

Pacific Sea Level and Geodetic Monitoring Project: Levelling & GNSS Monitoring Survey Report

Yaren, Nauru, October 2019

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1 Motivation

The Australian Bureau of Meteorology (Bureau), Geoscience Australia (GA) and the Pacific Community (SPC) work together on the Australian Aid funded Pacific Sea Level and Geodetic Monitoring Project (PSLGMP). The project is focused on determining the long-term variation in sea level through observation and analysis of changes in the height of the land using Global Navigation Satellite System (GNSS) data and changes in the sea level using tide gauges managed and operated by the Bureau. It is the role of GA and SPC to provide information about 'absolute' movement of the tide gauge (managed by Bureau) using GNSS to continuously monitor land motion and using levelling (SPC) to measure the height difference between the tide gauge and GNSS pillar every 18 months.

Land movement caused by earthquakes, subsidence and surface uplift have an important effect on sea level observations at tide gauges. For example, a tide gauge connected to a pier which is subsiding at a rate of 5 mm per year would be observed as a rate of 5 mm per year of sea level rise at the tide gauge. Because of this, it is important to measure, and account for, the movement of land when measuring 'absolute' sea level variation - the change in the sea level relative to the centre of the Earth. Relative sea level variation on the other hand is measured relative to local buildings and landmass around the coastline.

Geoscience Australia's work enables more accurate 'absolute' sea level estimates by providing observations of land motion which can be accounted for by Bureau when analysing the tide gauge data.

2 Introduction

This report provides the results of the GNSS monitoring survey & high precision level survey completed between the Sea Level Fine Resolution Acoustic Measuring Equipment (SEAFRAME) tide gauge and the GNSS Continuously Operation Reference Station (CORS) in Yaren, Nauru from 24th to 29th October 2019. It also provides an updated height of the tide gauge derived from GNSS time series analysis and precise levelling observations.

GNSS Monitoring Survey

A high precision geodetic terrestrial survey is undertaken to monitor the stability of the GNSS CORS monument. This survey is used to complement GNSS analysis by determining whether movement detected by GNSS analysis is caused by localised movement of the pillar, or movement of the land across a larger area. Local movement is monitored by examining and comparing the results of repeat surveys to the monument and permanent reference marks approximately 15 m from the GNSS monument.

Levelling Survey

The Total Station levelling technique is used to observe the difference in height between the Tide Gauge and GNSS monument. The levelling route includes a deep driven benchmark array in Nauru, which runs approximately 3.7 km. Previous levelling surveys have been conducted along this route using the Total Station differential levelling technique in 2005, 2007, 2009, 2010, 2012, 2013, 2015, 2016 and 2018. This report contains an analysis of the 2019 Total Station differential levelling and GNSS monitoring results as well as a combined comparison of the previous levelling surveys.

Personnel

Personnel involved in the GNSS monitoring and levelling surveys were Andrick Lal and Zulfikar Begg, from the Geodetic Survey at SPC. The GNSS time series analysis and derivation of the tide gauge ellipsoidal height was undertaken by the GNSS analysis team at Geoscience Australia

2.1 Site Description and Contacts

The levelling benchmark array between the GNSS CORS, and SEAFRAME is approximately 3.2kms long, running from the GNSS CORS pillar (NAUR) at the southern end of the runway, along its length, then along the main road, and through the phosphate offices, to the tide gauge at the harbour on the west side of the island.

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2.2 Survey Support

The survey team very much appreciated the assistance from the Republic of Nauru Lands Department, who received the survey equipment freight, and were able to make a staff member available to assist us during the survey work.

The PSLGM Survey team would also like to acknowledge the continued support of the Republic of Nauru Lands Department and appreciate the continued working relationship that has been in place since 2003.

3 Measurement Network

3.1 Terrestrial Network

The Total Station differential levelling survey was carried out between the GNSS CORS and the SEAFRAME Tide Gauge using the existing deep driven benchmark array. This consists of Primary deep driven benchmarks, and temporary holding marks (Table 3.1).

Table 3.1 The primary survey control network. Locality diagrams of these marks are provided in Appendix A.

Name	Description
NAU1	Deep driven benchmark
NAU2	Deep driven benchmark
NAU33	Deep driven benchmark
NAU36	Deep driven benchmark
NAU37	Deep driven benchmark
NAU38	Deep driven benchmark
NAU16	Deep driven benchmark
NAU15	SEAFRAME sensor benchmark
NAU14a	SEAFRAME Project benchmark
NAURBM	Reference benchmark for the GNSS CORS pillar
RM1	GNSS CORS reference mark 1
RM2	GNSS CORS reference mark 2
RM3	GNSS CORS reference mark 3

Upon inspection, all the deep driven benchmarks were located, found in good order, and undisturbed. Included in the survey were the temporary holding marks; 53, 104, 103, 110, 101, 47, 109, 44, 43, 107, 106, 100, and 28 (Fig 3.6). The benchmark, NAU14, was not found, and therefore replaced with a new benchmark, NAU14A (masonry nail in concrete).

3.1.1 PSLGMP Vertical Reference Frame Wiring Diagram

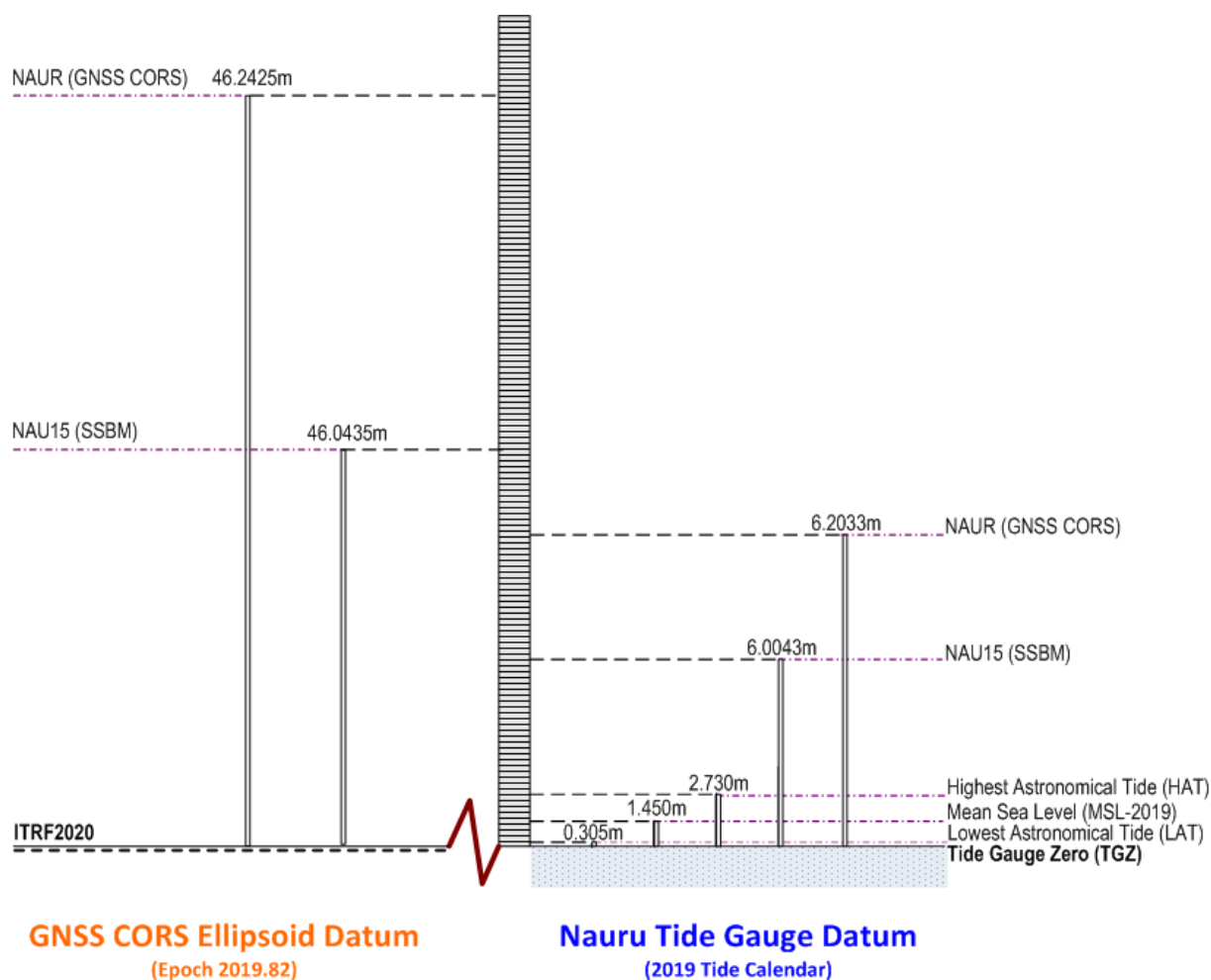


Figure 3.1 Wiring diagram depicting the offsets between surveyed marks. The left-hand side shows the height of the GNSS CORS pillar (NAUR), SEAFRAME sensor reference benchmark (NAU15), with respect to the International Terrestrial Reference Frame 2020 at epoch 2019.82. The right-hand side shows the height of NAUR, NAU15, and tidal datums with respect to tide gauge zero. For more information on tidal datums, please refer to [Pacific Sea Level and Geodetic Monitoring Project File information and Instructions \(bom.gov.au\)](http://bom.gov.au/pacific-sea-level-and-geodetic-monitoring-project-file-information-and-instructions)

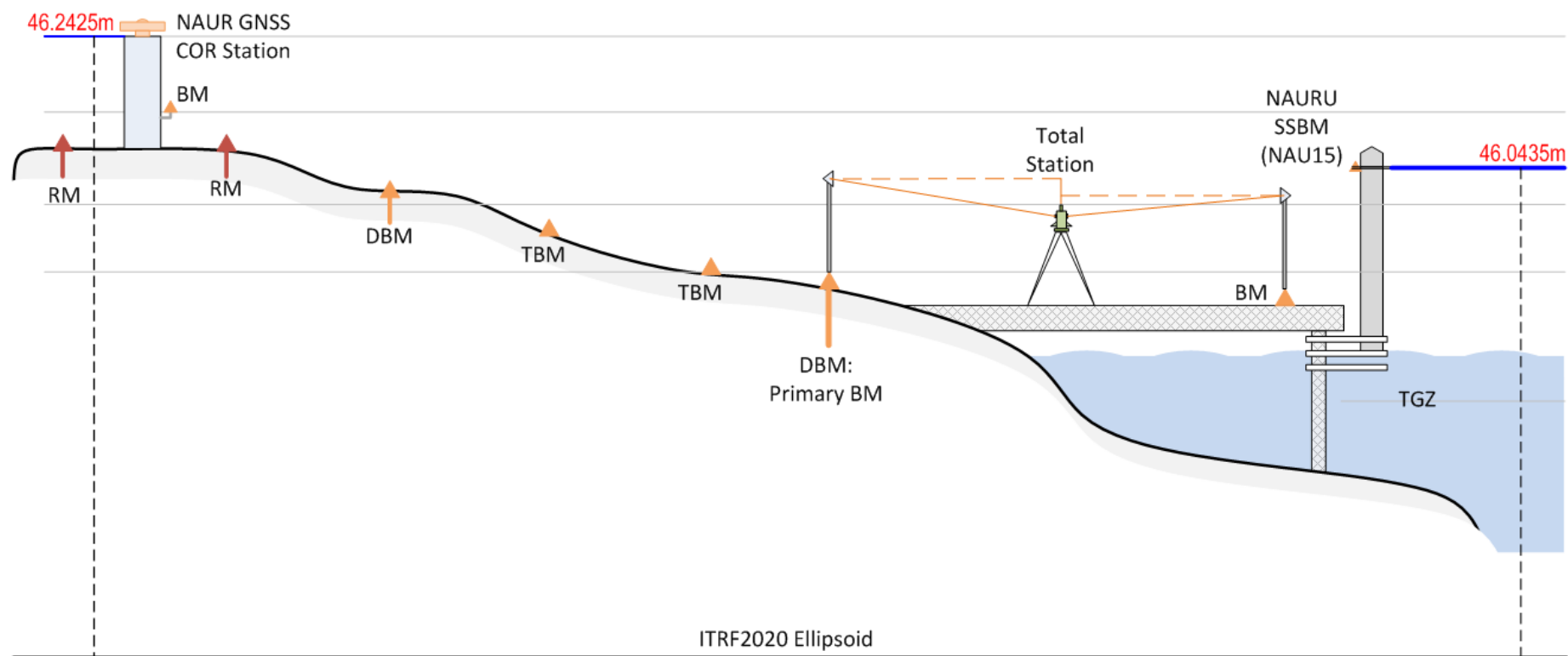


Figure 3.2 Simplified representation of the Total Station differential levelling survey carried out between the GNSS CORS and the SEAFRAME Tide Gauge.



Figure 3.3 Tide Gauge Station. The red circle denotes the location of the SEAFRAME sensor benchmark (NAU15) at the port harbour.



Figure 3.4 GNSS COR Station (NAUR) at the Yaren District in Nauru.

3.1.2 Levelling Benchmark Network

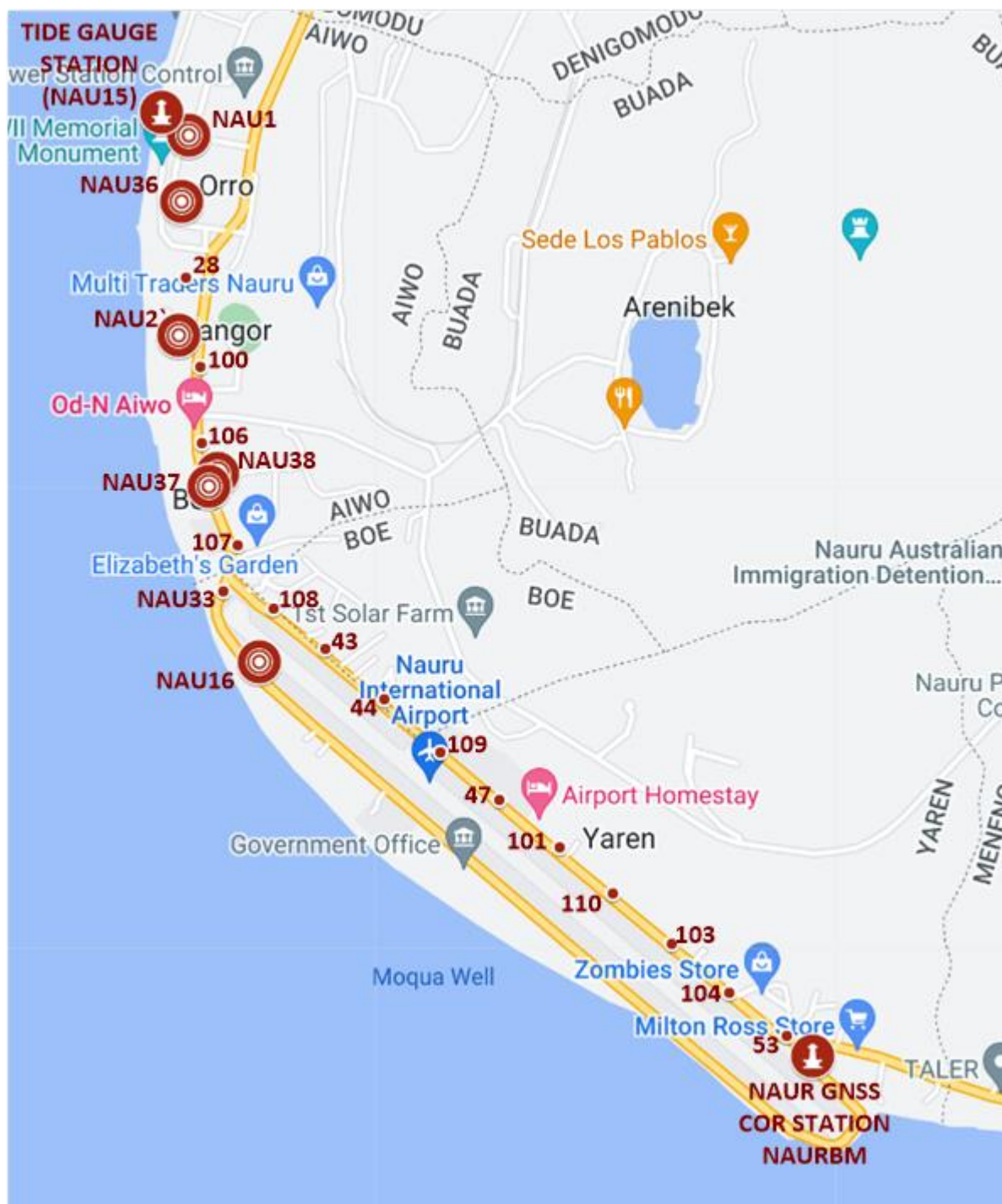


Figure 3.6 Levelling benchmark array. Source: Adopted from Google Maps.

3.1.3 GNSS CORS and Reference Marks

The GNSS CORS site is located within a compound adjacent to the southern end of the airport runway, at the intersection of the island circuit road and the airport bypass road. The site consists of a building to house the technical equipment and a 1.5 m GNSS CORS antenna pillar. The pillar is 25 metres from the GNSS CORS hut. Access can be arranged with the survey office who holds the key for the gate. Entry is unrestricted after initial visit to the Lands & Survey department.

Three primary deep driven Reference Marks (RM) benchmarks were placed at the time of installation at a distance of 20 m to 30 m from the GNSS monument at approximately 120 degree radial spacing from true north (Fig 3.6). The RM's consist of capped 20 mm stainless steel rods driven to refusal and are protected by 150mm PVC pipe within circular poly carbonate boxes.

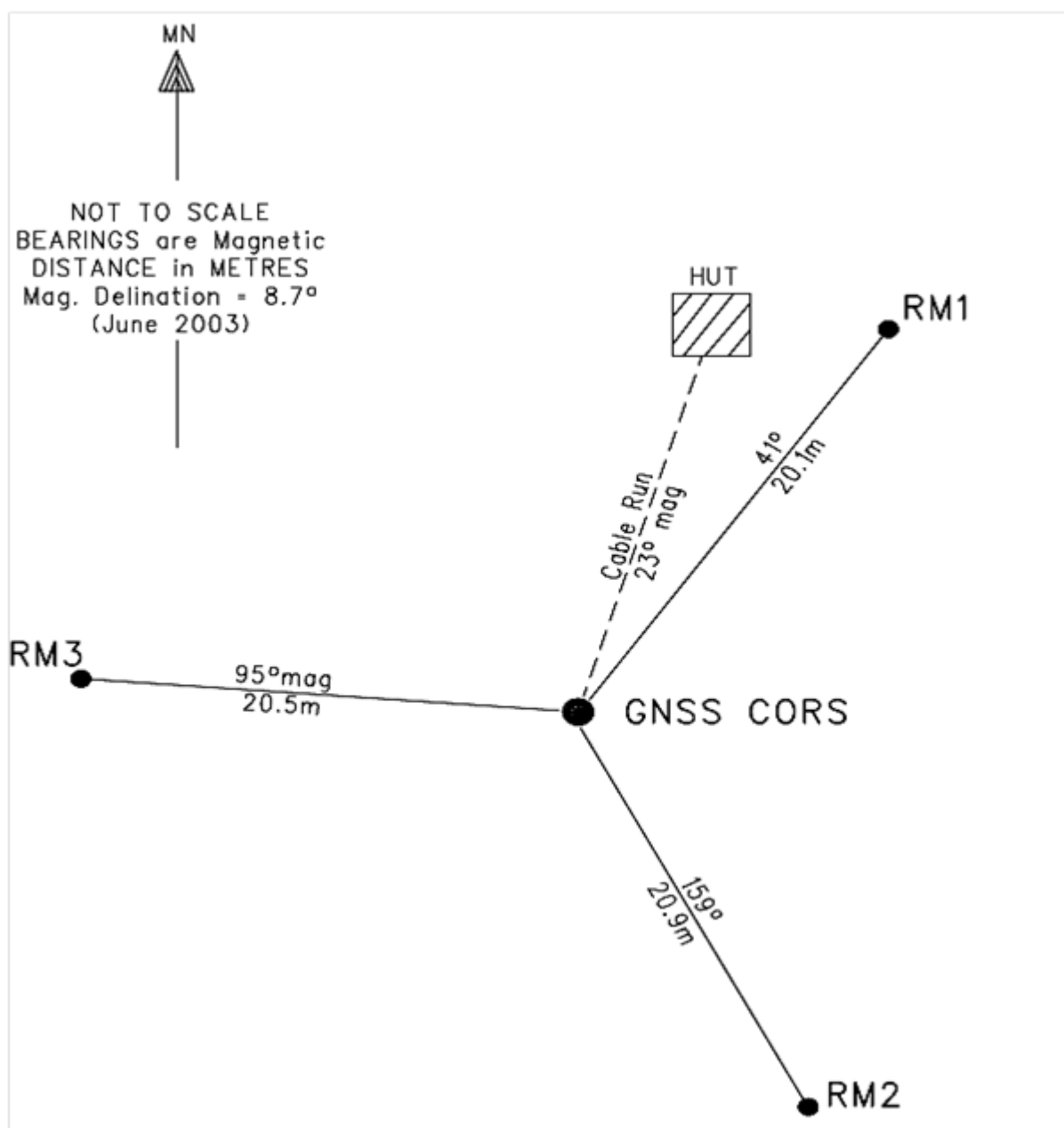


Figure 3.7 GNSS CORS site monitoring survey reference marks.

3.2 Datum

3.2.1 Survey Datum

The adopted reference point for the levelling survey is the levelling benchmark connected to the side of the GNSS CORS pillar (NAURBM) fixed at 0.0000m.

3.2.2 Historical Survey Datum

The datum for the previous surveys was the Nauru Island Datum (NID). And the adopted reference point from the levelling survey was NAU1 fixed at RL of 7.2930m. This value was determined by the National Tidal Centre Australia (NTCA) in 1993 by adopting the height from the old survey benchmark, UH1 with RL of 4.130m (NID) and in 1994 by adopting the height from the old survey benchmark, U2B with RL of 3.8894m NID.

4 Monitoring Survey

4.1 Background

A local monitoring survey is undertaken routinely to monitor for any local horizontal or vertical movement of the GNSS pillar, relative to the RMs. The RMs are all located within 30 m of the GNSS pillar. We acknowledge that this monitoring does not account for any movement over the wider area – i.e., movements that might be consistent across all RM's.

4.1.1 Methodology

The Total Station is used to observe and record all horizontal and vertical angles and slope distances in the network by setup and observation from each RM.

Two monitoring techniques can be used to determine movement of the GNSS monument.

The conventional 'Direct Method', involves removing the GNSS antenna and setting up the Total Station on the pillar to directly observe to a prism setup on a tripod over each RM. The Total Station is then moved to each RM in turn and observations are made directly to the pillar and other RMs from each setup. This method can also provide a direct observation to the height of the antenna mount, but obviously requires an interruption to the GNSS data when the antenna is removed, which is not ideal.

The 'Indirect Method' was developed to leave the antenna undisturbed. The symmetrical properties of the antenna are used to indirectly measure the centre of the antenna by triangulation from each RM. To measure the horizontal position of the Antenna Reference Point (ARP), angular direction observations are made to symmetrically coupled points on the external profile of the antenna (Figure 4.1) from each RM. The angular observations from all setups can be averaged and intersected to give a position of the central axis of the antenna by way of triangulation from the three RMs.

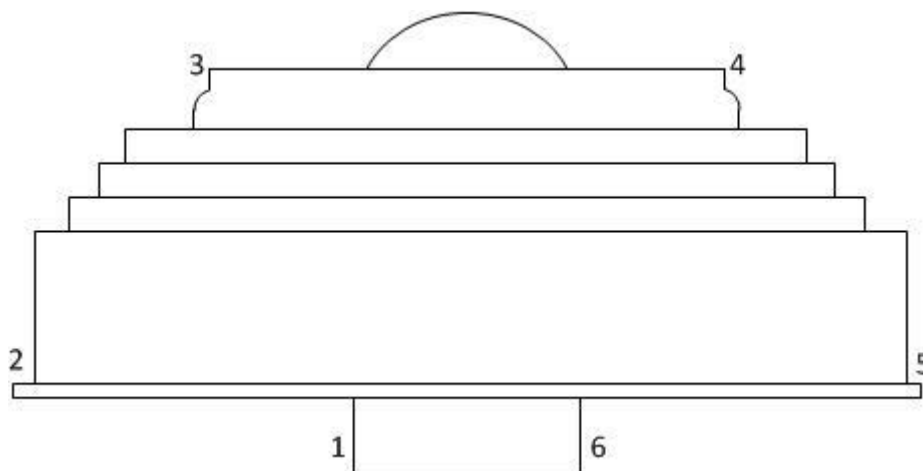


Figure 4.1 Symmetrical points on the antenna profile (LEIAR25.R3) observed from each RM.

The Indirect Method does not allow for a measurement of the vertical position of the monument. Instead, the result of the RM levelling survey (from each RM to the GNSS BM) is used, and then the known offset from the BM to the antenna mounting plate is applied.

Both techniques used will allow comparison to previous years, to monitor any movement of the pillar over time. The reduced observations are put into a least squares adjustment program, DynAdjust (Fraser et al., 2018), to determine the final coordinates by holding the point at the centre of the GNSS pillar plate fixed and calculating the relative movement of each RM in ΔE , ΔN & ΔU , and an estimate of the error.

To avoid introducing any discontinuities into the GNSS time-series it is preferred, where possible, that the Indirect Method of observation be used.

4.2 Horizontal Observations

The heights of the RMs are observed using the Total Station levelling (EDM height traversing) technique, with a Leica Total Station and two fixed height rods with precision reflectors (see Appendix C).

A horizontal control survey was conducted following the ICSM SP1 Guideline for Conventional Traverse Surveys (ICSM, 2021). Five sets of observations were completed at each standpoint; a set consists of a round of face left observations, followed by a round of face right observations to each of the visible survey marks. For each observation a horizontal direction, zenith angle and slope distance was recorded. At each instrument set-up atmospheric conditions (temperature, pressure and relative humidity) were recorded. Atmospheric conditions were applied during the post-processing stage and not directly into the Total Station. Instrument and target heights were measured using an offset tape.

4.3 Data Analysis and Results

4.3.1 Levelling Survey

Reduction of the digital data was computed using the Geoscience Australia levelling program "leveling1.exe" and LevellingFIELD_3.pl. This program computes the height difference between the two reflectors by taking the mean average of the measured height differences and also providing standard deviations and a misclose for the levelling loop. Refer to Section 5 for a detailed description of the levelling process.

Table 4.3.1 Reduced Level (RL) Shown is the height relative to NAURBM.

From	To	Rise (m)	Fall (m)	RL (m)	Dist (km)	Acc Dist (km)
NAURBM				0.0000		0.000
RM1	RM1	0.0000	-0.7124	-0.7124	0.020	0.020
RM2	RM2	0.0000	-0.1625	-0.8749	0.0355	0.0557
RM3	RM3	0.0978	0.0000	-0.7771	0.036	0.092
RM2	RM2	0.0000	-0.0980	-0.8750	0.036	0.000
RM1	RM1	0.1625	0.0000	-0.7125	0.0353	0
	NAURBM	0.7125	0.0000	-0.0001	0.020	0.000
	Sum:	0.9727	-0.9728			
	Misclose:		-0.0001	-0.0001	0.183	(Total Dist)
			ALLOWABLE (m):	0.0006	2 x Sqrt (km) test:	PASS

From	To	Rise (m)	Fall (m)	RL (m)	Dist (km)	Acc Dist (km)
NAURBM				0.0000		0.000
RM2	RM2	0.0000	-0.8749	-0.8749	0.021	0.021
NAURBM	NAURBM	0.8750	0.0000	0.0001	0.021	0.000
	Sum:	0.8750	-0.8749			
	Misclose:		0.0001	0.0001	0.041	(Total Dist)
			ALLOWABLE (m):	0.0003	2 x Sqrt (km) test:	PASS

From	To	Rise (m)	Fall (m)	RL (m)	Dist (km)	Acc Dist (km)
NAURBM				0.0000		0.000
RM3	RM3	0.0000	-0.7771	-0.7771	0.023	0.023
	NAURBM	0.7770	0.0000	-0.0001	0.023	0.000
	Sum:	0.7770	-0.7771			
	Misclose:		-0.0001	-0.0001	0.045	(Total Dist)
			ALLOWABLE (m):	0.0003	2 x Sqrt (km) test:	PASS

4.3.2 Geodetic Adjustment

All observations were combined into a geodetic adjustment using DynAdjust (Fraser et al., 2018). In the adjustment, the point on the GNSS pillar plate (NAUR) was tightly constrained to its ITRF2014 coordinates and aligned to NAUR-RM1 with an azimuth of 49° 42' 00.33", which had been determined in the 2003 survey by GNSS observation to RM1. The angular observations were given an uncertainty of 1.0" and the slope distances an uncertainty of 1.0 mm. The estimated coordinates and associated variance-covariance matrix were output in a SINEX file format and have been provided to Geoscience Australia.

The ITRF2014@2010.0 latitude and longitude coordinates adopted at NAUR as GNSS constraint are taken from the Geoscience Australia GNSS portal¹. The ellipsoidal height is the ITRF2020 height from the week of the survey. For more information on how this ellipsoidal height was computed, see Brown et al. (2020).

Table 4.3.2 Latitude, Longitude and Ellipsoidal Height (metres) for the GNSS & RM stations ITRF2014@2010.0 Latitude, Longitude coordinates, and ITRF2020@2019.82 ellipsoidal height were adopted at NAUR. CCC means all 3 dimensions (in XYZ) are constrained and FFF means they were all free,

Station	Constrain	Latitude	Longitude	Ellipsoidal height (m)
NAUR	CCC	-0° 33' 06.22602"	166° 55' 31.97186"	46.2425
RM1	FFF	-0° 33' 05.80272"	166° 55' 32.46768"	44.5818
RM2	FFF	-0° 33' 06.89241"	166° 55' 32.11098"	44.4185
RM3	FFF	-0° 33' 06.02644"	166° 55' 31.33880"	44.5176

Table 4.3.3 Earth Centred Cartesian coordinates and associated standard deviations (metres) for the GNSS & RM stations. ITRF2014@2010.00 Latitude, Longitude coordinates as per <https://gnss.ga.gov.au/network> ITRF2020@2019.82 ellipsoidal height were adopted at NAUR

Description	X	Y	Z	SD(e)	SD(n)	SD(up)
NAUR	-6212555.0485	1442787.0098	-61006.8008	0.0000	0.0000	0.0000
RM1	-6212557.0210	1442771.7287	-60993.5887	0.0005	0.0004	0.0006
RM2	-6212554.0529	1442781.3624	-61027.0557	0.0009	0.0006	0.0006
RM3	-6212549.9977	1442805.7001	-61000.4593	0.0006	0.0009	0.0006

Table 4.3.4 Difference in XYZ coordinates between the GNSS pillar and RMs (metres)

FROM	To	ΔE	ΔN	ΔU
NAUR	RM1	15.3312	-20.4705	-1.6608
NAUR	RM2	4.3017	-20.4684	-1.8240
NAUR	RM3	-19.5746	6.1301	-1.7250

¹ GNSS Network Portal ([ga.gov.au](https://gnss.ga.gov.au))

4.4 Assessment of Results

Table 4.4.1 and Figures 4.4.1 – 4.4.3 show the movement of the reference marks with respect to the GNSS pillar in ΔE , ΔN and ΔU . No obvious errors or movement are apparent in the time series.

Table 4.4.1 Topocentric vectors showing delta east, delta north and delta up between the GNSS pillar and each Reference Mark (metres).

YEAR	FROM	To	ΔE	ΔN	ΔU
2003	NAUR	RM1	15.3306	13.0013	-1.6593
2004	NAUR	RM1	15.3310	13.0016	-1.6592
2005	NAUR	RM1	15.3306	13.0012	-1.6607
2007	NAUR	RM1	15.3308	13.0015	-1.6609
2009	NAUR	RM1	15.3304	13.0011	-1.6609
2010	NAUR	RM1	15.3309	13.0015	-1.661
(2010 ID)	NAUR	RM1	15.3308	13.0015	-1.661
2015	NAUR	RM1	15.3302	13.0009	-1.6606
2016	NAUR	RM1	15.3298	13.0005	-1.6608
2018	NAUR	RM1	15.3312	13.0017	-1.6609
2019	NAUR	RM1	15.3305	13.0012	-1.6601
Ref RL	(as at 2016)		15.3306	13.0012	-1.6605

YEAR	FROM	To	ΔE	ΔN	ΔU
2003	NAUR	RM2	4.3036	-20.4697	-1.8218
2004	NAUR	RM2	4.3033	-20.4697	-1.8217
2005	NAUR	RM2	4.303	-20.4708	-1.8233
2007	NAUR	RM2	4.3039	-20.4705	-1.8234
2009	NAUR	RM2	4.3036	-20.4699	-1.8235
2010	NAUR	RM2	4.3031	-20.4695	-1.8235
(2010 ID)	NAUR	RM2	4.3031	-20.4696	-1.8235
2015	NAUR	RM2	4.3026	-20.4687	-1.8233
2016	NAUR	RM2	4.3017	-20.4678	-1.8234
2018	NAUR	RM2	4.3017	-20.4684	-1.8242
2019	NAUR	RM2	4.302	-20.4689	-1.8236
Ref RL	(as at 2016)		4.3031	-20.4696	-1.8230

YEAR	FROM	To	ΔE	ΔN	ΔU
2003	NAUR	RM3	-19.5738	6.1246	-1.7238
2004	NAUR	RM3	-19.5743	6.1257	-1.7236
2005	NAUR	RM3	-19.5744	6.1254	-1.7253
2007	NAUR	RM3	-19.5743	6.1247	-1.7254
2009	NAUR	RM3	-19.5745	6.1269	-1.7255
2010	NAUR	RM3	-19.5746	6.1258	-1.7256
(2010 ID)	NAUR	RM3	-19.5747	6.1257	-1.7255
2015	NAUR	RM3	-19.5741	6.126	-1.7255
2016	NAUR	RM3	-19.5753	6.1264	-1.7254
2018	NAUR	RM3	-19.5746	6.1301	-1.7253
2019	NAUR	RM3	-19.5754	6.1274	-1.7262
Ref RL	(as at 2016)		-19.5744	6.1257	-1.7251

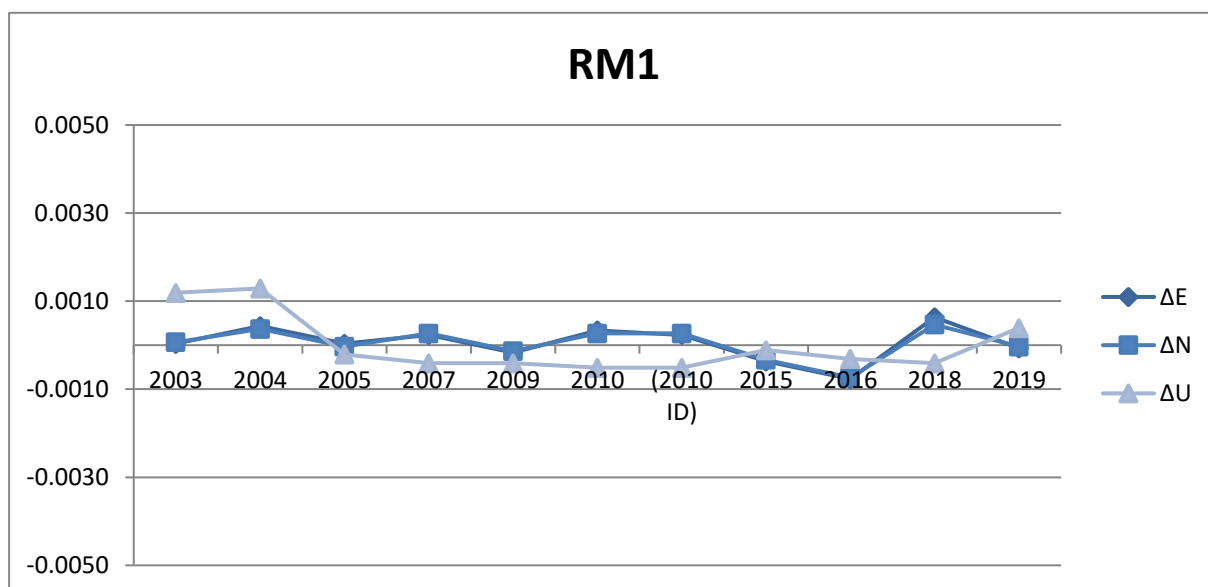


Figure 4.4.1 Time series of RM1 movement relative to GNSS pillar (0 = REF pre 2016 mean).

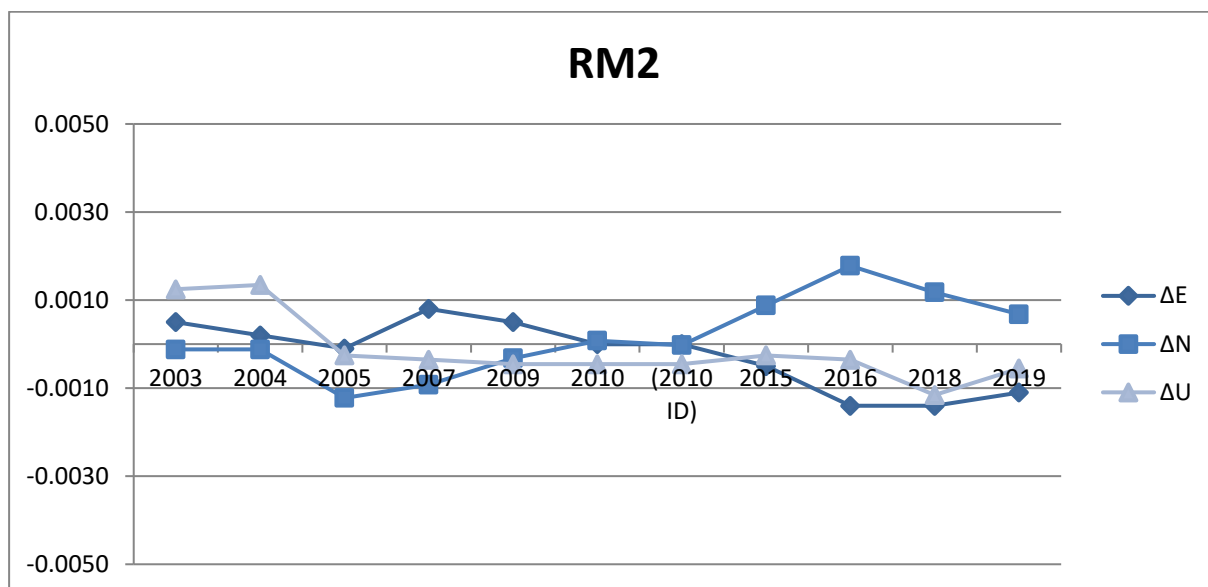


Figure 4.4.2 Time series of RM2 movement relative to GNSS pillar (0 = REF pre 2016 mean).

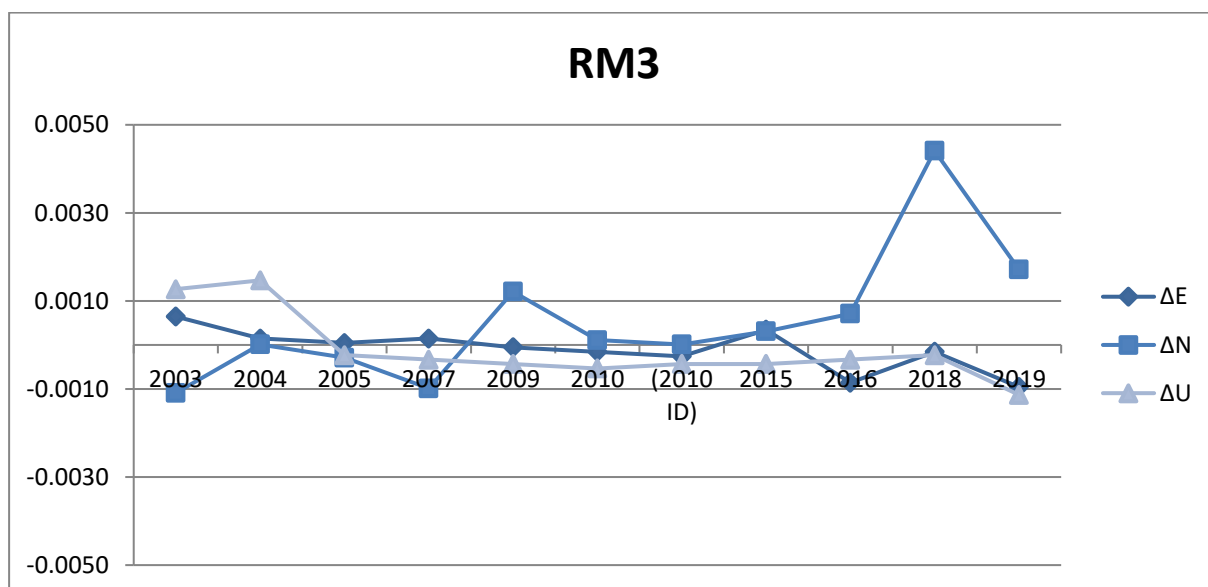


Figure 4.4.3 Time series of RM3 movement relative to GNSS pillar (0 = REF pre 2016 mean).

5 Tide Gauge Level Connection

5.1 Background

The Total Station differential levelling technique was performed in accordance with the SP1 levelling guidelines (ICSM, 2021). After reduction an internal precision of $1\text{mm}\sqrt{K}$ or better was achieved within each survey bay, where K is distance in kilometres.

5.2 Survey Methodology

The Total Station differential levelling technique was used for the Nauru tide gauge levelling survey. This technique uses a 'leap-frog' method which involves setting up a Total Station midway between two target/reflectors (on a reflector rod with bi-pod). The targets remain at a particular change point for the backsight and foresight observations and all levelling runs start and finish with the same reflector and reflector rod to eliminate any reflector rod 'zero error'.

This technique can also be performed using a single set-up / single rod configuration which was the case when levelling between benchmarks which are close together e.g., between the GNSS CORS RMs.

The levelling run was divided into bays between each holding benchmark. Observations were completed in both directions within the bay to close each loop along the way. This method provides a closure between bench marks and allows a hold point in the survey in case of severe weather, physical interference, or time restrictions while completing the survey across the week.

In support of the slope distance observations, the ambient temperature, pressure and humidity are recorded (Kestral 4000 pocket weather tracker) and input into the instrument to apply the first velocity correction to the observed distances (Rüeger & Brunner, 1982). Five rounds of observations are taken to the backsight and foresight targets from each instrument setup. The instrument measures slope distances ($\pm 1\text{mm}$) and vertical angle ($1''$) to derive height differences.

Table 5.2.2 contains the values of the constants or calibrated heights used throughout the analysis.

Table 5.2.2 Calibrations and constants.

Name	Value (m)	Description
NAUR (Ellipsoidal ht)	46.2425	Observed RL at the ARP of NAUR (Ellipsoidal) @ 2019.82
NAUR - NAURBM	-0.9484	Offset constant between GNSS Monument (Pillar plate) & GNSSBM
Primary Pole & 1/2m Pole	1.00048	Height difference between poles used (Calibrated September 2019)

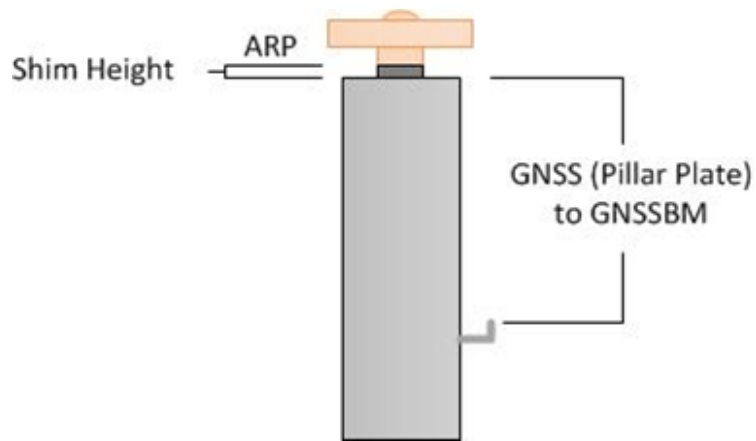


Figure 5.2.1 GNSS Pillar Offsets

5.3 Data Analysis and Results

Reduction of the digital data was computed using the Geoscience Australia levelling program “leveling1.exe” and LevellingFIELD_3.pl. This program computes the height difference between the two reflectors by taking the mean average of the measured height differences and providing standard deviations and a misclose for the input levelling loop.

The Reduced Level (RL) shown in Table 5.3.1 below is the height relative to NAURBM (GNSS BM)

Table 5.3.1 Reduced level data – NAUR (GNSS CORS) to NAU14A (Tide Gauge Benchmark)

From	To	Rise(m)	Fall (m)	RL (m)	Dist (km)	Acc Dist (km)
NAUR				0.9484		
NAURBM	NAURBM	0.0000	-0.9484	0.0000	0.000	0.000
53	53	0.2250	0.0000	0.2250	0.088	0.088
104	104	0.0000	-0.2889	-0.0639	0.192	0.280
103	103	0.4290	0.0000	0.3651	0.199	0.479
110	110	0.6543	0.0000	1.0194	0.202	0.681
101	101	0.0369	0.0000	1.0563	0.197	0.878
47	47	0.0000	-0.4721	0.5842	0.200	1.078
109	109	0.0000	-0.1319	0.4523	0.198	1.276
44	44	0.7781	0.0000	1.2304	0.200	1.476
43	43	0.0000	-0.5694	0.6610	0.204	1.680
108	108	0.9443	0.0000	1.6053	0.177	1.856
NAU33	NAU33	0.5450	0.0000	2.1503	0.179	2.035
107	107	0.3701	0.0000	2.5204	0.131	2.166
NAU38	NAU38	0.0000	-0.2074	2.3130	0.200	2.365
NAU37	NAU37	0.1266	0.0000	2.4396	0.056	2.422
106	106	0.3469	0.0000	2.7865	0.118	2.540
100	100	0.1638	0.0000	2.9503	0.200	2.740
NAU2	NAU2	0.0000	-0.4249	2.5254	0.167	2.907
28	28	0.0145	0.0000	2.5399	0.166	3.073
NAU36	NAU36	0.0000	-0.5810	1.9589	0.199	3.272
NAU1	NAU1	0.0737	0.0000	2.0326	0.185	3.457
NAU14A	NAU14A	0.0000	-3.3695	-1.3369	0.100	3.557
NAU1	NAU1	3.3696	0.0000	2.0327	0.101	
NAU36	NAU36	0.0000	-0.0740	1.9587	0.185	
28	28	0.5808	0.0000	2.5394	0.199	
NAU2	NAU2	0.0000	-0.0147	2.5247	0.167	
100	100	0.4250	0.0000	2.9497	0.167	
106	106	0.0000	-0.1642	2.7855	0.200	

NAU37	NAU37	0.0000	-0.3471	2.4384	0.118	
NAU38	NAU38	0.0000	-0.1268	2.3116	0.056	
107	107	0.2073	0.0000	2.5189	0.200	
NAU33	NAU33	0.0000	-0.3701	2.1488	0.131	
108	108	0.0000	-0.5453	1.6035	0.178	
43	43	0.0000	-0.9445	0.6590	0.177	
44	44	0.5695	0.0000	1.2286	0.204	
109	109	0.0000	-0.7784	0.4502	0.200	
47	47	0.1318	0.0000	0.5820	0.196	
101	101	0.4721	0.0000	1.0541	0.200	
110	110	0.0000	-0.0372	1.0169	0.197	
103	103	0.0000	-0.6546	0.3624	0.202	
104	104	0.0000	-0.4288	-0.0664	0.199	
53	53	0.2887	0.0000	0.2223	0.192	
NAURBM	NAURBM	0.0000	-0.2251	-0.0027	0.086	
NAUR	NAUR	0.9484	0.0000	0.9457	0.000	
	Sum:	11.7013	-11.7040			
	Misclose:		-0.0027	-0.0027	7.112	(Total Dist)
			<u>ALLOWABLE</u> <u>(m):</u>	0.0038	<u>2 x Sqrt (km)</u> <u>test:</u>	<u>PASS</u>

Table 5.3.2 Reduced level data – NAU14A (Tide Gauge Benchmark) to NAU15 (SEAFRAME sensor benchmark)

From	To	Rise(m)	Fall(m)	RL(m)	Dist (Km)	Acc Dist (km)
NAU14A				-1.3369	0.000	3.557
NAU15	NAU15	2.0864	0.0000	0.7494	0.010	3.567
NAU14A	NAU14A	0.0000	-2.0863	-1.3369	0.010	
	Sum:	2.0864	-2.0863			
	Misclose:		0.0000	0.0000	0.021	(Total Dist)
			<u>ALLOWABLE</u> <u>(m):</u>	0.0002	<u>2 x Sqrt (km)</u> <u>test:</u>	<u>PASS</u>

Table 5.3.3 Reduced level data – NAU33 to NAU16

From	To	Rise(m)	Fall(m)	RL(m)	Dist (Km)	Acc Dist (km)
NAU33				2.1496		2.035
NAU16	NAU16	0.0000	-0.7157	1.4338	0.131	2.166
	NAU33	0.7157	0.0000	2.1496	0.131	0.000
	Sum:	0.7157	-0.7157			
	Misclose:		0.0000	0.0000	0.262	(Total Dist)
			<u>ALLOWABLE</u> <u>(m):</u>	0.0007	<u>2 x Sqrt (km)</u> <u>test:</u>	<u>PASS</u>

Table 5.3.4 Measured height differences (in metres) between all BMs (ΔR_{L2019}).

	NAURBM	NAU16	NAU38	NAU37	NAU2	NAU36	NAU1	NAU15	RM1	RM2	RM3	NAUR	NAU14A
NAURBM	-	1.4338	2.3123	2.4390	2.5250	1.9588	2.0326	0.7494	-0.7125	-0.8749	-0.7771	0.9484	-1.3369
NAU16	-1.4338	-	0.8785	1.0052	1.0912	0.5250	0.5988	-0.6844	-2.1463	-2.3088	-2.2109	-0.4854	-2.7708
NAU38	-2.3123	-0.8785	-	0.1267	0.2127	-0.3535	-0.2797	-1.5629	-3.0248	-3.1873	-3.0894	-1.3639	-3.6492
NAU37	-2.4390	-1.0052	-0.1267	-	0.0860	-0.4802	-0.4064	-1.6896	-3.1515	-3.3140	-3.2161	-1.4906	-3.7759
NAU2	-2.5250	-1.0912	-0.2127	-0.0860	-	-0.5663	-0.4924	-1.7756	-3.2375	-3.4000	-3.3021	-1.5766	-3.8620
NAU36	-1.9588	-0.5250	0.3535	0.4802	0.5663	-	0.0738	-1.2094	-2.6712	-2.8337	-2.7359	-1.0104	-3.2957
NAU1	-2.0326	-0.5988	0.2797	0.4064	0.4924	-0.0738	-	-1.2832	-2.7451	-2.9076	-2.8097	-1.0842	-3.3696
NAU15	-0.7494	0.6844	1.5629	1.6896	1.7756	1.2094	1.2832	-	-1.4619	-1.6244	-1.5265	0.1990	-2.0864
RM1	0.7125	2.1463	3.0248	3.1515	3.2375	2.6712	2.7451	1.4619	-	-0.1625	-0.0646	1.6609	-0.6245
RM2	0.8749	2.3088	3.1873	3.3140	3.4000	2.8337	2.9076	1.6244	0.1625	-	0.0979	1.8233	-0.4620
RM3	0.7771	2.2109	3.0894	3.2161	3.3021	2.7359	2.8097	1.5265	0.0646	-0.0979	-	1.7255	-0.5599
NAUR	-0.9484	0.4854	1.3639	1.4906	1.5766	1.0104	1.0842	-0.1990	-1.6609	-1.8233	-1.7255	-	-2.2853
NAU14A	1.3369	2.7708	3.6492	3.7759	3.8620	3.2957	3.3696	2.0864	0.6245	0.4620	0.5599	2.2853	-

Table 5.3.5 Time-series of Reduced Levels (with respect to NAURBM)

Year	NAURBM	NAU16	NAU38	NAU37	NAU2	NAU36	NAU1	NAU14	NAU15	RM1	RM2	RM3	NAUR	NAU14A
1993.5					2.5267		2.0360	-1.4111	0.7474					
1994.2		1.4345			2.5279		2.0360	-1.4108	0.7471					
1995.2		1.4353			2.5279		2.0360	-1.4111	0.7468					
1996.7		1.4353			2.5266		2.0360	-1.4101	0.7478					
1997.9		1.4350			2.5279		2.0360	-1.4108	0.7457					
1999.5		1.4346			2.5265		2.0360	-1.4108	0.7462					
2000.6		1.4350			2.5271		2.0360	-1.4105	0.7480					
2002.4		1.4350	2.3141	2.4407	2.5268	1.9621	2.0360	-1.4106	0.7476					
2003.9	0.0000	1.4345	2.3144	2.4409	2.5272	1.9621	2.0360	-1.4098	0.7476	-0.7124	-0.8750	-0.7769		
2005.8	0.0000	1.4364	2.3152	2.4417	2.5276	1.9623	2.0360	-1.4092	0.7494					
2005.8	0.0000	1.4326	2.3116	2.4381	2.5238	1.9579	2.0317	-1.4137	0.7457	-0.7124	-0.8750	-0.7770	0.9484	
2007.5	0.0000	1.4319	2.3106	2.4374	2.5233	1.9567	2.0305	-1.4146	0.7445	-0.7126	-0.8750	-0.7771	0.9483	
2009.1	0.0000	1.4331	2.3121	2.4388	2.5243	1.9582	2.0326	-1.4125	0.7469	-0.7123	-0.8750	-0.7770	0.9485	
2010.5	0.0000	1.4313	2.3097	2.4366	2.5212	1.9548	2.0283	-1.4175	0.7414	-0.7124	-0.8750	-0.7770	0.9486	
2012.2	0.0000	1.4355	2.3142	2.4408	2.5266	1.9610	2.0350	-1.4099	0.7472	-0.7124	-0.8749	-0.7770		
2013.7	0.0000	1.4346	2.3133	2.4402	2.5258	1.9596	2.0337	-1.4116	0.7439	-0.7117	-0.8751	-0.7769	0.9484	
2015.3	0.000	1.4352	2.3142	2.4409	2.5262	1.9600	2.0339	-1.4114	0.7483	-0.7122	-0.8749	-0.7771	0.9484	
2016.6	0.000	1.4338	2.3127	2.4394	2.5261	1.9603	2.0341	-1.4111	0.7470	-0.7124	-0.8750	-0.7770	0.9484	
2018.3	0.000	1.4349	2.3143	2.4411	2.5274	1.9617	2.0354	-1.4090	0.7530	-0.7125	-0.8752	-0.7774	0.9484	
2019.8	0.000	1.4338	2.3123	2.4390	2.5250	1.9588	2.0326	#N/A	0.7494	-0.7125	-0.8749	-0.7771	0.9484	-1.3369

5.4 Comparison with previous surveys

All historic data has been readjusted relative to the benchmark attached to the base of the GNSS pillar (NAURBM) (Table 5.3.5). To investigate whether BMs have moved over time, the RLs from the 2019 survey (RL2019) have been compared to a reference height (RH) defined as the average of all previously calculated RLs. In cases where a site has undergone known movement (e.g., BM removed and reinstalled), the RH is the latest measured RL.

5.4.1 Difference in Reference Height values

Table 5.4.1.1 $\Delta RL_{REF} - \Delta RL_{2019}$ values (in metres). Shows the difference in height between two marks from the current survey compared to the reference height difference.

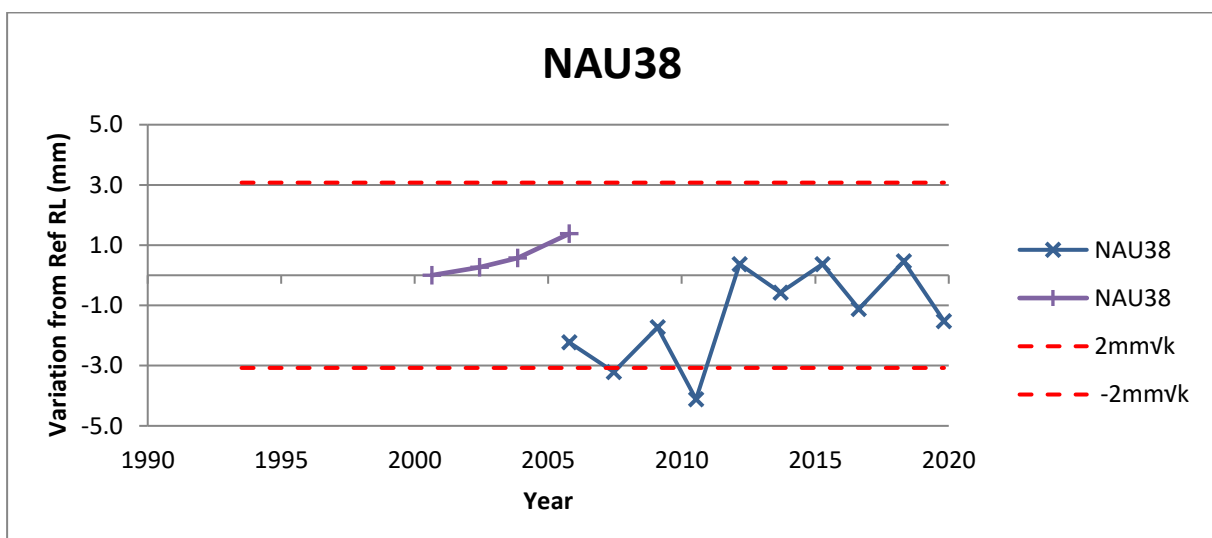
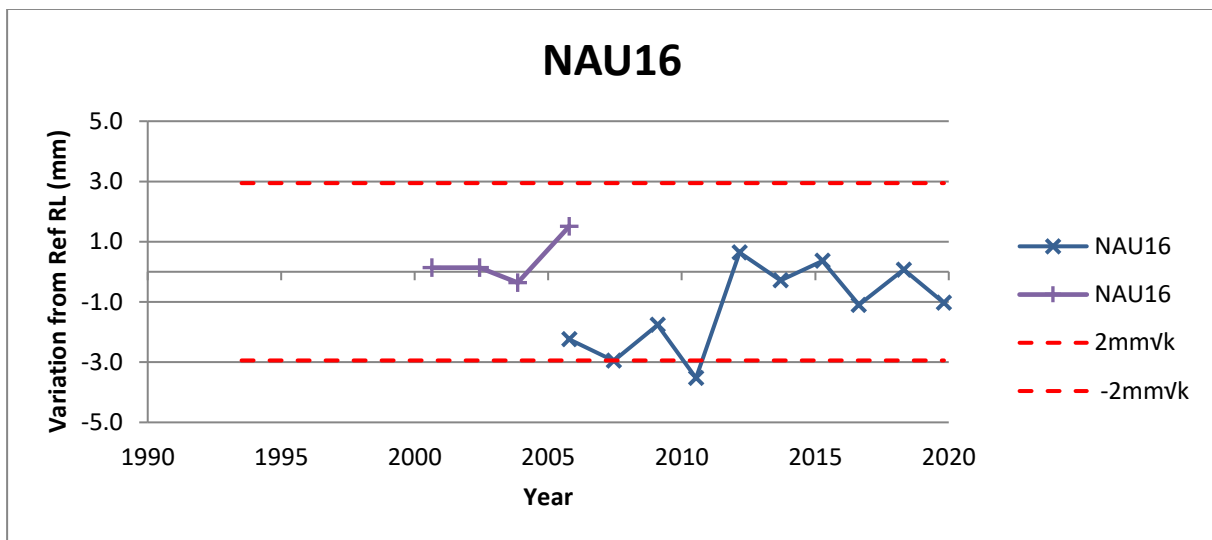
REF - 2019	NAURBM	NAU16	NAU38	NAU37	NAU2	NAU36	NAU1	NAU15	RM1	RM2	RM3	NAUR
NAURBM	-	0.0010	0.0015	0.0015	0.0017	0.0017	0.0032	-0.0023	0.0001	-0.0001	0.0001	0.0000
NAU16	-0.0010	-	0.0005	0.0005	0.0006	0.0007	0.0022	-0.0034	-0.0010	-0.0011	-0.0009	-0.0010
NAU38	-0.0015	-0.0005	-	0.0000	0.0001	0.0002	0.0017	-0.0039	-0.0015	-0.0016	-0.0014	-0.0015
NAU37	-0.0015	-0.0005	0.0000	-	0.0002	0.0002	0.0018	-0.0038	-0.0014	-0.0015	-0.0014	-0.0015
NAU2	-0.0017	-0.0006	-0.0001	-0.0002	-	0.0001	0.0016	-0.0040	-0.0016	-0.0017	-0.0016	-0.0017
NAU36	-0.0017	-0.0007	-0.0002	-0.0002	-0.0001	-	0.0015	-0.0040	-0.0016	-0.0018	-0.0016	-0.0017
NAU1	-0.0032	-0.0022	-0.0017	-0.0018	-0.0016	-0.0015	-	-0.0056	-0.0032	-0.0033	-0.0032	-0.0033
NAU15	0.0023	0.0034	0.0039	0.0038	0.0040	0.0040	0.0056	-	0.0024	0.0023	0.0024	0.0023
RM1	-0.0001	0.0010	0.0015	0.0014	0.0016	0.0016	0.0032	-0.0024	-	-0.0001	0.0000	-0.0001
RM2	0.0001	0.0011	0.0016	0.0015	0.0017	0.0018	0.0033	-0.0023	0.0001	-	0.0001	0.0001
RM3	-0.0001	0.0009	0.0014	0.0014	0.0016	0.0016	0.0032	-0.0024	0.0000	-0.0001	-	-0.0001
NAUR	0.0000	0.0010	0.0015	0.0015	0.0017	0.0017	0.0033	-0.0023	0.0001	-0.0001	0.0001	-

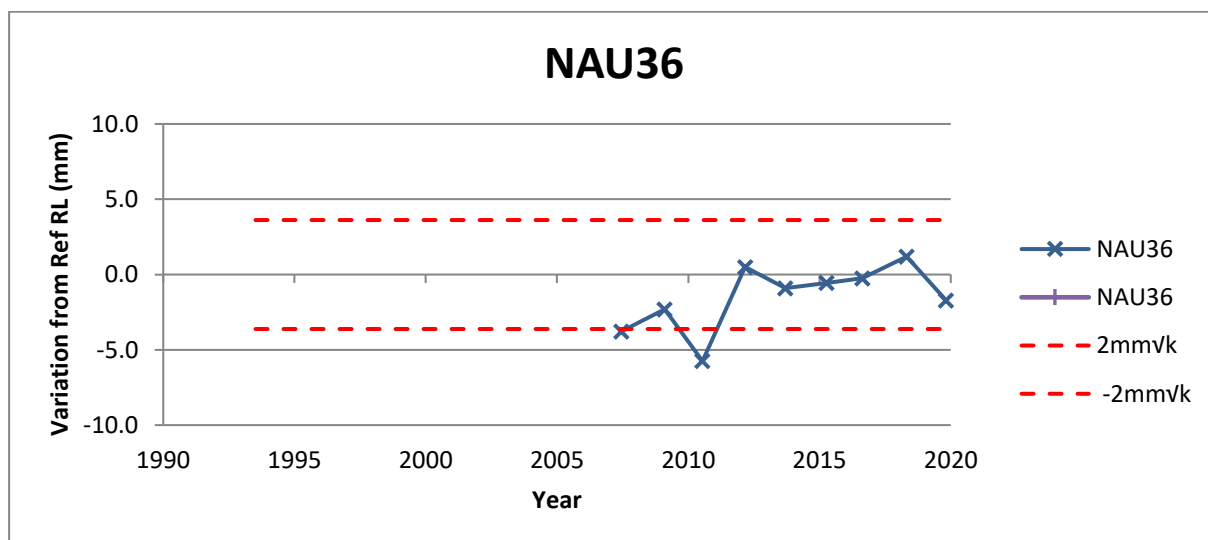
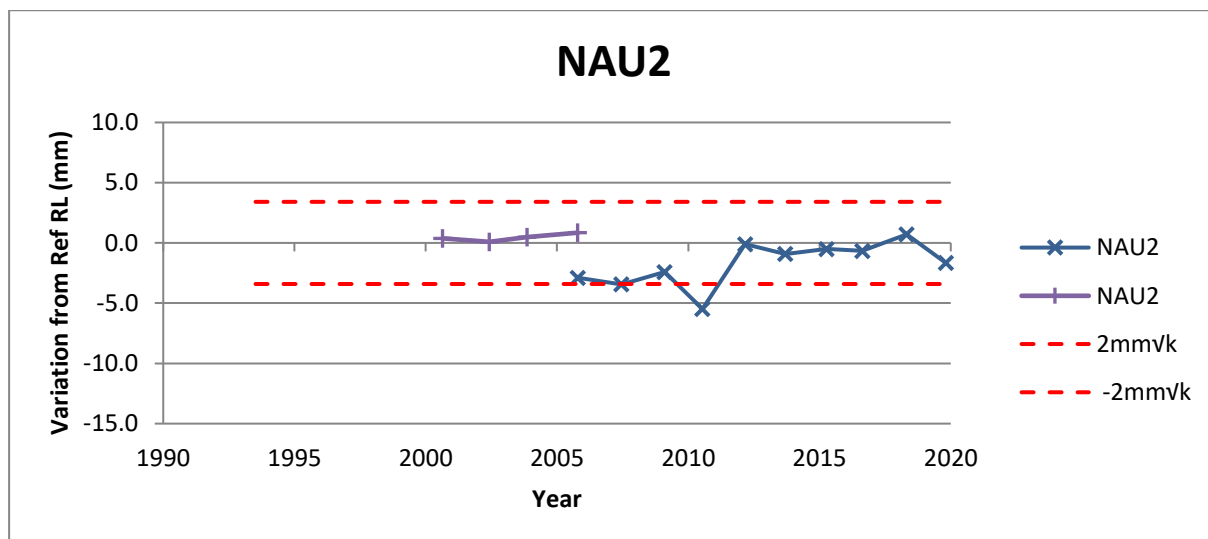
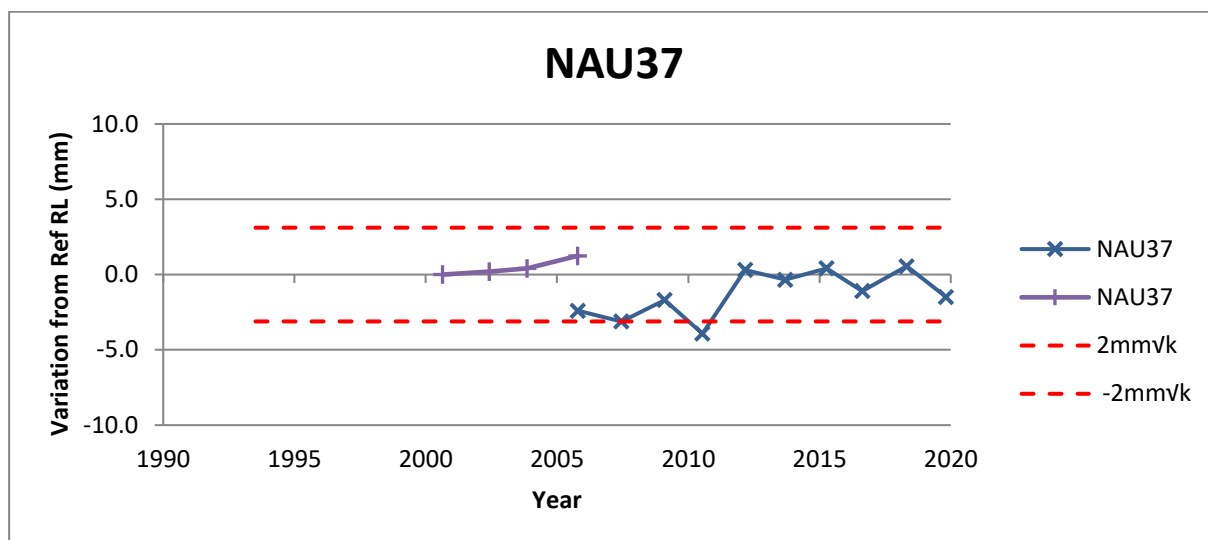
Table 5.4.1.1 values are calculated by subtracting the difference in height between RL_{2019} values (Table 5.3.5) from the difference in height between RL_{REF} values.

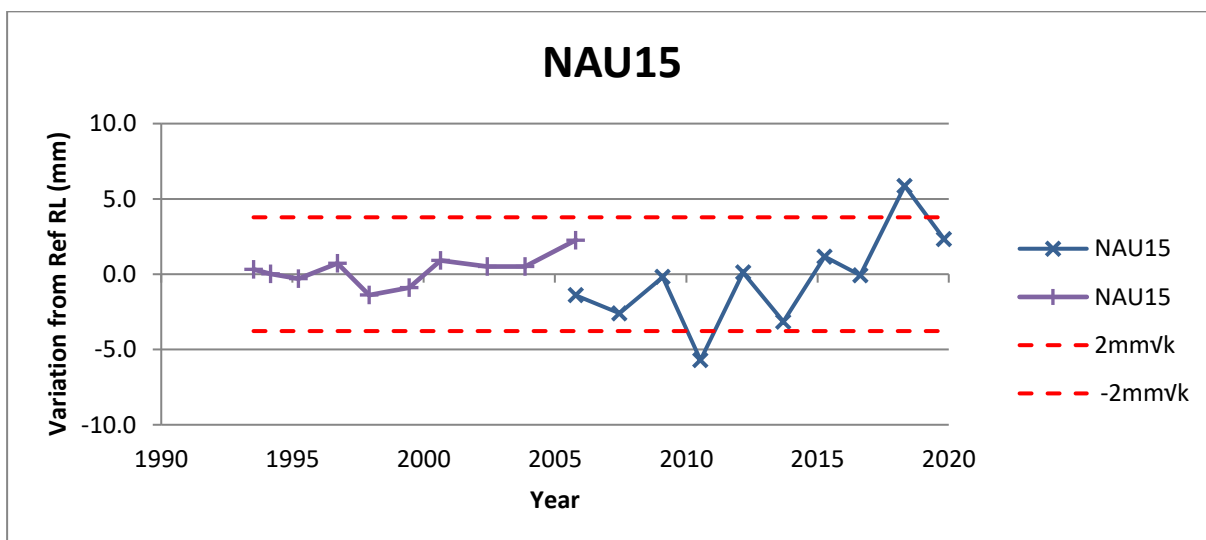
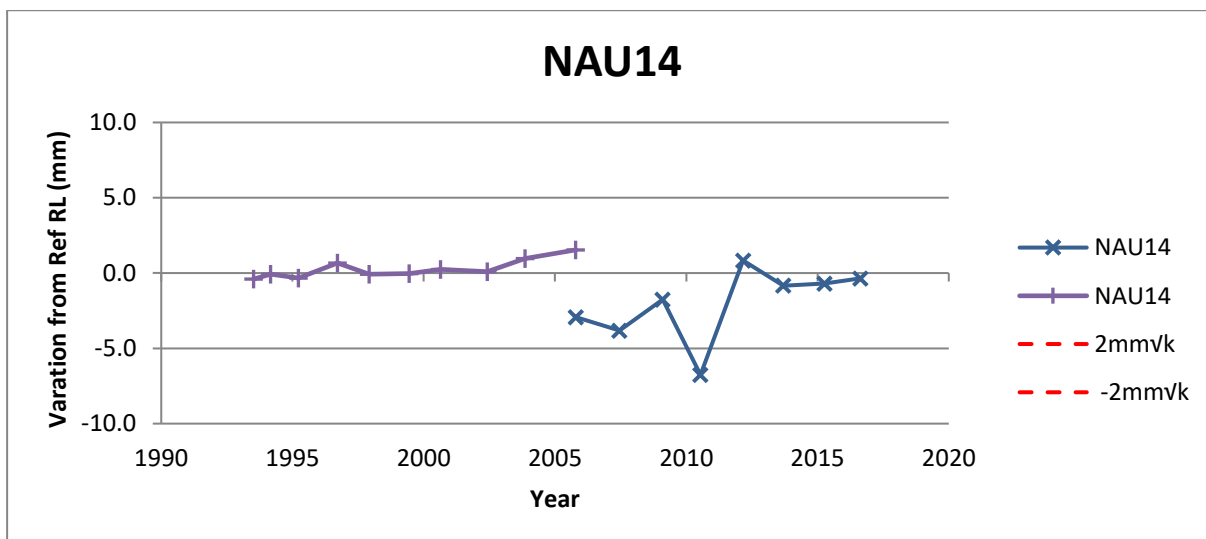
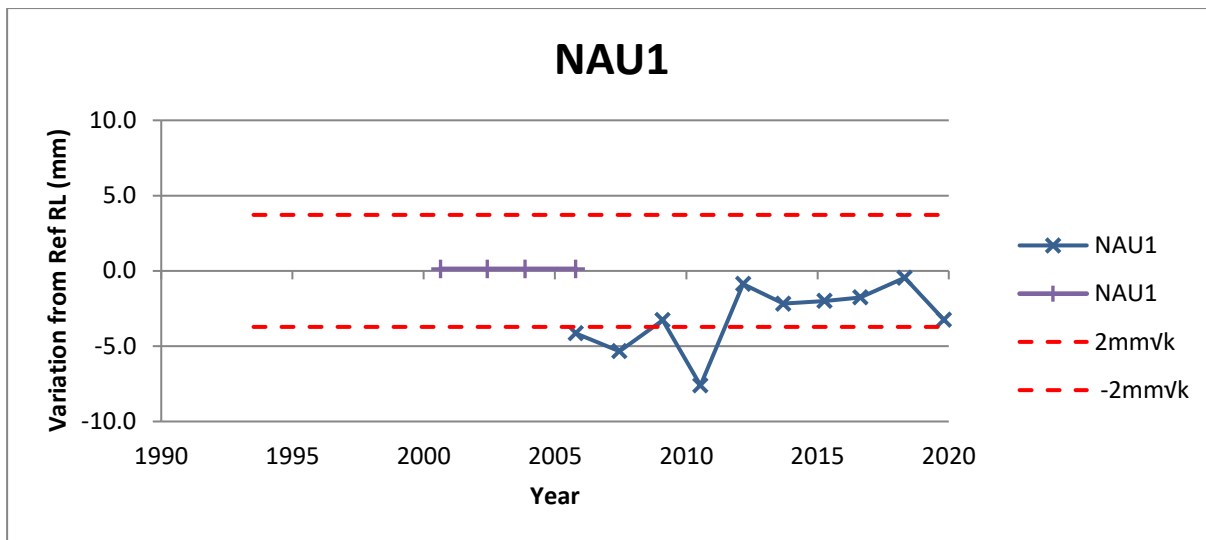
Comparing the change in relative heights between all benchmarks can help identify movement of a particular BM, inconsistency in survey, or even deformation between the GNSS pillar and levelling run BMs.

5.4.2 Time series charts for each BM

The change in RL over time can be used to detect trends of movement or survey errors. In the series of figures below, the red dashed lines represent the allowable error in height based on the levelling technique used and distance of the benchmark from the GNSS pillar ($2\text{mm}\sqrt{K}$). The purple line (with crosses) shows the results achieved using precise differential levelling [with a levelling instrument and survey staff] and the blue line with crosses show the levelling results based on the Total Station differential levelling technique.







6 Assessment of Results

After a full analysis of the monitoring and levelling survey results, the following conclusions can be drawn. There is one difference above 0.003 m:

- NAU15, which is likely due to:
 - localised movement where the tide gauge structure has been deteriorating over the years. There are developments of a new tide gauge facility underway to replace the current tide gauge station.

The survey from the primary BM (NAU1) to the Tide Gauge Station show that the tide gauge is stable, and no deformation has occurred since the previous survey.

The survey from the GNSS CORS pillar, NAURBM to the primary BM (NAU1) shows that the height difference between these two marks has remained consistent to previous years, and any variation is still within the expected tolerance of the survey.

Table 6.1 Comparison of results with Reference ΔH (m).

PT ID	Reference ΔH (m)	2019.82 Value (m)	Difference
NAURBM - Primary BM (NAU1)	2.0359	2.0326	-0.0033
NAU1 - TG Plaque BM (NAU14a)	-3.3728	-3.3696	-0.0033
NAU1 - TG ref pin (NAU15)	-1.2887	-1.2832	-0.0055
NAURBM - NAU1	2.0359	2.0326	-0.0033
NAU1 - NAU14a	-3.3728	-3.3696	-0.0033
NAU14a - NAU15	2.0841	2.0864	0.0022
NAUR - TG Plaque	-2.2853	-2.2853	0.0000
NAUR - TG BM	-0.2012	-0.1990	-0.0022
NAUR - TGZ	-6.2055	-6.2033	-0.0022

Table 6.2 List of height differences from NAURBM to primary benchmarks, and conversion to TGZ & ITRF2020.

PT ID	Reference RL (m)	2019.82 Value (m)	Difference	TGZ	ITRF2020
NAURBM	0.0000	0.0000	0.0000	5.2549	45.2941
NAU16	1.4350	1.4338	-0.0011	6.6887	46.7279
NAU38	2.3139	2.3123	-0.0016	7.5672	47.6064
NAU37	2.4406	2.4390	-0.0016	7.6939	47.7331
NAU2	2.5270	2.5250	-0.0019	7.7799	47.8191
NAU36	1.9600	1.9588	-0.0012	7.2137	47.2529
NAU1	2.0359	2.0326	-0.0033	7.2875	47.3267
NAU15	0.7472	0.7494	0.0022	6.0043	46.0435
RM1	-0.7124	-0.7125	-0.0001	4.5424	44.5817
RM2	-0.8750	-0.8749	0.0000	4.3799	44.4192
RM3	-0.7770	-0.7771	-0.0001	4.4778	44.5170
NAUR	0.9484	0.9484	0.0000	6.2033	46.2425
NAU14A	-1.3369	-1.3369	0.0000	3.9180	43.9572
TGZ	-5.2571	-5.2549	0.0022	0.0000	40.0392

7 Absolute height of the tide gauge

When combined, the GNSS and levelling data provide information about the absolute movement of the tide gauge. This information can be used by Bureau to translate relative sea level into absolute sea level.

7.1 GNSS time series analysis

The ellipsoidal height of the GNSS pillar is computed using Geoscience Australia's weekly cumulative GNSS solution and modelled using Chebyshev polynomials² (Figure 7.1). Uncertainty regions (95% confidence) were determined using the residuals with respect to the polynomial model. Large outliers (>50cm) were removed manually as they have a significant impact on the estimated uncertainties. The ellipsoidal heights are with respect to the International Terrestrial Reference Frame 2020 (ITRF2020).

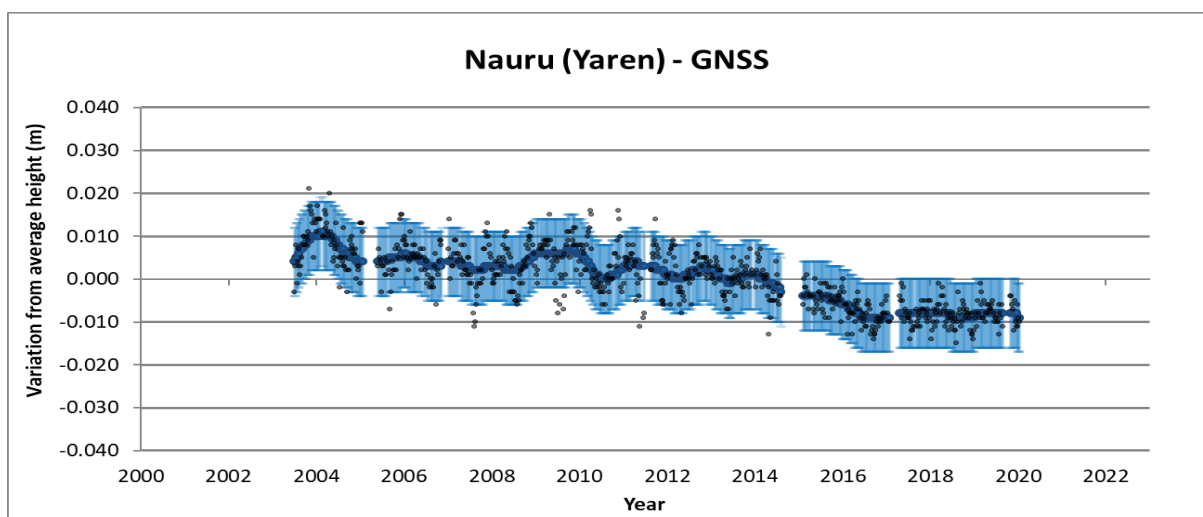


Figure 7.1 Time series of GNSS analysis (dark blue line) with 95%CI uncertainty (light blue lines).

² The order of the polynomial was determined iteratively by evaluating the significance of the improvement in model misfit Chi-Squared by an F test. The number of terms used in the preferred models ranged from 2 (i.e. linear) to 10 across the analysed time series and depends on the complexity of the observed signal.

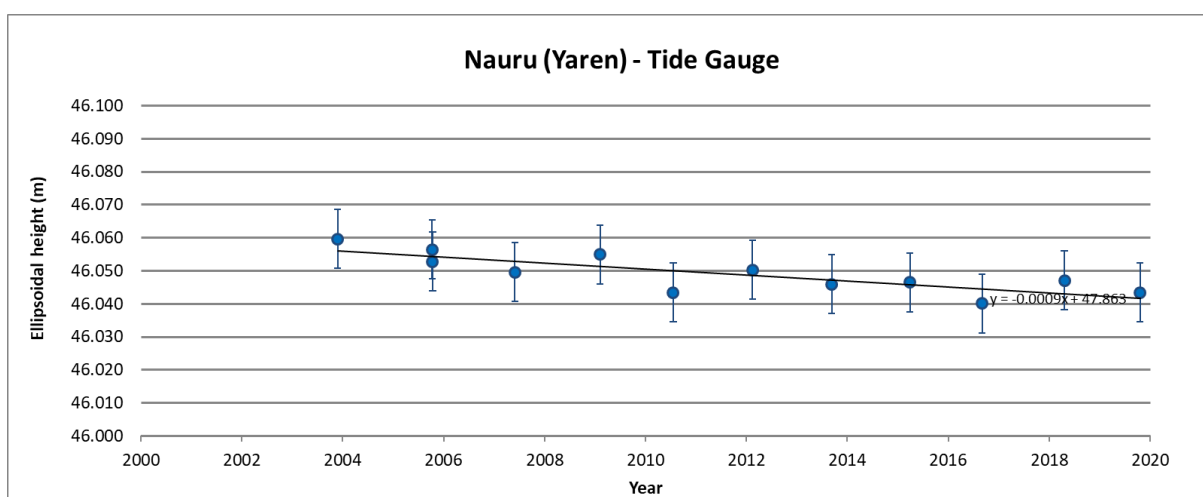


Figure 7.2 The height of the SEAFRAME sensor reference benchmark (with respect to ITRF2020).

The height of the SEAFRAME sensor reference benchmark is the sum of the ellipsoidal height of the GNSS pillar plate and the levelled height difference between the GNSS pillar plate and SEAFRAME sensor reference benchmark (Figure 7.2; Table 7.1)³. The height uncertainty is the combined uncertainty from the GNSS analysis and the levelling.

Table 7.1 Height of the SEAFRAME sensor reference benchmark (and 95%CI uncertainty) derived from GNSS time series analysis and levelling. Height is with respect to the International Terrestrial Reference Frame 2020.

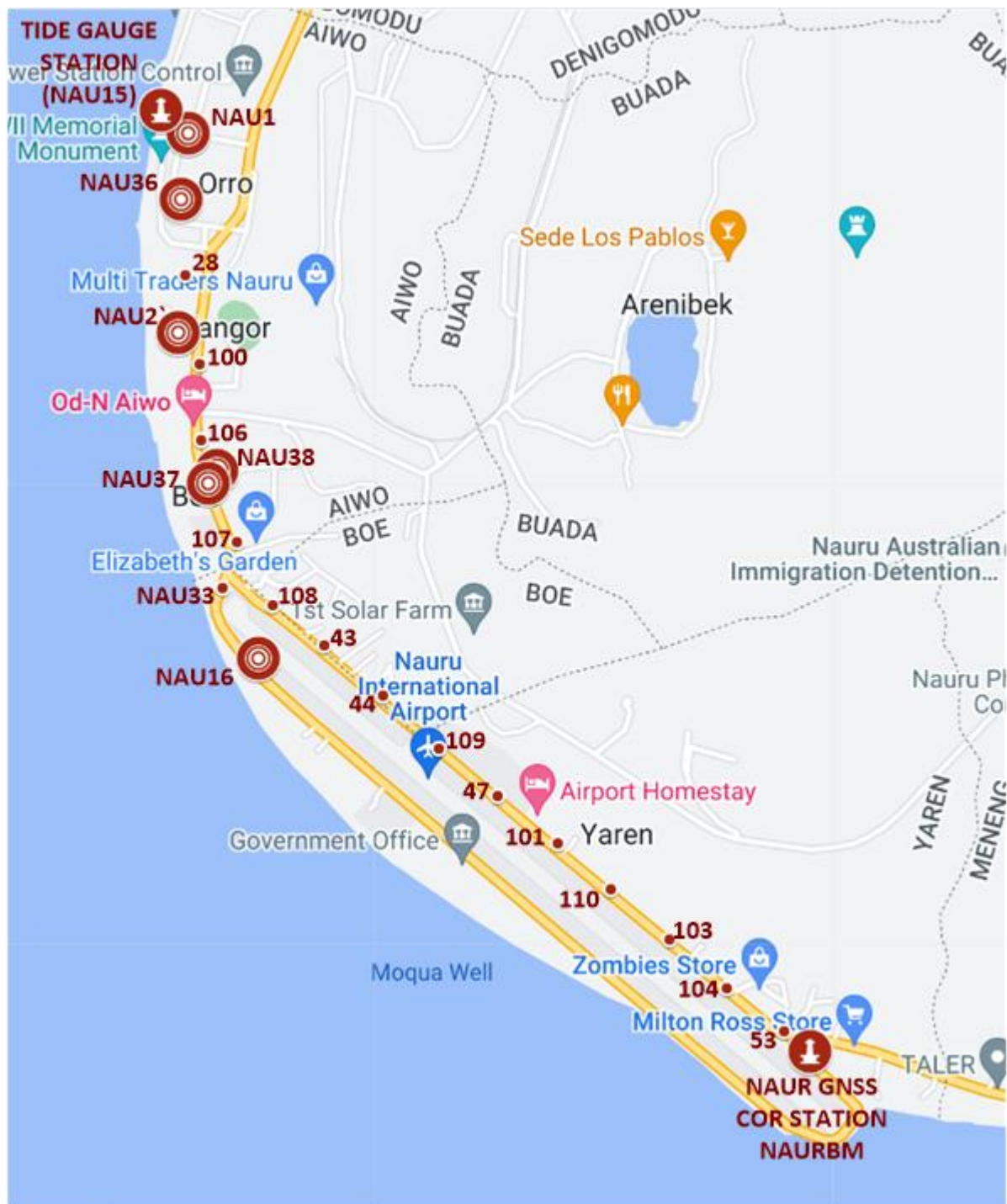
Date	Height (m)	Uncertainty (95%CI) (m)
2003.90	46.0597	0.0089
2005.77	46.0565	0.0089
2005.78	46.0528	0.0089
2007.41	46.0496	0.0089
2009.10	46.0550	0.0089
2010.55	46.0435	0.0089
2012.12	46.0503	0.0089
2013.69	46.0460	0.0089
2015.24	46.0465	0.0089
2016.67	46.0401	0.0089
2018.32	46.0471	0.0089
2019.82	46.0435	0.0089

³ It is recognised that the height of the SEAFRAME sensor reference benchmark is the sum of the geometric GNSS ellipsoidal height and the physical orthometric levelling height. No geoid corrections were applied to the levelling data because of the short distance of the levelling run and the lack of high resolution gravity data in this region.

8 References

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- Fraser, R., Leahy, F., Collier, P., 2018. *DynAdjust User's Guide Version 3.0*. Dynamic Network Adjustment Software.
- Intergovernmental Committee on Surveying and Mapping (ICSM) 2021, Guideline for Conventional Traverse Surveys – SP1 V2.2.
- Rüeger, J.M. & Brunner, F.K. 1982, '*EDM Height Traversing versus Geodetic Levelling*', The Canadian Surveyor, vol. 36, no. 1, pp. 69-87.
- Rueger, J. M., Brunner, F. K., 1981. *Practical Results from EDM-Height Traversing*. The Australian Surveyor. June 1981, Vol. 30, No 6.

Appendix A Locality Diagrams



Source: Adopted from Google Map

A 1 Deep Benchmarks



PACIFIC SEA LEVEL MONITORING PROJECT



Australian Government
Geoscience Australia

SURVEY BENCH MARK RECORD



SPC
Secretariat
of the Pacific
Community

Bench Mark Number: NAU1

Original Bench Mark Established by:
National Tide Centre Australia, Oceanographic Services,
Bureau of Meteorology, 25 college Rd, Kent Town, SA.

Date: 11/02/1992

Existing Bench Mark Established by:

Date:

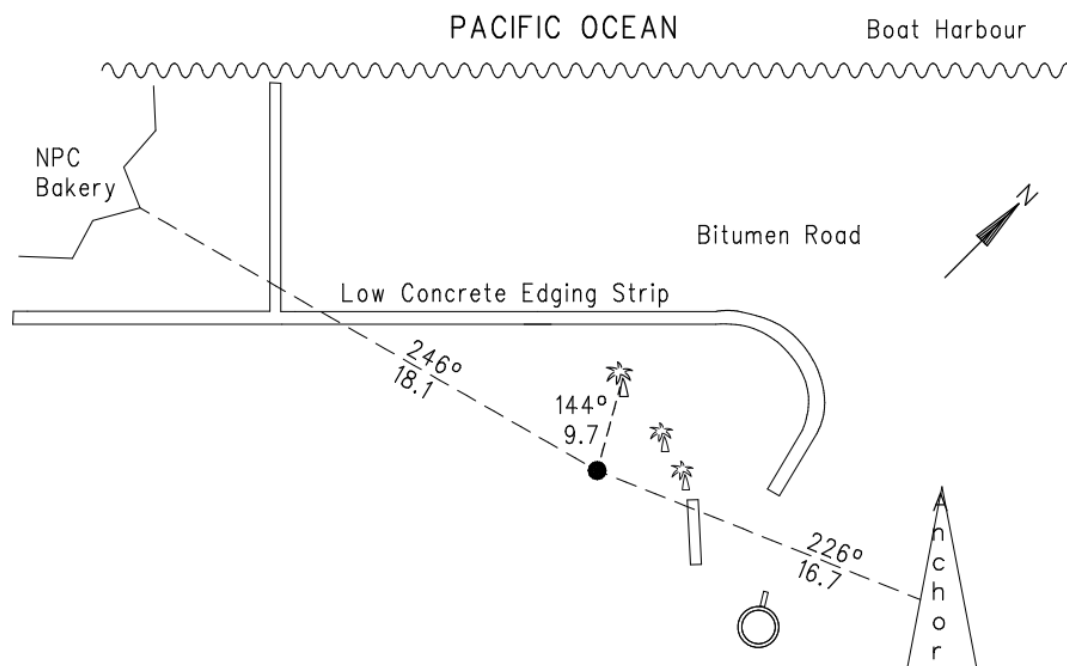
Notes / References: Deep Survey Benchmark
This survey mark is in a good locality for GNSS occupation.

Country: Nauru
Island: Nauru

City: Nauru

MARKING AND LOCALITY SKETCH

Bench Mark: 4.3m of 19mm diameter stainless steel capped rod driven to refusal. Rod sheathed with 50mm diameter PVC pipe, filled with bentonite, for 1.5m. Top of mark 0.2m below ground level. Locality sketch Mark approximately 100m from the tide gauge station.



NOT TO SCALE

Distances in Metres

Magnetic Bearings

Approved by: Geoscience Australia / SPC

Date: 2/9/2014

SURVEY BENCH MARK RECORD

Bench Mark Number: NAU2

Original Bench Mark Established by:
National Tidal Centre Australia, Oceanographic Services,
Bureau of Meteorology, 25 College Rd, Kent Town SA.

Date: 11/02/1992

Existing Bench Mark Established by:

Date:

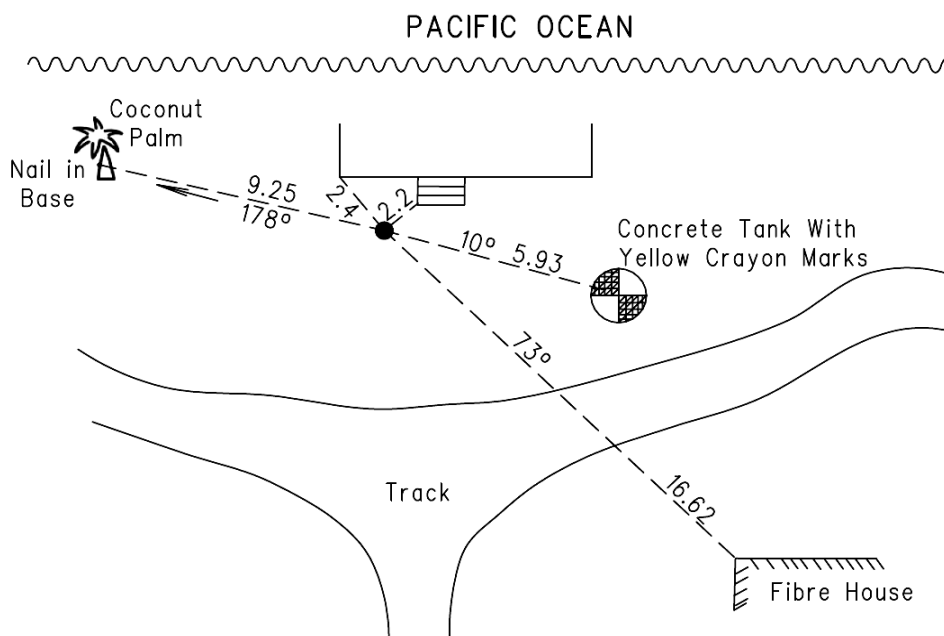
Notes / References: Deep Survey Benchmark
This survey mark is in a good locality for GPS occupation.

Country: Nauru
Island: Nauru

City: Nauru

MARKING AND LOCALITY SKETCH

Bench Mark: 4.8m of 19mm diameter stainless steel capped rod driven to refusal. Rod sheathed with 50mm diameter PVC pipe, filled with bentonite, for 1.8m. Top of mark 0.1m below ground level. Locality sketch Mark approximately 600m from the tide gauge station.



NOT TO SCALE

Distances in Metres

Magnetic Bearings

Approved by: Geoscience Australia / SPC

Date: November 2006

SURVEY BENCH MARK RECORD



Bench Mark Number: NAU16

Original Bench Mark Established by:

Date: 25/08/1993

National Tidal Centre Australia, Oceanographic Services,
Bureau of Meteorology, 25 College Rd, Kent Town SA.

Existing Bench Mark Established by:

Date:

Notes / References: Deep driven Benchmark

This survey mark is not in a good locality for GPS occupation

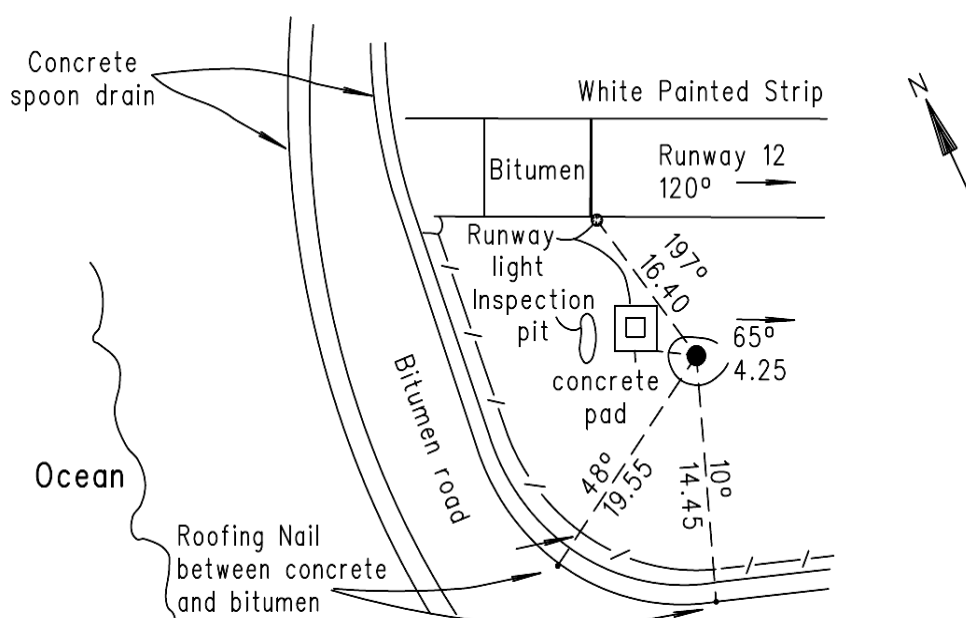
Country: Nauru

Island: Nauru

City: Nauru

MARKING AND LOCALITY SKETCH

Bench Mark: 4.3m of 19mm diameter stainless steel capped rod driven to refusal. Rod sheathed with 50mm diameter PVC pipe, filled with bentonite, for 0.5m. Top of mark 0.1m below ground level. Locality sketch Mark approximately 1500m from the tide gauge station.



NOT TO SCALE

Distances in Metres

Magnetic Bearings

Approved by: Geoscience Australia / SPC

Date: November 2006

SURVEY BENCH MARK RECORD



Bench Mark Number: NAU36

Original Bench Mark Established by:

Date: 03/06/2002

National Tidal Centre Australia, Oceanographic Services,
Bureau of Meteorology, 25 College Rd, Kent Town SA.

Existing Bench Mark Established by:

Date:

Notes / References: Deep Survey Benchmark

This survey mark is not in a good locality for GPS occupation.

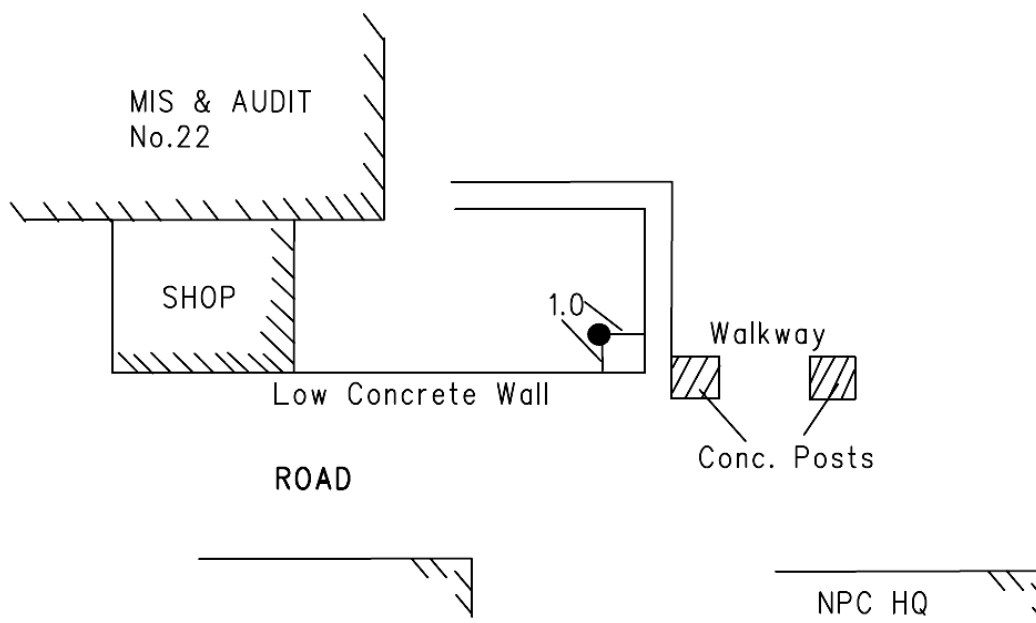
Country: Nauru

Island: Nauru

City: Nauru

MARKING AND LOCALITY SKETCH

Bench Mark: 4.2m of 19mm diameter stainless steel capped rod driven to refusal. Rod sheathed with 50mm diameter PVC pipe, filled with bentonite, for 0.5m. Top of mark 0.1m below ground level. Locality sketch Mark approximately 600m from the tide gauge station.



NOT TO SCALE

Distances in Metres

Magnetic Bearings

Approved by: Geoscience Australia / SPC

Date: November 2006

SURVEY BENCH MARK RECORD



Bench Mark Number: NAU37

Original Bench Mark Established by:

Date: 03/06/2002

National Tidal Centre Australia, Oceanographic Services,
Bureau of Meteorology, 25 College Rd, Kent Town SA.

Existing Bench Mark Established by:

Date:

Notes / References: Deep Survey Benchmark

This survey mark is not in a good locality for GPS occupation.

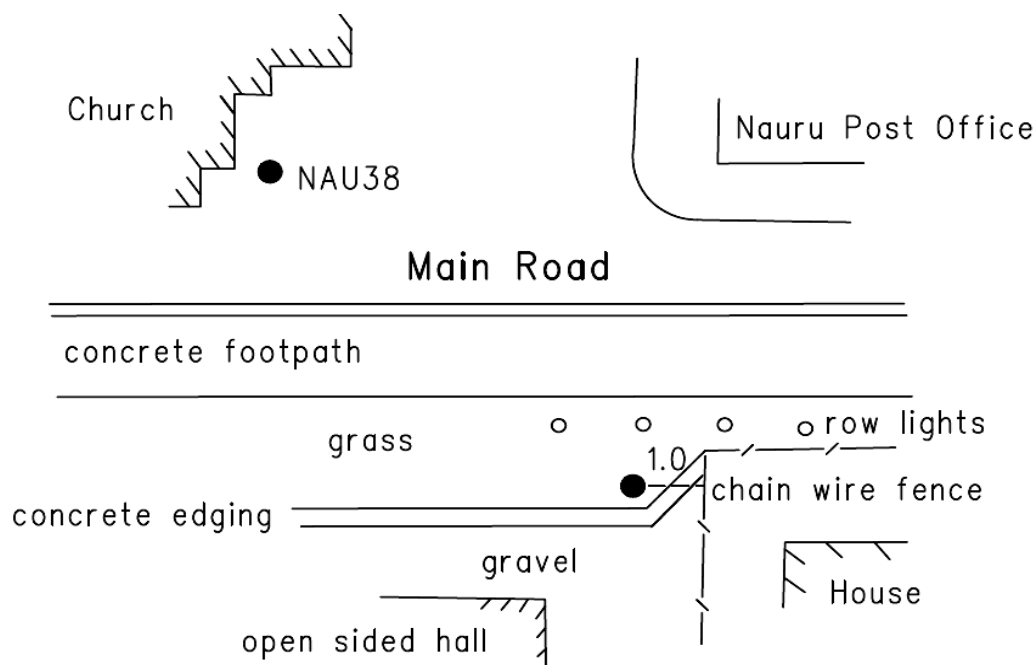
Country: Nauru

Island: Nauru

City: Nauru

MARKING AND LOCALITY SKETCH

Bench Mark: 3.3m of 19mm diameter stainless steel capped rod driven to refusal. Rod sheathed with 50mm diameter PVC pipe, filled with bentonite, for 0.5m. Top of mark 0.1m below ground level. Locality sketch Mark approximately 1200m from the tide gauge station.



NOT TO SCALE

Distances in Metres

Magnetic Bearings

Approved by: Geoscience Australia / SPC

Date: November 2006

SURVEY BENCH MARK RECORD



Bench Mark Number: NAU38

Original Bench Mark Established by:

Date: 03/06/2002

National Tidal Centre Australia, Oceanographic Services,
Bureau of Meteorology, 25 College Rd, Kent Town SA.

Existing Bench Mark Established by:

Date:

Notes / References: Deep Survey Benchmark

This survey mark is not in a good locality for GPS occupation.

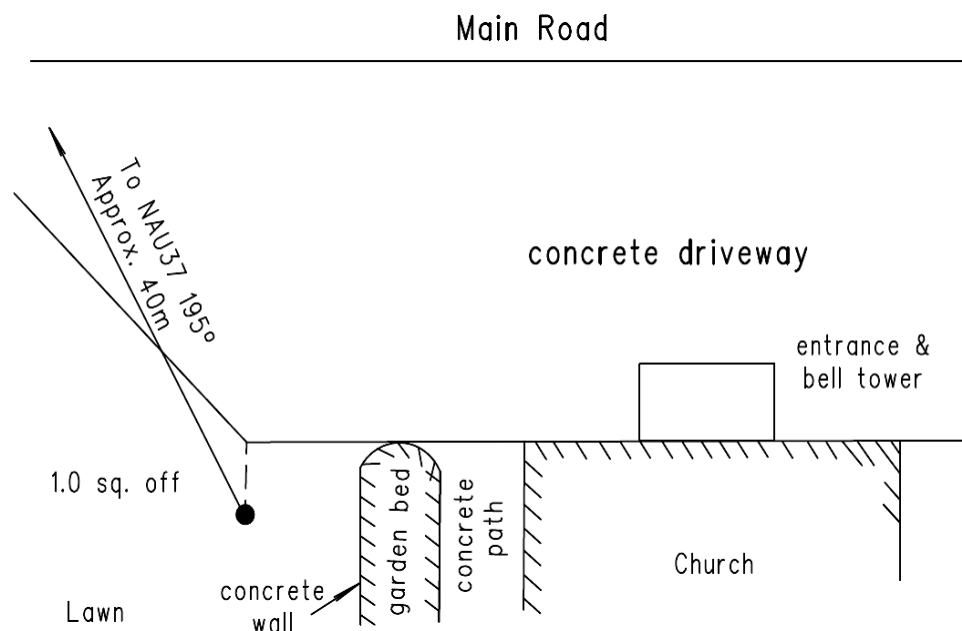
Country: Nauru

Island: Nauru

City: Nauru

MARKING AND LOCALITY SKETCH

Bench Mark: 3.3m of 19mm diameter stainless steel capped rod driven to refusal. Rod sheathed with 50mm diameter PVC pipe, filled with bentonite, for 0.5m. Top of mark 0.1m below ground level. Locality sketch Mark approximately 1200m from the tide gauge station.



NOT TO SCALE

Distances in Metres

Magnetic Bearings

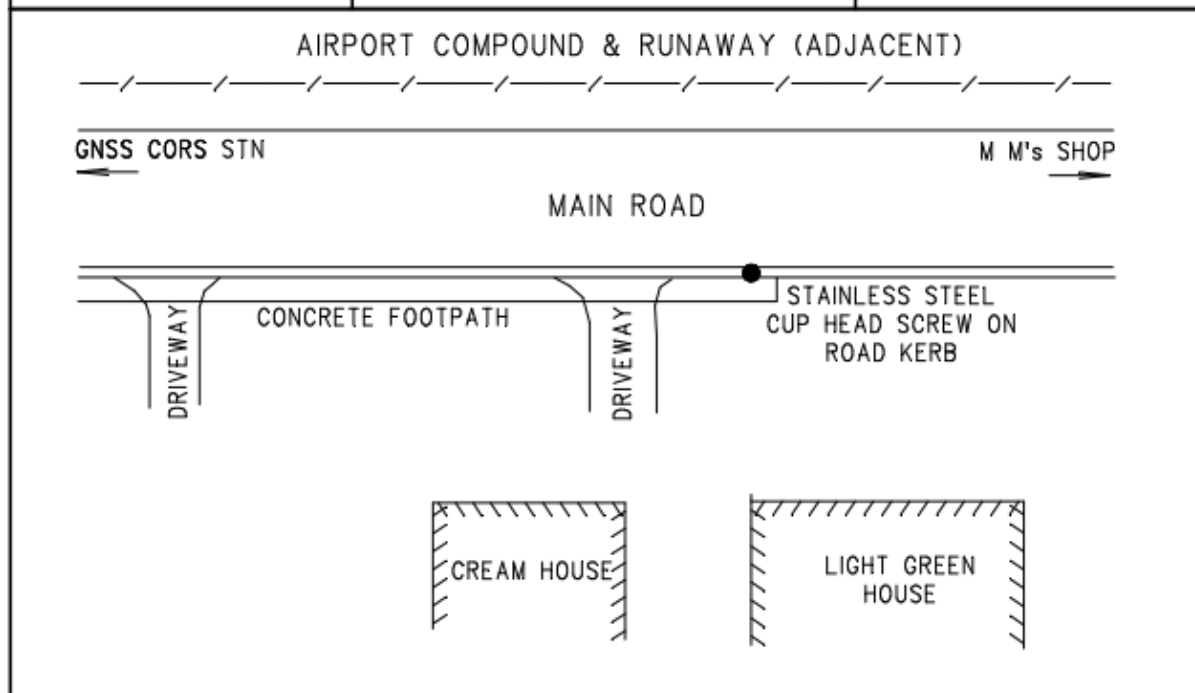
Approved by: Geoscience Australia / SPC

Date: November 2006

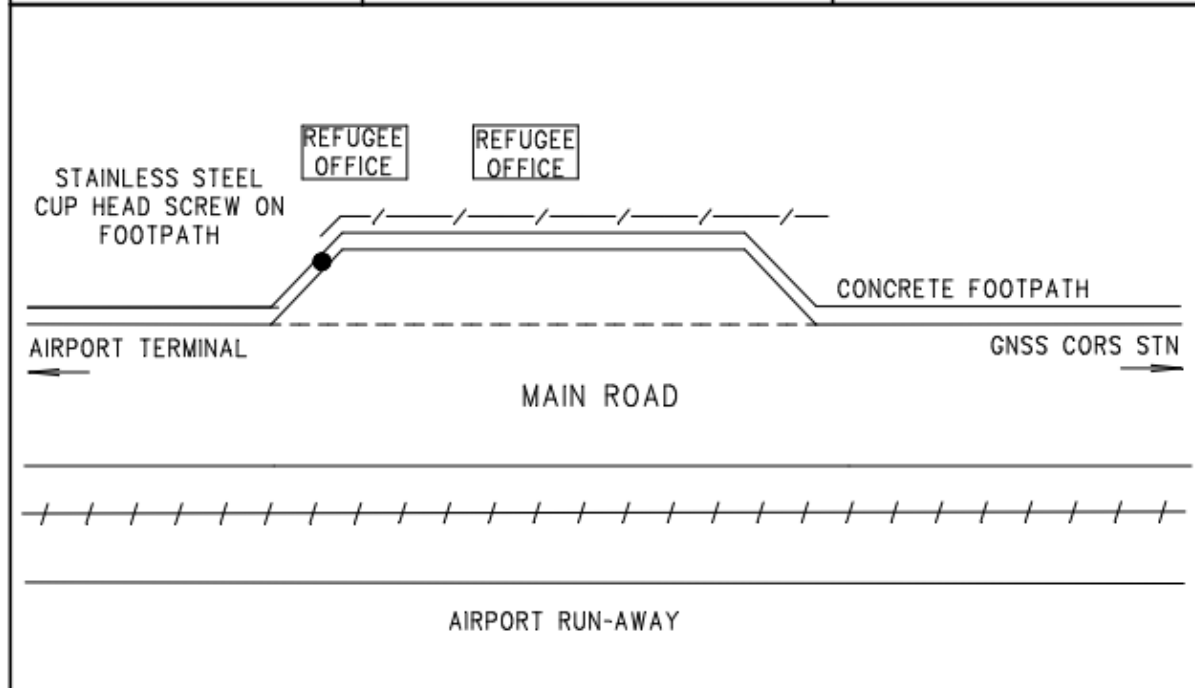
A 2 Temporary Holdings Marks

COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 53
PROJECT: PSLGM	SURVEYOR: S. Turner	DATE: 07-09-03
<p>The diagram for Point No. 53 shows an 'Airport' at the top with a 'chainwire fence' below it. A 'Bitumen Road' runs horizontally, with a black dot marking the point. To the left of the road is a 'two storey house' represented by a simple outline. To the right of the road is a 'basketball background' represented by a hatched rectangle. Further right is a 'green water tank' represented by a circle with a cross-hatch pattern.</p>		
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 104
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 04-06-07
<p>The diagram for Point No. 104 shows an 'AIRPORT COMPOUND & RUNAWAY (ADJACENT)' at the top with a dashed line below it. A 'GNSS CORS STN' is marked with a black dot on the left. A 'TO M M's SHOP' arrow points to the right. A 'MAIN ROAD' runs horizontally, with a 'CONCRETE FOOTPATH' below it. A 'STAINLESS STEEL CUP HEAD SCREW ON ROAD KERB' is marked with a black dot on the main road. An 'ACCESS ROAD' curves from the main road down to a 'VEHICLE JUNK YARD' represented by a rectangle.</p>		

COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 103
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 04-06-07



COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 110
PROJECT: PSLGM	SURVEYOR: AL, MK & VR	DATE: 24-08-16



COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 101
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 04-06-07
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 47
PROJECT: PSLGM	SURVEYOR: S. Turner	DATE: 10-09-12

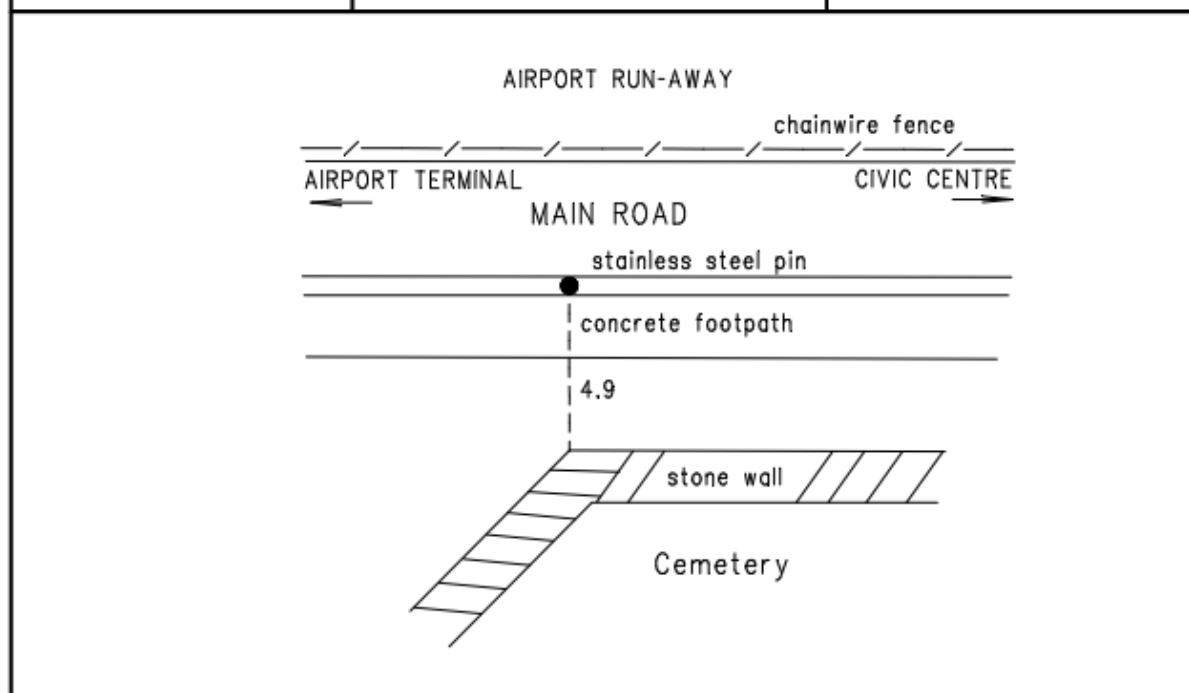
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 109
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 05-02-09

The diagram shows a site plan for Point 109. At the top, there is an 'AIRCRAFT PARKING BAY' and an 'AIRPORT TERMINAL' to its right. Below these is a 'MAIN ROAD' running horizontally. To the left of the road is the 'CIVIC CENTRE' and to the right is the 'GNSS CORS'. Below the road is the 'AIRPORT RUN-AWAY' area. Two entry points are marked: 'ENTRY 'A'' on the left and 'ENTRY 'B'' on the right. Between them is a 'STAINLESS STEEL CUP HEAD SCREW ON BASE OF RUNAWAY LIGHTS'. To the right of 'ENTRY 'B'' is a 'RED/WHITE BOX'.

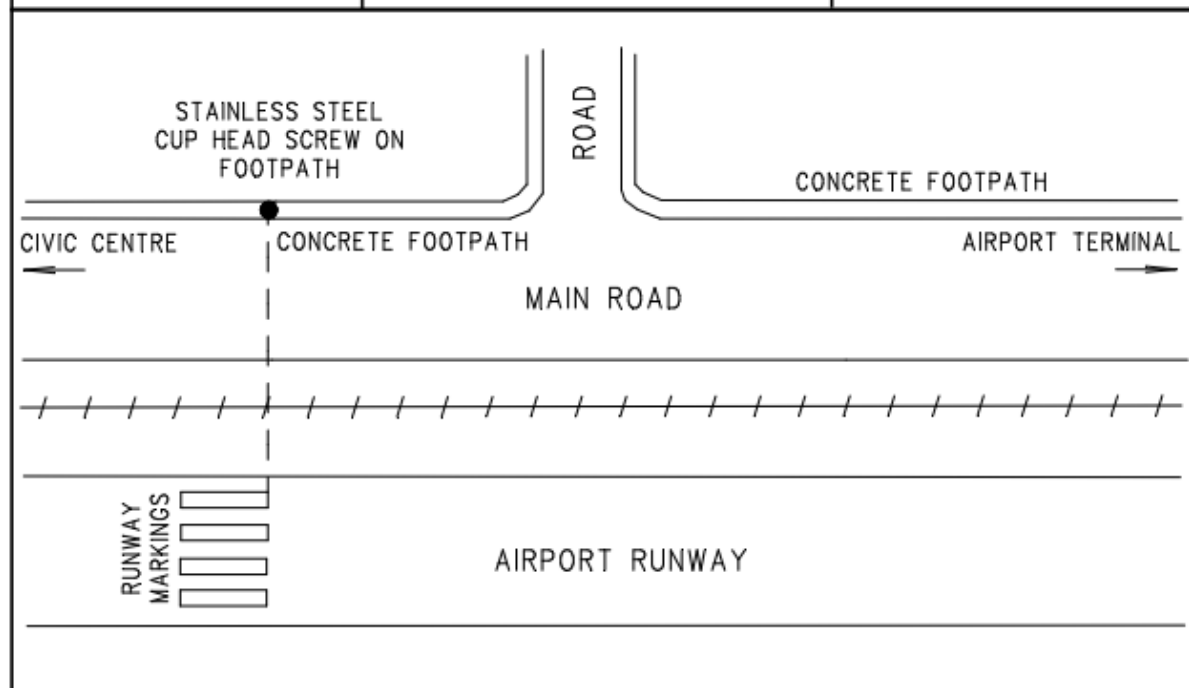
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 44
PROJECT: PSLGM	SURVEYOR: S. Turner	DATE: 07-09-03

The diagram shows a site plan for Point 44. At the top is the 'AIRPORT RUN-AWAY' area with a 'chainwire fence' indicated by a dashed line. Below the fence is a 'concrete kerb' and then a 'concrete footpath' shown with diagonal hatching. To the left of the footpath is the 'Community Hall'. To the right of the hall is a 'concrete retaining wall' shown with diagonal hatching. An 'iron post' is located on the retaining wall, with a 'stainless steel pin' attached to it.

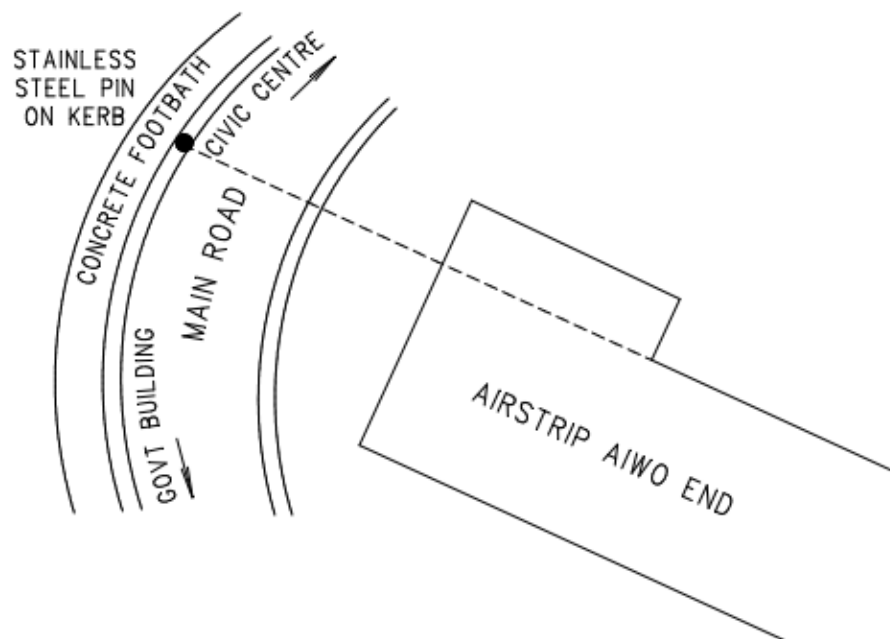
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 43
PROJECT: PSLGM	SURVEYOR: S Turner	DATE: 03-06-02



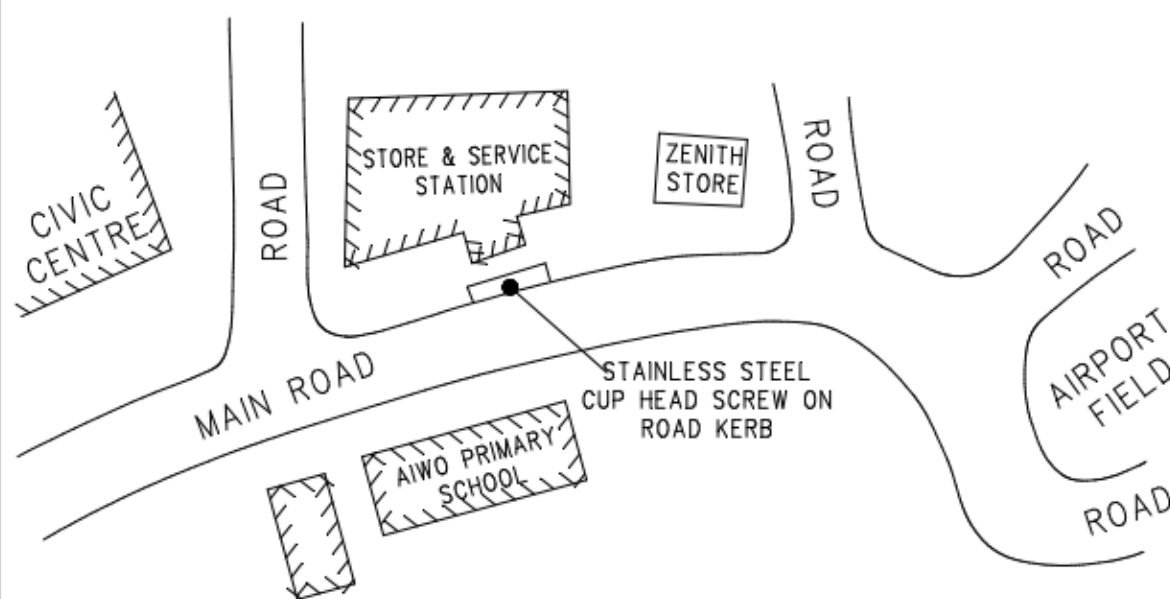
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 108
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 05-02-09



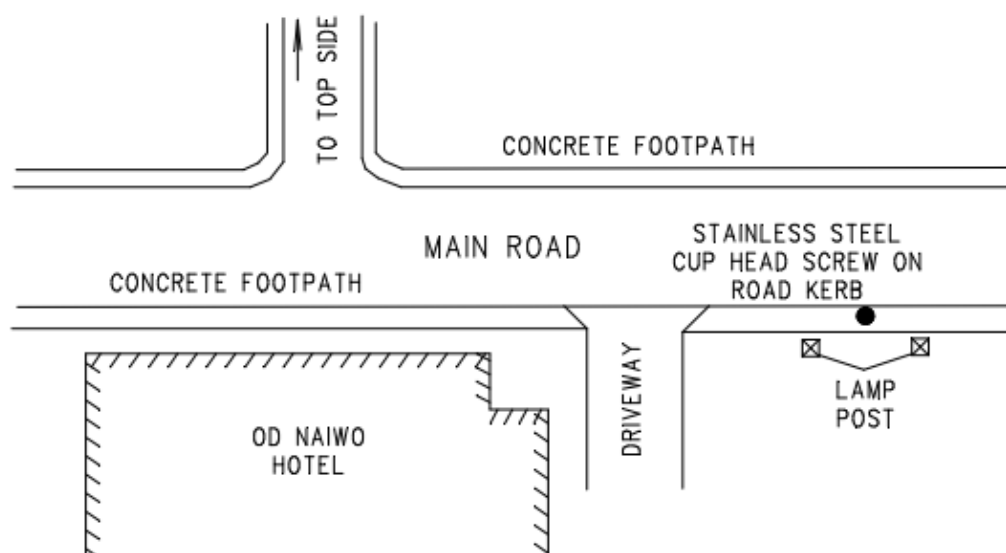
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. NAU33
PROJECT: PSLGM	SURVEYOR: S. Turner	DATE: 31-05-02



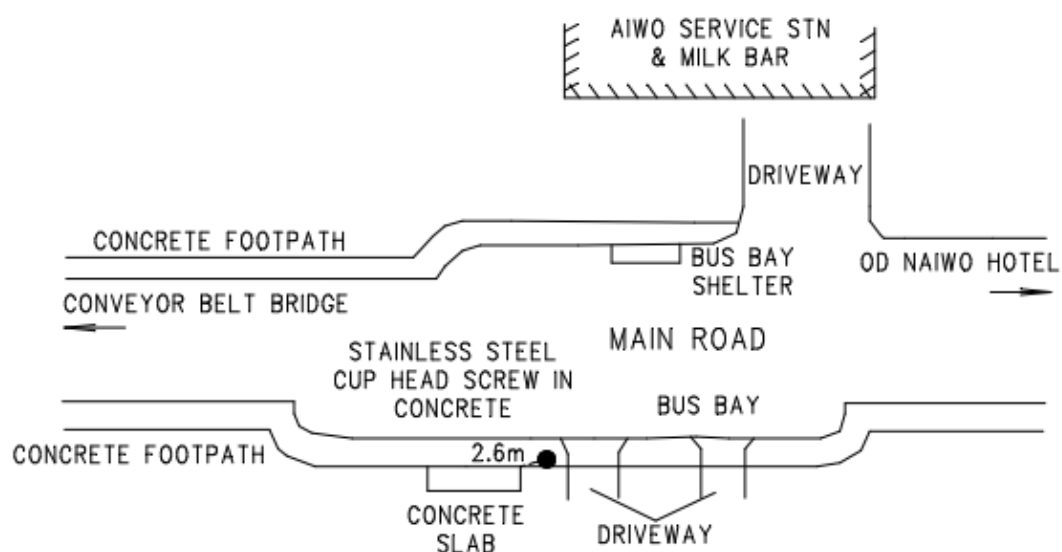
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 107
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 05-02-09



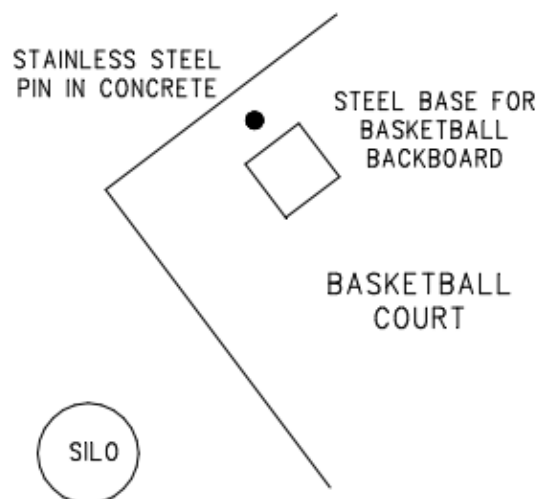
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 106
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 05-02-09



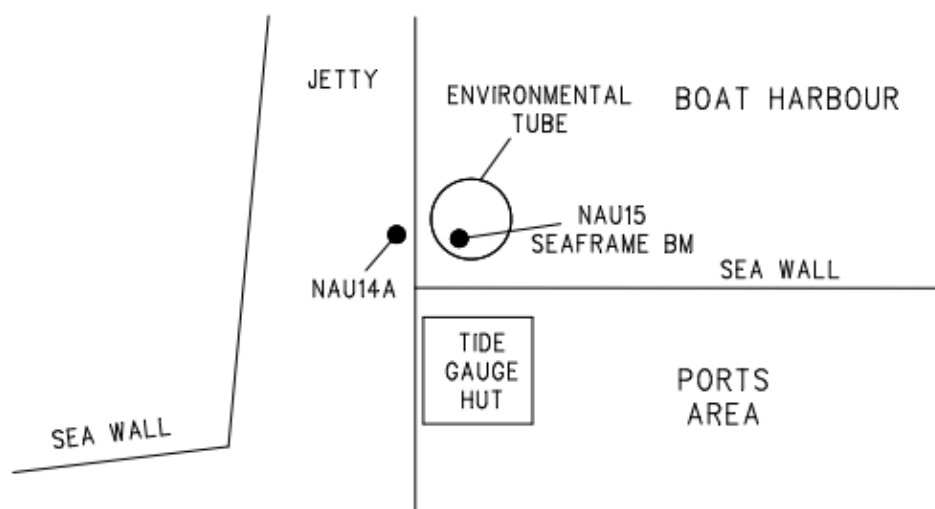
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 100
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 04-06-07



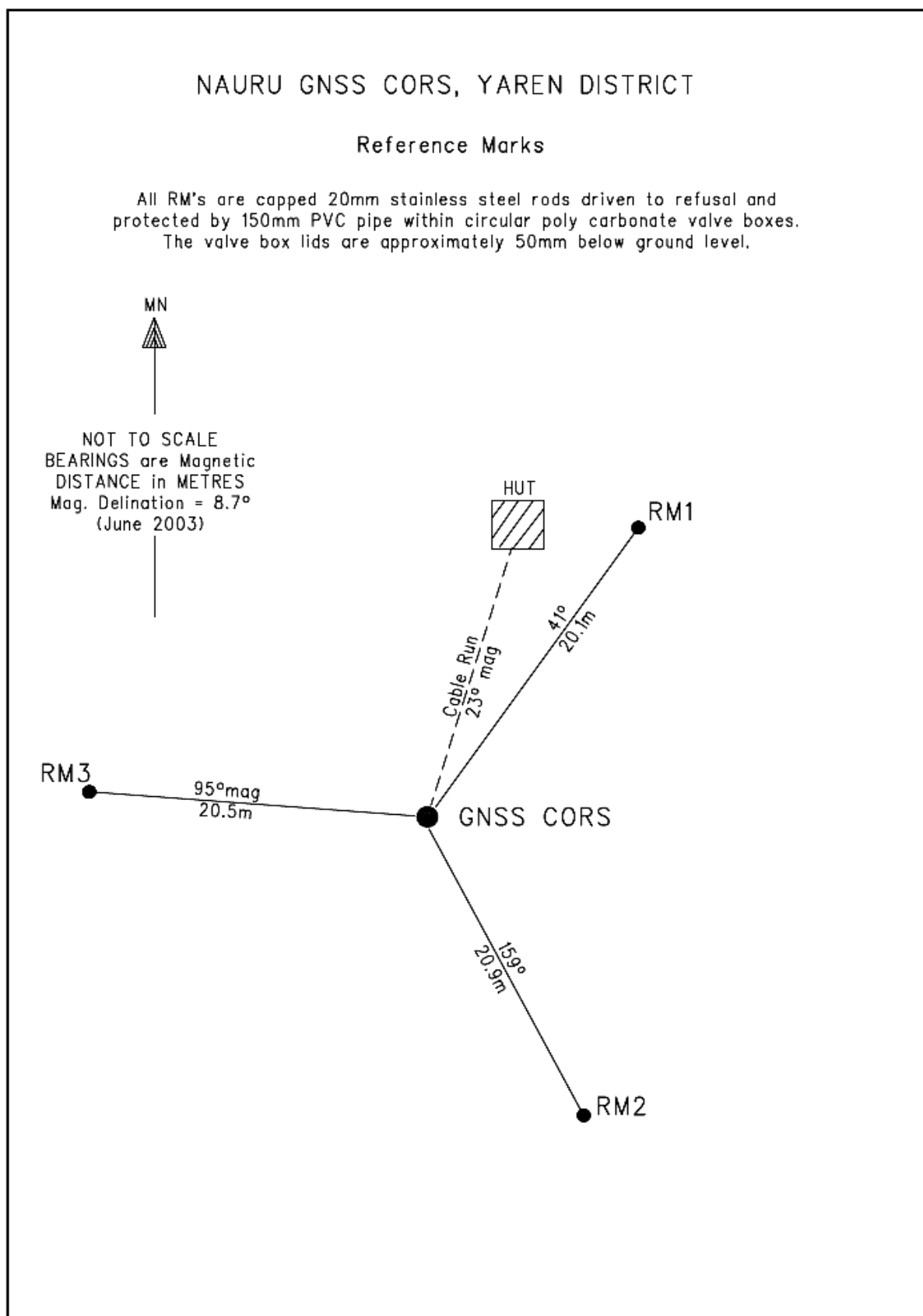
COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. 28
PROJECT: PSLGM	SURVEYOR: S. Turner	DATE: 31-05-02



COUNTRY: Nauru	ISLAND: Nauru DISTRICT: Aiwo/Boe	L. D. P. POINT NO. NAU 14A & 15
PROJECT: PSLGM	SURVEYOR: S. Yates & A Lal	DATE: 05-02-09



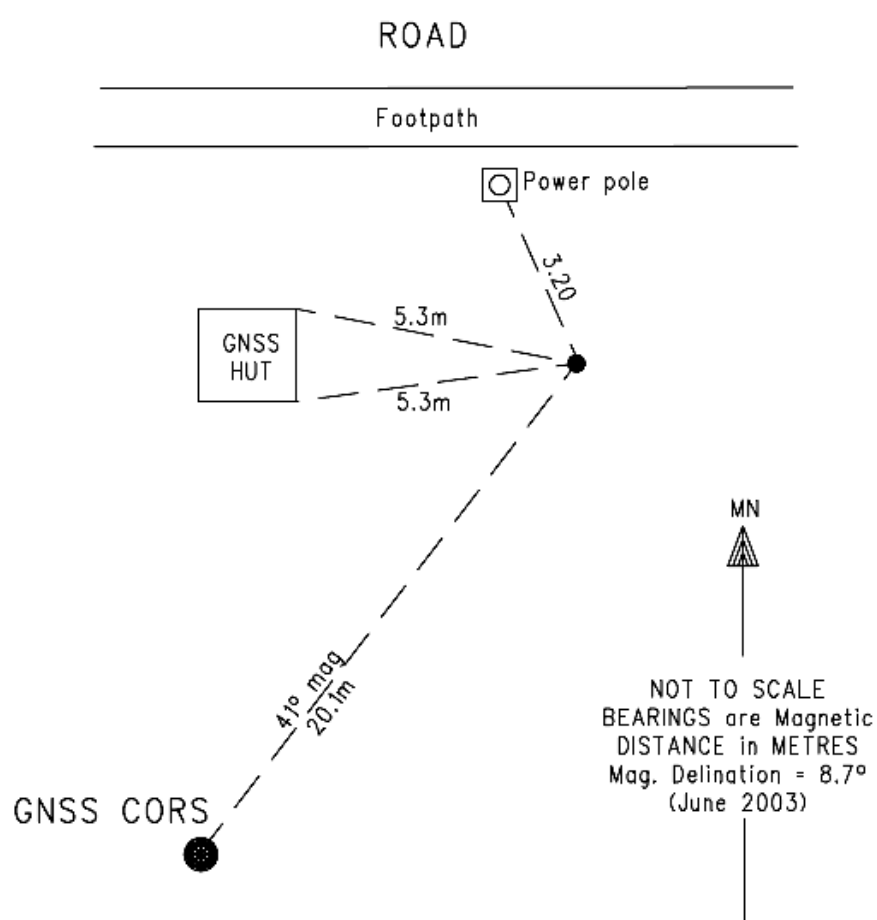
A 3. GNSS Site Reference Marks



NAURU GNSS CORS, YAREN DISTRICT - RM1

Reference Mark

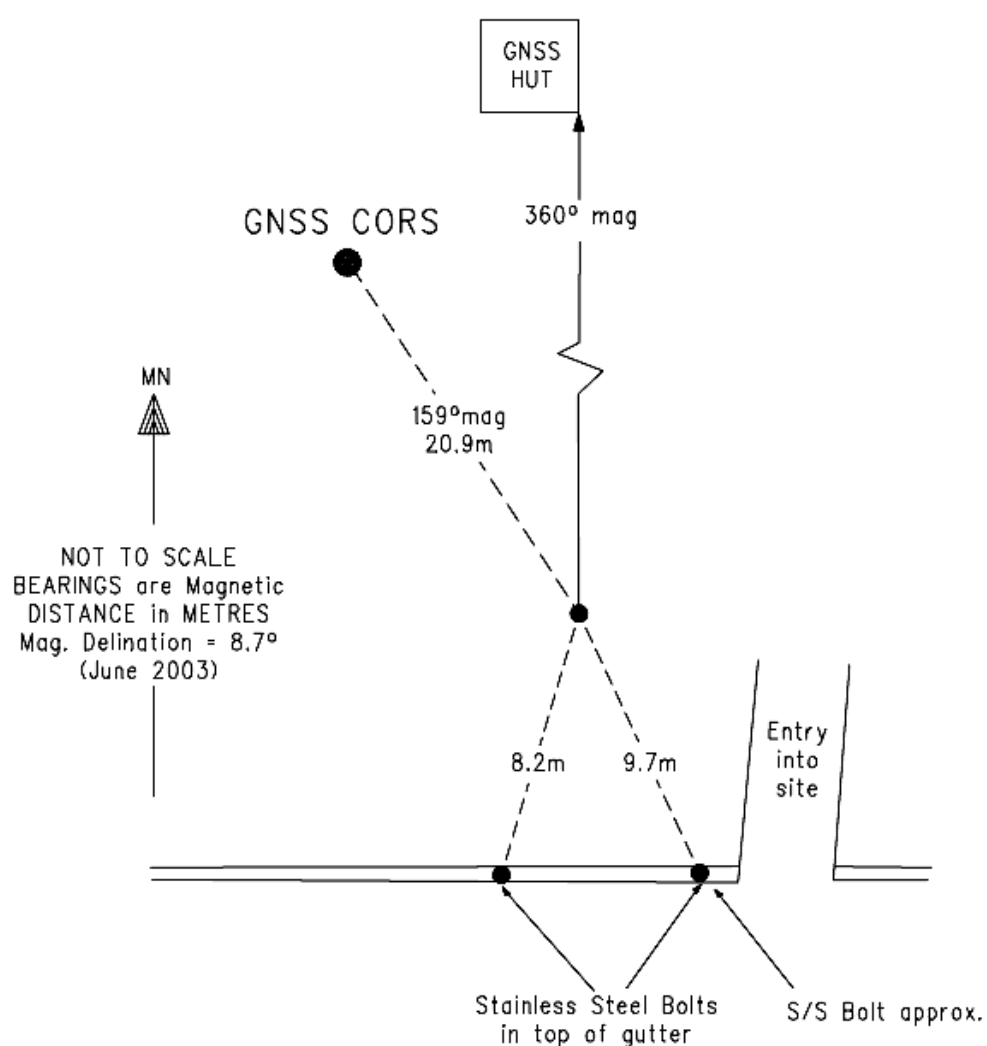
All RM's are capped 20mm stainless steel rods driven to refusal and protected by 150mm PVC pipe within circular poly carbonate valve boxes.
The valve box lids are approximately 50mm below ground level.



NAURU GNSS CORS, YAREN DISTRICT - RM2

Reference Mark

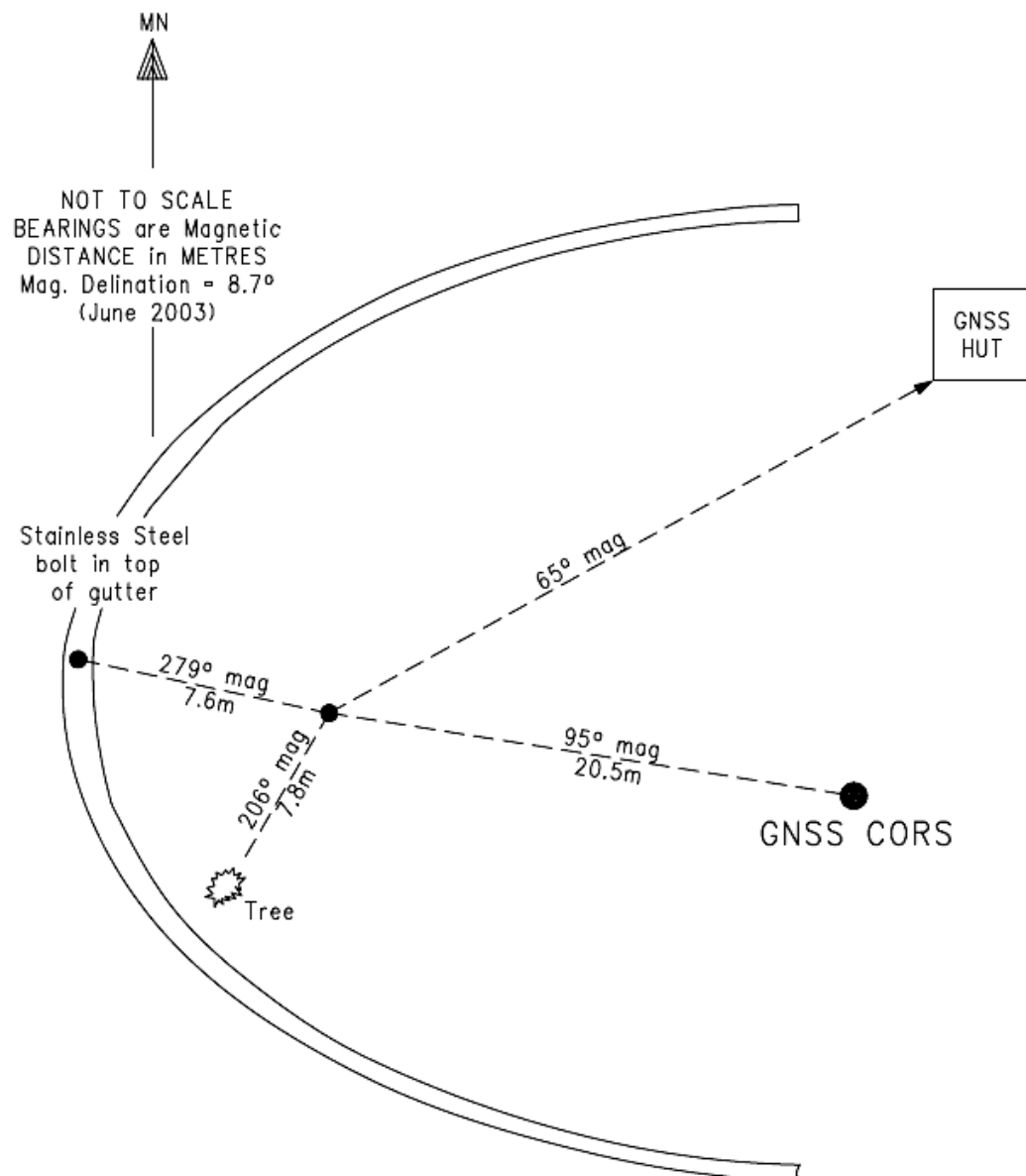
All RM's are capped 20mm stainless steel rods driven to refusal and protected by 150mm PVC pipe within circular poly carbonate valve boxes. The valve box lids are approximately 50mm below ground level.



NAURU GNSS CORS, YAREN DISTRICT - RM3

Reference Mark

All RM's are capped 20mm stainless steel rods driven to refusal and protected by 150mm PVC pipe within circular poly carbonate valve boxes. The valve box lids are approximately 50mm below ground level.



Appendix B Planning Aspects and Notes

Upon arranging travel to Nauru, make contact with the project support officer in country well in advance. Passport details provided to project focal point to allow entry on arrival, and a visa issued once in country. Copy of visa should be on hand before arrival into Nauru.

Prior arrangements with the local counterpart made for the clearance of the survey equipment from the Customs Authority when shipped across.

DHL Express commonly used for freight inwards and outward for the Survey Equipment into Nauru.

- For freight payment inwards and outward, cash payment recommended to avoid excess charges.
- The survey equipment freighted to the Land Management Division as the project focal point in the country.

When levelling across the airport apron, contact airport security, and airport manager, and advise them of the intended work. NAU19 exists on the concrete footing of a taxiway light, and security advised before stopping at this point. An escort to arrange to access NAU16.

Plan carefully around the arrival of international aircraft. Roads and access shut down for a few hours while the airport is operating.

Quantity	Item & description	Locations
1	Tool Box	Tide Gauge Station Hut.
2	<i>Prism Pole Clamps</i>	<i>Tools used by C&M Teams (BoM & SPC)</i>
1	<i>50m Measuring Tape</i>	
1	<i>Engineers Hammer</i>	
1	<i>Carpenters Hammer</i>	
1	<i>Set of Allen Keys</i>	
1	<i>Torx Drivers</i>	
2	<i>Multigrips pliers</i>	
1	<i>Set of Screw Drivers</i>	
1	PVC Pipe (1.2m)	GNSS COR Station Hut
1	<i>Aluminium GST6 tripod with Feet</i>	
1	PVC Pipe (1.7m)	GNSS COR Station Hut
1	<i>Ground Base Plate</i>	
4	<i>Telescopic-Bi-pods</i>	
2	<i>Stainless-steel levelling prism poles</i>	
1	<i>Half Stainless-steel levelling prism pole</i>	
3	Black Bags - Leica GST20 Telescopic Tripods	GNSS COR Station Hut
1	Green Bag - Leica GST40 Rigid Tripod	GNSS COR Station Hut
1	Spade	GNSS COR Station Hut
1	Crow Bar	GNSS COR Station Hut

Appendix C Equipment Specifications

Tachymeters, EDM and Theodolites

A Leica TM30 (S/N 361441) Total Station was used to record all angles and distance measurements.

Specification

- EDM (infrared) distance standard deviation of a single measurement (DIN 18723, part 6): 0.6 mm \pm 1 ppm.
- Angular standard deviation of a mean direction measured in both faces (DIN 18723, part 3): 0.3 mgon ($\approx 1^\circ$).

Calibration

The Leica TM30 electronic distance measuring instrument (Serial No. 361441) was calibrated by the Australian National Measurement Institute (NMI) in July 2013. It was found to have an average error of 0.44×10^{-6} mm, which has been added to the Total Station.

Meteorological Sensor

Description

A NK Kestrel 4000 Pocket Weather Tracker (S/N 625479) was used to record meteorological observations (temperature, pressure and relative humidity).

Specification

- Temperature is accurate to 1.0°C between -29.0°C and 70.0°C.
- Pressure is accurate to 1.5 mb at 25°C between 750 mb and 1100 mb.
- Relative humidity is accurate to 3.0%.

Forced Centring

Description

An FG0L30 (S/N 609030) zenith and nadir optical plummet was used to centre and level all instrument and target setups.

Specification

- Accuracy is 1:30 000 (1 mm at 30 m).

Targets and Reflectors

Description

The standard target kit includes:

- 4 x Leica GDF21 tribrachs.
- 4 x Leica GZR3 prism carriers with optical plummet.
- 4 x Leica GPH1P precision prisms.

Calibration

The additive constant for the Leica GPH1P precision prism is -34.4 mm which was applied directly into the Leica TM30 Total Station. All prisms were calibrated on a tripod baseline at Geoscience Australia in July 2009. Approximate prism corrections of 0.0 mm were applied to observations during data processing.

Precision Levelling

Levelling Instruments

Refer to section 2.1 for a description of the Leica TM30 Total Station

Levelling Rods

A fixed height stainless steel rod (SP Primary Pole) approximately 1.6 m in height with Leica style bayonet mount on top for mounting a precision prism was used with a Leica bi-pod for stability.

A fixed height short stainless-steel rod (SP 1/2m TG Pole) approximately 0.5 m in height with Leica style bayonet mount on top for mounting a precision prism was used.

A height offset between the pole (SP Primary Pole) and the short pole (SP 1/2m TG Pole) was determined by observing both on a low mark. Multi-set, dual face observations were used to eliminate collimation effects. The resulting height offset was 1.00054m.

Tripods

Description

Leica GST20 heavy-duty timber tripods with adjustable legs was used on all marks, with the exception of the pillars, during the monitoring survey.

A Leica rigid timber tripod was used to mount the TM30 on for the purpose of this levelling survey.

Note: Three Leica adjustable leg tripods was left on site in the current GNSS hut for carrying out the associated RM horizontal survey.

GNSS Equipment

At the time of the survey, the GNSS equipment in use at the GNSS CORS site was:

- Septentrio GNSS receiver SEPT POLARX4TR Firmware Version 2.9.6 (S/N 3007597)
- Leica Choke Ring antenna (LEIAR25.R3) - S/N 10150005