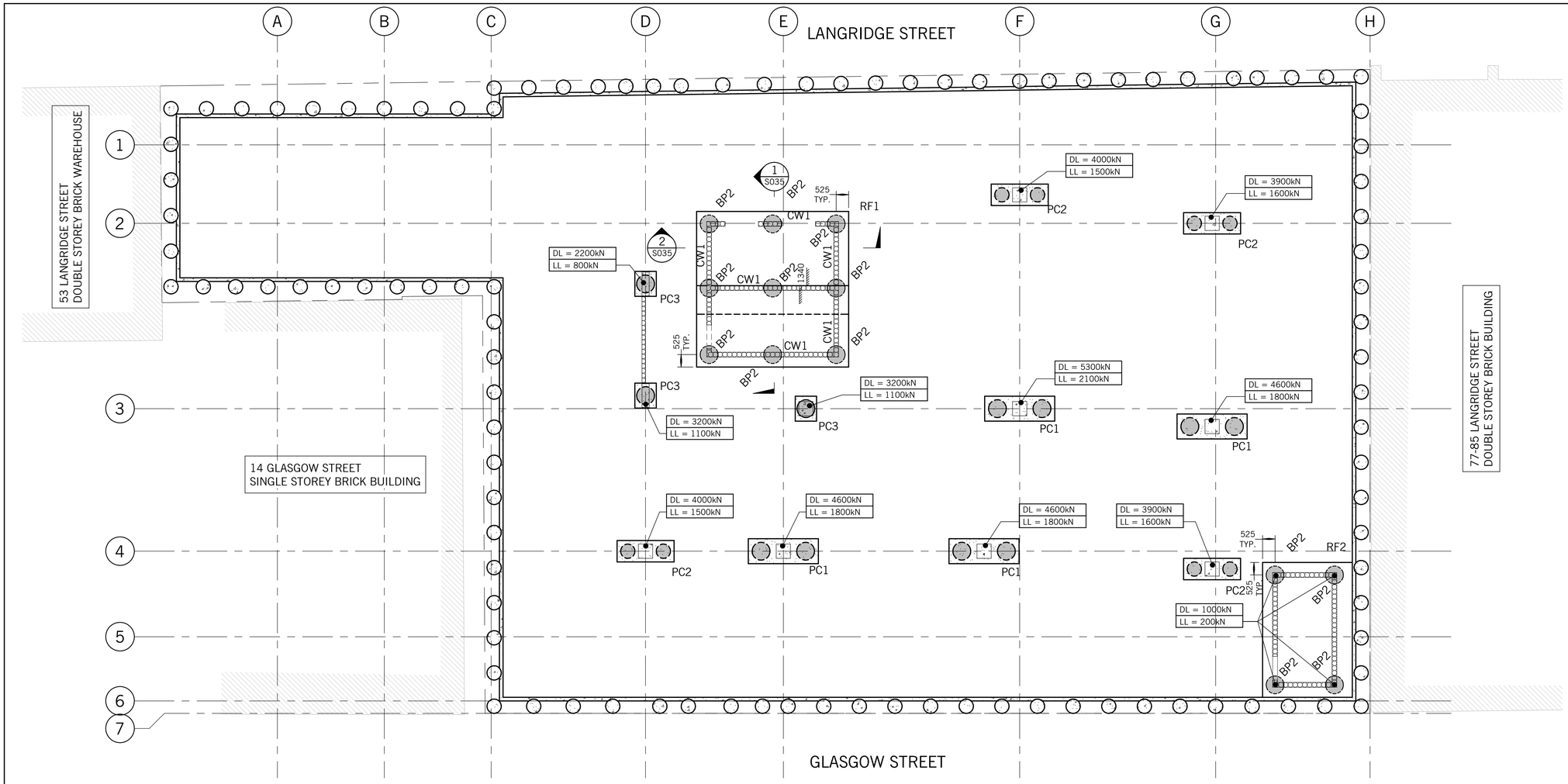

HYDROSTATIC SLAB NOTES:

THE BASEMENT 2 SLAB AND LOWER RETENTION WALLS ARE LOCATED BELOW THE WATER TABLE AND HAVE BEEN DESIGNED AS A HYDROSTATIC SLAB TO STRUCTURALLY RESIST THE HYDROSTATIC FORCES APPLIED. THEY HAVE NOT BEEN DESIGN AS A LIQUID RETAINING STRUCTURE AND AS SUCH RELIES ON A WATERPROOF MEDIUM MEMBRANE, WATERPROOF ADDITIVES OR SIMILAR, TO STOP WATER PERMEATING THROUGH THE CONCRETE OR POTENTIAL CRACKS IN THE STRUCTURE.

A WATERPROOFING CONSULTANT SHOULD BE ENGAGED TO ADVISE ON ALL WATERPROOFING REQUIREMENTS INCLUDING POTENTIAL MEMBRANES, CONCRETE ADDITIVES AND DETAILING OF ALL COLD JOINTS TO PILES, SHOTCRETE WALLS, SLABS, FOUNDATIONS AND WALLS. ALLOWANCE FOR 44kPa HEAD APPLIED TO THE FLOOR AND WALLS OF THE BASEMENT.

- ALL DETAILING OF MEMBRANES, WATER STOPS, ETC MADE HEREIN ARE INDICATIVE ONLY AND PENDING TO FURTHER SPECIALIST ADVICE.
- ALLOWANCE FOR POTENTIAL 50mm BLINDING LAYER TO BASEMENT SLAB SHOULD BE MADE PENDING CONFIRMATION OF THE WATERPROOFING SYSTEM ADOPTED AND SPECIFIC REQUIREMENTS.
- ALLOW FOR WATERPROOF ADMIXTURE IN CONCRETE BELOW THE WATERTABLE.
- POUR STRIP AND POUR SIZE TO BE CONSIDERING IN CONJUNCTION WITH WATERPROOFING STRATEGY ALONG WITH ADDITIONAL REINFORCEMENT TO CONTROL CRACK WIDTH

PILE SETOUT NOTES:

1. THE ENGINEER SHALL BE GIVEN 24 HOURS NOTICE TO RESPOND AND INSPECT THE NATURE AND CONDITION OF THE FOUNDATION MATERIAL PRIOR TO THE POURING OF PILES AND PILECAPS.
2. ALTERNATIVE DESIGN:
   ALTERNATIVE DESIGN(S) FOR THE FOUNDATION SYSTEM BASED UPON THE DESIGN LOADS NOMINATED ON THE DRAWINGS MAY BE SUBMITTED FOR APPROVAL. THE ALTERNATIVE DESIGN(S) WHEN PROPERLY INSTALLED, SHALL MEET THE PERFORMANCE REQUIREMENT AND COMPLY WITH THE PILING NOTES.THE DESIGN OF PILE CAPS SHALL BE INCLUDED IN THE ALTERNATIVE DESIGN.

   CALCULATIONS:
   SUBMIT CALCULATIONS TO SHOW THAT THE PROPOSED SYSTEM WILL MEET THE SPECIFIED PERFORMANCE REQUIREMENTS. ALSO SUBMIT THE SOURCES OF GEOTECHNICAL INFORMATION AND DESIGN PARAMETERS USED IN CALCULATIONS.

   THE CONTRACTOR SHALL PROVE BY CALCULATION AND OTHER SUPPORT DATA THAT THE FOUNDATION SYSTEM IS CAPABLE OF DISPERSING HORIZONTAL FORCES DUE TO EARTHQUAKE AND WIND, INTO THE SOIL STRATA. THE RESULTANT LATERAL DISPLACEMENT OF THE FOUNDATION SYSTEM SHALL BE WITHIN ACCEPTABLE LIMIT.

DL = 5000kN	DENOTES TOTAL WORKING DEAD LOAD (G) DL = 5000kN
LL = 1000kN	WORKING LIVE LOAD (Q) LL = 1000kN

* = ADDITIONAL POINT LOAD
ALL LOADS ARE WORKING LOADS.

FOUNDATION PLAN

SCALE: 1 : 100

PILE NOTES:

- THE LOADS PROVIDED ARE THE STRUCTURAL LOADS AT THE BOTTOM OF THE PILECAP. (i.e. THESE DO NOT INCLUDE THE ADDITIONAL LOAD DUE TO DOWNDRAG) REFER GEOTECHNICAL REPORT FOR INFORMATION.
- PILECAPS DRAWN ARE INDICATIVE ONLY. THE NUMBER AND TYPE OF PILES LOADING REQUIREMENTS ARE REQUIRED TO BE SUBMITTED BY THE PILING SUBCONTRACTOR FOR APPROVAL.
- CUT OFF LEVELS AT TOP OF PILES TO BE DETERMINED BY SUBCONTRACTOR AND ENGINEER, DEPENDENT ON PILECAP DESIGN.

PILING CONTRACTOR NOTE:

1. PILING CONTRACTOR SHALL ENSURE NO LIVE SERVICES IN PILE DESIGNATED AREA PRIOR TO PILING.
2. PILECAP SIZE AND PROFILE TO BE ADVISED SUBJECT TO PILING CONTRACTOR DESIGN PARAMETERS.

DESIGN LOAD:

HYDROSTATIC UPLIFT = 44 kPa AT DESIGN GROUND WATER LEVEL OF R.L. = 16.0 AHD
REFER GEOTECH REPORT FOR DETAILS

FOUNDATION SCHEDULE

MARK	SIZE	f'c	REINF. Rate	REMARKS
BP2	750ø	SEE REMARKS	SEE REMARKS	BY D&C CONTRACTOR
PC1	1200d x 1050 x 2950	40	120kg/m³	PILE CAP - 2 No. 750ø PILES
PC2	1200d x 900 x 2400	40	120kg/m³	PILE CAP - 2No. 600ø PILES
PC3	750d x 900 x 1050	40	120kg/m³	PILE CAP - 1No. 750ø PILES
RF1	1200d x AS NOTED ON PLAN	40	120kg/m³	RAFT
RF2	750d x AS NOTED ON PLAN	40	120kg/m³	RAFT

NOTE : 600ø IS EXPECTED TO CARRY 2700kN WORKING LOAD REFER GEOTECHNICAL REPORT.
750ø IS EXPECTED TO CARRY 5000kN WORKING LOAD REFER GEOTECHNICAL REPORT.
900ø IS EXPECTED TO CARRY 7000kN WORKING LOAD REFER GEOTECHNICAL REPORT.
PILE GROUP SHOWN ARE INDICATIVE ONLY AND MAY CHANGE TO SUIT D&C PILING CONTRACTOR FINAL SUBMISSION.

CONCRETE COLUMN SCHEDULE

MARK	SIZE	f'c (MPa)	REINF. RATE (kg/m³)	REMARKS
C1	300x1000	40-65 MPa (50 MPa AVERAGE)	220 kg/m³ AVERAGE	
C2	600x600	40-65 MPa (50 MPa AVERAGE)	220 kg/m³ AVERAGE	
C3	350x800	40-65 MPa (50 MPa AVERAGE)	220 kg/m³ AVERAGE	
C4	500x500	40-65 MPa (50 MPa AVERAGE)	220 kg/m³ AVERAGE	
C5	450x450	40-65 MPa (50 MPa AVERAGE)	220 kg/m³ AVERAGE	
C6	250x800	40-65 MPa (50 MPa AVERAGE)	220 kg/m³ AVERAGE	
C8	300x300	40-65 MPa (50 MPa AVERAGE)	220 kg/m³ AVERAGE	
FC1	300x700	40 MPa	100 kg/m³	NON LOAD BEARING FACADE COLUMN
REINFORCEMENT RATES NOMINATED ARE BASED ON AVERAGE RATES THROUGHOUT THE BUILDING				

DO NOT SCALE DRAWINGS, USE FIGURED DIMENSIONS

Rev.	Description	Eng.	Draft.	Date
P1	PRELIMINARY ISSUE	JV	JT	11.10.19

DRAWING REFERENCE	REFERENCE No.
DRAWING INDEX	S000
GENERAL NOTES	S001-S002
RETENTION	S010-S029
CONCRETE COLUMNS	S800-S819
IN-SITU WALLS	S820-S879
PRECAST WALLS	S880-S909
SLAB ON GROUND DETAILS	S950-S951
SUSPENDED CONCRETE SLABS	S955-S957
POST TENSIONING DETAILS	S960-S961
R.C. STAIR DETAILS	S970
MASONRY DETAILS	S980-S984
STEEL DETAILS	S990-S991

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CLIENT
DOMAIN HILL PROPERTY GROUP

PROJECT
PROPOSED OFFICE BUILDING
71 LANGRIDGE STREET,
COLLINGWOOD

TITLE
FOUNDATION PLAN

DATE	DESIGNED BY	CHECKED BY
SEP '19	JV	PM
SCALES AT A1	DRAWN BY	APPROVED BY
1:100	JT	PW
JOB No.	DRAWING No.	REV.
17040	S030	P1

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