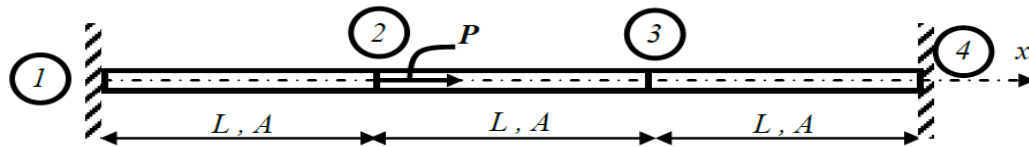


EXERCICE N°01**1. Matrice de Rigidité Globale**➤ *Matrices de Rigidité Élémentaire*

$$[k^{(e)}] = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[k^{(1)}] = [k^{(2)}] = [k^{(3)}] = \frac{AE}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

➤ *Assemblage des Matrices Élémentaires en Matrice Globale*

$$[K] = \sum_{i=1}^n [k^{(i)}] \quad \text{Soit :} \quad [K] = [k^{(1)}] + [k^{(2)}] + [k^{(3)}]$$

✓ *Concept de la Superposition (Méthode de Rigidité Directe)*

$$[k_{e\ 1-2}^{(1)}] = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{matrix} u_1 \\ u_2 \end{matrix} \quad [k_{e\ 2-3}^{(2)}] = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{matrix} u_2 \\ u_3 \end{matrix}$$

$$[k_{e\ 3-4}^{(3)}] = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{matrix} u_3 \\ u_4 \end{matrix}$$

$$[K] = \frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 2 & -1 & 0 \\ 0 & -1 & 2 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{matrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{matrix}$$

2. Déplacements u_2 et u_3

$$\{F\} = [K] \{U\}$$

$$\begin{Bmatrix} F_{1x} \\ F_{2x} \\ F_{3x} \\ F_{4x} \end{Bmatrix} = \frac{AE}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 2 & -1 & 0 \\ 0 & -1 & 2 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix}$$

$$\begin{Bmatrix} F_{2x} \\ F_{3x} \end{Bmatrix} = \frac{AE}{L} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix}$$

$$\begin{Bmatrix} P \\ 0 \end{Bmatrix} = \frac{AE}{L} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix}$$

$$u_2 = \frac{2 PL}{3 EA} \quad ; \quad u_3 = \frac{PL}{3 EA}$$

3. Réactions aux Nœuds 1 et 4

$$\{F\} = [K] \{U\}$$

$$\begin{Bmatrix} F_{1x} \\ F_{2x} \\ F_{3x} \\ F_{4x} \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 2 & -1 & 0 \\ 0 & -1 & 2 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix}$$

$$F_{1x} = -\frac{2}{3}P \quad , \quad F_{4x} = -\frac{1}{3}P$$

➤ *Vérification* : $F_{2x} = F_{1x} + F_{4x}$ OK !!!

EXERCICE N°02

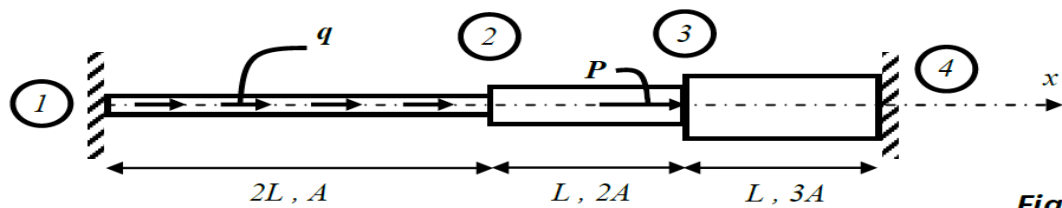


Figure 2

1. Matrice de Rigidité Globale

➤ *Matrices de Rigidité Élémentaire*

$$[k^{(1)}] = \frac{EA}{2L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \quad \text{soit :} \quad [k^{(1)}] = \frac{EA}{L} \begin{bmatrix} \frac{1}{2} & -\frac{1}{2} \\ -\frac{1}{2} & \frac{1}{2} \end{bmatrix}$$

$$[k^{(2)}] = \frac{2EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \quad \text{soit :} \quad [k^{(2)}] = \frac{EA}{L} \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix}$$

$$[k^{(3)}] = \frac{3EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \quad \text{soit :} \quad [k^{(3)}] = \frac{EA}{L} \begin{bmatrix} 3 & -3 \\ -3 & 3 \end{bmatrix}$$

➤ *Assemblage des Matrices Élémentaires en Matrice Globale*

$$[K] = \sum_{i=1}^n [k^{(i)}] \quad \text{Soit :} \quad [K] = [k^{(1)}] + [k^{(2)}] + [k^{(3)}]$$

✓ *Concept de la Superposition (Méthode de Rigidité Directe)*

$$[k_{e\,1-2}^{(1)}] = \frac{EA}{L} \begin{bmatrix} 1/2 & -1/2 \\ -1/2 & 1/2 \end{bmatrix} \begin{matrix} u_1 \\ u_2 \end{matrix} \quad [k_{e\,2-3}^{(2)}] = \frac{EA}{L} \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix} \begin{matrix} u_2 \\ u_3 \end{matrix}$$

$$[k_{e\,3-4}^{(3)}] = \frac{EA}{L} \begin{bmatrix} 3 & -3 \\ -3 & 3 \end{bmatrix} \begin{matrix} u_3 \\ u_4 \end{matrix}$$

$$[K] = \frac{EA}{L} \begin{bmatrix} 0.5 & -0.5 & 0 & 0 \\ -0.5 & 2.5 & -2 & 0 \\ 0 & -2 & 5 & -3 \\ 0 & 0 & -3 & 3 \end{bmatrix}$$

2. Déplacements Nodaux

$$\begin{Bmatrix} F_{1x} \\ F_{2x} \\ F_{3x} \\ F_{4x} \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 0.5 & -0.5 & 0 & 0 \\ -0.5 & 2.5 & -2 & 0 \\ 0 & -2 & 5 & -3 \\ 0 & 0 & -3 & 3 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix}$$

$$\begin{Bmatrix} E_{1x} = R_1 + qL \\ F_{2x} = qL \\ F_{3x} = P = 2qL \\ -F_{4x} = R_4 \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 0.5 & -0.5 & 0 & 0 \\ -0.5 & 2.5 & -2 & 0 \\ 0 & -2 & 5 & -3 \\ 0 & 0 & -3 & 3 \end{bmatrix} \begin{Bmatrix} u_1 = 0 \\ u_2 \\ u_3 \\ u_4 = 0 \end{Bmatrix}$$

$$\begin{Bmatrix} 1 \\ 2 \end{Bmatrix} qL = \frac{EA}{L} \begin{bmatrix} 2.5 & -2 \\ -2 & 5 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix}$$

$$qL \begin{Bmatrix} 1 \\ 2 \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 2.5 & -2 \\ -2 & 5 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix}$$

$$u_2 = \frac{18}{17} \frac{qL^2}{EA} \quad ; \quad u_3 = \frac{14}{17} \frac{qL^2}{EA}$$

3. Forces Nodales Globales

$$\begin{Bmatrix} F_{1x} = R_1 + qL \\ F_{2x} \\ F_{3x} \\ F_{4x} = R_4 \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 0.5 & -0.5 & 0 & 0 \\ -0.5 & 2.5 & -2 & 0 \\ 0 & -2 & 5 & -3 \\ 0 & 0 & -3 & 3 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix}$$

$$R_1 = -\frac{26}{17} qL \quad ; \quad R_4 = -\frac{42}{17} qL$$

EXERCICE N°03

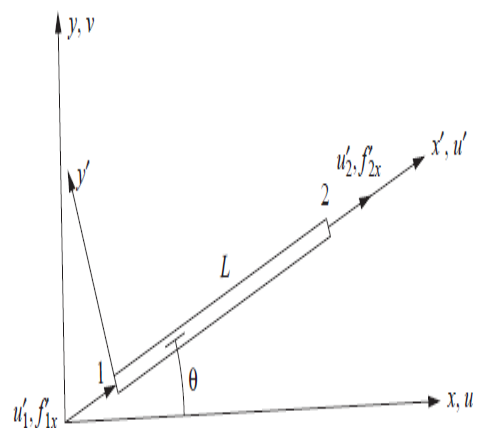
▪ Matrice de Rigidité Élémentaire

$$\begin{Bmatrix} f'_{1x} \\ f'_{2x} \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} u'_1 \\ u'_2 \end{Bmatrix}$$

$$[k_e] = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$\begin{Bmatrix} f'_{1x} \\ f'_{1y} \\ f'_{2x} \\ f'_{2y} \end{Bmatrix} = [K_e] \begin{Bmatrix} u'_1 \\ v'_1 \\ u'_2 \\ v'_2 \end{Bmatrix}$$

$$[k_e] = \frac{EA}{L} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$



▪ Matrice de Rigidité Globale dans le Repère Global

$$\begin{Bmatrix} f_{1x} \\ f_{1y} \\ f_{2x} \\ f_{2y} \end{Bmatrix} = [K] \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{Bmatrix}$$

➤ Le changement de base est alors possible en posant que :

$$[K] = [R_e]^T [k_e] [R_e]$$

avec :

$$[R_e] = \begin{bmatrix} C & S & 0 & 0 \\ -S & C & 0 & 0 \\ 0 & 0 & C & S \\ 0 & 0 & -S & C \end{bmatrix} \quad \text{où :} \quad C = \cos \theta, \quad S = \sin \theta$$

$$[K] = \frac{EA}{L} \begin{bmatrix} C^2 & CS & -C^2 & -CS \\ CS & S^2 & -CS & -S^2 \\ -C^2 & -CS & C^2 & CS \\ -CS & -S^2 & CS & S^2 \end{bmatrix}$$

$$\theta = 30^\circ, \quad C = \cos 30^\circ = \frac{\sqrt{3}}{2}, \quad S = \sin 30^\circ = \frac{1}{2}$$

$$[K]_{(KN/m)} = 7 \cdot 10^4 \begin{bmatrix} \frac{3}{4} & \frac{\sqrt{3}}{4} & \frac{3}{4} & \frac{\sqrt{3}}{4} \\ \frac{\sqrt{3}}{4} & \frac{1}{4} & \frac{\sqrt{3}}{4} & \frac{1}{4} \\ \frac{3}{4} & \frac{\sqrt{3}}{4} & \frac{3}{4} & \frac{\sqrt{3}}{4} \\ \frac{\sqrt{3}}{4} & \frac{1}{4} & \frac{\sqrt{3}}{4} & \frac{1}{4} \end{bmatrix}$$

EXERCICE N°04

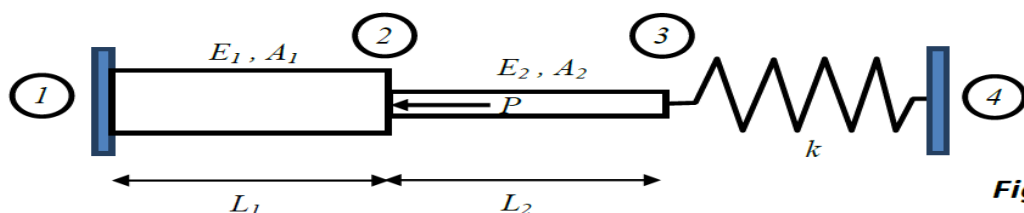


Figure 3

1. Matrice de rigidité

➤ *Elément 1*

$$[k^{(1)}] = \frac{E_1 A_1}{L_1} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

➤ *Elément 2*

$$[k^{(2)}] = \frac{E_2 A_2}{L_2} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

➤ **Elément 3**

$$[k^{(3)}] = k \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$k = \frac{E_1 A_1}{L_1}$$

$$[k^{(3)}] = \frac{E_1 A_1}{L_1} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$E_1 = 3E_2 = 3E \quad \text{et} \quad L_1 = L_2 = L$$

❖ Les sections droites sont de diamètres d_1 et d_2 avec $d_1 = 2d_2 = 2d$

$$A_1 = \frac{\pi d_1^2}{4} \quad \text{donc :} \quad A_1 = \pi d_2^2$$

$$\text{et} \quad A_2 = \frac{\pi d_2^2}{4} \quad \text{d'où :} \quad A_1 = 4 A_2$$

$$[k^{(1)}] = \frac{3\pi E d^2}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[k^{(2)}] = \frac{\pi E d^2}{4L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[k^{(3)}] = \frac{3\pi E d^2}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[K] = \frac{\pi E d^2}{L} \begin{bmatrix} 3 & -3 & 0 & 0 \\ -3 & 13/4 & -1/4 & 0 \\ 0 & -1/4 & 13/4 & -3 \\ 0 & 0 & -3 & 3 \end{bmatrix}$$

$$\begin{pmatrix} F_{1x} = R_1 \\ F_{2x} = -P \\ F_{3x} = 0 \\ F_{4x} = R_4 \end{pmatrix} = \frac{\pi E d^2}{4L} \begin{bmatrix} 12 & -12 & 0 & 0 \\ -12 & 13 & -1 & 0 \\ 0 & -1 & 13 & -12 \\ 0 & 0 & -12 & 12 \end{bmatrix} \begin{pmatrix} u_1 = 0 \\ u_2 \\ u_3 \\ u_4 = 0 \end{pmatrix}$$

$$\begin{Bmatrix} -P \\ 0 \end{Bmatrix} = \frac{\pi E d^2}{4L} \begin{bmatrix} 13 & -1 \\ -1 & 13 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix}$$

$$\begin{cases} -P = \frac{\pi E d^2}{4L} (13u_2 - u_3) \\ 0 = \frac{\pi E d^2}{4L} (-u_2 + 13u_3) \end{cases}$$

$$u_2 = -\frac{13}{42} \frac{PL}{\pi E d^2}$$

$$u_3 = -\frac{1}{42} \frac{PL}{\pi E d^2}$$

▪ Calcul des réactions

$$\begin{pmatrix} F_{1x} = R_1 \\ F_{2x} = -P \\ F_{3x} = 0 \\ F_{4x} = R_4 \end{pmatrix} = \frac{\pi E d^2}{4L} \begin{bmatrix} 12 & -12 & 0 & 0 \\ -12 & 13 & -1 & 0 \\ 0 & -1 & 13 & -12 \\ 0 & 0 & -12 & 12 \end{bmatrix} \begin{pmatrix} u_1 = 0 \\ u_2 = \dots \\ u_3 = \dots \\ u_4 = 0 \end{pmatrix}$$

$$F_{1x} = R_1 = \frac{\pi E d^2}{4L} (-12u_2) \dots \dots \dots R_1 = \frac{13 P}{14}$$

$$F_{4x} = R_4 = \frac{\pi d^2 E_2}{4L} (-12u_3) \dots \dots \dots R_4 = \frac{P}{14}$$

2. Les efforts internes dans chaque élément

$$\{f_e\} = [K_e] \{u_e\}$$

➤ **Élément 1**

$$\begin{pmatrix} f_{1x}^1 \\ f_{2x}^1 \end{pmatrix} = \frac{3 \pi E d^2}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{pmatrix} 0 \\ u_2 \end{pmatrix}$$

$$\begin{pmatrix} f_{1x}^1 \\ f_{2x}^1 \end{pmatrix} = \begin{pmatrix} \frac{13 P}{14} \\ -\frac{13 P}{14} \end{pmatrix}$$

Effort de Compression

➤ **Élément 2**

$$\begin{pmatrix} f_{2x}^1 \\ f_{3x}^2 \end{pmatrix} = \frac{\pi E d^2}{4 L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{pmatrix} u_2 \\ u_3 \end{pmatrix}$$

$$\begin{pmatrix} f_{2x}^1 \\ f_{3x}^2 \end{pmatrix} = \begin{pmatrix} -\frac{P}{14} \\ \frac{P}{14} \end{pmatrix}$$

Effort de Traction

➤ **Élément 3**

$$\begin{pmatrix} f_{3x}^3 \\ f_{4x}^3 \end{pmatrix} = \frac{3 \pi E d^2}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{pmatrix} u_3 \\ 0 \end{pmatrix}$$

$$\begin{pmatrix} f_{3x}^3 \\ f_{4x}^3 \end{pmatrix} = \begin{pmatrix} -\frac{P}{14} \\ \frac{P}{14} \end{pmatrix}$$

Effort de Traction