# Partitioned Finite Element Method for the Mindlin Plate as a Port-Hamiltonian system \*

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Abstract: The port-Hamiltonian framework allows for a structured representation and interconnection of distributed parameter systems described by Partial Differential Equations (PDE) from different realms. Here, the Mindlin-Reissner model of a thick plate is presented in a tensorial formulation. Taking into account collocated boundary control and observation gives rise to an infinite-dimensional port-Hamiltonian system (pHs). The Partitioned Finite Element Method (PFEM), already presented in our previous work, allows obtaining a structure-preserving finite-dimensional port-Hamiltonian system, and accounting for boundary control in a straightforward manner. In order to illustrate the flexibility of PFEM, both types of boundary controls can be dealt with: either through forces and momenta, or through kinematic variables. The discrete model is easily implementable by using the FEniCS platform. Computation of eigenfrequencies and vibration modes, together with time-domain simulation results demonstrate the consistency of the proposed approach.

Keywords: Port-Hamiltonian systems (pHs), Geometric Discretization, Mindlin-Reissner Plate, Partitioned Finite Element Method (PFEM), Symplectic Integration

# 1. INTRODUCTION

Distributed parameter systems are of relevant interest given the increased computational power available for simulations. The port-Hamiltonian (pH) framework allows for a structured representation of PDEs coming from different realms. Continuum mechanics, thermodynamics, electromagnetism can be all represented (Duindam et al. (2009); Jacob and Zwart (2012)). In order to simulate and control such systems, a finite dimensional representation of the distributed system has to be found and it can be advantageous to use a discretization procedure that preserves the port-Hamiltonian nature. If so the preservation, at a discrete level, of the energy balance and associated dynamical properties (e.g. stability, passivity, controllability, etc.) is guaranteed.

The first attempt to perform a structure-preserving discretization dates back to Golo et al. (2004), where the authors proposed a mixed finite element spatial discretization for 1D hyperbolic system of conservation laws. Pseudospectral methods relying on higher-order global polynomial approximations were studied in Moulla et al. (2012) and simplicial discretization based on discrete exterior

calculus was proposed in Seslija et al. (2012). A 2D finite difference method with staggered grids was used in Trenchant et al. (2018). Weak formulations which lead to Galerkin numerical approximations began to be explored in the last years. In Kotyczka et al. (2018) the prototypical example of hyperbolic systems of two conservation law was discretized by a weak formulation. The Finite Element Method is not the discretization technique of choice in the pH framework. In fact, these system have been studied in functional analysis mainly in one spatial dimension (Villegas (2007); Augner and Jacob (2016)), except for Kurula and Zwart (2015), but the analyses herein is only theoretical.

In this paper we recover the Mindlin plate port Hamiltonian formulation (Macchelli et al. (2005)). However, instead of using a purely vectorial representation for the co-energy and energy variables, each physical variable is accounted for considering its nature. Mathematically, this means that scalar, vectorial and tensorial objects will co-exist in the pH formulation of the Mindlin plate (Brugnoli et al. (2018)). The reader can consult (Grinfeld, 2015, Chapter 16) where tensorial calculus and Hamiltonian formalism are illustrated in the context of PDEs . The discretization procedure relies, in this article, on the partitioned Finite Element method (PFEM). This method, originally presented in Cardoso-Ribeiro et al. (2018), is an extension of the Mixed Finite Element Method to the case of pH systems and requires the integration by parts to be

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performed so that the symplectic structure is preserved. It consist in three different steps:

- (1) the system is first put into weak form;
- once the boundary control of interest is selected, the corresponding subsystem is integrated by parts;
- (3) the problem is discretized by using a Mixed Finite Element method.

The final system will contain boundary control and observation. Boundary conditions not appearing explicitly in the boundary term might be imposed strongly and included in the functional space or weakly (section 4.1). If the latter possibility is employed, the discretized system is an algebraic differential one (pHDAEs), which can be analyzed by referring to Beattie et al. (2018); van der Schaft (2013).

This paper starts with the presentation of the Mindlin Plate model in strong form as a port-Hamiltonian system. Then the weak form is presented for two different choices of boundary control. The discretization procedure is then detailed. Computation of modes considering different boundary conditions and temporal simulation are then performed by using FEniCS (Logg et al. (2012)).

### 2. PH FORMULATION OF THE MINDLIN PLATE

In this section the classical formulation of the Mindlin plate is recalled. Then the tensorial pH formulation is illustrated. The boundary variables are highlighted thanks to the energy balance.

#### 2.1 Notations

First, the differential operators needed for the following are recalled. For a vector field  $\boldsymbol{u}: \mathbb{R}^d \to \mathbb{R}^d$  the gradient is defined as

$$\operatorname{grad}(\boldsymbol{u}) = \nabla \boldsymbol{u} := \begin{pmatrix} \partial_{x_1} u_1 & \dots & \partial_{x_d} u_1 \\ \vdots & & \vdots \\ \partial_{x_1} u_d & \dots & \partial_{x_d} u_d \end{pmatrix}.$$

The symmetric part of this operator corresponds to the deformation gradient in continuum mechanics:

$$\operatorname{Grad}(\boldsymbol{u}) := rac{1}{2} \left( \nabla \boldsymbol{u} + \nabla^T \boldsymbol{u} \right).$$

For a vector field  $\boldsymbol{u}: \mathbb{R}^d \to \mathbb{R}^d$ , the divergence operator reads

$$\operatorname{div}(\boldsymbol{u}) = \nabla \cdot \boldsymbol{u} := \sum_{i=1}^d \partial_{x_i} u_i.$$

For a tensor field  $U : \mathbb{R}^d \to \mathbb{R}^{d \times d}$ , with elements  $u_{ij}$ , the divergence is a vector defined column-wise as

$$\mathrm{Div}(\boldsymbol{U}) = \nabla \cdot \boldsymbol{U} := \left(\sum_{i=1}^d \partial_{x_i} u_{ij}\right)_{j=1,\dots,d}.$$

Furthermore,  $\mathbb{V}$  denotes the space of  $\mathbb{R}^d$  vectors and  $\mathbb{S}$  denotes the space of symmetric  $d \times d$  matrices. The geometrical dimension for the problem of interest is d = 2.

### 2.2 Mindlin-Reissner Model for Thick Plates

The Mindlin Model is a generalization to the 2D case of the Timoshenko beam model and accounts for the shear deformation. Given an open and connected set  $\Omega \in \mathbb{R}^2$ , the classical equations for this model (Mindlin (1951)) are

$$\begin{cases} \rho h \frac{\partial^2 w}{\partial t^2} &= \operatorname{div}(\boldsymbol{q}), \\ \rho \frac{h^3}{12} \frac{\partial^2 \boldsymbol{\theta}}{\partial t^2} &= \boldsymbol{q} + \operatorname{Div}(\boldsymbol{M}), \end{cases}$$
(1)

where  $\rho$  is the material density, h is the plate thickness, the scalar w is the vertical displacement, the vector  $\boldsymbol{\theta} = (\theta_x, \theta_y)^T \in \mathbb{V}$  collects the deflection of the cross section along axes x and y respectively. The symmetric momenta tensor  $\boldsymbol{M} \in \mathbb{S}$  has components (x denotes index 1, y index 2)

$$\begin{split} m_{xx} &= D \left( \frac{\partial \theta_x}{\partial x} + \nu \frac{\partial \theta_y}{\partial y} \right), \\ m_{yy} &= D \left( \frac{\partial \theta_y}{\partial y} + \nu \frac{\partial \theta_x}{\partial x} \right), \\ m_{xy} &= D \frac{(1 - \nu)}{2} \left( \frac{\partial \theta_x}{\partial x} + \frac{\partial \theta_y}{\partial y} \right), \end{split}$$

while for the shear stress vector q, the components are

$$q_x = kGh\left(\frac{\partial w}{\partial x} - \theta_x\right), \quad q_y = kGh\left(\frac{\partial w}{\partial y} - \theta_y\right).$$

with  $\nu$  the Poisson's ratio, D the bending module, G the stiffness module and the correction factor k=5/6 (Mindlin (1951)). The variables conjugated to the momenta tensor and to the shear stress vector are the curvatures tensor K and the shear strain  $\epsilon_s$  respectively, defined as

$$K := Grad(\theta), \quad \epsilon_s := grad(w) - \theta.$$

The kinetic and potential energy density K and U read

$$\mathcal{K} = \frac{1}{2} \left\{ \rho h \left( \frac{\partial w}{\partial t} \right)^2 + \frac{\rho h^3}{12} \frac{\partial \boldsymbol{\theta}}{\partial t} \cdot \frac{\partial \boldsymbol{\theta}}{\partial t} \right\}, 
\mathcal{U} = \frac{1}{2} \left\{ \boldsymbol{M} : \boldsymbol{K} + \boldsymbol{q} \cdot \boldsymbol{\epsilon}_s \right\},$$
(2)

where  $M: K := \sum_{i,j} m_{ij} \kappa_{ij}$  is the tensor contraction. The Hamiltonian is easily written as

$$H = \int_{\Omega} (\mathcal{K} + \mathcal{U}) \, d\Omega. \tag{3}$$

## 2.3 Tensorial Port-Hamiltonian formulation

In order to rewrite the system as a port-Hamiltonian one, the energy variables have to be selected first. This choice is analogous to that of the pH Timoshenko beam model (Jacob and Zwart (2012)), but with the additional complication that the variables here are of scalar, vectorial and tensorial nature:

al nature:  

$$\alpha_w = \rho h \frac{\partial w}{\partial t}, \qquad \alpha_\theta = \frac{\rho h^3}{12} \frac{\partial \boldsymbol{\theta}}{\partial t},$$
  
 $\boldsymbol{A}_\kappa = \boldsymbol{K}, \qquad \alpha_{\epsilon_s} = \epsilon_s.$  (4)

The co-energy variables are found by computing the variational derivative of the Hamiltonian

$$e_{w} := \frac{\delta H}{\delta \alpha_{w}} = \frac{\partial w}{\partial t}, \qquad e_{\theta} := \frac{\delta H}{\delta \alpha_{\theta}} = \frac{\partial \theta}{\partial t},$$

$$E_{\kappa} := \frac{\delta H}{\delta A_{\kappa}} = M, \qquad e_{\epsilon_{s}} := \frac{\delta H}{\delta \alpha_{\epsilon_{s}}} = q.$$
(5)

Proposition 1. (Brugnoli et al. (2018)).

The variational derivative of the Hamiltonian with respect to the curvatures tensor is the momenta tensor

$$\frac{\delta H}{\delta \boldsymbol{A}_{\kappa}} = \boldsymbol{M}.$$

The port-Hamiltonian system is expressed as follows

$$\begin{cases}
\frac{\partial \alpha_w}{\partial t} &= \operatorname{div}(\boldsymbol{e}_{\epsilon_s}), \\
\frac{\partial \boldsymbol{\alpha}_{\theta}}{\partial t} &= \operatorname{Div}(\boldsymbol{E}_{\kappa}) + \boldsymbol{e}_{\epsilon_s}, \\
\frac{\partial \boldsymbol{A}_{\kappa}}{\partial t} &= \operatorname{Grad}(\boldsymbol{e}_{\theta}), \\
\frac{\partial \boldsymbol{\alpha}_{\epsilon_s}}{\partial t} &= \operatorname{grad}(\boldsymbol{e}_w) - \boldsymbol{e}_{\theta},
\end{cases}$$
(6)

If the variables are concatenated together, the formally skew-symmetric operator J can be highlighted

$$\frac{\partial}{\partial t} \begin{pmatrix} \alpha_w \\ \boldsymbol{\alpha}_{\theta} \\ \boldsymbol{A}_{\kappa} \\ \boldsymbol{\alpha}_{\epsilon_s} \end{pmatrix} = \underbrace{\begin{bmatrix} 0 & 0 & 0 & \text{div} \\ 0 & 0 & \text{Div } \boldsymbol{I}_{2\times 2} \\ 0 & \text{Grad } & 0 & 0 \\ \text{grad } -\boldsymbol{I}_{2\times 2} & 0 & 0 \end{bmatrix}}_{I} \begin{pmatrix} e_w \\ \boldsymbol{e}_{\theta} \\ \boldsymbol{E}_{\kappa} \\ \boldsymbol{e}_{\epsilon_s} \end{pmatrix}, \quad (7)$$

where  $I_{2\times 2}: \mathbb{V} \to \mathbb{V}$  is the identity operator.

Remark 1. It can be observed that the interconnection structure given by J in (7) mimics that of the Timoshenko beam in one spatial dimension.

Theorem 2. (Brugnoli et al. (2018)). The adjoint of the tensor divergence Div is -Grad, the opposite of the symmetric gradient.

The boundary values can be found by evaluating the time derivative of the Hamiltonian (Brugnoli et al. (2018))

$$\dot{H} = \int_{\partial\Omega} \{ w_t q_n + \omega_n m_{nn} + \omega_s m_{ns} \} \, \mathrm{d}s. \tag{8}$$

where s is the curvilinear abscissa and the last integral is obtained by applying the Green-Gauss theorem. The boundary variable are defined as follows

Shear Force 
$$q_n := \boldsymbol{e}_{\epsilon_s} \cdot \boldsymbol{n}$$
,  
Flexural momentum  $m_{nn} := \boldsymbol{E}_{\kappa} : (\boldsymbol{n} \otimes \boldsymbol{n})$  (9)  
Torsional momentum  $m_{ns} := \boldsymbol{E}_{\kappa} : (\boldsymbol{s} \otimes \boldsymbol{n})$ ,

where  $u \otimes v$  denotes the outer product of vectors equivalent to a matrix given by  $uv^T$ . Vectors n and s designate the normal and tangential unit vector to the boundary. The corresponding power conjugated variables are

Vertical velocity 
$$w_t := e_w$$
,  
Flexural rotation  $\omega_n := e_\theta \cdot n$  (10)  
Torsional rotation  $\omega_s := e_\theta \cdot s$ .

Remark 2. Obviously this result is equivalent to the one found in the vectorial case (see Macchelli et al. (2005)), but the tensorial formulation is more suitable from the point of view of functional analysis since it makes appear intrinsic operators (div, Div, grad, Grad). The operators are here defined in a pure Euclidian patch and are the specialization of the exterior derivation of forms on manifolds, which is the coordinate free operator transforming coenergy variables in energy variables in the classical distributed formulation (see Golo et al. (2004)).

Remark 3. System (7) has an associated Stokes-Dirac Structure (Macchelli et al. (2005)) and therefore dissipa-

tion and distributed forces can be added by introducing the specific ports.

### 3. WEAK FORM

In this section the first two steps of the PFEM method are carried out. First the system is put into weak form. Then depending on the choice of the boundary inputs a subset of equations is integrated by parts (see Sections. 3.1 and 3.2). The final weak form is then ready to be discretized to produce a finite dimensional pH system (see Sec. 4).

In order to put the system into weak form the first line of (7) is multiplied by  $v_w$  (multiplication by a scalar), the second line and the fourth by  $v_{\theta}$ ,  $v_{\epsilon_s}$  (scalar product in  $\mathbb{R}^2$ ) and the third one by  $V_{\kappa}$  (tensor contraction).

$$\int_{\Omega} v_w \frac{\partial \alpha_w}{\partial t} d\Omega = \int_{\Omega} v_w \text{div}(\boldsymbol{e}_{\epsilon_s}) d\Omega,$$
 (11)

$$\int_{\Omega} \boldsymbol{v}_{\theta} \cdot \frac{\partial \boldsymbol{\alpha}_{\theta}}{\partial t} d\Omega = \int_{\Omega} \boldsymbol{v}_{\theta} \cdot (\text{Div}(\boldsymbol{E}_{\kappa}) + \boldsymbol{e}_{\epsilon_{s}}) d\Omega, \quad (12)$$

$$\int_{\Omega} \mathbf{V}_{\kappa} : \frac{\partial \mathbf{A}_{\kappa}}{\partial t} d\Omega = \int_{\Omega} \mathbf{V}_{\kappa} : \operatorname{Grad}(\mathbf{e}_{\theta}) d\Omega, \tag{13}$$

$$\int_{\Omega} \boldsymbol{v}_{\epsilon_s} \cdot \frac{\partial \boldsymbol{\alpha}_{\epsilon_s}}{\partial t} d\Omega = \int_{\Omega} \boldsymbol{v}_{\epsilon_s} \cdot (\operatorname{grad}(\boldsymbol{e}_w) - \boldsymbol{e}_{\theta}) d\Omega. \quad (14)$$

To simplify the notation, all test and unknown functions can be collected in one variable

$$v := (v_w, \mathbf{v}_{\theta}, \mathbf{V}_{\kappa}, \mathbf{v}_{\epsilon_s}),$$

$$\alpha := (\alpha_w, \mathbf{\alpha}_{\theta}, \mathbf{A}_{\kappa}, \mathbf{\alpha}_{\epsilon_s}),$$

$$e := (e_w, \mathbf{e}_{\theta}, \mathbf{E}_{\kappa}, \mathbf{e}_{\epsilon_s}),$$
(15)

so that the previous system is rewritten compactly as

$$\left(v, \frac{\partial \alpha}{\partial t}\right) = (v, Je),\tag{16}$$

where the bilinear form  $(v, u) = \int v \cdot u \, d\Omega$ , is the inner product on space  $\mathscr{L}^2(\Omega) := L^2(\Omega) \times L^2(\Omega; \mathbb{V}) \times L^2(\Omega; \mathbb{S}) \times L^2(\Omega; \mathbb{V})$ . The operator J was defined in equation (7). It can be decomposed into the sum of three operators

$$J = J_{\text{div}} + J_{\text{grad}} + J_{I}, \tag{17}$$

where  $J_{\text{div}}$ ,  $J_{\text{grad}}$ ,  $J_{I}$  contain only the divergence operators (div, Div), gradient operators (grad, Grad) and algebraic part (identity matrices) respectively.

The integration by part has to be performed so that the final bilinear form on the right-hand side remains skew-symmetric. Obviously, since J is skew-symmetric  $J_{\rm div}=-J_{\rm grad}^*$ , where  $A^*$  is the formal adjoint of operator A. Depending on which of the two differential operators  $J_{\rm div}$  and  $J_{\rm grad}$  is chosen for the integration by parts, two different boundary controls can arise. Other choices are possible but to simplify the presentation only these two cases are illustrated here.

## 3.1 Boundary control through forces and momenta

If the operator  $J_{\text{div}}$  is integrated by parts (meaning that the right-hand side of equations (11), (12) is integrated by parts) then

$$(v, Je) = j_{\text{grad}}(v, e) + f_N(v), \tag{18}$$

where now the bilinear form

$$j_{\text{grad}}(v, e) = (J_{\text{div}}^* v, e) + (v, J_{\text{grad}} e) + (v, J_{I} e)$$

is skew symmetric and can be expressed as follows

$$j_{\text{grad}}(v, e) := -\int_{\Omega} \operatorname{grad}(v_w) \cdot \boldsymbol{e}_{\epsilon_s} d\Omega$$

$$-\int_{\Omega} \left\{ \operatorname{Grad}(\boldsymbol{v}_{\theta}) : \boldsymbol{E}_{\kappa} - \boldsymbol{v}_{\theta} \cdot \boldsymbol{e}_{\epsilon_s} \right\} d\Omega$$

$$+ \int_{\Omega} \boldsymbol{V}_{\kappa} : \operatorname{Grad}(\boldsymbol{e}_{\theta}) d\Omega$$

$$+ \int_{\Omega} \boldsymbol{v}_{\epsilon_s} \cdot (\operatorname{grad}(\boldsymbol{e}_w) - \boldsymbol{e}_{\theta}) d\Omega.$$
(19)

The linear functional  $f_N(v)$  represents the boundary term associated with forces and momenta. The subscript Ndenotes the fact that classical Neumann conditions appear as boundary input. It reads

$$f_N(v) = \int_{\partial\Omega} \left\{ v_w q_n + v_{\omega_n} m_{nn} + v_{\omega_s} m_{ns} \right\} ds, \qquad (20)$$

where  $v_{\omega_n} = \boldsymbol{v}_{\theta} \cdot \boldsymbol{n}$ ,  $v_{\omega_s} = \boldsymbol{v}_{\theta} \cdot \boldsymbol{s}$ . In this first case, the boundary controls  $u_{\partial}$  and the corresponding output  $y_{\partial}$ 

$$oldsymbol{u}_{\partial} = egin{pmatrix} q_n \ m_{nn} \ m_{ns} \end{pmatrix}_{\partial\Omega}, \qquad oldsymbol{y}_{\partial} = egin{pmatrix} w_t \ \omega_n \ \omega_s \end{pmatrix}_{\partial\Omega}.$$

# 3.2 Boundary control through kinematic variables

Alternatively, if the operator  $J_{\text{grad}}$  is integrated by parts (meaning that the right-hand side of equations (13), (14) is integrated by parts) then

$$(v, Je) = j_{\text{div}}(v, e) + f_D(v),$$
 (21)

where now the bilinear form

$$j_{\text{div}}(v, e) = (J_{\text{grad}}^* v, e) + (v, J_{\text{div}} e) + (v, J_{I} e)$$

is skew symmetric and can be expressed as follows

$$j_{\text{div}}(v, e) := + \int_{\Omega} v_w \text{div}(\boldsymbol{e}_{\epsilon_s}) d\Omega$$

$$+ \int_{\Omega} \boldsymbol{v}_{\theta} \cdot (\text{Div}(\boldsymbol{E}_{\kappa}) + \boldsymbol{e}_{\epsilon_s}) d\Omega$$

$$- \int_{\Omega} \text{Div}(\boldsymbol{V}_{\kappa}) \cdot \boldsymbol{e}_{\theta} d\Omega$$

$$- \int_{\Omega} \{\text{div}(\boldsymbol{v}_{\epsilon_s}) e_w + \boldsymbol{v}_{\epsilon_s} \cdot \boldsymbol{e}_{\theta}\} d\Omega.$$
(22)

The linear functional  $f_D(v)$  represents the boundary term associated with linear and angular velocities. The subscript D denotes the fact that classical Dirichlet conditions appear as boundary input. It reads

$$f_D(v) = \int_{\partial \Omega} \{ v_{q_n} w_t + v_{m_{nn}} \omega_n + v_{m_{ns}} \omega_s \} ds,$$
 (23)

where  $v_{m_{nn}} = \mathbf{V}_{\kappa} : (\mathbf{n} \otimes \mathbf{n}), \ v_{m_{ns}} = \mathbf{V}_{\kappa} : (\mathbf{s} \otimes \mathbf{n})$  and  $v_{q_n} = \mathbf{v}_{\epsilon_s} \cdot \mathbf{n}$ . In this second case, the boundary controls  $u_{\partial}$  and the corresponding output  $y_{\partial}$  are

$$m{u}_{\partial} = \begin{pmatrix} w_t \\ \omega_n \\ \omega_s \end{pmatrix}_{\partial\Omega}, \qquad m{y}_{\partial} = \begin{pmatrix} q_n \\ m_{nn} \\ m_{ns} \end{pmatrix}_{\partial\Omega}.$$

# 4. DISCRETIZATION PROCEDURE

In this section the numerical implementation is detailed. First the inclusion of generic homogeneous boundary conditions is discussed in § 4.1. Then the construction of the finite dimensional system is explained in 4.2, followed by a suitable selection of the finite element space (section 4.3).

# 4.1 Boundary conditions

For the numerical study, the first discretization procedure, which makes use of the skew-symmetric bilinear form  $j_{\text{grad}}$ , is used. The problem to be discretized is the following

$$\left(v, \frac{\partial \alpha}{\partial t}\right) = j_{\text{grad}}(v, e) + f_N(v).$$
 (24)

The variables have to be regular enough so that the bilinear form  $j_{\rm grad}(v,e)$  is defined. This is the case if

$$(e_w, v_w) \in H^1(\Omega), (e_\theta, v_\theta) \in H^1(\Omega; \mathbb{V}).$$
 (25)

For what concerns the other variables the trace operator has to be defined. A suitable choice is the space  $H^1$ :

$$(\boldsymbol{E}_{\kappa}, \boldsymbol{V}_{\kappa}) \in H^{1}(\Omega, \mathbb{S}), \quad (\boldsymbol{e}_{\epsilon_{s}}, \boldsymbol{v}_{\epsilon_{s}}) \in H^{1}(\Omega; \mathbb{V}).$$
 (26)

For this formulation the Neumann boundary conditions (BC) are weakly imposed by the boundary term  $f_N(v)$ . The possible homogeneous boundary conditions that will be considered are:

- Clamped (C) on  $\Gamma_C \subseteq \partial\Omega$ :  $w_t = 0$ ,  $\omega_n = 0$ ,  $\omega_s = 0$ ;
- Simply supported hard (S) on  $\Gamma_S \subseteq \partial \Omega$ :  $w_t =$ 0,  $m_{nn}=0$ ,  $\omega_s=0$ ; Free (F) on  $\Gamma_F \subseteq \partial \Omega$ :  $q_n=0$ ,  $m_{nn}=0$ ,  $m_{ns}=0$ .

Necessarily, it must hold  $\partial\Omega = \overline{\Gamma}_C \cup \overline{\Gamma}_S \cup \overline{\Gamma}_F$ . The set  $\Gamma_C$  or  $\Gamma_S$  could be empty. The classical Dirichlet BC can be imposed strongly or weakly. In the first case, this means that the functional spaces for  $e_w$ ,  $e_\theta$  incorporate the Dirichlet BC. If on the contrary the boundary conditions are imposed weakly, the linear functional  $f_N(v)$  becomes a bilinear form so that Lagrange multipliers are introduced into the problem:

$$b_D(v,\lambda) = + \int_{\Gamma_C} \left\{ v_w \lambda_{q_n} + v_{\omega_n} \lambda_{m_{nn}} + v_{\omega_s} \lambda_{m_{ns}} \right\} ds$$
$$+ \int_{\Gamma_S} \left\{ v_w \lambda_{q_n} + v_{\omega_s} \lambda_{m_{ns}} \right\} ds. \tag{27}$$

The Lagrange multiplier correspond to the reaction forces and momenta at the boundary where Dirichlet conditions hold. The Dirichlet BC are then imposed weakly by nullifying the bilinear form

$$b_D(v_\lambda, e) = + \int_{\Gamma_C} \left\{ v_{q_n} w_t + v_{m_{nn}} \omega_n + v_{m_{ns}} \omega_s \right\} ds$$
$$+ \int_{\Gamma_C} \left\{ v_{q_n} w_t + v_{m_{ns}} \omega_s \right\} ds. \tag{28}$$

The other quantities of interest were defined in (10). For homogeneous BC it holds  $b_N(v_\lambda, e) = 0$ . The final system reads

$$\begin{cases}
\left(v, \frac{\partial \alpha}{\partial t}\right) = j_{\text{grad}}(v, e) + b_D(v, \lambda) \\
0 = b_D(v_\lambda, e)
\end{cases}$$
(29)

#### 4.2 Finite Dimensional System

An equivalent vectorial formulation for the tensorial variables is now used to illustrate the discretization procedure. In fact the inner product of symmetric tensors  $\int V_{\kappa}: E_{\kappa} d\Omega$  can be equivalently rewritten by making use of a vectorial formulation

$$\int \boldsymbol{V}_{\kappa} : \boldsymbol{E}_{\kappa} d\Omega = \int \boldsymbol{v}_{\kappa} : \boldsymbol{e}_{\kappa} d\Omega,$$

where  $\mathbf{v}_{\kappa} = (v_{xx}, v_{yy}, \sqrt{2} v_{xy})^T$  and  $\mathbf{e}_{\kappa} = (e_{xx}, e_{yy}, \sqrt{2} e_{xy})^T$ . The Div and Grad operator are now replaced by their equivalent representation for vectors:

$$\operatorname{Div}_{v} = \begin{pmatrix} \frac{\partial}{\partial x} & 0 & \frac{\partial}{\partial y} \\ 0 & \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{pmatrix}, \quad \operatorname{Grad}_{v} = \begin{pmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{pmatrix}. \tag{30}$$

The co-energy and test functions are discretized using the same basis (Galerkin method) and each  $e_*$  variable is discretized using an  $N_*$ -dimensional space

$$v_{w} = \boldsymbol{\phi}_{w}^{T}(x, y)\boldsymbol{v}_{w}, \quad e_{w} = \boldsymbol{\phi}_{w}^{T}(x, y)\boldsymbol{e}_{w}(t),$$

$$\boldsymbol{v}_{\theta} = \boldsymbol{\Phi}_{\theta}^{T}(x, y)\boldsymbol{v}_{\theta}, \quad \boldsymbol{e}_{\theta} = \boldsymbol{\Phi}_{\theta}^{T}(x, y)\boldsymbol{e}_{\theta}(t),$$

$$\boldsymbol{v}_{\kappa} = \boldsymbol{\Phi}_{\kappa}^{T}(x, y)\boldsymbol{v}_{\kappa}, \quad \boldsymbol{e}_{\kappa} = \boldsymbol{\Phi}_{\kappa}^{T}(x, y)\boldsymbol{e}_{\kappa}(t),$$

$$\boldsymbol{v}_{\epsilon_{s}} = \boldsymbol{\Phi}_{\epsilon_{s}}^{T}(x, y)\boldsymbol{v}_{\epsilon_{s}}, \quad \boldsymbol{e}_{\epsilon_{s}} = \boldsymbol{\Phi}_{\epsilon_{s}}^{T}(x, y)\boldsymbol{e}_{\epsilon_{s}}(t),$$

$$(31)$$

where  $\phi_w \in \mathbb{R}^{N_w}$ ,  $\Phi_{\theta} \in \mathbb{R}^{N_{\theta} \times 2}$ ,  $\Phi_{\kappa} \in \mathbb{R}^{N_{\kappa} \times 3}$ ,  $\phi_{\epsilon_s} \in \mathbb{R}^{N_{\kappa} \times 2}$ . The skew-symmetric bilinear  $j_{\text{grad}}(v, e)$  form is replaced by the skew-symmetric matrix

$$J_d = \begin{pmatrix}
0 & 0 & 0 & -D_{\text{grad}}^T \\
0 & 0 & -D_{\text{Grad}}^T & -D_0^T \\
0 & D_{\text{Grad}} & 0 & 0 \\
D_{\text{grad}} & D_0 & 0 & 0
\end{pmatrix}, (32)$$

where

$$\mathbf{D}_{Grad} = \int_{\Omega} \mathbf{\Phi}_{\kappa} \operatorname{Grad}_{v}(\mathbf{\Phi}_{\theta}^{T}) d\Omega, 
\mathbf{D}_{grad} = \int_{\Omega} \mathbf{\Phi}_{\epsilon_{s}} \operatorname{grad}(\boldsymbol{\phi}_{w}^{T}) d\Omega, 
\mathbf{D}_{0} = -\int_{\Omega} \mathbf{\Phi}_{\epsilon_{s}} \mathbf{\Phi}_{\theta}^{T} d\Omega.$$
(33)

For a linear port Hamiltonian system, like the one under consideration, the energy and co-energy variables are related by a symmetric coercive and therefore invertible operator Q, such that  $\alpha = Qe$ . The  $\alpha$  variables are deduced from the co-energy variables by using  $\alpha = Q^{-1}e$ . The following relations hold

$$\alpha_{w} = \rho h e_{w}, \qquad \alpha_{\kappa} = C e_{\kappa},$$

$$\alpha_{\theta} = \frac{\rho h^{3}}{12} e_{\theta}, \qquad \alpha_{\epsilon_{s}} = \frac{1}{Ghk} e_{\epsilon_{s}},$$
(34)

where C is the inverse of the bending stiffness matrix or compliance matrix. The mass matrix reads M= $\operatorname{diag}[M_w, M_\theta, M_\kappa, M_{\epsilon_s}], \text{ where }$ 

$$\mathbf{M}_{w} = \int_{\Omega} \rho h \boldsymbol{\phi}_{w} \boldsymbol{\phi}_{w}^{T} d\Omega, \quad \mathbf{M}_{\kappa} = \int_{\Omega} \boldsymbol{\Phi}_{\kappa} \boldsymbol{C} \boldsymbol{\Phi}_{\kappa}^{T} d\Omega, 
\mathbf{M}_{\theta} = \int_{\Omega} \frac{\rho h^{3}}{12} \boldsymbol{\Phi}_{\theta} \boldsymbol{\Phi}_{\theta}^{T} d\Omega, \quad \mathbf{M}_{\epsilon_{s}} = \int_{\Omega} \frac{1}{Ghk} \boldsymbol{\Phi}_{\epsilon_{s}} \boldsymbol{\Phi}_{\epsilon_{s}}^{T} d\Omega.$$
(35)

To deal with generic boundary conditions the Lagrange multipliers and corresponding test functions have to be discretized as well

$$\lambda_{q_n} = \boldsymbol{\phi}_{q_n}^T(x, y) \boldsymbol{\lambda}_{q_n}, \qquad v_{q_n} = \boldsymbol{\phi}_{q_n}^T(x, y) \boldsymbol{v}_{q_n},$$

$$\lambda_{m_{nn}} = \boldsymbol{\phi}_{m_{nn}}^T(x, y) \boldsymbol{\lambda}_{m_{nn}}, \quad v_{m_{nn}} = \boldsymbol{\phi}_{m_{nn}}^T(x, y) \boldsymbol{v}_{m_{nn}},$$

$$\lambda_{m_{ns}} = \boldsymbol{\phi}_{m_{ns}}^T(x, y) \boldsymbol{\lambda}_{m_{ns}}, \quad v_{m_{ns}} = \boldsymbol{\phi}_{m_{ns}}^T(x, y) \boldsymbol{v}_{m_{ns}},$$
(24)

where  $\phi_{q_n} \in \mathbb{R}^{N_{q_n}}$ ,  $\phi_{m_{nn}} \in \mathbb{R}^{N_{m_{nn}}}$ ,  $\phi_{m_{ns}} \in \mathbb{R}^{N_{m_{ns}}}$ . It is now possible to construct the following matrices

$$\boldsymbol{b}_{w} = \int_{\Gamma_{C}} \boldsymbol{\phi}_{w} \boldsymbol{\phi}_{q_{n}}^{T} \, \mathrm{d}s + \int_{\Gamma_{S}} \boldsymbol{\phi}_{w} \boldsymbol{\phi}_{q_{n}}^{T} \, \mathrm{d}s,$$

$$\boldsymbol{b}_{\omega_{n}} = \int_{\Gamma_{C}} (\boldsymbol{\Phi}_{\theta} \boldsymbol{n}) \boldsymbol{\phi}_{m_{nn}}^{T} \, \mathrm{d}s,$$

$$\boldsymbol{b}_{\omega_{s}} = \int_{\Gamma_{C}} (\boldsymbol{\Phi}_{\theta} \boldsymbol{s}) \boldsymbol{\phi}_{m_{ns}}^{T} \, \mathrm{d}s + \int_{\Gamma_{S}} (\boldsymbol{\Phi}_{\theta} \boldsymbol{s}) \boldsymbol{\phi}_{m_{ns}}^{T} \, \mathrm{d}s.$$
(37)

The final system reads

$$\begin{pmatrix} \boldsymbol{M} & 0 \\ 0 & 0 \end{pmatrix} \frac{\mathrm{d}}{\mathrm{d}t} \begin{pmatrix} \boldsymbol{e} \\ \boldsymbol{\lambda} \end{pmatrix} = \begin{pmatrix} \boldsymbol{J}_d & \boldsymbol{B}_D \\ -\boldsymbol{B}_D^T & 0 \end{pmatrix} \begin{pmatrix} \boldsymbol{e} \\ \boldsymbol{\lambda} \end{pmatrix}, \quad (38)$$

where  $e, \lambda$  are the concatenation of the co-energy variables and Lagrange multipliers and  $\boldsymbol{B}_D = [\boldsymbol{b}_w \ \boldsymbol{b}_{\omega_n} \ \boldsymbol{b}_{\omega_s}]$ . Furthermore  $\boldsymbol{e} \in V^h \subset H^1(\Omega) \times H^1(\Omega; \mathbb{V}) \times H^1(\Omega, \mathbb{S}) \times H^1(\Omega, \mathbb{V})$ . This finite dimensional system is a descriptor pH system, as explained in Beattie et al. (2018).

Remark 4. If non null boundary conditions on momenta and forces have to be imposed on the boundary, i.e.

- $\begin{array}{ll} \bullet & q_n = \overline{q}_n \text{ on } \Gamma_{q_n} \subseteq \Gamma_F; \\ \bullet & m_{nn} = \overline{m}_{nn} \text{ on } \Gamma_{m_{nn}} \subseteq (\Gamma_F \cup \Gamma_S); \\ \bullet & m_{ns} = \overline{m}_{ns} \text{ on } \Gamma_{m_{ns}} \subseteq \Gamma_F; \end{array}$

the linear functional  $f_N(v)$  is replaced by the vector

$$\mathbf{f}_N = (\mathbf{f}_{N,w}^T, \mathbf{f}_{N,\theta}^T, 0, 0)^T,$$
 (39)

$$\mathbf{f}_{N,w} = \int_{\Gamma_{q_n}} \boldsymbol{\phi_w} \, \overline{q}_n \, \mathrm{d}s,$$

$$\mathbf{f}_{N,\theta} = \int_{\Gamma_{m,n}} \boldsymbol{\Phi_{\theta} n} \, \overline{m}_{nn} \, \mathrm{d}s + \int_{\Gamma_{m,n}} \boldsymbol{\Phi_{\theta} s} \, \overline{m}_{ns} \, \mathrm{d}s.$$
(40)

$$\begin{pmatrix} \boldsymbol{M} & 0 \\ 0 & 0 \end{pmatrix} \cdot \frac{\mathrm{d}}{\mathrm{d}t} \begin{pmatrix} \boldsymbol{e} \\ \boldsymbol{\lambda} \end{pmatrix} = \begin{pmatrix} \boldsymbol{J}_d & \boldsymbol{B}_D \\ -\boldsymbol{B}_D^T & 0 \end{pmatrix} \begin{pmatrix} \boldsymbol{e} \\ \boldsymbol{\lambda} \end{pmatrix} + \begin{pmatrix} \boldsymbol{f}_N \\ 0 \end{pmatrix} \quad (41)$$

Remark 5. Distributed forces and dissipation are easily included inside this model. If e.g. gravity has to be included then the corresponding forcing term is given by

$$\mathbf{f}_g = (\mathbf{f}_{g,w}^T, 0, 0, 0)^T, \quad \mathbf{f}_{g,w} = -\int_{\Omega} \phi_{\mathbf{w}} \, \rho h g \, d\Omega, \quad (42)$$

where  $g = 9.81 \, m/s^2$  is the gravity acceleration. This contribution is then added to the system, as for the case of non null Neumann boundary condition.

4.3 Finite Element Choice

We denote by

$$H_r^k(\mathbb{P}_l) = \{ v \in H^k(\Omega) | v_{|T} \in \mathbb{P}_l \ \forall T \in \mathbb{T}_r \}$$

the finite element space which is a subspace of  $H^k(\Omega)$ , based on the shape function space of piecewise polynomials of degree l, over the mesh  $\mathbb{T}_r = \bigcup_i T_i$ . The cells  $T_i$  are triangles. These spaces can be scalar-valued, vector-valued or symmetric matrix valued, depending on the variables to be discretized. The parameter r is the average size of a mesh element. For our study all the variables are discretized by the same finite element space. The analysis were conducted using two different finite element spaces:

- (1) the first order Lagrange polynomials  $H_r^1(\mathbb{P}_1)$ ;
- (2) the second order Lagrange polynomials  $H_r^1(\mathbb{P}_2)$ .

#### 5. NUMERICAL RESULTS

In this section the computation of the eigenvalues of a square plate for several boundary conditions is presented. Three different time-domain simulations follow.

#### 5.1 Eigenvalues Computation

The test case for this analysis is a simple square plate of side L, a benchmark problem which has been studied in Dawe and Roufaeil (1980); Huang and Hinton (1984); Durán et al. (1999) for different boundary conditions:

- All clamped CCCC;
- Simply supported hard SSSS;
- Half clamped half simply supported SCSC;
- All clamped but one side free CCCF.

Two different thickness to length ratios are considered, h/L = 0.1 representative of a thick plate and h/L = 0.01representative of a thin plate. In order to compare our results the computed frequencies  $\omega_{mn}^h$  in the following nondimensional form:

$$\widehat{\omega}_{mn}^{h} = \omega_{mn}^{h} L \left( \frac{2(1+\nu)\rho}{E} \right)^{1/2} \tag{43}$$

m and n being the numbers of half-waves occurring in the modes shapes in the x and y directions, respectively. The only parameters which influence the results are the Poisson's ratio  $\nu = 0.3$ , the correction factor k, whose value is taken equal to k = 0.8601 for CCCC and CCCF, k = 0.8333 for SSSS and k = 0.822 for SCSC (for comparison purposes) and the thickness-to-span ratio h/L. The reported non-dimensional frequencies are independent of the remaining geometrical and physical parameters. The error is computed as

$$\varepsilon = \frac{\operatorname{abs}(\widehat{\omega}_{mn}^h - \omega_{mn}^{DR})}{\omega_{mn}^{DR}},\tag{44}$$

where  $\omega_{mn}^{DR}$  are the eigenvalues computed in Dawe and Roufaeil (1980) using an analytical procedure. The results obtained using  $H_r^1(\mathbb{P}_1), H_r^1(\mathbb{P}_2)$  are reported in Tables 1, 2 respectively, together with the results from Huang and Hinton (1984) (H-H) and Dawe and Roufaeil, 1980 (D-R). For linear polynomials the mesh consist of a regular grid with 10 and 20 elements by side. For quadratic polynomials the mesh consist of a regular grid with 5 and 10 elements by side. Ten elements in  $H^1_r(\mathbb{P}_1)$  and 5 in  $H^1_r(\mathbb{P}_2)$  have the same degree of freedom n=968. Analogously, 20 elements in  $H^1_r(\mathbb{P}_1)$  and 10 in  $H^1_r(\mathbb{P}_2)$  have the same degree of freedom n = 3528. In the thick plate case the error is limited to 2% in each for any choice of the finite element space. On the contrary the thin case exhibit worse results, particularly for the polynomials of order 1. This must be linked to the shear locking phenomenon since  $M_{\kappa} \propto h^{-3}$  mixes with  $M_{\epsilon_s} \propto h^{-1}$ . Higher order elements allow to overcome the problem but this issue is still present as the results deteriorate in the thin case.

# 5.2 Time-Domain simulations

In this analysis we consider a square plate, subject either to a non null shear force on the boundaries either to a distributed force over the domain. The physical and simulation setting are reported in Tab. 3.

Ten elements for each side and second order Lagrange polynomials are used. The symplectic Störmer-Verlet integrator is employed, so that when no solicitation is applied to the system, the Hamiltonian is preserved. Three different simulation with different BC are considered. The initial condition are set to zero for each variables. For the first simulation the set of BC reads:

- $x = 0 \to w_t = 0$ ,  $\omega_n = \omega_s = 0$ ,  $x = 1 \to q_n = 0$ ,  $m_{nn} = m_{ns} = 0$ ,  $y = 0 \to q_n = f(t)$ ,  $m_{nn} = m_{ns} = 0$ ,
- $y = 1 \rightarrow q_n = -f(t), m_{nn} = m_{ns} = 0,$

where

$$f(t) = \begin{cases} 10^6 \ [Pa \cdot m], & \forall t < 0.2 \, t_{\text{fin}}, \\ 0, & \forall t \ge 0.2 \, t_{\text{fin}}. \end{cases}$$
(45)

The set of BC for the second simulation is

- $x=0 \rightarrow w_t=0, \, \omega_n=\omega_s=0,$
- $x = 1 \rightarrow q_n = g(y, t), m_{nn} = m_{ns} = 0,$
- $\bullet \ y = 0 \to w_t = 0, \ \omega_n = \omega_s = 0,$
- $y=1 \rightarrow w_t=0, \, \omega_n=\omega_s=0$

$$g(y,t) = \begin{cases} 10^6 \sin\left(\frac{2\pi}{L}y\right) & [Pa \cdot m], \quad \forall t < 0.5 \, t_{\text{fin}}, \\ 0, & \forall t \ge 0.5 \, t_{\text{fin}}. \end{cases}$$
(46)

For the third simulation a clamped plate subject to gravity is considered. Since the solicitation admits a potential the Hamiltonian does not represent the total energy, that now includes the potential energy

$$E_p = \int_{\Omega} \rho h g w \, d\Omega, \quad w: \text{ vertical dislpacement.}$$
 (47)

Snapshots of the vertical displacement are reported in Figs. 1, 2, 3. This field is obtained from the velocity field  $e_w$ by applying the trapezoidal rule integration. The output is consistent with the imposed BC and with the physical intuition of the observed phenomenon. The Hamiltonian trend is shown in Fig. 4. For the first and second simulation once the solicitation is set to zero the system is closed and therefore the total energy of the system is preserved (see Figs 4a, 4b). For the third simulation the conserved quantity is the total energy  $E_{\text{tot}} = H + E_p$ , as Fig. 4c shows.

# 6. CONCLUSION

This work represents a first attempt to simulate bidimensional mechanical systems using the Hamiltonian formalism. The obtained results look convincing, but the model presented herein is not yet fully understood. In particular identifying the proper functional spaces where variables live and the optimal mixed finite element choice is still to be done. Future developments will include the interconnection of this mechanical model with simpler ones, to benefit from the modularity of the port-Hamiltonian framework. Furthermore, to avoid the shear locking phenomenon, the Kirchhoff-Love model will be addressed.

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BCs	Mode	N = 10	N = 20	Н-Н	D-R	BCs	Mode	N = 10	N = 20	Н-Н	D-R
CCCC	$\widehat{\omega}_{11}$	1.5999	1.5917	1.591	1.594	CCCC	$\widehat{\omega}_{11}$	0.1967	0.1765	0.1754	0.1754
	$\widehat{\omega}_{21}$	3.0615	3.0410	3.039	3.046		$\widehat{\omega}_{21}$	0.4030	0.3604	0.3574	0.3576
	$\widehat{\omega}_{12}$	3.0615	3.0410	3.039	3.046		$\widehat{\omega}_{12}$	0.4030	0.3604	0.3574	0.3576
	$\widehat{\omega}_{22}$	4.3161	4.2682	4.263	4.285		$\widehat{\omega}_{22}$	0.6431	0.5358	0.5264	0.5274
	$\widehat{\omega}_{11}$	0.9324	0.9324	0.930	0.930	SSSS	$\widehat{\omega}_{11}$	0.1706	0.1128	0.0963	0.0963
SSSS	$\widehat{\omega}_{21}$	2.2227	2.2226	2.219	2.219		$\widehat{\omega}_{21}$	0.3576	0.2660	0.2406	0.2406
مممم	$\widehat{\omega}_{12}$	2.2227	2.2226	2.219	2.219		$\widehat{\omega}_{12}$	0.3576	0.2660	0.2406	0.2406
	$\widehat{\omega}_{22}$	3.4142	3.3608	3.405	3.406		$\widehat{\omega}_{22}$	0.5803	0.4442	0.3847	0.3848
	$\widehat{\omega}_{11}$	1.3111	1.3013	1.300	1.302	SCSC	$\widehat{\omega}_{11}$	0.1864	0.1487	0.1411	0.1411
SCSC	$\widehat{\omega}_{21}$	2.4155	2.3966	2.394	2.398		$\widehat{\omega}_{21}$	0.3649	0.2829	0.2668	0.2668
5050	$\widehat{\omega}_{12}$	2.9082	2.8871	2.885	2.888		$\widehat{\omega}_{12}$	0.3987	0.3485	0.3377	0.3377
	$\widehat{\omega}_{22}$	3.8906	3.8458	3.839	3.852		$\widehat{\omega}_{22}$	0.6075	0.4933	0.4604	0.4608
	$\widehat{\omega}_{\frac{1}{2}1}$	1.0855	1.0982	1.081	1.089	CCCF	$\widehat{\omega}_{\frac{1}{2}1}$	0.1238	0.1166	0.1166	0.1171
CCCF	$\left \begin{array}{c} \widehat{\omega}_{\frac{3}{2}1}^2 \end{array}\right $	1.7636	1.7461	1.744	1.758		$\widehat{\omega}_{\frac{3}{2}1}^2$	0.2207	0.1954	0.1949	0.1951
	$\widehat{\omega}_{\frac{1}{2}2}^2$	2.6696	2.6575	2.657	2.673		$\widehat{\omega}_{\frac{1}{2}2}^2$	0.3204	0.3078	0.3080	0.3093
	$\widehat{\omega}_{\frac{5}{2}1}^2$	3.2248	3.1997	3.197	3.216		$\widehat{\omega}_{\frac{5}{2}1}^2$	0.4144	0.3751	0.3736	0.3740

Table 1. Eigenvalues for h/L=0.1 (left) and h/L=0.01 (right) using  $\mathbb{P}_1$  reference,  $\blacksquare \ \varepsilon < 2\%, \ \blacksquare \ \varepsilon < 5\%, \ \blacksquare \ \varepsilon < 15\%, \ \blacksquare \ \varepsilon < 30\%, \ \blacksquare \ \varepsilon < 50\%, \ \blacksquare \ \varepsilon < 80\%.$ 

	BCs	Mode	N=5	N = 10	Н-Н	D-R	BCs
	CCCC	$\widehat{\omega}_{11}$	1.5976	1.5914	1.591	1.594	
		$\widehat{\omega}_{21}$	3.0584	3.0405	3.039	3.046	CCCC
	0000	$\widehat{\omega}_{12}$	3.0677	3.0405	3.039	3.046	0000
		$\widehat{\omega}_{22}$	4.3109	4.2662	4.263	4.285	
		$\widehat{\omega}_{11}$	0.9304	0.9302	0.930	0.930	
	SSSS	$\widehat{\omega}_{21}$	2.2223	2.2194	2.219	2.219	SSSS
		$\widehat{\omega}_{12}$	2.2224	2.2194	2.219	2.219	ממממ
		$\widehat{\omega}_{22}$	3.4128	3.4061	3.405	3.406	
		$\widehat{\omega}_{11}$	1.3053	1.3004	1.300	1.302	
	SCSC	$\widehat{\omega}_{21}$	2.4040	2.3946	2.394	2.398	SCSC
	SCSC	$\widehat{\omega}_{12}$	2.9060	2.8858	2.885	2.888	SCSC
		$\widehat{\omega}_{22}$	3.8721	3.8415	3.839	3.852	
		$\widehat{\omega}_{\frac{1}{2}1}$	1.0845	1.0797	1.081	1.089	
	CCCF	$\widehat{\omega}_{\frac{3}{2}1}^2$	1.7559	1.7425	1.744	1.758	CCCI
		$\widehat{\omega}_{\frac{1}{2}2}^{\frac{1}{2}1}$	2.6762	2.6547	$  _{2.657}$	2.673	
			3.2186	3.1954	3.197	3.216	
		$\widehat{\omega}_{\frac{5}{2}1}$	5.2160	5.1954	5.197	5.210	

BC	)s	Mode	N=5	N = 10	H-H	D-R
		$\widehat{\omega}_{11}$	0.1872	0.1762	0.1754	0.1754
CC	ccc	$\widehat{\omega}_{21}$	0.3725	0.3598	0.3574	0.3576
		$\widehat{\omega}_{12}$	0.4055	0.3598	0.3574	0.3576
		$\widehat{\omega}_{22}$	0.6043	0.5335	0.5264	0.5274
		$\widehat{\omega}_{11}$	0.0963	0.0963	0.0963	0.0963
999	SSSS	$\widehat{\omega}_{21}$	0.2422	0.2406	0.2406	0.2406
l DDI		$\widehat{\omega}_{12}$	0.2430	0.2406	0.2406	0.2406
		$\widehat{\omega}_{22}$	0.3874	0.3848	0.3847	0.3848
	SCSC	$\widehat{\omega}_{11}$	0.1492	0.1418	0.1411	0.1411
SCS		$\widehat{\omega}_{21}$	0.2827	0.2683	0.2668	0.2668
DO.		$\widehat{\omega}_{12}$	0.3608	0.3394	0.3377	0.3377
		$\widehat{\omega}_{22}$	0.4940	0.4654	0.4604	0.4608
		$\widehat{\omega}_{\frac{1}{2}1}$	0.1197	0.1169	0.1166	0.1171
CC	CCCF	$\widehat{\omega}_{\frac{3}{2}1}^2$	0.2092	0.1960	0.1949	0.1951
		$\widehat{\omega}_{\frac{1}{2}2}^2$	0.3188	0.3089	0.3080	0.3093
		$\widehat{\omega}_{\frac{5}{2}1}^2$	0.3938	0.3757	0.3736	0.3740

Table 2. Eigenvalues for h/L=0.1 (left) and h/L=0.01 (right) using  $\mathbb{P}_2$  reference,  $\bullet$   $\varepsilon<2\%, \bullet$   $\varepsilon<5\%, \bullet$   $\varepsilon<15\%.$ 

Plat	e Parameters	Simulation Settings			
E	70 [GPa]	Integrator	Störmer-Verlet		
$\rho$	$2700 [kg/m^3]$	$\Delta t$	0.001[ms]		
$\nu$	0.35	$t_{ m fin}$	1 [ms]		
k	5/6	N <sup>◦</sup> Samples	100		
h/L	0.1	N° Elements	10		
L	1 [m]	FE space	$H^1_{r=L/10}(\mathbb{P}_2)$		

Table 3. Simulations settings and physical parameters

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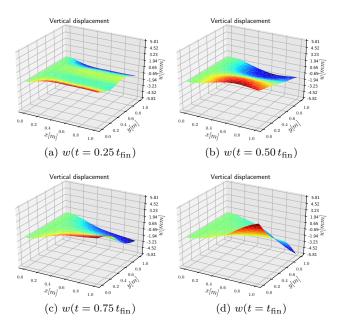


Fig. 1. Snapshots for Simulation  $n^{\circ}1$ .

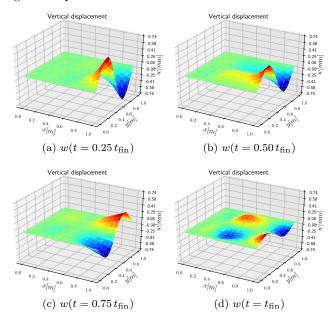


Fig. 2. Snapshots for Simulation  $n^{\circ}2$ .

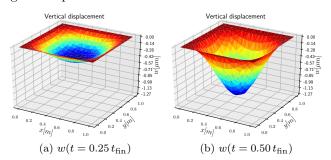


Fig. 3. Snapshots for Simulation  $n^{\circ}3$ 

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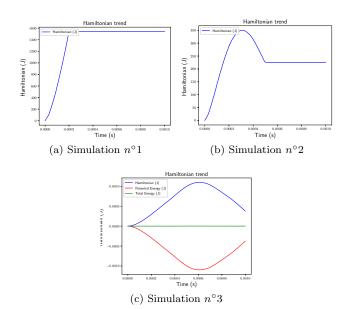


Fig. 4. Hamiltonian trend for the three simulations

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