

Design of a reservoir hydropower scheme

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Task 1: Determine the design of a gravity dam, height H to satisfy safety factors against sliding and overturning.

Finding measurement

Stability analysis

Load Combination

1 - Usual load combination

2 - Flood load case

3 - Extreme case : earthquake

Annex: calculation code

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Finding measurement

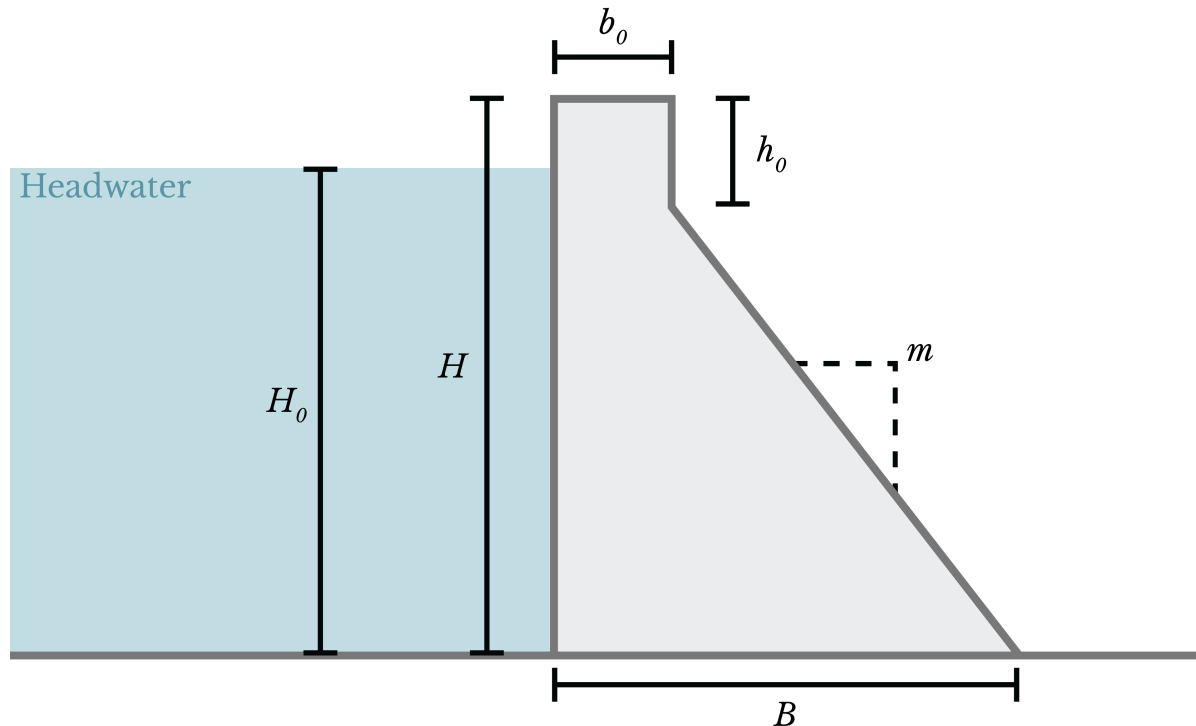


Figure 1 : Introduction to notations for the gravity dam measurements

According to the Bureau of Reclamation of the USA department of the interior "Design of gravity dams" paper we can calculate the missing dimensions according to the current standards.

$$m = \frac{B}{H} = \frac{b_0}{h_0} \simeq 0,8$$

$$b_0 = \frac{H}{10} \geq 8m$$

We can therefore calculate :

- $B = m \cdot H = 0,8 \times 71 = 56,8m$
- $b_0 = \frac{H}{10} = \frac{71}{10} = 7,1m \leq 8$, so we choose $b_0 = 8m$
- $h_0 = \frac{b_0}{m} = \frac{8}{0,8} = 10m$

Stability analysis

Now we have to figure out the weakpoints and the critical failure modes. We can identify three of them:

- **Sliding** : too much pressure causes the dam to move in the horizontal plane. The following safety factor is then considered: $n_{SL} = \frac{\sum V}{\sum H}$. Where $\sum V$ represent the sum of the vertical efforts and $\sum H$ for the horizontal ones. To meet the safety requirements $n_{SL} > 3$ for the usual load case.
- **Overturning** : the dam rotates around a fixed point. In our case and as shown in the diagram the lower end on the right. The safety factor is described as, $n_{OV} = \frac{M_{stab}}{M_{dest}} > 2$.
- **Crushing** : in this case we are looking at compression and tension failure.

- G_1 and G_2 represent the **dead load** of the dam. The following formulas describing the effort are then considered :
 - $\gamma_{conc} = 24 \text{ kN/m}^3$
 - $G_1 = b_0 \cdot H \cdot \gamma_{conc} \cdot e = 13632 \text{ kN}$
 where e is the thickness of the study section ($e = 1 \text{ m}$), so we won't consider it in the next equations
 - $G_2 = \frac{1}{2}(B - b_0)(H - H_0)\gamma_{conc} = 35721 \text{ kN}$
 - $r_{G1} = B - \frac{b_0}{2} = 52,8 \text{ m}$
 - $r_{G2} = \frac{2}{3}(B - b_0) = 32,5 \text{ m}$
- P_w the **hydrostatic pressure** on the dam :
 - $\gamma_w = 10 \text{ kN/m}^3$
 - $P_w = \gamma_w \cdot \frac{H_0^2}{2} = 21125 \text{ kN}$
 - $r_w = \frac{1}{3} \cdot H_0 = 21,7 \text{ m}$
- P_u the **uplift pressure** on the dam :

- To reduce the effect of uplift, draingaes to the ground are installed in the dam. This reduction is associated with the coefficient $\lambda = 0,85$
- $P_u = \lambda \cdot H_0 \cdot \gamma_w \cdot \frac{B}{2} = 15691 \text{ kN}$
- $r_u = \frac{2}{3} \cdot B = 37,8 \text{ m}$
- P_s the **silt pressure** on the dam :
 - $\gamma_s = 18 \text{ kN/m}^3$
 - $H_s = \frac{H_0}{3} = 21,6 \text{ m}$
 - $P_s = \gamma_s \cdot \frac{H_s^2}{2} = 4225 \text{ kN}$
 - $r_s = \frac{H_s}{3} = 7,2 \text{ m}$

We can therefore calculate our safety factors :

$$n_{SL} = \frac{G_1 + G_2 - P_u}{P_w - P_s} = 1,99$$

$$n_{OV} = \frac{G_1 \cdot r_{G1} + G_2 \cdot r_{G2}}{P_w \cdot r_w + P_s \cdot r_s + P_u \cdot r_u} = 1,74$$

We can now conclude that the current design of the dam **does not satisfy the safety criteria** because $n_{SL} < 3$ and $n_{OV} < 2$.

2 - Flood load case

In this second case, we are considering the same problem as the previous case, whereas the reservoir is at his flood level : $H_0 = H$

For this case the safety factors we have to check have different values : $n_{SL} > 2$ and $n_{OV} > 1,5$.

By doing the same calculation we can calculate our new safety factors :

$$n_{SL} = 1,6$$

$$n_{OV} = 1,5$$

For this second case we can conclude that a flood scenario could be critical for the dam can **the safety critria are not respected.**

3 - Extreme case : earthquake

For this last case, we are considering the following load combination :

- the dead weight of the dam,
- the reservoir at its operational level,
- the silt presence,
- the uplift pressure,
- the seismic failure,

For the modelisation of the seismic forces we are using the Chopra method and for our calculus we are looking at the worst scenario possible.

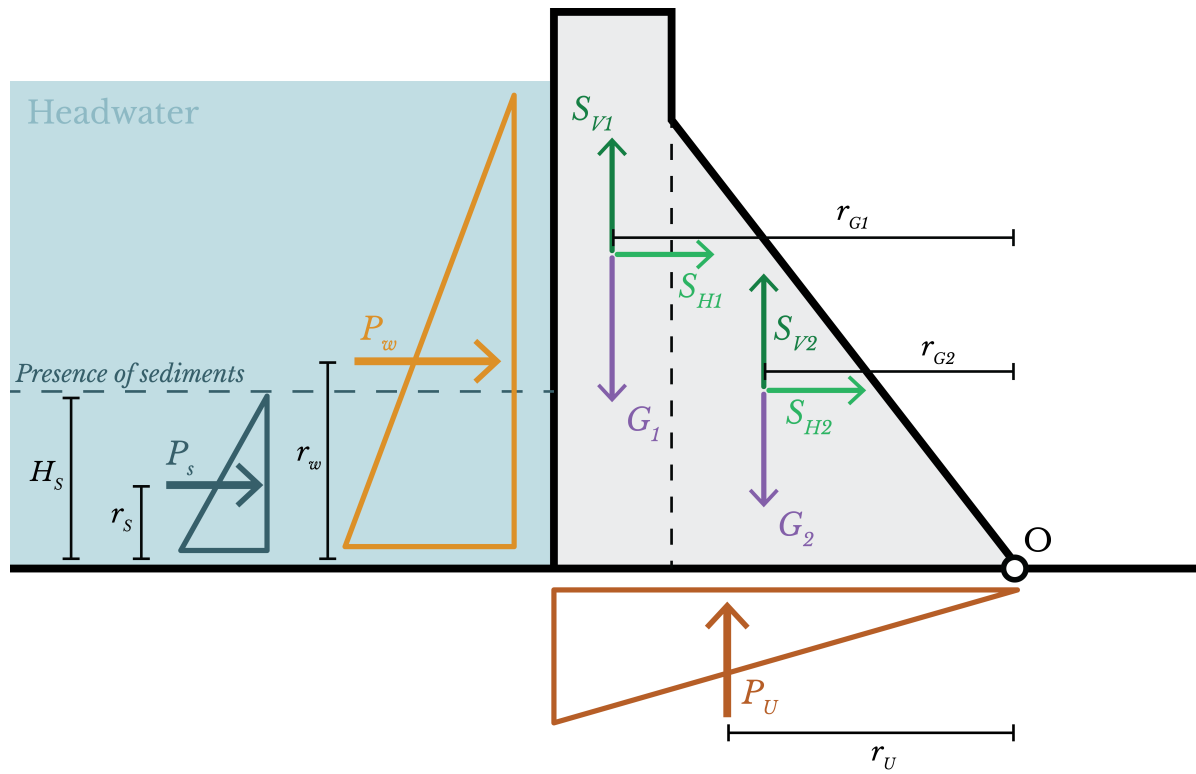


Figure 3 : Efforts on the dam for the earthquake case

As for dead loads, we divide our calculation in two parts :

- S_V represent the vertical **effort of an earthquake** and S_H the horizontal ones :
 - $\alpha = 0,1g$ the seismic coefficient
 - $S_{V1} = \alpha \cdot G_1 = 13372 \text{ kN}$
 - $S_{H1} = \frac{2}{3} \alpha \cdot G_1 = 8915 \text{ kN}$
 - $S_{V2} = \alpha \cdot G_2 = 35042 \text{ kN}$
 - $S_{H2} = \frac{2}{3} \alpha \cdot G_2 = 23361 \text{ kN}$

Our safety factors are : $n_{SL} > 1,3$ and $n_{OV} > 1,2$.

We can calculate them :

$$n_{SL} = \frac{G_1 + G_2 - P_u - S_{V1} - S_{V2}}{P_w - P_s - S_{H1} - S_{H2}} = 0,95$$

$$n_{OV} = \frac{(G_1 + S_{V1}) \cdot r_{G1} + (G_2 + S_{V2}) \cdot r_{G2}}{P_w \cdot r_w + P_s \cdot r_s + P_u \cdot r_u + S_{H1} \cdot r_{G1} + S_{H2} \cdot r_{G2}} = 1,6$$

As expected this case **do not satisfy every safety criteria** . Even though the overturning possibility respect its safety factor.

Annex: calculation code

```
"Finding measurement"
H=71
m=0.8
B=m*H
b0=H/10
if b0<=8:
    b0=8
h0=b0/m

"Load combination"
#dead load
gconc=24
H0=H-6
G1=b0*H*gconc
G2=0.5*(B-b0)*(H-h0)*gconc
rG1=B-(b0/2)
rG2=(2/3)*(B-b0)
#hydrostatic pressure
gw=10
Pw=gw*0.5*H0*H0
rw=(1/3)*H0
#uplift pressure
gamma=0.85
Pu=gamma*H0*gw*B/2
ru=(2/3)*B
#silt pressure
gs=18
Hs=H0/3
Ps=gs*0.5*Hs*Hs
rs=Hs/3
#earthquake forces
a=9.81*0.1
Sv1=a*G1
Sv2=a*G2
Sh1=(2/3)*a*G1
Sh2=(2/3)*a*G2

"security factors check"
nsl=(G1+G2-Pu-Sv1-Sv2)/(Pw-Ps-Sh1-Sh2)
print("nsl=",nsl)
nov=((G1+Sv1)*rG1+(G2+Sv2)*rG2)/(Pw*rw+Ps*rs+Pu*ru+Sh1*rG1+Sh2*rG2)
print("nov=",nov)
```