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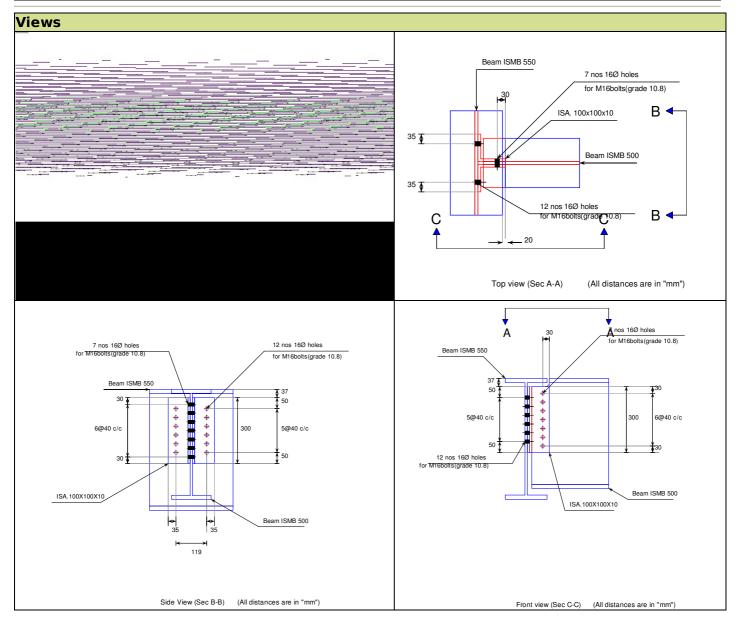
Design Conclusion			
Cleat Angle	Pass		
Cleat Angle			
Connection Properties			
Connection			
Connection Title	Double Angle Web Cleat		
Connection Type	Shear Connection		
Connection Category			
Connectivity	Beam-Beam		
Beam Connection	Bolted		
Column Connection	Bolted		
Loading (Factored Load)			
Shear Force (kN)	250.0		
Components			
Column Section	ISMB 550		
Material	Fe 410		
Beam Section	ISMB 500		
Material	Fe 410		
Hole	STD		
Cleat Section	ISA 100X100X10		
Thickness (mm)	10		
Cleat Leg Size B (mm)	100		
Cleat Leg Size A (mm)	100		
Hole	STD		
Bolts on Beam			
Type	HSFG		
Grade	10.8		
Diameter (mm)	16		
Bolt Numbers	7		
Columns (Vertical Lines)	1		
Bolts Per Column	7		
Gauge (mm)	0		
Pitch (mm)	40		
End Distance (mm)	30		
Edge Distance (mm)	30		
Bolts on Column	luoro		
Type	HSFG		
Grade	10.8		
Diameter (mm)	16		
Bolt Numbers	12		
Columns (Vertical Lines)	1		
Bolts Per Column	6		
Gauge (mm)	0		
Pitch (mm)	40		
End Distance (mm)	50.0		
Edge Distance (mm)	35.1		
Assembly Classes (1999)	20		
Column-Beam Clearance (mm)	20		

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	eam Connectivity		
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{\rm dsb} = ((2*1000*0.6126*16*16)/(\sqrt{3}*1.25*1000) = 84.311 [cl. 10.3.3]$	
Bolt bearing capacity (kN)		$V_{dsb} = (2.5*0.5*16*10.0*1000)/(1.25*1000) = 163.2$ [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{dsb}$ = (2.5*0.5*18.0*10.2*410)/(1.25*1000) = 66.912 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{dsb} = (2.5*0.5*18.0*10*410)/(1.25*1000) = 66.912$ [cl. 10.3.4]	
Bearing capacity (kN)		Min (163.2, 66.912, 131.2) = 66.912	Pass
Bolt capacity (kN)		Min (84.311, 66.912) = 66.912	Pass
Critical Bolt Shear (kN)	≤66.912	37.558	Pass
No. of bolts	250.0/66.912 = 3.7	7	Pass
No.of column(s)	≤2	1	
No. of bolts per column		7	
Bolt pitch (mm)	$\geq$ 2.5* 16 = 40, $\leq$ Min(32*10.2, 300) = 300 [cl. 10.2.2]	40	
Bolt gauge (mm)	$\geq$ 2.5*16 = 40, $\leq$ Min(32*10.2, 300) = 300 [cl. 10.2.2]	0	
End distance (mm)	$\geq$ 1.7*18.0 = 30.6, $\leq$ 12*10.2 = 122.4 [cl. 10.2.4]	30	
Edge distance (mm)	$\geq$ 1.7*18.0 = 30.6, $\leq$ 12*10.2 = 122.4 [cl. 10.2.4]	30	Pass
Block shear capacity (kN)	250.0	V <sub>db</sub> = 328.946 [cl. 6.4.1]	
Cleat height (mm)	≥0.6*500.0=300.0, ≤500.0- 17.2-17.0-19.3-18.0- 5=423.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	0.0	Pass
	$(2*84.311*40^2)/(40*1000) =$ 8.75	$M_{\rm d} = (1.2*250*Z)/(1000*1.1) = 270.0$ [cl. 8.2.1.2]	Pass

Design Check: Column Connectivity				
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		$V_{\rm dsb} = ((1000*0.6126*16*16)/(\sqrt{3}*1.25*1000) = 42.156$ [cl. 10.3.3]	)	
Bolt bearing capacity (kN)		$V_{dsb} = (2.5*0.5*16*10.0*1000)/(1.25*1000) = 160.0 [cl. 10.3.4]$		
Bearing capacity of Primary beam web (kN)		$V_{dsb}$ = (2.5*0.5*18.0*11.2*410)/(1.25*1000) = 142.434 [cl. 10.3.4]		
Bearing capacity of cleat leg (kN)		$V_{\text{dsb}} = (2.5*0.5*18.0*10*410)/(1.25*1000) = 73.8$ [cl. 10.3.4]		
Bearing capacity (kN)		Min (160.0, 142.434, 73.8) = 142.434	Pass	
Bolt capacity (kN)		Min (42.156, 142.434) = 42.156	Pass	
No. of bolts	250.0/66.912 = 5269.5	12	Pass	
No.of column(s)	≤2	1		
No. of bolts per column		6		
Bolt pitch (mm)	$\geq$ 2.5* 16 = 40, $\leq$ Min(32*10.0, 300) = 300 [cl. 10.2.2]	40		
5 5 .	$\geq$ 2.5*16 = 40, $\leq$ Min(32*10.0, 300) = 300 [cl. 10.2.2]	0		
End distance (mm)	[cl. 10.2.4]	50.0		
Edge distance (mm)	$\geq$ 1.7*18.0 = 30.6, $\leq$ 12*10.0 = 120.0 [cl. 10.2.4]	35.1	Pass	
Block shear capacity (kN)	250.0	V <sub>db</sub> = 337.128 [cl. ]		
	≥0.6*500.0=300.0, ≤500.0- 17.2-17.0-19.3-18.0- 5=423.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	35.1	Pass	
	$(2*42.156*40^2)/(40*1000) =$ 8.75	$M_{\rm d} = (1.2*250*Z)/(1000*1.1) = 270.0$ [cl. 8.2.1.2]	Pass	

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Additional Comments	