

Product Category: 09.22.16 - Non-Structural Metal Framing

Product Name: 250PWS134-19 PrimeWall Stud 20 EQ

Available Finish: G40
 (G60/G90 coatings available upon request)
 *Other standard coatings referenced in ASTM A1003

Web Depth: 2-1/2 in

Flange Width: 1-11/32 in

Return Lip: 0.406 in

Design Thickness: 0.0200 in

Gauge: 19 mils or 20 EQ

Yield stress, Fy: 55 ksi

Weight: 0.39 lb/ft

Gross Section Properties

Cross sectional area (A)	0.115 in ²
Moment of inertia (I _{xx})	0.119 in ⁴
Section Modulus (S _{xx})	0.095 in ³
Radius of gyration (R _x)	1.017 in
Gross moment of inertia (I _{yy})	0.031 in ⁴
Gross Radius of gyration (R _y)	0.515 in


Effective Section Properties

Moment of inertia for deflection (I _{xx})	0.111 in ⁴
Section modulus (S _{xx})	0.075 in ³
Allowable bending moment (M _{a-L})	2.480 In-k
Allowable bending moment from distortional buckling (M _{a-D})	2.02 In-k
Allowable strong axis shear away from punch-out (V _{ag})	311 lb
Allowable strong axis shear at punch out (V _{aNet})	222 lb

Torsional Properties

St. Venant torsion constant (J x 1000)	0.015 in ⁴
Warping constant (C _w)	0.047 in ⁶
Distance from shear center to neutral axis (X _o)	-1.152 in
Distance from shear center to mid-plane of web (m)	0.685 in
Radii of gyration (R _o)	1.621 in
Torsional flexural constant (β)	0.495
Unbraced Length (L _u)	27.6 in

Interior Non-Structural, Non-Composite Wall Height Table Notes

- Calculations are based on AISI Standard, North American Specification for the Design of Cold-Formed Steel Structural Members, 2016 edition (AISI S100-2016). All calculations are based on allowable strength design (ASD).
- When provided, factory punchouts will be located along the centerline of the webs of the members and will have a minimum center-to-center spacing of 24 inches. Punchouts for members > 2.5 inches deep are a maximum of 1.5 inches wide x 4 inches long. Members with depths 2.5" and smaller are maximum 3/4" wide x 4 inches long.
- For deflection determination, use the effective moment of inertia.
- The effective moment of inertia for deflection is calculated at a stress which results in a section modulus such that the stress times the section modulus at that stress is equal to the allowable local buckling moment, M_{a-L}.
- Tabulated gross and torsional properties are based on the full, unreduced section away from punchouts
- Effective X-X Axis properties of all stud and joist sections based on punched sections.
- Where section designations include a superscript '1', web height-to-thickness exceeds 200. Web stiffeners are required at all supports and concentrated loads.
- Where effective properties are not listed for a section, web depth-to-thickness or flange width-to-thickness limits from the AISI S100 are exceeded. Only gross properties are available.
- Allowable bending moment and moment of inertia for 6" studs based on the direct strength method (DSM).

Non-Composite Limiting Heights – Fully Braced

Spacing (inches)	5psf L/120	5psf L/240	5psf L/360	7.5psf L/120	7.5psf L/240	7.5psf L/360	10psf L/120	10psf L/240	10psf L/360
12	14' 6"	11' 7"	10' 2"	12' 7"	10' 2"	8' 10"	11' 5"	9' 2"	8' 1"
16	13' 2"	10' 7"	9' 3"	11' 5"	9' 2"	8' 1"	10' 1"	8' 4"	7' 4"
24	11' 5"	9' 2"	8' 1"	9' 6"	8' 0"	7' 0"	8' 2"	7' 3"	6' 5"

Additional Information

MRI Steel Framing, LLC is an SFIA member. MRI acts in accordance with the product and quality standards required by the SFIA program.

MRI meets or exceeds ASTM C645, C754, A653, and A1003.

Current LEED credits available upon request

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