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Designer	LoremIpsum	Job Number	123
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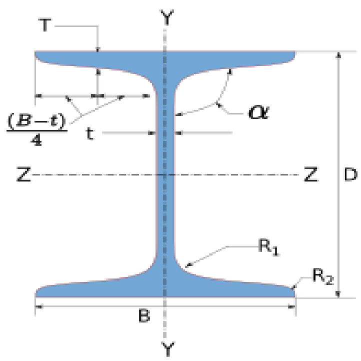
1 Input Parameters

Module	Beam Coverplate Connection
MainModule	Moment Connection
Moment(kNm)*	5.0
Shear (kN)*	62.0
Axial (kN) *	151.0

Section



Beam Section *		MB 450	
Preferences		Outside + Inside	
Material *		E 250 (Fe 410 W)A	
Ultimate strength, fu (MPa)		410	
Yield Strength , fy (MPa)	250	R1(mm)	15.0
Mass	72.4	R2(mm)	7.5
Area(mm2) - A	9220.0	Iz(mm4)	303580000.0
D(mm)	450.0	Iy(mm4)	8070000.0
B(mm)	150.0	rz(mm)	181.0
t(mm)	9.4	ry(mm)	30.0
T(mm)	17.4	Zz(mm3)	1349300.0
FlangeSlope	98	Zy(mm3)	108000.0

Bolt Details

Diameter (mm)*	[12.0, 16.0, 20.0, 24.0, 30.0, 36.0]
Grade *	[3.6, 4.6, 4.8, 5.6, 5.8, 6.8, 8.8, 9.8, 10.9, 12.9]
Type *	Friction Grip Bolt
Bolt hole type	Standard
Slip factor (µ_f)	0.3
Type of edges	a - Sheared or hand flame cut
Gap between beam and support (mm)	10.0
Are the members exposed to corrosive influences	False

2 Design Checks

2.1 Member Capacity

Check	Required	Provided	Remarks
Axial Capacity Member Ac (kN)		$A_c = \frac{A * f_y}{\gamma_{m0} * 10^3}$ $= \frac{9220.0 * 250}{1.1 * 10^3}$ $= 2095.45$	
Shear Capacity Member Sc (kN)		$S_c = \frac{A_v * f_y}{\sqrt{3} * \gamma_{mo} * 10^3}$ $= \frac{415.2 * 9.4 * 250}{\sqrt{3} * 1.1 * 10^3}$ $= 512.12$	
Plastic Moment Capacity Pmc (kNm)		$Pmc = \frac{\beta_b * Z_p * fy}{\gamma_{mo} * 10^6}$ $= \frac{1 * 405118.94 * 250}{1.1 * 10^6}$ $= 92.07$	
Moment Deformation Criteria Mdc (kNm)		$Mdc = \frac{1.5 * Z_e * fy}{1.1 * 10^6}$ $= \frac{1.5 * 1349300.0 * 250}{1.1 * 10^6}$ $= 459.99$	
Moment Capacity Member Mc (kNm)		$M_c = min(Pmc, Mdc)$ $= min(92.07, 459.99)$ $= 92.07$	

2.2 Load Consideration

Check	Required	Provided	Remarks
Applied Axial Load Au (kN)	$Ac_{min} = 0.3 * A_c$ $= 0.3 * 2095.45$ $= 628.64$ $Ac_{max} = Ac$ $= 2095.45$	$A_u = 628.64$	Pass
Applied Shear Load Vu (kN)	$Vc_{min} = 0.6 * S_c$ $= 0.6 * 512.12$ $= 307.27$ $Vc_{max} = Sc$ $= 512.12$	$V_u = 307.27$	Pass
Applied Moment Load Mu (kNm)	$Mc_{min} = 0.5 * M_c$ $= 0.5 * 92.07$ $= 46.04$ $Mc_{max} = Mc$ $= 92.07$	$M_u = 46.04$	Pass
Forces Carried by Web		$A_w = Axial\ force\ in\ web$ $= \frac{(D - 2 * T) * t * Au}{A}$ $= \frac{(450.0 - 2 * 17.4) * 9.4 * 628.64}{9220.0}$ $= 266.11\ kN$ $M_w = Moment\ in\ web$ $= \frac{Z_w * Mu}{Z}$ $= \frac{405118.94 * 46.04}{1551600.0}$ $= 12.02\ kNm$	
Forces Carried by Flange		$A_f = Axial\ force\ in\ flange$ $= \frac{Au * B * T}{A}$ $= \frac{628.64 * 150.0 * 17.4}{9220.0}$ $= 177.95\ kN$ $M_f = Moment\ in\ flange$ $= Mu - M_w$ $= 46.04 - 12.02$ $= 34.02\ kNm$ $F_f = flange\ force$ $= \frac{M_f * 10^3}{D - T} + A_f$ $= \frac{34.02 * 10^3}{450.0 - 17.4} + 177.95$ $= 256.59\ kN$	

2.3 Initial Member Check

Check	Required	Provided	Remarks
Flange Tension Yielding Capacity (kN)	$F_f = 256.59$	$T_{dg} = \frac{l * t * f_y}{\gamma_{mo}}$ $= \frac{1 * 150.0 * 17.4 * 250}{1.1}$ $= 593.18$	Pass
Web Tension Yielding Capacity (kN)	$A_w = 266.11$	$T_{dg} = \frac{l * t * f_y}{\gamma_{mo}}$ $= \frac{1 * 415.2 * 9.4 * 250}{1.1}$ $= 887$	Pass

2.4 Initial flange plate height check

Check	Required	Provided	Remarks
flange_plate.Height	Outer.b >= 50	$Outer.b = 150.0$	Pass
flange_plate.InnerHeight	Inner.b >= 50	$inner.b = \frac{B - t - (2 * r_1)}{2}$ $= \frac{150.0 - 9.4 - (2 * 15.0)}{2}$ $= 55.3$	Pass

2.5 Flange plate thickness

Check	Required	Provided	Remarks
Thickness (mm)*	$T = 8.7$	$t_f = 12.0$	Pass
Plate Area check (mm2)	$pt.area >=$ $connected\ member\ area * 1.05$ $= 2740.5$	$outer.b = B$ $= 150.0$ $inner.b = \frac{B - t - (2 * r_1)}{2}$ $= \frac{150.0 - 9.4 - (2 * 15.0)}{2}$ $= 55.3$ $pt.area = (150.0 + (2 * 55.3)) * 12.0$ $= 3127.2$	Pass

2.6 Web plate thickness

Check	Required	Provided	Remarks
Thickness (mm)*	$t = 4.7$	$t_w = 6.0$	Pass
Plate Area check (mm2)	$pt.area >=$ $connected\ member\ area * 1.05$ $= 3604.52$	$web\ b = D - (2 * T) - (2 * r_1)$ $= 450.0 - (2 * 17.4) - (2 * 15.0)$ $= 365.2$ $pt.area = 6.0 * 2 * 365.2$ $= 4382.4$	Pass

2.7 Web Spacing Checks

Check	Required	Provided	Remarks
Min.Diameter (mm)		$d = 12.0$	
Min. Gauge (mm)	$p/g_{min} = 2.5\ d$ $=2.5 * 12.0 \quad = 30.0$	$g = 30$ ($Row\ Limit$ $(r_l) = 2$)	
Min. Edge Distance (mm)	$e/e`_{min} = [1.5\ or\ 1.7] * d_0$ $= 1.7 * 13.0 = 22.1$	25	
Spacing Check	$depth = 2 * e + (r_l - 1) * g$ $= 2 * 25 + (2.0 - 1) * 30$ $= 80.0$	365.2	Pass

2.8 Flange Spacing Checks

Check	Required	Provided	Remarks
Min.Diameter (mm)		$d = 12.0$	
Min. Gauge (mm)	$p/g_{min} = 2.5\ d$ $=2.5 * 12.0 \quad = 30.0$	$g = 0.0\ (Row\ Limit\ (r_l) = 1)$	
Min. Edge Distance (mm)	$e/e`_{min} = [1.5\ or\ 1.7] * d_0$ $= 1.7 * 13.0 = 22.1$	25	
Spacing Check	$depth = 2 * e + (r_l - 1) * g$ $= 2 * 25 + (1.0 - 1) * 30$ $= 50.0$	55.3	Pass

2.9 Flange Bolt Checks

Check	Required	Provided	Remarks
Diameter (mm)	Bolt Quantity Optimisation	$d = 12.0$	
Grade	Bolt Grade Optimisation	12.9	
Bolt.fu		1200.0	
Bolt.fy		1080.0	
Hole Diameter (mm)		$d_0 = 13.0$	
Slip Resistance		$V_{dsf} = \frac{\mu_f\ n_e\ K_h\ F_o}{\gamma_{mf}}$ $Where, F_o = 0.7 * f_{ub} A_{nb}$ $V_{dsf} = \frac{0.3 * 1 * 1.0 * 0.7 * 1200.0 * 0.0}{1.25}$ $= 33.99$	
No of Bolts	$R_u = \sqrt{V_u^2 + A_u^2}$ $n_{trial} = R_u / V_{bolt}$ $R_u = \frac{\sqrt{0.0^2 + 256.59^2}}{33.99}$ $= 16$	16	
No of Columns		$n_c = 8$	
No of Rows		$n_r = 2$	
Min. Pitch (mm)	$p/g_{min} = 2.5\ d$ $= 2.5 * 12.0 \quad = 30.0$	30	Pass
Max. Pitch (mm)	$p/g_{max} = \min(32\ t,\ 300\ mm)$ $= \min(32 *\ 12.0,\ 300\ mm)$ $= 300$	30	Pass
Min. Gauge (mm)	$p/g_{min} = 2.5\ d$ $= 2.5 * 12.0 \quad = 30.0$	0.0	N/A
Max. Gauge (mm)	$p/g_{max} = \min(32\ t,\ 300\ mm)$ $= \min(32 *\ 12.0,\ 300\ mm)$ $= 300$	0.0	N/A
Min. End Distance (mm)	$e/e`_{min} = [1.5\ or\ 1.7] * d_0$ $= 1.7 * 13.0 = 22.1$	25	Pass
Max. End Distance (mm)	$e/e`_{max} = 12\ t\ \varepsilon$ $\varepsilon = \sqrt{\frac{250}{f_y}}$ $e/e`_{max} = 12\ * 12.0 * \sqrt{\frac{250}{250}}$ $= 144.0$	25	Pass
Min. Edge Distance (mm)	$e/e`_{min} = [1.5\ or\ 1.7] * d_0$ $= 1.7 * 13.0 = 22.1$	27.65	Pass

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Check	Required	Provided	Remarks
Max. Edge Distance (mm)	$e/e`_{max} = 12\ t\ \varepsilon$ $\varepsilon = \sqrt{\frac{250}{f_y}}$ $e/e`_{max} = 12\ * 12.0 * \sqrt{\frac{250}{250}}$ $= 144.0$	27.65	Pass
Bolt Capacity post Long Joint (kN)	$if\ l \geq 15 * d\ then\ V_{rd} = \beta_{ij} * V_{db}$ $if\ l < 15 * d\ then\ V_{rd} = V_{db}$ $where,$ $l = ((nc\ or\ nr) - 1) * (p\ or\ g)$ $\beta_{ij} = 1.075 - l/(200 * d)$ $but\ 0.75 \leq \beta_{ij} \leq 1.0$	$l\ = ((nc\ or\ nr) - 1) * (p\ or\ g)$ $lc = 2 * ((\frac{8}{2} - 1) * 30 + 25) + 10.0$ $= 240.0$ $lr = 2 * ((\frac{2}{2} - 1) * 0.0 + 27.65$ $+ 15.0) + 9.4 = 94.7$ $l\ = 240.0$ $15 * d = 15 * 12.0 = 180.0$ $since,\ l \geq 15 * d$ $then\ V_{rd} = \beta_{ij} * V_{db}$ $\beta_{ij} = 1.075 - 240.0/(200 * 12.0)$ $= 0.98$ $V_{rd} = 0.98 * 33.99 = 33.31$	
Capacity (kN)	32.07	33.31	Pass

2.10 Web Bolt Checks

Check	Required	Provided	Remarks
Slip Resistance		$V_{dsf} = \frac{\mu_f\ n_e\ K_h\ F_o}{\gamma_{mf}}$ $Where, F_o = 0.7 * f_{ub} A_{nb}$ $V_{dsf} = \frac{0.3 * 1 * 1.0 * 0.7 * 1200.0 * 84.3}{1.25}$ $= 33.99$	
No of Bolts	$R_u = \sqrt{V_u^2 + A_u^2}$ $n_{trial} = R_u / V_{bolt}$ $R_u = \frac{\sqrt{307.27^2 + 266.11^2}}{33.99}$ $= 24$	60	
No of Columns		$n_c = 6$	
No of Rows		$n_r = 10$	
Min. Pitch (mm)	$p/g_{min} = 2.5\ d$ $= 2.5 * 12.0 \quad = 30.0$	30	Pass
Max. Pitch (mm)	$p/g_{max} = \min(32\ t,\ 300\ mm)$ $= \min(32 *\ 6.0,\ 300\ mm)$ $= 192.0$	30	Pass
Min. Gauge (mm)	$p/g_{min} = 2.5\ d$ $= 2.5 * 12.0 \quad = 30.0$	30	Pass
Max. Gauge (mm)	$p/g_{max} = \min(32\ t,\ 300\ mm)$ $= \min(32 *\ 6.0,\ 300\ mm)$ $= 192.0$	30	Pass
Min. End Distance (mm)	$e/e`_{min} = [1.5\ or\ 1.7] * d_0$ $= 1.7 * 13.0 = 22.1$	25	Pass
Max. End Distance (mm)	$e/e`_{max} = 12\ t\ \varepsilon$   $\varepsilon = \sqrt{\frac{250}{f_y}}$   $e/e`_{max} = 12\ * 6.0 * \sqrt{\frac{250}{250}}$ $= 72.0$	25	Pass
Min. Edge Distance (mm)	$e/e`_{min} = [1.5\ or\ 1.7] * d_0$ $= 1.7 * 13.0 = 22.1$	25	Pass
Max. Edge Distance (mm)	$e/e`_{max} = 12\ t\ \varepsilon$   $\varepsilon = \sqrt{\frac{250}{f_y}}$   $e/e`_{max} = 12\ * 6.0 * \sqrt{\frac{250}{250}}$ $= 72.0$	25	Pass

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Check	Required	Provided	Remarks
Parameters required for bolt force (mm)		$l_n = length\ available$ $l_n = (n_r - 1) * g$ $= (10 - 1) * 30$ $= 270$ $y_{max} = l_n/2$ $= 270/2$ $= 135.0$ $x_{max} = p * (\frac{n_c}{2} - 1)/2$ $= 30 * (\frac{6}{2} + -1)/2$ $= 30.0$	
Moment Demand (kNm		$M_d = (V_u * ecc + M_w)$ $= \frac{(307.27 * 10^3 * 75.0 + 12.02 * 10^6)}{10^6}$ $= 35.07$	
Bolt.Force		$vbv = V_u/(n_r * n_c)$ $= \frac{307.27}{(10 * 6)}$ $= 10.24$ $tmh = \frac{M_d * y_{max}}{\Sigma r_i^2}$ $= \frac{35.07 * 135.0}{240.75}$ $= 19.66$ $tmv = \frac{M_d * x_{max}}{\Sigma r_i^2}$ $= \frac{35.07 * 30.0}{240.75}$ $= 4.37$ $abh = \frac{A_u}{(n_r * n_c)}$ $= \frac{266.11}{(10 * 6)}$ $= 8.87$ $vres = \sqrt{(vbv + tmv)^2 + (tmh + abh)^2}$ $= \sqrt{(10.24 + 4.37)^2 + (19.66 + 8.87)^2}$ $= 32.06$	

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Check	Required	Provided	Remarks
Bolt Capacity post Long Joint (kN)	$if\ l \geq 15 * d\ then\ V_{rd} = \beta_{ij} * V_{db}$ $if\ l < 15 * d\ then\ V_{rd} = V_{db}$ $where,$ $l = ((nc\ or\ nr) - 1) * (p\ or\ g)$ $\beta_{ij} = 1.075 - l/(200 * d)$ $but\ 0.75 \leq \beta_{ij} \leq 1.0$	$l = ((nc\ or\ nr) - 1) * (p\ or\ g)$ $lc = 2 * ((\frac{6}{2} - 1) * 30 + 25) + 10.0$ $= 180.0$ $lr = (10 - 1) * 30 = 270$ $l = 270$ $15 * d = 15 * 12.0 = 180.0$ $since,\ l \geq 15 * d$ $then\ V_{rd} = \beta_{ij} * V_{db}$ $\beta_{ij} = 1.075 - 270/(200 * 12.0)$ $= 0.96$ $V_{rd} = 0.96 * 33.99 = 32.63$	
Capacity (kN)	32.06	32.63	Pass

2.11 Inner and Outer flange plate Checks

Check	Required	Provided	Remarks
Min. Plate Height (mm)	$min\ flange\ plate\ ht = beam\ width$ $= 150.0$	150.0	Pass
Min. Plate Length (mm)	$2[2 * e_{min} + (\frac{bolt\ lines}{2} - 1) * p_{min})]$ $+ \frac{gap}{2}]$ $= 2 * [(2 * 22.1 + (\frac{8}{2} - 1) * 30.0$ $= + \frac{10.0}{2}]$ $= 278.4$	290.0	Pass
Min. Inner Plate Height (mm)	$= \frac{B - t - (2 * R1)}{2}$ $= \frac{150.0 - 9.4 - 2 * 15.0}{2}$ $= 55.3$	55.3	Pass
Max. Inner Plate Height (mm)	$= \frac{B - t - (2 * R1)}{2}$ $= \frac{150.0 - 9.4 - 2 * 15.0}{2}$ $= 55.3$	55.3	Pass
Min. Inner Plate Length (mm)	$2[2 * e_{min} + (\frac{bolt\ lines}{2} - 1) * p_{min})]$ $+ \frac{gap}{2}]$ $= 2 * [(2 * 22.1 + (\frac{8}{2} - 1) * 30.0$ $= + \frac{10.0}{2}]$ $= 278.4$	290.0	Pass
Min.Plate Thickness (mm)	$t_w = 8.7$	12.0	Pass

2.12 Member Checks

Check	Required	Provided	Remarks
Flange Tension Yielding Capacity (kN)		$T_{dg} = \frac{l * t * f_y}{\gamma_{mo}}$ $= \frac{1 * 150.0 * 17.4 * 250}{1.1}$ $= 593.18$	
Flange Tension Rupture Capacity (kN)		$T_{dn} = \frac{0.9 * A_n * f_u}{\gamma_{m1}}$ $= \frac{0.9 * (150.0 - 2 * 13.0) * 17.4 * 410}{1.25}$ $= 636.92$	
Flange Block Shear Capacity (kN)		$T_{db1} = \frac{A_{vg} f_y}{\sqrt{3}\gamma_{m0}} + \frac{0.9 A_{tn} f_u}{\gamma_{m1}}$ $T_{db2} = \frac{0.9 * A_{vn} f_u}{\sqrt{3}\gamma_{m1}} + \frac{A_{tg} f_y}{\gamma_{m0}}$ $T_{db} = min(T_{db1}, T_{db2}) = 630.9$	
Flange Tension Capacity (kN)	$f_f = 256.59$	$T_d = min(T_{dg}, T_{dn}, T_{db})$ $= min(593.18, 636.92, 630.9)$ $= 593.18$	Pass
Web Tension Yielding Capacity (kN)		$T_{dg} = \frac{l * t * f_y}{\gamma_{mo}}$ $= \frac{1 * 415.2 * 9.4 * 250}{1.1}$ $= 887.02$	
Web Tension Rupture Capacity (kN)		$T_{dn} = \frac{0.9 * A_n * f_u}{\gamma_{m1}}$ $= \frac{0.9 * (415.2 - 10 * 13.0) * 9.4 * 410}{1.25}$ $= 791.4$	
Web Block Shear Capacity (kN)		$T_{db1} = \frac{A_{vg} f_y}{\sqrt{3}\gamma_{m0}} + \frac{0.9 A_{tn} f_u}{\gamma_{m1}}$ $T_{db2} = \frac{0.9 * A_{vn} f_u}{\sqrt{3}\gamma_{m1}} + \frac{A_{tg} f_y}{\gamma_{m0}}$ $T_{db} = min(T_{db1}, T_{db2}) = 703.61$	
Web Tension Capacity (kN)	$A_w = 266.11$	$T_d = min(T_{dg}, T_{dn}, T_{db})$ $= min(887.02, 791.4, 703.61)$ $= 703.61$	Pass

2.13 Flange Plate Capacity Checks in axial-Outside/Inside

Check	Required	Provided	Remarks
Tension Yielding Capacity (kN)		$T_{dg} = \frac{l*t*f_y}{\gamma_{mo}}$ $= \frac{1*260.6*12.0*250}{1.1}$ $= 710.73$	
Tension Rupture Capacity (kN)		$T_{dn} = \frac{0.9*A_n*f_u}{\gamma_{m1}}$ $= \frac{0.9*(260.6-2*13.0)*12.0*410}{1.25}$ $= 831.05$	
Block Shear Capacity (kN)		$T_{db1} = \frac{A_{vg}f_y}{\sqrt{3}\gamma_{m0}} + \frac{0.9A_{tn}f_u}{\gamma_{m1}}$ $T_{db2} = \frac{0.9*A_{vn}f_u}{\sqrt{3}\gamma_{m1}} + \frac{A_{tg}f_y}{\gamma_{m0}}$ $T_{db} = min(T_{db1}, T_{db2}) = 977.66$	
Plate Tension Capacity (kN)	$f_f = 256.59$	$T_d = min(T_{dg}, T_{dn}, T_{db})$ $= min(710.73, 831.05, 977.66)$ $= 710.73$	Pass

2.14 Web Plate Capacity Checks in Axial

Check	Required	Provided	Remarks
Tension Yielding Capacity (kN)		$T_{dg} = \frac{l * t * f_y}{\gamma_{mo}}$ $= \frac{1 * 320 * 6.0 * 250}{1.1}$ $= 503.87$	
Tension Rupture Capacity (kN)		$T_{dn} = \frac{0.9 * A_n * f_u}{\gamma_{m1}}$ $= \frac{0.9 * (320 - 10 * 13.0) * 6.0 * 1200.0}{1.25}$ $= 673.06$	
Block Shear Capacity (kN)		$T_{db1} = \frac{A_{vg} f_y}{\sqrt{3}\gamma_{m0}} + \frac{0.9 A_{tn} f_u}{\gamma_{m1}}$ $T_{db2} = \frac{0.9 * A_{vn} f_u}{\sqrt{3}\gamma_{m1}} + \frac{A_{tg} f_y}{\gamma_{m0}}$ $T_{db} = min(T_{db1}, T_{db2}) = 898.23$	
Plate Tension Capacity (kN)	$A_w = 266.11$	$T_d = min(T_{dg}, T_{dn}, T_{db})$ $= min(872.73, 673.06, 898.23)$ $= 673.06$	Pass

2.15 Web Plate Capacity Checks in Shear

Check	Required	Provided	Remarks
Shear yielding Capacity (V_dy) (kN)		$V_{dy} = \frac{A_v * f_y}{\sqrt{3} * \gamma_{mo}} = \frac{1 * 320 * 6.0 * 250}{\sqrt{3} * 1.1} = 503.87$	
Shear Rupture Capacity (V_dn) (kN)		$V_{dn} = \frac{0.9 * A_{vn} * f_u}{\sqrt{3} * \gamma_{m1}} = \frac{0.9 * (320 - (3.0 * 13.0)) * 6.0 * 410}{\sqrt{3} * 1.25} = 388.59$	
Block Shear Capacity in Shear (V_db) (kN)		$T_{db1} = \frac{A_{vg} f_y}{\sqrt{3}\gamma_{m0}} + \frac{0.9 A_{tn} f_u}{\gamma_{m1}}$ $T_{db2} = \frac{0.9 * A_{vn} f_u}{\sqrt{3}\gamma_{m1}} + \frac{A_{tg} f_y}{\gamma_{m0}}$ $T_{db} = min(T_{db1}, T_{db2}) = 582.57$	
Plate Shear Capacity (kN)	$V_u = 307.27$	$V_d = min(V_{dy}, V_{dn}, V_{db}) = min(503.87, 388.59, 898.23) = 388.59$	Pass

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3 3D View

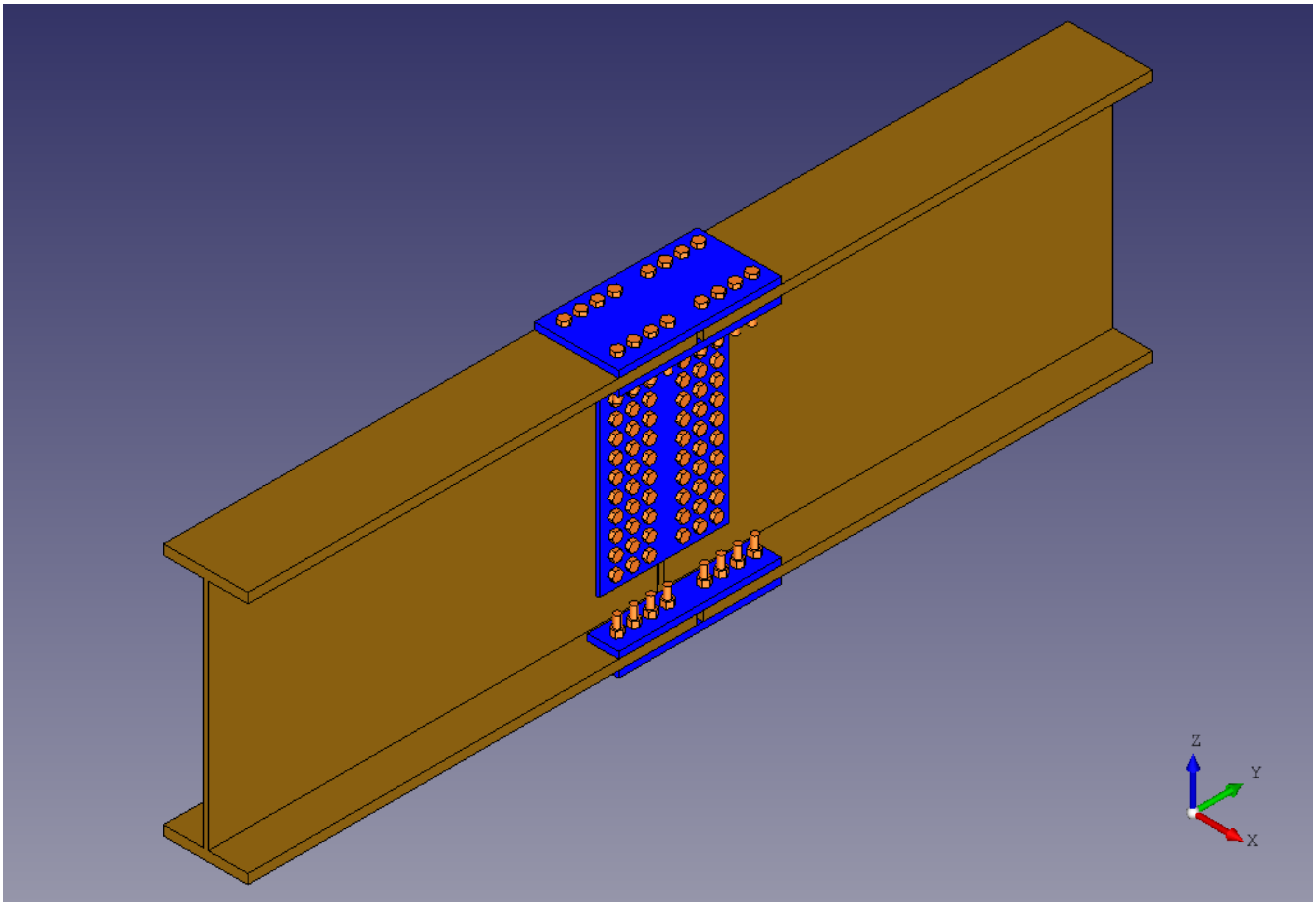

Figure 1: 3D View