyword)

PARAMETERS (keyword and/or value)

METHOD

arclength Selects the arclength method for solving the nonlinear equations associated with a nonlinear static

or an implicit dynamic computation. If this option is not selected, then the default algorithm for

solving the nonlinear quations is a standard Newton-Raphson iteration method (string).

PARAMETERS

maxitr	For a nonlinear static or an implicit dynamic computation, specifies the maximum number of
	Newton-like iterations to be performed (per time-step) (integer). The default value is 100.
nltol	For a nonlinear static or an implicit dynamic computation, specifies the tolerance for convergence

of the Newton-like method (float). The default value is 1e-06.

dlambda float float Specifies the loading parameters to use in a nonlinear static or implicit dynamic analysis. The first

float entered is the load fraction increment, and the second float is the total load factor. For example, "dlambda $0.25\ 1.0$ " would mean to run 4 load steps each applying an incremental load of $0.25*(Applied\ Force)$ per load step. On the other hand, "dlambda $0.25\ 2.0$ " means 8 load steps each applying an incremental load of $0.25*(Applied\ Force)$ per load step. This can help the

convergence of Newton-Raphson.

fitalg For structural problems, specifies the fitting algorithm for the corotational formulation. 8 or fitalg

= 1 (default value) the tangent stiffness matrix 8s not consistent but the corotational framework is

most robust. For fitalg = 2, the tangent stiffness matrix is consistent (integer).

nlmat For structural problems, specifies a materially or geometrically + materially nonlinear analysis. In

that case, appropriate element types must be specified in the TOPOLOGY command. If this keyword is

absent, the NONLINEAR command requests a geometrically nonlinear analysis (string).

Next: OUTPUT6, Previous: NONLINEAR

74 OUTPUT OF RESULTS (OUTPUT completely spelled out)

Command Statement: OUTPUT

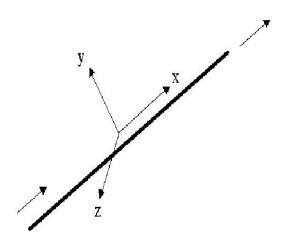
The OUTPUT command statement is used to signal that the following lines of data correspond to specifying which results are to be outputted. For vector results such as displacement fields, this command forces **AERO-S** to output only the three translational components.

The shared memory version of **AERO-S** generates ASCII output files. Most of these are in a format that is suitable for postprocessing by the **XPost** sofware, but some are in a format that is suitable for postprocessing by **gnuplot**. The distributed memory version of **AERO-S** generates binary output files except for the case of selective nodal output which it generates in ASCII format. The **SOWER** software can be used to convert the binary output files into ASCII output files, in a format suitable for postprocessing by **XPost**. All "nodal" output files are generated in the **gnuplot** format. Examples using the various OUTPUT commands can be found in FEM.d/fem_examples/Output.d/.

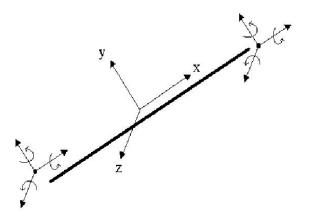
Note 1: In addition to the time-instances implied by the specified value of the parameter INCREMENT, any result specified for output is also outputted at the beginning of a simulation if INCREMENT is non zero, and at the of a simulation if **AERO-S** exits gracefully.

Note 2: The strain and stress outputs are not available for the bar and beam elements, except for STRAINXX and STRESSXX

Note 3: The internal forces and moments and STRAINXX and STRESSXX for the bar and beam elements are outputted in the local frames using the sign conventions graphically depicted below.



Sign conventions for the output of the internal forces in a bar element



Sign conventions for the output of the internal forces in a beam element

Note 3: For nonlinear structural analysis, the stresses and strains for the corotational elements are outputted on the deformed configuration and in the local frames.

Note 4: For nonlinear structural analysis, the stresses and strains for the total Lagrangian elements are outputted on the undeformed configuration and in the global frame.

Note 5: For nonlinear structural analysis, the internal forces and moments and STRAINXX and STRESSXX for the bar and beam corotational elements are outputted in the local frames associated with the deformed configuration.

Note 6: When multiple outputs are expected, they are printed one set after the other in the same output file.

Note 7: To visualize the directions of principal strains and stresses, the auxiliary code PDIREC should be used to generate the node set and element set used by **XPost** for this purpose. When runnig PDIREC, the user is asked to enter the name of the mesh **XPost** file (for example, the one obtained from executing **AERO-S** with the "-t" command), a **AERO-S** result file, the type of that result for labeling purposes (eg. SP1DIREC), and a characteristic length to be used for scaling the length of the elements representing the directions to be visualized. PDIREC can also be used for visualizing any other vector output (eg. GDISPLAC or GVELOCIT).

Note 8: The vocabulary used for the request of stresses and strains that are associated with the elements listed below differs from the standard continuum mechanics notation. See below for specific notations.

The input format of this command can be as follows.

_	
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Юl	TEUL

RESULT	[FORMAT]	PATHANDFILENAME	INCREMENT	[NODE_NUM BER]	[OPTION]	[]	
[OPTION]							

or

RESULT	[FORMAT]	PATHANDFILENAME	INCREMENT	[N NODE_NUMBER]	[OPTION]	[]	
[OPTION]							

or

RESULT	[FORMAT]	PATHANDFILENAME	INCREMENT	[NG GROUP_NUMBER]	[OPTION]	[]
[OPTION]						

RESULT	The following results can be outputted:
GDISPLAC	Generalized nodal displacements.
DISPLACX	Nodal displacements in the \mathcal{I} direction.
DISPLACY	Nodal displacements in the \boldsymbol{y} direction.
DISPLACZ	Nodal displacements in the z direction.
ROTATIOX	Nodal rotations in the \mathcal{I} direction.
ROTATIOY	Nodal rotations in the y direction.
ROTATIOZ	Nodal rotations in the \mathbb{Z} direction.
DISPLMOD	Euclidean norms of the nodal displacements.
ROTATMOD	Euclidean norms of the nodal rotations.
GDISPMOD	Euclidean norms of the generalized nodal displacements.
GVELOCIT	Nodal velocities.
GACCELER	Nodal accelerations.

GTEMPERA Nodal temperatures.

GTEMPVEL Nodal first time-derivatives of the temperature.

Strains in the global xyz-axes.

GEIGENPA Eigenvalues and Eigenvectors. If singular modes are found during an eigen analysis, they are saved in the

file FNAME associated with GEIGENPA before the non-singular eigen results.

GEIGSLSH Sloshing Eigenvalues and Eigenvectors (displacement potential). If a singular mode is found during the

analysis, it is saved in the file FNAME associated with GEIGSLSH before the non-singular eigen results.

SLSHDISP True fluid nodal displacements associated with the computed sloshing modes.

STRAINXX Strains in the global xyz-axes.

STRAINYY Strains in the global xyz-axes.

STRAINZZ Strains in the global xyz-axes.

STRAINXY Strains in the global xyz-axes.

STRAINYZ Strains in the global xyz-axes.

STRAINVM von Mises strain.

STRAINZX

EFFPSTRN Effective plastic strain. Currently, this result is available only for the Belytschko-Tsay element (type

= 16)

STRAINP1 First principal strains.

STRAINP2 Second principal strains.

STRAINP3 Third principal strains.

EP2DIREC Directions of first principal strains.

EP2DIREC Directions of second principal strains.

EP3DIREC Directions of third principal strains.

STRESSXX (Second-Piola) stresses in the global xyz-axes.

STRESSYY (Second-Piola) stresses in the global xyz-axes.

STRESSZZ (Second Piola) stresses in the global xyz-axes.

STRESSXY (Second Piola) stresses in the global xyz-axes.

STRESSYZ (Second Piola) stresses in the global xyz-axes.

STRESSZX (Second Piola) stresses in the global xyz-axes.

STRESSXX (Second Piola) stresses in the global xyz-axes.

STRESSXM (Second Piola yon Mises stress in the element.

STRESSP1 (Second Piola) first principal stresses. (Second Piola) second principal stresses. STRESSP2 STRESSP3 (Second Piola) third principal stresses. Directions of first principal stresses. SP1DIREC SP2DIREC Directions of second principal stresses. SP3DTREC Directions of third principal stresses. HEATFLXX Temperature flux along global x. HEATFLXY Temperature flux along global y. HEATFLXZ Temperature flux along global z.

GRDTEMPX Temperature gradient along global x.

GRDTEMPY Temperature gradient along global y.

ENERGIES

GRDTEMPZ

Temperature gradient along global z.

INXFORCE

Internal forces along local x-axis.

INXFORCE

Internal forces along local y-axis.

INXFORCE

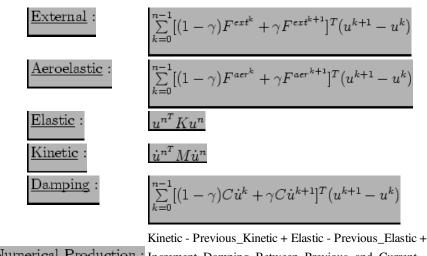
Internal forces along local z-axis.

AXMOMENT Applied (internal) moments along local x-axis.

AYMOMENT Applied (internal) moments along local y-axis.

AZMOMENT Applied (internal) moments along local z-axis.

External, Aeroelastic, Elastic, Kinetic, Damping, and Numerical Production energies. The Numerical Production energy corresponds to the energy conservation error for a conservative system. When positive, it suggests a numerically unstable scheme. When negative, it suggests the presence of numerical damping in the scheme. Using the standard notation and that of the DYNAMICS section, these energies are computed as follows:



Numerical Production:

Kinetic - Previous_Kinetic + Elastic - Previous_Elastic +

Increment_Damping_Between_Previous_and_Current

Increment_Exterior_Between_Previous_and_Current

AEROFORX Aeroelastic nodal forces along global x-axis at t^{n+1} , after application of the boundary conditions (see DYNAMICS).

AEROFORY Aeroelastic nodal forces along global y-axis at t^{n+1} , after application of the boundary conditions (see <u>DYNAMICS</u>).

AEROFORZ Aeroelastic nodal forces along global z-axis at t^{n+1} , after application of the boundary conditions (see DYNAMICS).

AEROMOMX Aeroelastic nodal moments along global x-axis at t^{n+1} , after application of the boundary

conditions (see <u>DYNAMICS</u>).

AEROMOMY Aeroelastic nodal moments along global y-axis at t^{n+1} , after application of the boundary

conditions (see $\underline{DYNAMICS}$).

AEROMOMZ Aeroelastic nodal moments along global z-axis at t^{n+1} , after application of the boundary

conditions (see **DYNAMICS**).

RAEROFOR Aeroelastic resultant forces along global x, y, and z axes (integrated over the wet surface of the

structure) at t^{n+1} , before application of the boundary conditions (see <u>DYNAMICS</u>).

RAEROTFL Fluid heat fluxes transferred to the structure at t^{n+1} .

Nodal values of the acoustic pressure computed by an acoustic analysis in the time domain. Nodal values of the first time-derivative of the acoustic pressure computed by an acoustic analysis in the time domain. RIROBOFF Acoustic pressure field at locations specified using the KIRLOC command and evaluated using the KIRLOC command and evaluated using the KIRLOC command and evaluated using the Command HSCB. The corresponding output file contains on each line five words of the form: X-ORDINATE_1 are the X, Y, and Z coordinates of the 1-th point specified using the command HSCB. The corresponding output file contains on each line five words of the form: X-ORDINATE_1 are the X, Y, and Z coordinates of the 1-th point specified using the command KIRLOC. RFAI(U_1) and IMAG(U_1) x or the real and imaginary parts of the acoustic pressure field computed at the 1-th specified point using the Kirchhoff integral method. FARFIELD FA	GHELMHOL	Real or complex acoustic pressure scattered field computed by an acoustic analysis in the frequency domain. The real part is always output first, then, if it is non uniformly zero, the imaginary part is output as a separate frame but in the same output file.
the time domain. Notal values of the second time-derivative of the acoustic pressure computed by an acoustic analysis in the time domain. KIRCHHOFF Acoustic pressure field at locations specified using the KIRLOC command and evaluated using the Kirchhoff integral. This result can be outputted only if the surface of the scatterer is defined using the command HSCB. The corresponding output file contains on each line five words of the form: X-ORDINATE_I are the X, Y, and Z-ORDINATE_I Are the DINATE_I IREAL(U_I) IMAG(U_I). X, Y, and Z-ORDINATE_I are the X, Y, and Z-ORDINATE_I are the X, Y, and Z-ORDINATE_I specified using the command KIRLOC, REAL(U_I) and IMAG(U_I) are the real and imaginary parts of the acoustic pressure field computed at the I-th specified point using the Kirchhoff integral method. FARFIEID Farfield pattern of the acoustic pressure field. This field can be output only if the HSCB command is also specified. In this case, the output file contains on each line four words of the form " a low specified. In this case, the output file contains on each line four words of the form " a low specified. In this case, the output file contains on each line four words of the form " a low specified. In this case, the output file contains on each line four words of the form " a low specified. In this case, the output file contains on each line four words of the form " a low specified. In this case, the output file contains on each line four words of the form " a low specified. MCDECORD Projections using a mass-based metric of a transient solution on the eigen modes saved by the MODE command in the file EIGENMODES. This field can be output only if the MODE command is also specified. MCDEEROR Relative truncation error using the two-norm of the representation of a transient solution by its projection on the eigen modes saved by the MODE command in the file EIGENMODES. This field can be output only if the MODE command is also specified. MCDALENE MCDALENE MODALENE MODALENE MODALENE MODALENE MODALENE MO	GACOUPRE	Nodal values of the acoustic pressure computed by an acoustic analysis in the time domain.
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	TANGENTIAL_TRACTION_MAG	
	CDIRNORX	

CDIRNORY y component of the normal direction for the constraint at a node. If multiple constraints exist at a

node, the value is for the last constraint.

CDIRNORZ z component of the normal direction for the constraint at a node. If multiple constraints exist at a

node, the value is for the last constraint.

CDIRTANX x component of the tangential direction for the constraint at a node. If multiple constraints exist at a

node, the value is for the last constraint.

CDIRTANY y component of the tangential direction for the constraint at a node. If multiple constraints exist at a

node, the value is for the last constraint.

CDIRTANZ z component of the tangential direction for the constraint at a node. If multiple constraints exist at a

node, the value is for the last constraint.

SLIP_MAG Incremental slip at a node. If multiple constraints exist at a node, the value is for the last constraint.

NODAL_DISSIPATION Frictional energy dissipated at a node.

CONTACT_AREA Contact area for a node.

GAP_CUR Normal gap for the current time. If multiple constraints exist at a node, the value is for the last

constraint.

GAP_OLD Normal gap from the previous time. If multiple constraints exist at a node, the value is for the last

constraint.

FORMAT This optional field can be used to override the default format for outputting a numerical value by one

which consists of 2 integer numbers F and P separated by a blank (integers). F specifies the field width — that is, the total number of digits to be printed including the decimal point, the "E" of the exponent and the sign of the exponent, but excluding the sign of the output value. P is the number of digits after the decimal and before the exponent. For example, 8 2 results in printing the output value

1523 as 0.15E+03.

FNAME User specified filename associated with result (characters).

INCREMENT Output increment for transient and eigenvalue results (integer). For a FARFIELD output result, this

field specifies, in the three-dimensional case, the number of longitudinal directions where the farfield is to be evaluated. In this case, (INCREMENT/2)+1) latitudinal directions are also considered and therefore the farfield is evaluated at ((INCREMENT/2)+1))*INCREMENT points uniformly distributed in spherical coordinates. In two-dimensions, the farfield is evaluated at INCREMENT points that are evenly distributed on a circle. For transient and quasi-static analyses, the final time-instance is always outputted for the requested fields — except for the aeroelastic forces and moments — independently

of the value of INCREMENT.

N Sub-command keyword (character) signaling that the next entry is a node identifying number.

NODE_NUMBER When this optional field is used, with or without the sub-command keyword N, only the computed

results associated with the node NODE_NUMBER are outputted in the same format as otherwise, except that the header of the **XPost** file is ommitted for easier use of a visualization software such as

gnuplot (integer).

NG Sub-command keyword (characters) signaling that the next entry is a group identifying number (see

GROUPS).

GROUP_NUMBER When this optional field is used together with the sub-command keyword NG, only the computed

results associated with the nodes belonging to the group of nodes GROUP_NUMBER (see <u>GROUPS</u>) are outputted in the following format which is suitable for a visualization software such as gnuplot

(integer).

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FLOAT-1

NODE_NUMBER-1 X Y Z RESULT-1 RESULT-2 ... RESULT-n

NODE_NUMBER-2 X Y Z RESULT-1 RESULT-2 ... RESULT-n

FLOAT-2

NODE_NUMBER-1 X Y Z RESULT-1 RESULT-2 ... RESULT-n

NODE_NUMBER-2 X Y Z RESULT-1 RESULT-2 ... RESULT-n

...

where FLOAT-i is an i -th time-instance in the case of a dynamic analysis, frequency-value (number of cycles per second) in the case of a structural dynamics frequency sweep analysis (see IMPEDANCE), or wavenumber in the case of an acoustic (Helmholtz) frequency sweep analysis (see <u>HELMHOLTZ</u>), NODE_NUMBER-j is the J -th node number of group GROUP_NUMBER, X, Y

and Z are the \boldsymbol{z} , \boldsymbol{y} , and \boldsymbol{z} coordinates of NODE_NUMBER-j, and RESULT-1 RESULT-2 ...

RESULT-n are all the computed results pertaining to NODE NUMBER-i and the chosen instance of RESULT.

Any of the following optional fields can be specified after the previous optional and/or non-optional entries documented above:

NDTYPE is an optional keyword that can take one of the following values: MEAN, STDV, or PDF. It should be used only when an intrusive or non intrusive non deterministic analysis is performed. When MEAN is selected and a non intrusive non deterministic analysis is performed, the mean value of the RESULT is outputted using the number of realizations NUM_RLZ specified in the NONINPC command. When STDV is chosen and a non intrusive non deterministic analysis is performed, the standard deviation of the RESULT is outputted using the number of realizations NUM_RLZ specified in the NONINPC command. When PDF is specified, the probability density function is outputted using a number of realizations equal to the integer set in the suboptional field NUM_OUT_RLZ (integer). First, the shape functions are evaluated at NUM_OUT_RLZ realizations of the random system. Then, NUM_OUT_RLZ realizations of the non deterministic RESULT are reconstructed from these evaluations using $v^{(j)} = \sum_{i=0}^{P-1} v_i^{(j)} \psi_i^{(j)}$ in the

case of a displacement or $\sigma^{(j)} = \sum_{i=0}^{P-1} \sigma_i^{(j)} \psi_i^{(j)}$ in the case of a stress field σ and written in the

output file (note that the case of a von Mises stress is treated slightly differently because it is a nonlinear function of the stress tensor components). A histogram can be constructed from these results.

STR_AVG_OPTION

If the output of a stress or strain field is requested, the user can set this optional field to one of the following values NODALFULL, NODALPARTIAL, ELEMENTAL (characters) to specify a type of stress/strain averaging. If NODALFULL is specified, the stress or strain is averaged at each node using all elements attached to this node. In this case, the contributions of the beam and bar elements are first transformed to the global frame. If NODALPARTIAL is specified, the averaging is performed using only

OPTION

NDTYPE

[NUM_OUT_RLZ]

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the elements that are neither of a bar or a beam type. If ELEMENTAL is specified, the stress or strain is output without any averaging at each node of each element. Note however that currently, the ELEMENTAL option is not supported when a FETI solver is invoked.

STR_SHELL_OPTION

This optional field is applicable only when the output of a stress/strain field of a shell element is requested. It can take any of the following values UPPER, MEDIAN, LOWER (characters). If UPPER is specified, the stresses are computed on the upper surface of the element. If MEDIAN is specified (default value), they are computed on the median surface of the element. If LOWER is specified, they are computed on the lower surface of the element.

STR_BEAM_OPTION

This optional field is applicable only when the output of STRAINXX or STRESSXX of a beam element is requested. It specifies two real numbers (real). The first (second) real number represents the positive or negative y(z) coordinate in percentage of the maximum positive (top fiber) or negative (bottom fiber) y(z) coordinate of the beam cross section, in the local frame attached to the beam, of the fiber at which $\begin{bmatrix} \epsilon_{xx} \end{bmatrix}$ or $\begin{bmatrix} \sigma_{xx} \end{bmatrix}$ is requested. The default value for both real numbers is zero.

COMPLEX_OUTPUT_OPTION

[NSTEPS]

STRAINXX

These optional and suboptional fields are applicable only when the output is complex-valued. COMPLEX_OUTPUT_OPTION can take any of the following values REALIMAG, MODPHASE, ANIM (characters). When set to REALIMAG, the real parts of the complex-valued output are printed first and the imaginary parts are printed next. When set to MODPHASE, the moduli ρ of the output values are first printed and

then the phases θ are printed. When set to ANIM, the values of $\rho\cos(\theta-\phi_1)$ are printed first,

followed by the values of $\rho\cos(\theta-\phi_2)$, etc, concluding with the values of $\rho\cos(\theta-\phi_N)$,

where ϕ_i is defined as $\frac{2\pi}{N}$ and N is specified in the suboptional field NSTEPS (integer).

For a shell element, the following definitions prevail

Nodal membrane strains in local xyz-axes. STRAINYY STRAINZZ Nodal membrane strains in local xyz-axes. STRAINXY Nodal bending curvatures in local xyz-axes. STRAINYZ Nodal bending curvatures in local xyz-axes. Nodal bending curvatures in local xyz-axes. STRAINZX STRESSXX Nodal membrane stress resultants in local xyz-axes. STRESSYY Nodal membrane stress resultants in local xyz-axes. STRESSZZ Nodal membrane stress resultants in local xyz-axes. STRESSXY Nodal bending stress resultants in local xyz-axes. Nodal bending stress resultants in local xyz-axes. STRESSYZ STRESSZX Nodal bending stress resultants in local xyz-axes.

Nodal membrane strains in local xyz-axes.

Next: PITA, Previous: OUTPUT

75 OUTPUT OF RESULTS 6 COLUMNS (OUTPUT6 completely spelled out)

Command Statement: OUTPUT6

The OUTPUT6 command has the same options and format of the OUTPUT command. Its outcome differs from that of OUTPUT only when vector results such as generalized displacements fields are to be outputted. In that case, the OUTPUT6 command forces FEM to output all six components of the generalized displacement field in a format slightly different from that of the file created by OUTPUT, in that the node number is included at the beginning of each line.

Note 1: When multiple outputs are expected, they are printed one set after the other in the same output file.

Next: PRELOAD, Previous: OUTPUT6

76 PARALLEL-IN-TIME ALGORITHM (PITA)

Command Statement: PITA

The PITA command statement is used to request the parallelization in time of the time-integration algorithm specified under the DYNAMIC command. This option is supported only when executing the distributed version of the **AERO-S** executable. The simultaneous presence of the command statement NONLINEAR in the input file automatically triggers the nonlinear version of the PITA algorithm. By default, the initialization of the time-slices is performed by running the chosen time-integrator on the coarse time-grid. Alternatively, the IDISP6PITA (see <u>IDISP6PITA</u>) and IVEL6PITA (see <u>IVEL6PITA</u>) commands can be used to specify the seed displacements and velocities. The imput format is given below.

Note 1: The current implementation allows only geometric nonlinearities.

Note 2: The foundations of the PITA methodology are described in [Farhat and al.] for the linear case and [Cortial and Farhat] for the nonlinear case.

PITA

J_RATIO MAX_ITER PITA_KEYWORDS

or

J_RATIO	MAX_ITER	MAX_SLICES_ON_CPU
PITA_KEYWORDS		

J_RATIO Ratio between the time-steps on the fine and coarse time-grids. Also, number of time-steps

in each time-slice (integer).

MAX_ITER Maximum number of outer PITA iterations (integer).

MAX_SLICES_ON_CPU Maximum number of time-slices assigned to a processor (integer). If the number of

time-slices is larger than the number of available CPUs, the execution of the PITA is organized in phases. In each phase, up to MAX_SLICES_ON_CPU time-slices are assigned to each processor, starting with the first one. The remaining time-slices are then progressively activated in subsequent phases while the active time-slices reach convergence and free new

slots. The default value is 1.

PITA_KEYWORDS None, one or several of the following optional keywords

NOFORCE Informs **AERO-S** about the absence of any external force so that a special version of the

PITA can be executed for optimal performance (linear case only).

CONSTFORCE Informs **AERO-S** that all external forces are time-independent so that a special version of

the PITA is executed for optimal performance (linear case only).

CKCOARSE Instructs **AERO-S** to propagate the remainder of the correction step on the coarse time-grid

(linear case only). This option should be used for testing purposes only. It is not

recommended in general.

LOCALBASES Instructs **AERO-S** to use the local basis updating scheme instead of the global one

(nonlinear case only). This option may improve performance but at the expense of

time-accuracy.

Next: PRESSURE, Previous: PITA

77 PRELOAD

Command Statement: PRELOAD

The PRELOAD command statement can be used to specify a preload within one or more truss elements. The preload is an axial force within the bar and affects the element stiffness matrix. It is assumed that the nodal coordinates of the model NODES include the deformations due to the preloading. Preload affects both linear and nonlinear, static, transient, and eigen analyses.

Preload is currently active only for truss elements. Prescribed preloads for other element types are currently ignored. The default preload of all truss elements is zero. When outputting the internal forces of bar elements, note that the axial force will include the preload.

The following two formats are supported and can be mixed.

PRELOAD

ELEMENT# PRELOAD_VALUE

STARTING_ELEMENT# ENDING_ELEMENT# PRELOAD_VALUE

ELEMENT# Element number where a preload is to be specified (integer).

PRELOAD_VALUE# Value of the axial preload (float).

STARTING_ELEMENT# First element of a sequence of elements where the preload has the same constant value (integer).

Last element of a sequence of elements where the preload has the same constant value (integer).

Next: **QSTATICS**, Previous: **PRELOAD**

78 PRESSURE

Command Statement: PRESSURE

The PRESSURE command can be used to specify a piece-wise uniform pressure field on beam and/or shell elements, or surface elements defined in <u>SURFACETOPO</u>. It can be positioned in the input file anywhere after the <u>TOPOLOGY</u> command. For a beam element (type 6 or 7), the pressure is applied in the Y-direction of the **local frame** specified in the <u>EFRAMES</u> command or implied by the third node technique for beam elements (see <u>TOPOLOGY</u>).

For a shell element (type 8, 88, 20, or 2020) or a surface element defined in <u>SURFACETOPO</u>, the pressure field is applied normal to the surface of the element using the usual convention of a positive outward normal — that is, if the three nodes of a shell element are denoted by a, b, and c and numbered in this order, the normal is computed as $\vec{ab} \wedge \vec{ac}$. Furthermore, a positive value of PRESSURE_VALUE implies a force in the direction of the normal.

In a geometrically nonlinear analysis, the generated pressure forces are of the **follower** type for shell elements and surface elements defined in <u>SURFACETOPO</u>; for beam elements they can either be of the follower type (default) or non-follower type, depending on a compilation switch that must be set for this purpose.

Note 1: AERO-S always generates pressure loads using a consistent method.

Note 2: The piece-wise uniform pressure field represented by the pressure value PRESSURE_VALUE can be varied in time using the command <u>MFTT</u>.

The following three formats are available for this command and can be mixed.

PRESSURE

ELEMENT# PRESSURE_VALUE

STARTING_ELEMENT# ENDING_ELEMENT# PRESSURE_VALUE

SURFACI	E SURFACE#	PRESSURE_VALUE
	ELEMENT#	Element number where a pressure field is to be specified (integer).
	PRESSURE_VALUE#	Value of the piece-wise uniform pressure field in that element (float).
	STARTING_ELEMENT#	First element of a sequence of elements where the piece-wise uniform pressure field has the same value (integer).
	ENDING_ELEMENT#	Last element of a sequence of elements where the piece-wise uniform pressure field has the same value (integer).
	SURFACE	Keyword indicating that a surface defined in <u>SURFACETOPO</u> is to be identified next by its integer dentification number (characters).
	SURFACE#	Integer identification of the surface defined in <u>SURFACETOPO</u> where the uniform pressure value PRESSURE_VALUE is to be applied (integer).

Next: RANDOM, Previous: PRESSURE

79 QUASISTATICS ANALYSIS

Command Statement:	QSTATICS
--------------------	----------

The QSTATICS command statement is used to signal that the subsequent data lines correspond to the following quasistatics *iterative* algorithm for solving Ax=b, where A can represent a diagonal block of a coupled system and can be singular

$$\Delta \tilde{x} = b^{n+1}$$

$$\alpha^{n+1} = \alpha^n + \beta R^T b^{n+1}$$

$$\bar{x}^{n+1} = \bar{x}^n + \theta(\tilde{x} - \bar{x}^n)$$

$$x^{n+1} = \bar{x}^{n+1} + R\alpha^{n+1}$$

This command is currently implemented only for linear analysis, and is useful only for a multidisciplinary analysis (aerothermal, aeroelastic, aerothermoelastic).

For single discipline analysis, it is at best ($\theta = 1$) a direct solution method.

The second and fourth steps of the iterative above solution algorithm are currently enabled only for an aerothermal (thermostructure-thermofluid) analysis in which the thermostructure problem is singular. In that case, *A* represents the matrix of the thermostructure subsystem and *R* represents a basis of its null space.

Note 1: An equation solver must be specified under the STATICS command.

The input format of this command can be as follows.

QSTATICS



 θ

Underrelaxation factor $0 < \theta \le 1$



Underrelaxation factor $0 < \beta \le 1$

tolqs

Convergence tolerance.

maxqs

Maximum number of iterations.

delta

Time-step equivalent-value of the increment between any two iterations. This parameter is taken into account only in: (a) the presence of a user-defined "control.C" file (see <u>ACTUATORS</u>, see <u>SENSORS</u>, see <u>USDD</u>, and see <u>USDF</u>.), and/or (b) the time-stamp in the output format for quasi-static analysis. It is the equivalent of a time-step and is used to convert the current iteration into current time. The default value is 0. When delta is specified to a non-zero value, $\underline{\theta}$ is automatically set to $\underline{\theta = 1}$ and convergence is not checked because in that case, convergence is reached in one iteration and it is understood that the purpose of the quasistatics analysis is to solve a series of consecutive static problems whose right-hand sides are set by a "control.C" file.

Next: READMODE, Previous: QSTATICS

80 RANDOM

Command Statement: RANDOM

The RANDOM command statement is used to signal that some materials have properties with random values. All random material properties are assumed to have a Gaussian probability distribution.

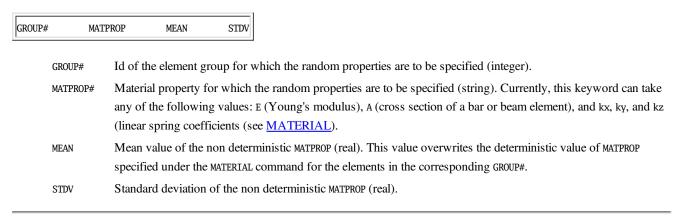
Note 1: Currently, an input file can specify only one random material property per group.

Note 2: To ensure the positivity of the material properties at every realization, the following non-Gaussian model is used

 ${\rm MATPROP = MEAN + \frac{STDV}{\sqrt{2}}(\xi^2-1)}$ with the constraint $\frac{STDV}{\sqrt{2}} < {\rm MEAN}$. Here $\boxed{\xi}$ denotes a standard normal random variable.

The input format of this command is given below.

RANDOM



Next: REBUILD, Previous: RANDOM

81 READING EIGEN MODES

Command Statement:	READMODE
--------------------	----------

The READMODE command can be used for inputing a modal basis and modal initial conditions (see <u>IDISPLACEMENTS</u>, <u>IVELOCITIES</u>) for dynamic computations (see <u>DYNAMICS</u>), or performing a "Multi-Ping-Pong" analysis (see <u>AERO</u>).

READMODE pathandfilename

pathandfilename

Name of the file containing the modes to be read (characters). The format of this file is as follows. The first line contains the number of modes to be read. The second line specifies the number of nodes of the underlying mesh. The modes follow afterwards. Each mode can be specified by a frequency (number of cycles per second) on a separate line and a number of lines equal to the number of nodes, each containing the 3 or 6 values of the mode for the 3 translational or all 6 degrees of freedom of that node. Alternatively, each mode can also be specified by a frequency (number of cycles per second) on a separate line and a number of lines equal to the number of nodes, each containing the node number followed by 3 or 6 values of the mode for the 3 translational or all 6 degrees of freedom of that node.

Next: RENUMBERING, Previous: READMODE

82 REBUILD



The REBUILD command statement is used only in nonlinear analyses to specify when to rebuild the tangent stiffness matrix.

REBUILD nsteps

nsteps

Specifies the number of Newton iterations after which the tangent stiffness matrix is to be continuously rebuilt/updated (integer). The default is 1.

Next: RESTART, Previous: REBUILD

83 RENUMBERING

Command Statement: RENUMBERING

The RENUMBERING command statement is used to specify the type of node renumbering to be performed on the mesh for either or both of the skyline and sparse solvers. The input format is given below.

RENUMBERING

TYPE

TYPE

For FEM, the "rcm" and "sloan" schemes are available for the skyline solvers, and the "esmond" (minimal degree ordering) and "metis" (recursive spectral bisection ordering) are available for the (esmond) sparse solver. When a FETI solver is used, two renumbering schemes can be specified if a skyline as well as a sparse solver are to be used by this FETI solver. In this case, the two renumbering schemes can be specified in any order, each on a separate line (string).

Next: TRBM, Previous: RENUMBERING

84 RESTART

Command Statement: RESTART

The RESTART command is used to request saving computational data in a RESTART file in order to enable later the restart of a dynamics simulation, and/or initializing a dynamics computation using computational results previously saved in a RESTART file (restart data). The processes of creating and reading from a RESTART file are specified on separate lines using different syntaxes.

Note 1: In addition to the time-instances implied by the specified value of the parameter INCREMENT, the restart data is also saved at the of a simulation if **AERO-S** exits gracefully.

The syntax for invoking this command is given below.

PATHANDFILENAME1 INCREMENT PATHANDFILENAME2 EXTENSION

INCREMENT An integer number that specifies at every how many time-integration steps the "refem.restart" file

will be updated. For an updating restart, this number specifies at every how many iterations the file

"restart.upd" will be updated.

PATHANDFILENAME1

INCREMENT

This command, which can be combined with the following one, instructs **AERO-S** to

save/overwrite the restart data into the specified file PATHANDFILENAME1 (string) every INCREMENT

(integer) time-steps. An example using RESTART can be found in FEM.d/fem_examples/Restart.d/

PATHANDFILENAME2

EXTENSION

This command, which can be combined with the previous one, instructs FEM to read the restart data from the specified file PATHANDFILENAME2 (string) and append the extension EXTENSION (string) (e.g. ".2") to all output filenames specified under the command OUTPUT. An example using RESTART

can be found in FEM.d/fem_examples/Restart.d/

Next: RBMFILTER, Previous: RESTART

85 RIGID BODY (AND OTHER ZERO ENERGY) MODES

Command Statement:

TRBM

The TRBM command is used to specify a tolerance for monitoring small pivots during the factorization of a matrix. "Small" pivots are deemed to be zero pivots. Therefore, they are associated with rigid body modes (or zero energy modes in general). When this command is specified in the **AERO-S** input file and a direct solver is specified under the <u>STATICS</u> command, the generalized inverse of the stiffness, conductivity, mass (when an initial acceleration is to be computed to satisfy the governing equation (see <u>DYNAMICS</u>)), or other relevant matrix is computed in factored form by eliminating the equation associated with a deemed zero pivot and setting the corresponding unknown to zero. This command can be used together with the GRBM command.

Note 1: See **GRBM** for an alternative option for analyzing singular systems.

Note 2: If both the <u>GRBM</u> and <u>TRBM</u> commands are specified in the same **AERO-S** input file, the one specified last is chosen. However, if that one is <u>TRBM</u>, it does not de-activate the <u>GRBM</u> command but simply overwrites the value of its second tolerance, VALUE_2, by the value of the tolerance specified under <u>TRBM</u>.





VALUE

Tolerance for monitoring the zero pivots of a matrix during its factorization. The default value is

1.0e-16 (real).

Next: SENSORS, Previous: TRBM

86 RIGID BODY MODES FILTER

Command Statement: RBMFILTER

This command is suitable for structure, aeroelastic, and aerothermoelastic simulations involving an unrestrained (or partially restrained) object when: 1) the effect of the rigid body modes is not sought-after, 2) prescribing Dirichlet (displacement) boundary conditions to remove the rigid body modes is not desirable. When this command is specified in the input file and the structural problem contains rigid body modes, a non-trivial projector is constructed. In a static or quasistatic analysis, the transpose of this projector is applied to the (possibly variable) right hand-side to make sure that the external load is self-equilibrated and therefore admissible. In a dynamic analysis: a) this projector is applied to the initial solution (initial displacement *and* velocity fields), and b) at each time-step, the transpose of this projector is applied to the (time-dependent) right- hand side of the equation being solved. In all cases, the net effect is to work with (and output) a solution where the structural rigid body modes have been filtered out.

The constructed projector can be written as $P = I - R(R^TQR)^{-1}R^TQ$ where I is the identity matrix, R is the matrix of the

rigid body modes of the structure (always computed with the GRBM method), and Q is either the identity matrix for static analysis, or the mass matrix for quasistatic and dynamic analyses. Hence, in a quasistatic analysis, this command requires the density to be present in the material properties specified under the MATERIAL command. An example input file using this command can be found in FEM.d/fem_examples/Rbm_Filter.d.

Note 1: This command works with all solvers except the FETI type solvers. It is implemented for linear static, linear quasistatic, linear dynamic, and nonlinear dynamic analyses. It is not currently implemented for nonlinear static analysis.

Note 2: This command is not active when a modal dynamic analysis is performed (see <u>DYNAMICS</u>). The reason is that one can achieve the same objective simply by not including the rigid body modes in the input for the READMODE command.

The input format of this command is specified below.

RBMFILTER

level Leaving this blank is equivalent to choosing the value of 1.

- 1 This is the default value. In this case, the projector is used as described above, in both linear and nonlinear analyses.
- 2 This value is applicable only for linear structural dynamics problems. In this case, the projector is also applied at each time-step to the outcome of the solution of the system of dynamic equations. As a result, the rigid body modes are also filtered directly from the displacement and velocity fields. This is a safety mesasure in case the previous option does not work properly, for example, because of some ill-conditioning.

Next: SLOSH, Previous: RBMFILTER

87 SENSORS *S*

Command Statement: SENSORS

The SENSORS command statement is used to specify to FEM the degrees of freedom to be observed and whose structural state is to be passed to the user defined control subroutine "control.C" (see <u>ACTUATORS</u>). An example input file using the SENSORS command can

be found in FEM.d/fem_examples/Control.d

Note 1: For nonlinear analyses, sensor information on the velocity and acceleration of a nodal degree of freedom is not currently available for use in the "control.C" file. These are currently passed as zero.





NODE# Node number where the sensor is to be placed (integer).

DOF# Degree of freedom local number where the sensor is to be placed (integer).

Next: <u>SZEM</u>, Previous: <u>SENSORS</u>

88 SLOSHING PROBLEMS



The SLOSH command statement is used to specify the computation of the sloshing modes of an incompressible and inviscid fluid whose free surface is orthogonal to a specified gravity field. The computational approach assumes linear kinematics, is based on a fluid displacement potential, and does not involve the fluid material properties. It requires identifying a free surface using free-surface elements (element type 302 in two dimensions and element type 312 in three dimensions, see <u>TOPOLOGY</u>) but does not require any other boundary condition.

Hence, this command should be used in conjunction with the EIGEN and STATIC commands in order to solve the arising eigenvalue problem.

SLOSH



slgrav

Magnitude (scalar) of the gravitational acceleration whose direction must be orthogonal to the free surface of the problem. FEM uses this specified value to convert the sloshing eigenvalues to sloshing frequencies (number of cycles per second) for output (real).

Next: STATICS, Previous: SLOSH

89 SLOSHING ZERO ENERGY MODE

Command Statement: SZEM

This command is effective only for a sloshing eigen computation involving a zero energy mode. It is used to request the computation of the constant potential mode using a physics-based algorithm (rather than the tolerance-based algorithm associated with the TRBM command). It should be used in conjunction with the SLOSH and related commands.



Next: SURFACETOPO, Previous: SZEM

90 STATICS

Command Statement: STATICS

The STATICS command statement is used to signal that the following data lines correspond to the selected equation solver to be used within the chosen analysis. The input format is given below.

Note 1: Except when otherwise specified, the solvers proposed below are available only for symmetrical systems of equations.

Note 2: Among all solvers proposed below, only the spooles and mumps sparse direct solvers (with pivoting turned on) and the gmres and FETI DP iterative solvers are suitable for the solution of indefinite systems. Such systems arise: (1) in conjunction with the

HELM, IMPEDANCE">HMPC or
TIEDSURFACES command, or the usage of a positive shift in the
EIGEN command (see SHIFT in that command), and (2) whenever a structural model includes rigid and/or joint elements (see TOPOLOGY).

Note 3: The Lagrange multiplier method for enforcing constraints leads to an indefinite system of equations. Currently, only the spooles and mumps direct solvers with pivoting turned on and the gmres and FETI DP iterative solvers support this approach for solving constrained systems (see CONSTRAINTS).

Note 4: Among all FETI methods, only FETI DP is maintained for nonlinear structural static and dynamic analyses.

Note 5: When a non deterministic analysis is performed using the intrusive version of the Polynomial Chaos method, only the PCG, BCG, and CR solvers can be selected for solving the resulting large system of equations.

STATICS

METHOD (keyword)
PARAMETERS (keyword and/or value)

METHOD

direct For a skyline direct solver (characters). This is also the default solver (except when

the INPC command is used) and is available for frequency-domain acoustic

(Helmholtz) problems.

skyline For a skyline direct solver (characters). This is also the default solver and is available

for frequency-domain acoustic (Helmholtz) problems.

sparse For Esmond's sparse direct solver (characters). This solver is also available for

frequency-domain acoustic (Helmholtz) problems.

sgisparse (completely spelled out) For SGI's sparse direct solver (characters). Runs only on SGI machines.

spooles (completely spelled out) For the SPOOLES sparse direct solver (characters). Cannot be used for any system

of equations with less than 24 unknowns.

spooles pivot (completely spelled out) For the SPOOLES sparse direct solver with pivoting option turned on (characters).

spooles unsymmetric (completely For the SPOOLES unsymmetric sparse direct solver (characters).

spelled out)

mumps (completely spelled out) For the MUMPS sparse symmetric positive definite direct solver (characters).

mumps pivot (completely spelled out) For the MUMPS sparse general symmetric direct solver with pivoting option turned

on (characters).

mumps unsymmetric (completely spelled For the MUMPS unsymmetric sparse direct solver (characters).

out)

sgisky (completely spelled out) For SGI's skyline direct solver (characters). Runs only on SGI machines.

frontal (completely spelled out) For a frontal direct solver (characters).

pcg For a preconditioned conjugate gradient solver (characters).

bcg For a bi-conjugate gradient solver (characters).
cr For a conjugate residual solver (characters).

gmres For a generalized minimum residual solver (characters). Applicable to symmetric and

unsymmetric systems.

FETI For a one-level FETI solver (default) (characters).

FETI 1 For a FETI-1 (one-level FETI method) solver (characters).

FETI 2 OLD For a FETI-2 solver using a "full" coarse problem implementation (characters).

FETI 2 NEW For a FETI-2 solver using a "sparse" coarse problem implementation (characters).

FETI DP For a FETI-DP (FETI dual-primal method) solver (characters).

FETI DPH For a FETI-DPH (FETI dual-primal method with augmentation basis) solver

(characters). This solver is to be used only with the IMPEDANCE command or the HELM command. Depending on whether the system to be solved is damped or not, or contains a complex boundary condition or multipoint constraint or not, this solver

performs in the real or complex domain.

PARAMETERS (pcg/bcg/cr/gmres solvers,

in any order)

precno Specifies the preconditioner (characters).

Unpreconditioned (integer).

1 Diagonal scaling (integer). This option should not be used when solving an indefinite

system arising in conjunction with the usage of the <u>LMPC</u> or <u>TIEDSURFACES</u> command, or whenever a structural model includes rigid and/or joint elements (see <u>TOPOLOGY</u>), because in all these cases, the system matrix will contain zero

diagonal entries.

2 Incomplete block diagonal scaling (integer). This option is available only for a system

of equations resulting from the intrusive version of the Polynomial Chaos method. In this case, the subsystems of equations associated with the diagonal blocks can be

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solved either using the sparse solver (default), or the FETI DP method with its acceleration for multiple right sides. If the FETI DP method is chosen as a block diagonal solver, then its keyword and the keywords associated with its options should be specified after all other parameters of the main (outer) solver have been

specified.

tolpcg Error tolerance for the convergence of the pcg solver (real).

tolbcg Error tolerance for the convergence of the bcg solver (real).

tolcr Error tolerance for the convergence of the cr solver (real).

tolgmres Error tolerance for the convergence of the gmres solver (real).

maxitr Maximum number of iterations to be performed (integer).

PARAMETERS (spooles solver, in any

order)

spooles_renum Specifies the renumbering (integer, default value is 0).

0 Best of nested dissection and multisection.

1 Multiple minimum degree.

Multisection.Nested dissection.

spooles_scale Specifies the scaling (integer, default value is 0).

0 No scaling.

1 Symmetric scaling.

spooles_tau Upper bound on the magnitude of the largest element in L or U when pivoting

enabled (real 1.0). If this number is too small the results will be wrong. The default

value of 100 is generally safe if scaling is used (see spooles_scale).

spooles_msglvl Message output level (integer ≥ 0 , default value is 0).

orderings, where n is the number of equations in the system to be solved. This parameter is used to control the incomplete nested dissection process. Any subgraph whose weight is less than maxdomainsize is not split further (integer ≥ 0 , default

value is 24).

spooles_maxzeros is the maximum number of zeros allowed in a supernode/front

(real > 0 and ≤ 1.0 , default value is 0.04).

spooles_maxsize Maximum number of internal columns in supernode/front (integer > 0, default value

is 64).

PARAMETERS (mumps solver, in any

order)

mumps_icntl # Value for ICNTL(#), see mumps documentation for specific details (integer).

mumps_cnt1 # Value for CNTL(#), see mumps documentation for specific details (real).

PARAMETERS (FETI, FETI-DP, FETI-DPH solvers, in any order)

sparse Specifies Esmond's sparse direct method as the local (subdomain and Dirichlet

preconditioner) solver (characters). The default is a skyline solver which can also be invoked by specifying skyline instead of sparse on this line. Note also that there is

another mechanism for specifying this option (see below).

local_solver

skyline Selects the skyline direct method as the local solver (subdomain and Dirichlet

preconditioner problems) (characters). This is also the default choice.

sparse Selects Esmond's sparse direct method as the local solver (characters).

spooles Selects the SPOOLES sparse direct method as the local solver (characters).

spooles pivot Selects the SPOOLES sparse direct method as the local solver with pivoting option

turned on (characters).

mumps Selects the MUMPS sparse direct method as the local solver (characters).

mumps pivot Selects the MUMPS sparse direct method as the local solver with pivoting option

turned on (characters).

coarse_solver

blockskyline Selects the block-skyline direct method as the coarse solver. This is also the default

choice (characters).

skyline Selects the skyline direct method as the coarse solver (characters).

sparse Selects Esmond's (sequential) sparse direct method as the coarse solver (characters).

psparse Selects the parallel sparse direct method as the coarse solver (characters).

spooles Selects the SPOOLES sparse direct method as the coarse solver (characters).

spooles pivot Selects the SPOOLES sparse direct method as the local solver with pivoting option

turned on (characters).

mumps Selects the MUMPS sparse direct method as the coarse solver (characters).

mumps pivot Selects the MUMPS sparse direct method as the local solver with pivoting option

turned on (characters).

precno Specifies the local preconditioner (integer or characters).

0 or noprec Unpreconditioned (integer).

1 or lumped Lumped preconditioner (integer). This is also the default preconditioner.

2 or dirichlet Dirichlet preconditioner (integer).

projector

1 Identity based projector (integer). This is also the default projector.

2 Preconditioner based projector (integer). Uses the selected preconditioner for building

the so-called Q matrix.

4 Superlumped projector (integer).

scaling

1 or stiffness Stiffness based scaling (integer). This is also the default scaling procedure.

2 or topology (subdomain connectivity) based scaling (integer).

version

The FETI (also known as FETI-1) method (integer). 1 The two-level FETI (also known as FETI-2) method (integer). nocoarse This option is only for dynamics. When specified, the FETI algorithm is executed without any "coarse grid". This keyword is useful only for the FETI-2, FETI-DP, and FETI-DPH methods. It corners specifies the treatments of the corners and corner dofs for the contruction by these iterative methods of their respective coarse problems. For the FETI-2 method, the user can define both the corner and corner dof selection algorithms. For the FETI-DP and FETI-DPH methods, the corners are automatically chosen by AERO-S but the user can still control the number of dofs at these corners. In that case, a corner is defined by FETI-2 as a crosspoint, and only the three active ср3 translational dof attached at each corner node are included in the construction of the FETI-2 or FETI-DP and FETI-DPH coarse problems. This is the default value for nodes with 3 dofs. Here, a crosspoint is defined as a point that belongs to more than four subdomains. ср6 In that case, a corner is defined by FETI-2 as a crosspoint, and all six active dof attached at each corner node are included in the construction of the FETI-2 or FETI-DP and FETI-DPH coarse problems. This is the default value for nodes with 6 dofs. Here, a crosspoint is defined as a point that belongs to more than two subdomains. In that case, a corner is defined by FETI-2 as the beginning or end of and edge, and be3 only the three active translational dof attached at each corner node are included in the construction of the FETI-2 or FETI-DP and FETI-DPH coarse problems. Note that a crosspoint is also the beginning or end of an edge. In that case, a corner is defined by FETI-2 either as the beginning or end of and edge, be6 and all six active dof attached at each corner node are included in the construction of the FETI-2 or FETI-DP and FETI-DPH coarse problems. Note that a crosspoint is also the beginning or end of an edge. This keyword is useful only for the FETI-DP and FETI-DPH methods. It specifies augment the augmentation of the "coarse grid" by various methods defined by the following commands. EdgeGs trans/all In that case, the "coarse grid" is augmented using extra equations generated by the rigid body modes (rbms) of the subdomain interfaces, and organized edge-by-edge.

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Here an edge refers to an interface between two subdomains and not to the usual geometric edge. The "trans/all" option denotes the equation type where "trans" refers

Gs trans/all

WeightedEdgeGs trans/all

to the translational rbms, and "all" refers to both the translational and roational rbms applied per edge.

In that case, the "coarse grid" is augmented using extra equations generated by the traces of the subdomain rigid body modes (rbms) on the subdomain interface boundaries, and organized subdomain-by-subdomain. The "trans/all" option denotes the equation type where "trans" refers to the translational rbms, and "all" refers to both the translational and roational rbms.

In that case, the "coarse grid" is augmented using extra equations generated by the weighted rigid body modes (rbms) of the subdomain interfaces, and organized edge-by-edge. Here an edge refers to an interface between two subdomains and not to the usual geometric edge. The "trans/all" option denotes the equation type where "trans" refers to the translational rbms, and "all" refers to both the translational and roational rbms applied per edge. The weights are similar to those used for scaling the residuals. Hence, they are based on stiffness considerations if the scaling option is set to stiffness, or on topological considerations if the scaling option is set to topology.

EdgeWs numdir

elastodynamic (or modelled dynamic shell) problems. In the latter case, the factor 6 comes from the fact that for each direction, there are 2 shear waves and 1 pressure wave, and each of these three waves has a cosine mode as well as a sine mode. The implemented directions are chosen according to the following scheme. In two dimensions, a sector is discretized into n sectors, with n an even integer. A direction is defined by connecting the center of the circle to a point on the circle delimiting a sector. Since both the cosine and sine modes are included, only one direction for each pair of opposite directions needs be retained, which results in a total of n/2 directions. Hence, n is chosen to be 2*numdir. In three dimensions, a cube is discretized into $n \times n \times n$ points. A direction is defined by connecting the center of the cube to a

point lying on a face of the cube. Since both the cosine and sine modes are included, only one direction for each pair of opposite directions needs be retained, which results in a total of $n^2 + 4(n-1)(n/2-1) + 2(n-1)mod(n,2)$ directions.

Hence, n is chosen so that $n^2 + 4(n-1)(n/2-1) + 2(n-1)mod(n,2)$

is as close as possible to numdir, with

is 1.0e-02.

$$n^2 + 4(n-1)(n/2-1) + 2(n-1)mod(n,2) \ge numdir.$$

interf_solver

This option is currently available only for the FETI-DP and FETI-DPH solvers. It speficies the Krylov method to be used with these algorithms for solving the interface problem (characters).

CG

Turns on the CG algorithm as an interface problem solver. This is the default value of interf_solver.

CGAL

Turns on Dostal's Augmented Lagrangian CG algorithm with adaptive precision control as an interface problem solver.

GMRES

Turns on the GMRES algorithm as an interface problem solver.

GCR

Turns on the GCR algorithm as an interface problem solver. This option is more computationally efficient when performing frequency or wavenumber sweeps (see IMPEDANCE, and/or see HELMHOLTZ) as the GCR algorithm is optimized for the systems with multiple right sides that arise during the reconstruction of solutions in frequency/ wavenumber bands.

orthotol

This option is currently available only for the FETI-DPH solver. It specifies the tolerance to be used for filtering out "small" vectors during the local Gram-Schmidt-like orthogonalization of the augmentation vectors. The default value

mpc_type

This keyword is useful only for the FETI-DP and FETI-DPH methods. It specifies the algorithm to be used for handling potential multipoint constraints (MPCs) (characters). The default is primal when the number of MPCs is less or equal to 1000, and dual otherwise.

dual

In this case, the MPCs are enforced by Lagrange multipliers and are satisfied only at convergence (characters).

primal

In this case, the MPCs are put in the coarse problem and are satisfied at every iteration (characters).

mpc_precn

This keyword is useful only for the FETI-DP and FETI-DPH methods, in the presence of MPCs, and when mpc_type is set to dual (characters). The default is tblock when running a single thread process, and full otherwise.

full

In this case, the CC^T matrix, where C denotes the constraint matrix, is treated as a single block matrix (characters).

tblock

In this case, the potential algebraic block-structure of the CC^T matrix, is exploited (characters).

sblock mblock In this case, the subdomain-structure of the CC^T matrix, is exploited (characters). In this case, the mortar-interface-structure of the CC^T matrix, is exploited (characters).

diag In this case, the CC^T matrix is approximated by its diagonal (characters).

mpc_scaling

1 or stiffness Stiffness based scaling (integer).

2 or topology (subdomain connectivity) based scaling (integer). This is also the default

scaling procedure.

cct_solver

skyline Selects the skyline direct method as the coarse solver. This is also the default choice

(characters).

sparse Selects Esmond's (sequential) sparse direct method as the coarse solver (characters).

cct_tol Specifies the tolerance to be used for detecting singularities in the solution of the

 CC^T problems of the FETI-DP and FETI-DPH methods in the presence of MPCs

(real). The default is 1.0E-12.

kryprec This option is only for nonlinear problems. It turns on the Krylov preconditioner

enrichment for the solution of nearby problems in a Newton method (characters).

Turns on the Krylov preconditioner and gives to the maxorth parameter (see below)

the scope of the entire nonlinear analysis.

2 Turns on the Krylov preconditioner and gives to the maxorth parameter (see below)

the scope of a load-step in a nonlinear analysis.

global_cor_rbm_tol Specifies the tolerance to be used for detecting singularities in the solution of the

corner-based coarse problems of the FETI-DP and FETI-DPH methods (real). The

default is 1.0E-6.

global_rbm_tol Specifies the tolerance to be used for detecting the global rigid body modes when

using a FETI method (real). The default is 1.0E-6.

maxorth Specifies the maximum total number of reorthogonalization vectors used for

accelerating FETI and/or enriching its chosen preconditioner during a simulation (integer). For nonlinear problems, if the kryprec option is not used, this total is to be understood per Newton iteration, as the orthogonalization vectors are flushed when the tangent operator is rebuilt. Also for nonlinear problems, if the kryprec option is used, priority in the accumulation of vectors is given to enriching the preconditioner. The default is maxitr (see below). To turn-off this reorthogonalization option, set

maxorth to 1.

tolfeti Error tolerance for the convergence of the feti solver (real). The default value is

1.0e-06.

maxitr Maximum number of iterations to be performed (integer). The default value is 500.

Next: TETT, Previous: STATICS

91 SURFACE TOPOLOGY

Command Statement: SURFACETOPO

The SURFACETOPO command statement can be used to define a surface on a body by describing its faceted connectivity. Such a surface can be paired with another similar surface using another command statement in order to define a surface-to-surface interaction (see
TIEDSURFACES">TIEDSURFACES, CONTACTSURFACES, FSINTERFACE, AERO), or can be referred to by another command statement to define a surface boundary condition (see <a href="https://preces.ps.//pr

This command can be repeated as many times as there are surfaces. Its format is as follows.

SURFACETOPO	ID_NUMBER	SHELL_THICKNESS	t
ID_NUMBE	R	Surface id r	number (integer).
SHELL_THICKNESS			byword indicating that the surface identified by ID_NUMBER defines the fa shell (string).
t		•	of shell surface (real).

FACE#	FACETYPE	CONNECTIVITY_NODES
FA	ACE#	Face id number whose type and connectivity are to be specified (integer).
FA	ACETYPE	
	1	4-node quadrilateral.
	2	8-node quadrilateral.
	3	3-node triangle.
	4	6-node triangle.
	5	9-node quadrilateral.
	6	12-node quadrilateral.
	7	10-node triangle.

Next: THERMOE, Previous: SURFACETOPO

92 THERMAL EXPANSION TEMPERATURE TABLE

Command Statement:	TETT
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The TETT command statement can be used to describe the evolution of the coefficient of thermal expansion with temperature, for a given material. This evolution can be specified here in a curve (or table) defined by pairs of temperature and coefficient of thermal expansion values. Linear interpolation is used for "in between" points, and the extrema values are adopted for "outside" points. Several curves can be specified, one after the other. Each curve is identified by an "id number" as described below.



CURVE

curve_id "Id number" for the following curve (or table) (integer).

T_1 A specified temperature value (float).

TE_1 A specified coefficient of thermal expansion value at temperature T_1 (float).

Next: THERMOH, Previous: TETT

93 THERMOELASTICITY

Command Statement: THERMOE

The THERMOE command statement is used to indicate that FEM is to interact with FEM-HEAT to perform a *transient* thermoelastic (thermostructure-structure vibration) *one-way* coupled simulation.

The syntax for invoking this option is given below.

THERMOE

Next: TIEDSURFACES, Previous: THERMOE

94 THERMOELASTICITY: FEM-HEAT (FEM-HEAT Only)

Command Statement: THERMOH

The THERMOH command statement is used to indicate that FEM-HEAT is to interact with FEM to perform a *transient* thermoelastic (thermostructure-structure vibration) *one-way* coupled simulation.

The syntax for invoking this option is given below.

THERMOH

Next: TOPOLOGY, Previous: THERMOH

95 TIED SURFACES

Command Statement: TIEDSURFACES

The TIEDSURFACES command statement can be used to tie — that is, enforce perfect contact between — pairs of surfaces defined using the <u>SURFACETOPO</u> command. Surface interactions are detected using ACME's search module.

Note 1: For the enforcement of tied surface constraints by the Lagrange multiplier method in explicit dynamic analysis, the discrete kinematic constraint equations are defined using a node-to-segment approach and the constraint forces are computed using ACME's enforcement module.

Note 2: For the enforcement of tied surface constraints by either the Lagrange multiplier method in all but explicit dynamic analyses, or the penalty or elimination method in all analyses, the discrete kinematic constraint equations are defined using a segment-to-segment (mortar) approach.

Note 3: The enforcement of tied surface constraints by the Lagrange multiplier method in all but explicit dynamic analyses is supported only by the FETI-DP family of solvers, the GMRES solver, and the SPOOLES and MUMPS direct sparse solvers with pivoting enabled (see <u>STATICS</u>).

TIEDSURFACES

SURF_PAIR_ID# MASTER SLAVE

or, for statics, frequency response, eigenvalue problems and dynamics with an implicit scheme

SURF_PAIR_ID# MASTER SLAVE CONSTRAINT_METHOD MORTAR_TYPE NORMAL_TOL TANGENTIAL_TOL

or, for dynamics with an explicit scheme

SURF_PAIR_ID# MASTER SLAVE CONSTRAINT_METHOD KPART_TYPE NORMAL_TOL TANGENTIAL_TOL NUM_ITER CONVERG_TOL

SURF_PAIR_ID# Id number of the surface pair to be described (integer).

MASTER Identification of the master (mortar method) surface (see <u>SURFACETOPO</u>) (integer).

SLAVE Identification of the slave (mortar method) surface (see <u>SURFACETOPO</u>) (integer).

CONSTRAINT_METHOD Method for enforcing the associated constraints (characters). The default method is set in

CONSTRAINTS and used whenever this entry is omitted.

multipliers The Lagrange multiplier method.

elimination The elimination method.

[beta]	penalty	The penalty method. The parameter beta should be a large positive number, typically of the order of 10^8 (no default value is provided).
	MORTAR_TYPE	Mortar type: $0 = \text{standard}$, $1 = \text{dual}$, default value is 0 (integer).
	NORMAL_TOL	Normal search tolerance used by ACME to identify interactions, default value is 0.1 (float) (see Figs. 1.2 and 1.3 in Section 1.3 of ACME's User Reference Manual).
	TANGENTIAL_TOL	Tangential search tolerance used by ACME to identify interactions, default value is 0.001 (float) (see Figs. 1.2 and 1.3 in Section 1.3 of ACME's User Reference Manual).
	KPART_TYPE	Kinematic partitioning type for explicit dynamics: $0 = $ fixed, $1 = $ automatic, default value is $0 = $ (integer).
	NUM_ITER	Maximum number of predictor-corrector iterations to be performed at each time step for explicit dynamics, default value is 5 (integer).
	CONVERG_TOL	Convergence tolerance of the predictor-correction iteration loop for explicit dynamics, default value is $1.0\text{e-}10$ (float).

Next: <u>USDF</u>, Previous: <u>TIEDSURFACES</u>

96 TOPOLOGY *S*

Comn	nand Statement:	TOPOLOGY

The TOPOLOGY command statement is used to signal that the following data lines correspond to the connectivity and type of each element. The input format is given below. There should be as many lines as there are elements in the system.

Note 1: In the absence of the NONLINEAR command in the input file, all element types imply linear kinematics and a linear elastic material even when otherwise specified.

Note 2: In the absence of the NONLINEAR command in the input file or the absence of the keyword nlmat under that command, all elements imply a linearly elastic material even when otherwise specified. The properties of linearly elastic materials are specified under the MATERIAL command.

Note 3: Materially and geometrically + materially nonlinear elements are supported only by AERO-S.

Note 4: The complete description of elements with a nonlinear material can be performed using both the MATERIAL and MATLAW commands. The usage of such elements requires the specification in the input file of the NONLINEAR command, and the presence of the nlmat keyword under that command.

Note 5: AERO-S supports a run-time generation of beam frames (see <u>EFRAMES</u>) that is activated when a third node in the local *x-z* plane is found in the definition of a beam element within this TOPOLOGY command. The only requirement for the third node is that it does not be colinear with the other two beam nodes. Using this third node option releaves the user from specifying the EFRAMES command. An example illustrating the third node option can be found in FEM.d/fem_examples/Third_Node.d.

Note 6: Geometric stiffnesses are currently implemented only for the truss, beam, shell, 4-noded tetrahedron, 8-noded brick, and spring elements. Some of these are based on the corotational method, some on the Lagrangian method. Mixing the corresponding geometric stiffness matrices is not a problem.

Note 7: Thermal loads (see <u>TETT</u>) are currently implemented only for the Euler beam, 4-noded plane stress/plane strain, 4-noded as well as 10-noded tetrahedron, 8-noded brick, and 6-noded penta (prism with triangular cross section) elements.

Note 8: For thermal analysis, **AERO-S** constructs the contribution of convection effects to the right-hand side vector of the resulting system of equations using the <u>CONVECTION</u> command. However, it constructs the contribution of convection effects to the stiffness matrix using the "boundary convection" elements (type 47, 48 and 49) which must be superposed to the edges or faces of existing thermal elements. The data for these boundary convection elements (depth, area, convection coefficients and ambient temperature) must be specified in the <u>MATERIAL</u> command.

Note 9: Currently, only the SPOOLES and MUMPS sparse direct solvers and the FETI-DP(H) solvers can handle rigid elements.

Note 10: Rigid elements are internally converted by **AERO-S** to linear multipoint constraints (see <u>LMPC</u>) in the following cases: (a)during an eigen analysis if the GRBM (see <u>GRBM</u>) is present in the input file, except for element types 66, 73 and 74, (b) during a nonlinear analysis, except for element type 65, and (c) when the FETI-DP equation solver is specified.^M

Note 11: For DEM (Discontinuous Enrichment Method) and DGM (Discontinuous Galerkin Method) elements, Q, T, and H designate a quadrialteral, a triangle in two dimensions and a tetrahedron in three dimensions, and an hexahedron, respectively. In the notation Q-X-Y, T-X-Y, and H-X-Y, X denotes the number of enrichment functions in the element and Y denotes the number of Lagrange multiplier degrees of freedom per edge or face of the element. In the notation X1xX2-Y for elastodynamics, X1 denotes the number of directions of the plane wave enrichment functions, X2 denotes the number of enrichment functions per wave direction (X2 = 2 (one pressure and one shear wave) in two dimensions and X2 = 3 (one pressure and two shear waves) in three dimensions), and Y denotes the number of Lagrange multiplier degrees of freedom per edge of face of the element. The connectivity (geometry and local node numbering) of all Q, T, and H DEM and DGM elements is that of Q, T, and H higher-order isoparametric elements, respectively. Hence, for each DEM element, the degree of the polynomial field can be deduced from the number of nodes of that element.

Note 12: The fluid elements (type 301, 302, 311, 312, 321, and 331) are active only in the following cases: (1) a mass computation using the <u>MASS</u> command, (2) a sloshing eigen computation using the <u>SLOSH</u> and related commands, (3) a hydroelastic eigenvalue computation using the <u>EIGEN</u>, <u>HEFRS</u>, <u>HEFSB</u> and related commands.

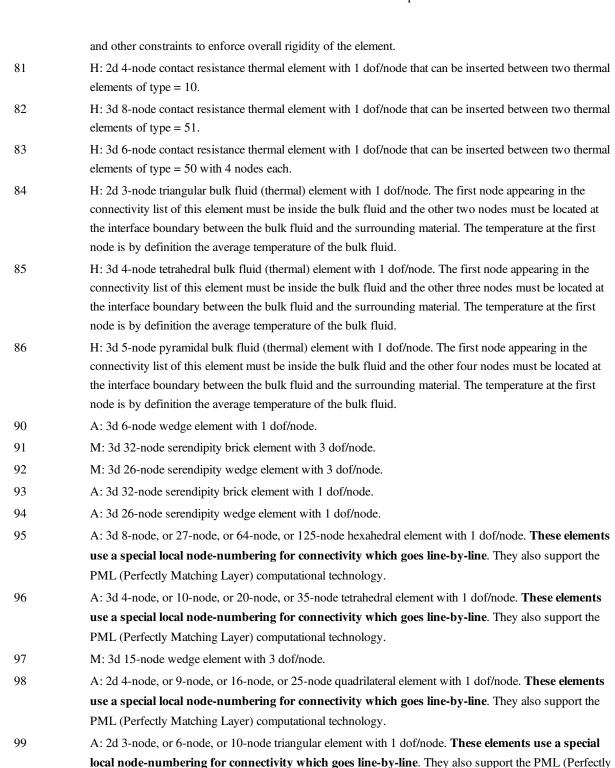
ELEMENT#	ELEMENT_TYPE	CONNECTIVITY_NODES
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ELEMENT#

Element identification number whose type and connectivity are to be specified (integer). A (*) indicates the element is not yet fully implemented and/or validated.

ELEMENT_TYPE:	M=Mechanics - A=Acoustic (Helmholtz or Time-Domain) - H=Thermal - C=Coupled Thermoelastic - F=Fluid
1	M: 3d truss (bar) element with 3 dof/node.
2	M: 2d 4-node plane stress/plane strain quad element with 2 dof/node. Note that only a lumped mass matrix is available for this element .
3	H: 3d 4-node quad element with 1 dof/node.
4	M: 2d 3-node triangular element with 2 dof/node. Note that only a lumped mass matrix is available
	for this element.
6	M: 3d Bernoulli beam element with 6 dof/node.
7	M: 3d Timoshenko beam element with 6 dof/node. Note that only a lumped mass matrix is available
	for this element.
8	M: 3d 3-node triangular AQR shell element with 6 dof/node. Note that only a lumped mass matrix is available for this element .
88	M: 3d 4-node shell element element with 6 dof/node that splits into 2 triangular shell elements of type 8. Note that only a lumped mass matrix is available for this element .
9	H: 3d 2-node lineal element with 1 dof/node.
10	H: 2d 4-node quad element with 1 dof/node.
11	M: 3d torsion spring element with 3 dof/node.
12	C: 3d thermomelastic truss element with 4 dof/node.
13	C: 2d thermoelastic quad element with 3 dof/node.
14	C: 3d thermoelastic Bernoulli beam element with 7 dof/node.
16	M: 3d Belytschko-Tsay quadrilateral shell element with 6 dof/node and 1-point quadrature rule. Currently, this element is available only for explicit dynamic analyses.
17	M: 3d 8-node brick element with 3 dof/node.
18	M: 3d 4-node tetrahedra element with 6 dof/node. Note that only a lumped mass matrix is available for this element .
19	M: 3d 3-node triangular membrane element with 6 dof/node. (Only in-plane and drilling stiffnesses). Note that only a lumped mass matrix is available for this element.
20	M: 3d 3-node triangular composite or orthotropic shell element with 6 dof/node. Note that only a
	lumped mass matrix is available for this element.
2020	M: 3d 4-node composite or orthotropic shell element with 6 dof/node. Note that only a lumped mass
	matrix is available for this element.
21	M: 3d translational spring-link element with 3 dof/node.
22	M: 3d torsion spring-link element with 3 dof/node.
23	M: 3d 4-node tetrahedral element with 3 dof/node.
24	M: 3d 6-node pentahedral element (prism with triangular cross section) with 3 dof/node.
25	M: 3d 10-node tetrahedral element with 3 dof/node.
30	A: 2d 4-node quad element with 1 dof/node.
31	A: 2d 4-node quad GLS element with 1 dof/node.
32*	A: 2d 8-node quad element with 1 dof/node.

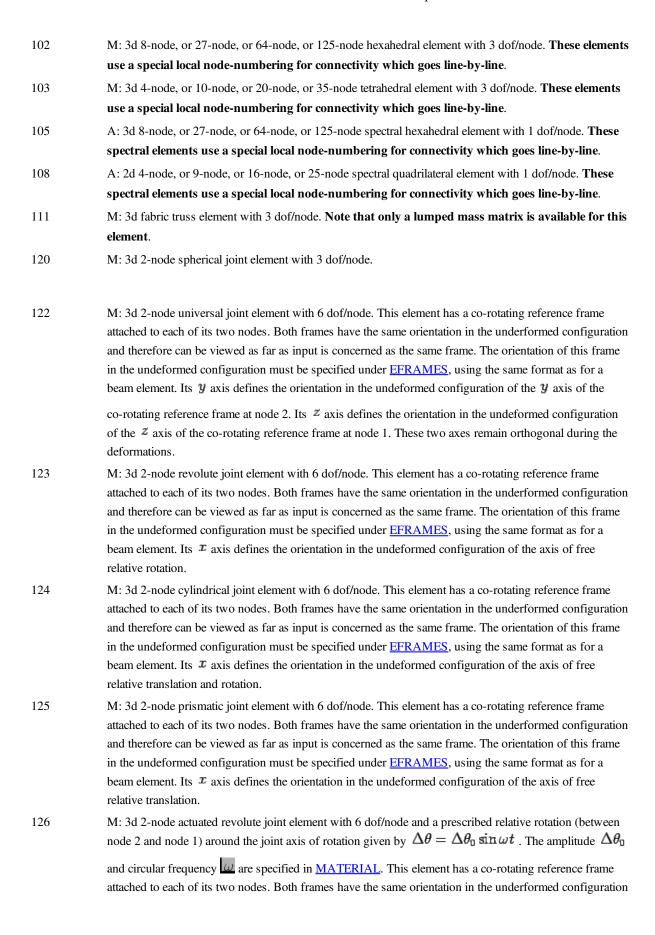
33*	A: 2d 4-node quad bubble element with 1 dof/node.
34*	A: 2d 4-node quad two-level bubble element with 1 dof/node.
35	A: 2d 3-node triangular element with 1 dof/node.
36	A: 2d 3-node triangular GLS element with 1 dof/node.
38*	A: 2d 6-node triangular element with 1 dof/node.
40	A: 3d 4-node tetrahedral element with 1 dof/node.
41	A: 3d 4-node tetrahedral GLS element with 1 dof/node.
42	A: 3d 10-node tetrahedral element with 1 dof/node.
44	A: 3d 8-node brick GLS element with 1 dof/node.
45*	A: 3d 8-node brick element with 1 dof/node.
46	H: 3d 3-node triangular heat conduction element.
4646	H: 3d 4-node quadrilateral heat conduction element that splits into 2 triangular elements of type 46.
47	H: 3d 2-node lineal element with 1 dof/node for boundary convection (see Note 8).
48	H: 3d 4-node quad element with 1 dof/node for boundary convection (see Note 8).
49	H: 3d 3-node triangular element with 1 dof/node for boundary convection (see Note 8).
50	H: 3d 4-node, or 10-node, or 20-node, or 35-node tetrahedral element with 1 dof/node. These elements
	use a special local node-numbering for connectivity which goes line-by-line.
51	H: 3d 8-node brick element with 1 dof/node.
52*	M: 3d 6-node triangular shell element.
53	H: 2d 3-node triangular heat conduction element.
56	H: 3d 2-node heat radiation element.
57	H: 3d 3-node triangular heat radiation element.
58	H: 3d 4-node quadrilateral heat radiation element.
65	M: 3d 2-node rigid truss (bar) element; enforces constant length of the element.
66	M: 3d 2-node rigid beam element; enforces constant length of the element, equal rotations of the cross sections at its two nodes, and other constraints between its rotational and translational dofs.
67	M: 3d 2-node rigid link (translational and rotational); for each dof at one node, enforces equality to the
	corresponding dof at the other node.
68	M: 3d 2-node rigid translational link; for each translational dof at one node, enforces equality to the corresponding dof at the other node.
69	M: 3d 2-node rigid rotational link; for each rotational dof at one node, enforces equality to the corresponding dof at the other node.
70	M: 3d 8-node rigid brick element with 3 dof/node; enforces constant distance between each pair of its nodes (special case of element type 71).
71	M: 3d rigid element (lineal, plane, or solid) with 3 dof/node and anywhere from 3 to 32 nodes per element; enforces constant distance between each pair of its nodes.
72	M: 3d 20-node brick element.
73	M: 3d 3-node rigid shell element; equivalent to three rigid beam elements, each defined by an edge of the element.
74	M: 3d rigid element (lineal, plane or solid) with 6 dof/node and anywhere from 3 to 32 nodes per element; enforces constant distance between each pair of its nodes, equal values of the rotational dofs at all nodes,



M: 2d 4-node, or 9-node, or 16-node, or 25-node quadrilateral element with 3 dof/node. **These elements** use a special local node-numbering for connectivity which goes line-by-line.

Matching Layer) computational technology.

M: 2d 3-node, or 6-node, or 10-node triangular element with 3 dof/node. These elements use a special local node-numbering for connectivity which goes line-by-line.



and therefore can be viewed as far as input is concerned as the same frame. The orientation of this frame in the undeformed configuration must be specified under EFRAMES, using the same format as for a beam element. Its $\boldsymbol{\mathcal{I}}$ axis defines the orientation in the undeformed configuration of the axis of forced relative rotation.

M: 3d 2-node actuated primsatic joint element (or prismatic actuator) with 6 dof/node and a prescribed relative translation (between node 2 and node 1) along the joint axis of translation given by $\Delta u = \Delta u_0 \sin \omega t$. The amplitude Δu_0 and circular frequency ω are specified in MATERIAL.

This element has a co-rotating reference frame attached to each of its two nodes. Both frames have the same orientation in the underformed configuration and therefore can be viewed as far as input is concerned as the same frame. The orientation of this frame in the undeformed configuration must be specified under EFRAMES, using the same format as for a beam element. Its ** axis defines the orientation in the undeformed configuration of the axis of forced relative translation.

128	M: 3d 4-node plane stress/plane strain quad element with 3 dof/node. Note that only a lumped mass
	matrix is available for this element

- M: 3d 3-node plane stress/plane strain triangular element with 3 dof/node. **Note that only a lumped** mass matrix is available for this element.
- 201 M: 3d materially nonlinear 8-node brick element with 3 dof/node.
- 202 M: 3d geometrically (Green-Lagrange) + materially nonlinear 8-node brick element with 3 dof/node.
- F: 2d 4-node sloshing (fluid) quadrilateral element with 1 dof/node.
- F: 2d 2-node free-surface (fluid) element with 1 dof/node for two-dimensional sloshing computations using element type = 301.
- F: 3d 4-node sloshing (fluid) tetrahedron element with 1 dof/node.
- F: 3d 3-node free-surface (fluid) triangular element with 1 dof/node for three-dimensional sloshing computations using element type = 311.
- F: 2d 4-node hydroelastic vibration (fluid) quadrilateral element with 1 dof/node.
- F: 3d 4-node hydroelastic vibration (fluid) tetrahedral element with 1 dof/node.
- 1100 A: 2d 4-node Helmholtz DGM element Q-4-1.
- 1101 A: 2d 4-node Helmholtz DGM element Q-8-2.
- 1102 A: 2d 4-node Helmholtz DGM element Q-16-4.
- 1103 A: 2d 4-node Helmholtz DGM element Q-32-8.
- 1104 A: 2d 4-node Helmholtz DGM element Q-8-2star with additional enrichment by evanescent wave functions XXX NOT READY YET XXX.
- 1110 A: 2d 3-node Helmholtz DGM element T-4-1.
- 1111 A: 2d 3-node Helmholtz DGM element T-8-2.
- A: 2d 4-node Helmholtz DEM element Q-4-1.
- 1121 A: 2d 4-node Helmholtz DEM element Q-8-2.
- 1122 A: 2d 4-node Helmholtz DEM element Q-16-4.
- 1123 A: 2d 4-node Helmholtz DEM element Q-32-8.
- 1130 A: 2d 3-node Helmholtz DEM element T-4-1.
- 1131 A: 2d 3-node Helmholtz DEM element T-8-2.

1150	A: 3d 8-node Helmholtz DGM element H-6-1.
1151	A: 3d 8-node Helmholtz DGM element H-26-4.
1152	A: 3d 8-node Helmholtz DGM element H-56-8.
1153	A: 3d 8-node Helmholtz DGM element H-98-12.
1160	A: 2d 3-node Helmholtz DGM element T-6-1.
1161	A: 2d 3-node Helmholtz DGM element T-26-4.
1162	A: 2d 3-node Helmholtz DGM element T-56-8.
1170	A: 3d 8-node Helmholtz DEM element H-6-1.
1171	A: 3d 8-node Helmholtz DEM element H-26-4.
1172	A: 3d 8-node Helmholtz DEM element H-56-8.
1173	A: 3d 8-node Helmholtz DEM element H-98-12.
1200	M: 2d 4-node Elastodynamic DGM element Q-4x2-2.
1201	M: 2d 4-node Elastodynamic DGM element Q-16x2-8.
1220	M: 2d 4-node Elastodynamic DEM element Q-4x2-2.
1250	M: 3d 4-node Elastodynamic DGM element H-6x3-3.
1251	M: 3d 4-node Elastodynamic DGM element H-26x3-15.
1252	M: 3d 4-node Elastodynamic DGM element H-50x3-28.
	These should be listed in a stacked fashion on a single line.
CONNECTIVITY_NODES	

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Next: <u>USDD</u>, Previous: <u>TOPOLOGY</u>

97 USER DEFINED FORCES *S*

Command Statement:	USDF
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The purpose of the USDF command is to specify nodal forces via a user-defined subroutine. In this case, the user should: 1) write his/her own algorithm for specifying the nodal forces within a subroutines named "control.C", 2) compile this subroutine and link it using the makefile provided for this command, and 3) use the LOAD command (see LOAD) to activate it. An example input file using the USDF command can be found in FEM.d/fem\examples/USDF.d/.

User-defined nodal forces are not assembled neither at the element level nor at the subdomain level. They can be combined with those specified under the FORC and GRAV command statments but will not be amplified by the MFTT or HFTT tables. In case of a conflict between this command and the FORC and GRAV commands, the specified nodal forces are simply added. A more detailed description of the "control.C" subroutine is given in its template located in the Control.d directory of **AERO-S**.

The syntax for invoking this option is given below.

Note 1: Similarly, this command can be used to specify time-variant prescribed Neumann boundary conditions (or source terms) for a time-domain acoustic simulation.





NODE# Node number where the force is specified (integer).

DOF# Degree of freedom local number where the force is specified (integer).

Note 1: All forces must be specified in the computational basis.

Note 2: The USDF command is redundant with the ACTUATORS command (see <u>ACTUATORS</u>) in the sense that ACTUATORS can achieve whatever USDF can achieve. However, the reverse is not true.

Next: WEIGHTS, Previous: USDF

98 USER DEFINED PRESCRIBED DISPLACEMENTS *S*



The purpose of the USDD command is to specify time-variant prescribed displacements for any node and its associated dofs using a user-defined subroutine. In this case, the user should: 1) write his/her own algorithm for specifying the prescribed displacement field within a subroutine named "control.C", 2) compile this subroutine and link it with **AERO-S** using the makefile that is provided for this purpose, and 3) use the LOAD command (see <u>LOAD</u>) to activate it. An example input file using the USDD command can be found in FEM.d/fem_examples/USDD.d/.

This command can be used simultaneously with the time-invariant command DISP for prescribing displacements at a node. In the event of a conflict between the DISP and USDD commands, USDD prevails. A more detailed description of the "control.C" subroutine is given in its template located in the Control.d directory of **AERO-S**.

The syntax for invoking this option is given below.

Note 1: Similarly, this command can be used to specify time-variant prescribed Dirichlet boundary conditions for a time-domain acoustic simulation by setting DOF# to 8 (see below).

USDD

NODE# DOF#

NODE# Node number where the displacement is specified (integer).

DOF#

Degree of freedom local number where the displacement is specified (integer).

Next: YMTT, Previous: USDD

99 WEIGHTS



The WEIGHTS command statement can be used to modify some or all of the weights attributed by default to an element type (see <u>TOPOLOGY</u>). These weights are exploited by the mesh partitioning algorithm (see <u>DECOMPOSE</u>) to achieve load balance when generating the subdomains.

Note 1: Variable weights attributed by default to elements with a variable number of nodes cannot be modified by this command.

The input format of this command is given below.

WEIGHTS

ELEMENT_TYPE#	DESIRED_WEIGHT
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ELEMENT_TYPE# This is the element type id number as in the TOPOLOGY command (integer).

DESIRED_WEIGHT This is the desired weight to be attributed to this element type (integer).

The weight default values are as follows:

M=Mechanics H=Thermal C=Coupled Thermoelastic F=Fluid A=Acoustic W = default weight

- 1 M: 3d truss (bar) element with 3 dof/node (W = 1).
- 2 M: 2d 4-node quad element with 2 dof/node (W = 2).
- 3 H: 3d 4-node quad element with 1 dof/node (W = 2).
- 4 M: 2d 3-node triangular element with 2 dof/node (W = 2).
- 6 M: 3d Bernoulli beam element with 6 dof/node (W = 1).
- 7 M: 3d Timoshenko beam element with 6 dof/node (W = 1).
- 8 M: $3d\ 3$ -AQR shell element with 6 dof/node (**W** = **3**).
- 88 M: 3d 4-node shell element element with 6 dof/node (splits into 2 elements of type 8) (W = 4).
- 9 H: 3d 2-node lineal element with 1 dof/node (W = 1).
- H: 2d 4-node quad element with 1 dof/node ($\mathbf{W} = \mathbf{2}$).
- 11 M: 3d torsion spring element with 3 dof/node (W = 1).

53

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12
              C: 3d thermomelastic truss element with 4 dof/node (W = 1).
13
              C: 2d thermoelastic quad element with 3 dof/node (W = 2).
14
              C: 3d thermoelastic Bernoulli beam element with 7 dof/node (W = 1).
16
              M: 3d Belytschko-Tsay shell element with 6 dof/node (W = 3).
17
              M: 3d 8-node brick element with 3 dof/node (W = 3).
18*
              M: 3d 4-node shear panel element with 3 dof/node (W = 1).
19
              M: 3d 3-node membrane element with 6 dof/node (W = 3).
              (only in-plane and drilling stiffnesses) (W = 1).
              M: 3d 3-node composite or orthotropic shell element with 6 dof/node (W = 3).
20
2020
              M: 3d 4-node composite or orthotropic shell element with 6 dof/node (splits into 2 elements of type 20) (W =
              3).
21
              M: 3d translational spring-link element with 3 dof/node (W = 1).
22
              M: 3d torsion spring-link element with 3 dof/node (W = 1).
23
              M: 3d 4-node tetrahedral element with 3 dof/node (W = 3).
24
              M: 3d 6-node pentahedral element with 3 dof/node (W = 3).
25
              M: 3d 10-node tetrahedral element with 3 dof/node (W = 4).
30
              A: 2d 4-node quad element with 1 dof/node (W = 1).
31*
              A: 2d 4-node quad GLS element with 1 dof/node (W = 1).
32*
              A: 2d 8-node quad element with 1 dof/node (W = 3).
33*
              A: 2d 4-node quad BUBBLE element with 1 dof/node (W = 1).
34*
              A: 2d 4-node quad two-level BUBBLE element with 1 dof/node (W = 1).
35
              A: 2d 3-node triangular element with 1 dof/node (W = 1).
36
              A: 2d 3-node triangular GLS element with 1 dof/node (W = 1).
38*
              A: 2d 6-node triangular element with 1 dof/node (W = 3).
40
              A: 3d 4-node tetrahedral element with 1 dof/node (W = 2).
41*
              A: 3d 4-node tetrahedral GLS element with 1 dof/node (W = 2).
42
              A: 3d 10-node tetrahedral element with 1 dof/node (W = 3).
44
              A: 3d 8-node brick GLS element with 1 dof/node (W = 3).
45
              A: 3d 8-node brick element with 1 dof/node (W = 3).
46
              H: 3d 3-node triangular heat element (W = 2).
4646
              H: 3d 4-node triangular heat element (splits into 2 elements of type 46) (W = 3).
47
              H: 3d 2-node lineal element with 1 dof/node for boundary convection (W = 1).
48
              H: 3d 4-node quad element with 1 dof/node for boundary convection (W = 2).
49
              H: 3d 3-node triangular element with 1 dof/node for boundary convection (\mathbf{W} = \mathbf{2}).
50
              H: 3d 4-node tetrahedral element with 1 dof/node (W = 3).
51
              H: 3d 8-node brick element with 1 dof/node (W = 4).
52*
              M: 3d 6-node triangular shell element (W = 1).
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H: 2d 3-node triangular heat element (W = 2).

- H: 3d 2-node heat radiation element (W = 1).
- H: 3d 3-node triangular heat radiation element (W = 2).
- H: 3d 4-node quadrilateral heat radiation element (W = 2).
- 65 M: 3d 2-node rigid truss (bar) element (W = 1).
- M: 3d 2-node rigid beam element (W = 1).
- 67 M: 3d rigid link (translational and rotational) (W = 1).
- 68 M: 3d rigid translational link (W = 1).
- 69 M: 3d rigid rotational link (W = 1).
- 70 M: 3d rigid plane or solid element with 3 dof/node (3-node to 20-node element) (W = 1).
- 71 M: 3d rigid plane or solid element with 3 dof/node and anywhere from 3 to 20 nodes per element (W = 1).
- 72 M: 3d 20-node brick element (W = 4).
- 73 M: 3d 3-node rigid shell element (W = 1).
- 74 M: 3d rigid plane or solid element with 6 dof/node (3-node to 32-node element) (W = 1).
- H: 2d 4-node contact resistance thermal element with 1 dof/node that can be inserted between two thermal elements of type = 10 (W = 2).
- H: 3d 8-node contact resistance thermal element with 1 dof/node that can be inserted between two thermal elements of type = 51 (W = 4).
- H: 3d 6-node contact resistance thermal element with 1 dof/node that can be inserted between two thermal elements of type = 50 with 4 nodes each ($\mathbf{W} = \mathbf{4}$).
- H: 2d 3-node triangular bulk fluid (thermal) element with 1 dof/node. The first node appearing in the connectivity list of this element must be inside the bulk fluid and the other two nodes must be located at the interface boundary between the bulk fluid and the surrounding material. The temperature at the first node is by definition the average temperature of the bulk fluid (W = 2).
- H: 3d 4-node tetrahedral bulk fluid (thermal) element with 1 dof/node. The first node appearing in the connectivity list of this element must be inside the bulk fluid and the other three nodes must be located at the interface boundary between the bulk fluid and the surrounding material. The temperature at the first node is by definition the average temperature of the bulk fluid ($\mathbf{W} = \mathbf{3}$).
- H: 3d 5-node pyramidal bulk fluid (thermal) element with 1 dof/node. The first node appearing in the connectivity list of this element must be inside the bulk fluid and the other four nodes must be located at the interface boundary between the bulk fluid and the surrounding material. The temperature at the first node is by definition the average temperature of the bulk fluid (**W** = **4**).
- A: 3d 6-node wedge element with 1 dof/node ($\mathbf{W} = 3$).
- 91 M: 3d 32-node serendipity brick element with 3 dof/node ($\mathbf{W} = \mathbf{6}$).
- 92 M: 3d 26-node serendipity wedge element with 3 dof/node ($\mathbf{W} = \mathbf{5}$).
- A: 3d 32-node serendipity brick element with 1 dof/node (W = 5).
- A: 3d 26-node serendipity wedge element with 1 dof/node (W = 4).
- A: 3d 8-node, or 27-node, or 64-node, or 125-node hexahedral element with 1 dof/node ($\mathbf{W} = 3, 4, 5, 6$).
- A: 3d 4-node, or 10-node, or 20-node, or 35-node tetrahedral element with 1 dof/node (W = 1, 2, 3, 4).
- 97 M: 3d 15-node serendipity wedge element with 3 dof/node (W = 4).
- A: 2d 4-node, or 9-node, or 16-node, or 25-node quadrilateral element with 1 dof/node (W = 2, 3, 4, 5).

99 A: 2d 3-node, or 6-node, or 10-node triangular element with 1 dof/node (W = 2, 3, 4). 100 M: 2d 4-node, or 9-node, or 16-node, or 25-node quadrilateral element with 3 dof/node (W = 2, 3, 4, 5). 101 M: 2d 3-node, or 6-node, or 10-node triangular element with 3 dof/node (W = 2, 3, 4). 102 M: 3d 8-node, or 27-node, or 64-node, or 125-node hexahedral element with 3 dof/node (W = 2, 3, 4, 5). 103 M: 3d 4-node, or 10-node, or 20-node, or 35-node tetrahedral element with 3 dof/node (W = 1, 2, 3, 4). 105 A: 3d 8-node, or 27-node, or 64-node, or 125-node spectral hexahedral element with 1 dof/node ($\mathbf{W} = \mathbf{2}, \mathbf{3}, \mathbf{4}, \mathbf{4}$ 5). 108 A: 2d 4-node, or 9-node, or 16-node, or 25-node spectral quadrilateral element with 1 dof/node (W = 2, 3, 4, **5**). 111 M: 3d fabric truss element with 3 dof/node (W = 1). 120 M: $3d\ 2$ -node spherical joint element with $3\ dof/node\ (W = 1)$. 122 M: 3d 2-node universal joint element with 6 dof/node (W = 1). 123 M: 3d 2-node revolute joint element with 6 dof/node (W = 1). 124 M: 3d 2-node cylindrical joint element with 6 dof/node (W = 1). 125 M: 3d 2-node prismatic joint element with 6 dof/node (W = 1). 126 M: 3d 2-node actuated revolute joint element with 6 dof/node (W = 1). 127 M: 3d 2-node actuated prismatic joint element with 6 dof/node (W = 1). 201 M: 3d materially nonlinear 8-node brick element with 3 dof/node (W = 3). 202 M: 3d geometrically (Green-Lagrange) + materially nonlinear 8-node brick element with 3 dof/node (W = 3). 301 F: 2d 4-node sloshing (fluid) quadrilateral element with 1 dof/node (W = 1). 302 F: 2d 2-node free-surface (fluid) element with 1 dof/node for two-dimensional sloshing computations using element type = 301 (W = 1). 311 F: 3d 4-node sloshing (fluid) tetrahedron element with 1 dof/node (W = 3). 312 F: 3d 3-node free-surface (fluid) triangular element with 1 dof/node for three-dimensional sloshing computations using element type = 311 (W = 2). 321 F: 2d 4-node hydroelastic vibration (fluid) quadrilateral element with 1 dof/node ($\mathbf{W} = \mathbf{2}$). 331 F: 3d 4-node hydroelastic vibration (fluid) tetrahedral element with 1 dof/node (W = 3). 1100 A: 2d 4-node Helmholtz DGM element Q-4-1 (W = 1). 1101 A: 2d 4-node Helmholtz DGM element Q-8-2 (W = 2). 1102 A: 2d 4-node Helmholtz DGM element Q-16-4 (W = 4). 1103 A: 2d 4-node Helmholtz DGM element Q-32-8 (W = 8). 1104 A: 2d 4-node Helmholtz DGM element Q-8-2star with additional enrichment by evanescent wave functions XXX NOT READY YET XXX (W = 2). 1110 A: 2d 3-node Helmholtz DGM element T-4-1 (W = 1). 1111 A: 2d 3-node Helmholtz DGM element T-8-2 (W = 2). 1120 A: 2d 4-node Helmholtz DEM element Q-4-1 (W = 1). 1121 A: 2d 4-node Helmholtz DEM element Q-8-2 (W = 2). 1122 A: 2d 4-node Helmholtz DEM element Q-16-4 (W = 4).

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A: 2d 4-node Helmholtz DEM element Q-32-8 (W = 8).

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1130	A: 2d 3-node Helmholtz DEM element T-4-1 ($\mathbf{W} = 1$).
1131	A: 2d 3-node Helmholtz DEM element T-8-2 ($\mathbf{W} = 2$).
1150	A: 3d 8-node Helmholtz DGM element H-6-1 ($W = 1$).
1151	A: 3d 8-node Helmholtz DGM element H-26-4 ($\mathbf{W} = 4$).
1152	A: 3d 8-node Helmholtz DGM element H-56-8 ($\mathbf{W} = 8$).
1153	A: 3d 8-node Helmholtz DGM element H-98-12 ($\mathbf{W} = 12$).
1160	A: 2d 3-node Helmholtz DGM element T-6-1 ($W = 1$).
1161	A: 2d 3-node Helmholtz DGM element T-26-4 ($W = 4$).
1162	A: 2d 3-node Helmholtz DGM element T-56-8 ($W = 8$).
1170	A: 3d 8-node Helmholtz DEM element H-6-1 ($W = 1$).
1171	A: 3d 8-node Helmholtz DEM element H-26-4 ($W = 4$).
1172	A: 3d 8-node Helmholtz DEM element H-56-8 ($\mathbf{W} = 8$).
1173	A: 3d 8-node Helmholtz DEM element H-98-12 ($\mathbf{W} = 12$).
1200	M: 2d 4-node Elastodynamic DGM element Q-4x2-2 ($\mathbf{W} = 2$).
1201	M: 2d 4-node Elastodynamic DGM element Q-16x2-8 ($\mathbf{W} = 8$).
1220	M: 2d 4-node Elastodynamic DEM element Q-4x2-2 ($\mathbf{W} = 2$).
1250	M: 3d 4-node Elastodynamic DGM element H-6x3-3 ($W = 3$).
1251	M: 3d 4-node Elastodynamic DGM element H-26x3-15 ($\mathbf{W} = 15$).
1252	M: 3d 4-node Elastodynamic DGM element H-50x3-28 ($W = 28$).

Previous: WEIGHTS

100 YOUNGS MODULUS TEMPERATURE TABLE

Command Statement: YMTT

The YMTT command statement can be used to describe the evolution of Young's modulus with temperature, for a given material. This evolution can be specified here in a curve (or table) defined by pairs of temperature and Young's modulus values. Linear interpolation is used for "in between" points, and the extrema values are adopted for "outside" points. Several curves can be specified, one after the other. Each curve is identified by an "id number" as described below.

YMTT

CURVE curve_id
T_1 YM_1
.

|--|

CURVE

curve_id "Id number" for the following curve (or table) (integer).

T_1 A specified temperature value (float).

YM_1 A specified Young's modulus value at temperature T_1 (float).