GENERAL NOTES:

1. DETAILS OF CONSTRUCTION NOT SHOWN SHALL BE OF SAME NATURE AS THOSE SHOWN FOR SIMILAR CONDITIONS. REFER TO THE TYPICAL DETAIL SHEETS FOR TYPICAL DETAILS OF CONSTRUCTION. TYPICAL DETAILS APPLY TO ALL CONSTRUCTION UNLESS SPECIFICALLY NOTED OR SHOWN OTHERWISE. WHERE CONDITIONS RE□UIRE MODIFICATIONS OF A TYPICAL DETAIL, THE CONTRACTOR SHALL SUBMIT MODIFIED DETAIL FOR APPROVAL BY THE ENGINEER OF RECORD PRIOR TO FABRICATION AND INSTALLATION. DETAILS OF CONSTRUCTION NOT SHOWN SHALL BE OF SAME NATURE AS THOSE SHOWN FOR SIMILAR CONSTRUCTION.
2. CONTRACTOR SHALL CONSIDER THE PROJECT SPECIFICATIONS A PART OF THE CONTRACT DOCUMENTS. WHERE INFORMATION IS CONFLICTING, SPECIFIC DETAILS SHALL GOVERN OVER TYPICAL DETAILS WHICH SHALL GOVERN OVER THESE NOTES WHICH SHALL GOVERN OVER SPECIFICATIONS.
3. ALL DIMENSIONS ON STRUCTURAL DRAWINGS SHALL BE CHECKED AGAINST ARCHITECTURAL DIMENSIONS. DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE OMITTED OR NOT CLEAR, CONTACT THE ARCHITECT (ARCH) OR STRUCTURAL ENGINEER OF RECORD (SEOR). ALL DIMENSIONS RELATED TO EXISTING CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR. DIMENSIONS ARE TO THE FACE OF STUDS, AND TO CENTERLINE OF COLUMNS UNO.
4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO IMMEDIATELY NOTIFY THE SEOR OF ANY CONFLICTS BETWEEN THE STRUCTURAL DRAWINGS AND OTHER DRAWINGS□OR EXISTING CONDITIONS NOT SHOWN OR DIFFERENT FROM THOSE SHOWN ON DRAWINGS PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE BUILDING THAT IS IN CONFLICT UNTIL THE CONFLICT IS RESOLVED WITH THE AFFECTED PARTIES.
5. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE SHOWN THEY DO NOT INDICATE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE CONSTRUCTION AND ALL ADJACENT PROPERTIES DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT ARE NOT LIMITED TO BRACING, SHORING OF LOADS DUE TO CONSTRUCTION E□UIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT OR SEOR SHALL NOT INCLUDE OBSERVATION OF THE ABOVE ITEMS.
6. SUBSTITUTION RE□UESTS FOR MATERIALS SPECIFIED ON THE STRUCTURAL DRAWINGS MAY BE CONSIDERED WITH MATERIALS HAVING E□UIVALENT OR GREATER CAPACITY AND PERFORMANCE. CURRENT EVALUATION REPORTS AND PRODUCT INFORMATION SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER DEMONSTRATING THE RE□UIRED CAPACITY AND PERFORMANCE OF THE MATERIAL TO BE SUBSTITUTED. WRITTEN APPROVAL FROM THE SEOR SHALL BE OBTAINED PRIOR TO THE SUBSTITUTION OF ANY MATERIAL SPECIFIED ON THE STRUCTURAL DOCUMENTS.
7. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COMPLY WITH THE PERTINENT SECTIONS OF THE "CONSTRUCTION SAFETY ORDERS" ISSUED BY THE STATE OF CALIFORNIA, LATEST EDITION, AND ALL OSHA RE□UIREMENTS AS THEY APPLY TO THIS PROJECT. THE ARCHITECT, SEOR, AND THE OWNER DO NOT ACCEPT ANY RESPONSIBILITY FOR THE CONTRACTOR'S FAILURE TO COMPLY WITH THESE RE□UIREMENTS.
8. ALL WORK IS NEW (N) UNLESS INDICATED AS EXISTING (E).
9. CONSTRUCTION MATERIALS SHALL BE DISTRIBUTED WHEN PLACED ON THE STRUCTURE SUCH THAT LOADS DO NOT EXCEED DESIGN LIVE LOADS OR RESULT IN AN UNBALANCED CONDITION.
10. REFER TO THE PROJECT SPECIFICATIONS FOR SHOP DRAWING RE□UIREMENTS AND SUBMITTALS.
11. CORE DRILLS RE□UIRED SHALL NOT CUT ANY REINFORCING. THE CONTRACTOR IS TO COORDINATE WORK OF ALL TRADES TO ENSURE COMPLIANCE. ALL CORE DRILLS ARE TO BE PRESENTED TO THE IOR FOR VERIFICATION. THE IOR IS TO DOCUMENT CORES EXAMINED INDICATING AN ABSENCE OF REINFORCING.

STRUCTURAL DESIGN CRITERIA:

1. CODES:
   ALL WORK SHALL BE IN CONFORMANCE WITH THE CALIFORNIA BUILDING CODE (CBC) 2016 EDITION, INCLUDING ALL AMENDMENTS. ALL STANDARDS USED SHALL BE THE LATEST VERSION APPROVED BY THE CODE ENFORCEMENT AGENCY ON THE DATE OF THE PERMIT ISSUANCE UNLESS SPECIFICALLY NOTED OTHERWISE.
2. DESIGN LIVE LOADS:

DEAD LOADS (UNREDUCIBLE, UNO):	
a. ROOF	
BUILDING ROOF	20 PSF
CANOPY	15 PSF
LIVE LOADS (UNREDUCIBLE, UNO):	
a. ROOF	
BUILDING ROOF	20 PSF
CANOPY	20 PSF

3. WIND DESIGN INFORMATION:

I □1.0	OCCUPANCY CAT. II	K□ □0.85	K_d □0.85	K□ □1.0
BASIC WIND SPEED V□ □ 110 MPH (3 SEC GUST)			EXPOSURE C	
INTERNAL PRESSURE COEFF. □□/- 0.18				
WALL C&C DESIGN PRESSURE	□□□22.4 PSF □ 15 FT HEIGHT			
	GC□□SEE ASCE7-10 FIG 30.4-1			
	GC□i □□/- 0.18			
PARAPET C&C DESIGN PRESSURE	□□□22.4 PSF □ 15 FT HEIGHT			
	GC□□SEE ASCE7-10 FIG 30.9-1			
	GC□i □□/- 0.00 (NO POROSITY)			

4. SEISMIC DESIGN INFORMATION:

I □1.25	OCCUPANCY CAT. III		SEISMIC DESIGN CATEGORY □E	
S_S □2.301	S_1 □0.768	S_{DS} □1.534	S_{D1} □0.768	
E□UIVALENT LATERAL FORCE PROCEDURE				
SPECIAL REINFORCED MASONRY WALL				
R □5	C_S □0.38	MAX STORY DRIFT Δ = 0.010 h		

FOUNDATION AND SLAB ON GRADE NOTES:

1. SEE SOIL REPORT BY: GEOTECHNICAL ENGINEERING GROUP, CITY OF LOS ANGELES
   JOB NO.: 17-074 DATED: AUGUST 22, 2017
   ADDENDUM: NOVEMBER 9, 2017
   SUPPLEMENT: OCTOBER 25, 2018
2. FOR CONTINUOUS AND PAD FOUNDATIONS,
   ALLOWABLE VERTICAL BEARING PRESSURE: 2500 PSF
   (UNDERLAIN BY AT LEAST 30 INCHES OF COMPACTED SOIL)
   ALLOWABLE VERTICAL BEARING PRESSURE □SHORT TERM: 2500□1.33 □3333 PSF
   (UNDERLAIN BY AT LEAST 30 INCHES OF COMPACTED SOIL)
   ALLOWABLE LATERAL BEARING PRESSURE: 275 PSF PER FT OF DEPTH
   DESIGN COEFFICIENT OF FRICTION FOR SLIDING: 0.35
3. THE BEARING VALUE SHOWN ABOVE IS FOR THE TOTAL OF DEAD AND FRE□UENTLY APPLIED LIVE LOADS AND MAY BE INCREASED BY ON THIRD FOR SHORT DURATION LOADING. ADD RECOMMENDATION FOR POLE FOOTING ON SUPPLIED SOILS REPORT.
4. THE CONTRACTOR SHALL CONFORM TO ALL RECOMMENDATIONS AND CONDITIONS INDICATED IN THE SOIL REPORT. THE GEOTECHNICAL ENGINEER SHALL OBSERVE ALL FOOTING EXCAVATIONS PRIOR TO PLACING CONCRETE.
5. SUBSURFACE SOIL PREPARATION:
   A. ALL EXISTING UNDOCUMENTED FILL SHALL BE REMOVED AND RECOMPACTED. ALL TOPSOILS SHALL BE REMOVED AS RE□UIRED BY THE GEOTECHNICAL ENGINEER.
   B. GEOTECHNICAL ENGINEER SHALL BE RETAINED DURING THE OVEREXCAVATION PROCESS. THE ACTUAL DEPTH OF REMOVAL WILL BE DETERMINED DURING GRADING OPERATIONS.
   C. OFFSITE FILL MATERIAL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT.
6. SPREAD FOOTINGS ARE CENTERED UNDER WALLS AND COLUMNS, UNO.
7. FOOTING ELEVATIONS ARE NOTED ON THE PLANS AND DETAILS AND SHALL BE USED FOR BIDDING. IN ANY CASE, FOOTINGS SHALL BEAR ON FIRM UNDISTURBED SOIL OR ENGINEERED FILL, IN ACCORDANCE WITH THE SOIL REPORT AND DETAILS SHOWN.
8. CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC. ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
9. FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE MECHANICALLY COMPACTED IN LAYERS WITH THE APPROVAL OF THE GEOTECHNICAL ENGINEER. FLOODING IS NOT PERMITTED.
10. ALL TRENCHES SHALL COMPLY WITH APPLICABLE OSHA RE□UIREMENTS.
11. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED BUT NOT BEHIND RETAINING WALLS BEFORE CONCRETE OR MASONRY ATTAINS ITS FULL DESIGN STRENGTH.
12. THE DESIGN OF ALL RETAINING WALLS AND SUBTERRANEAN BUILDING WALLS INDICATED ON THESE DRAWINGS IS BASED ON DRAINED SOILS.
13. CONSTRUCTION JOINTS (CJ) AND SAWCUT (SC) JOINTS IN SLABS SHALL OCCUR WHERE LOCATED ON PLANS AND DETAILS. CJ'S SHALL HAVE FORMED POUR STOPS. CONSTRUCTION JOINTS IN WALLS AND FOOTINGS NEED NOT OCCUR AT THE SAME LOCATION, UNO.
14. SEE ARCHITECT'S PLANS FOR LOCATIONS OF SLAB SLOPES, DEPRESSIONS, CURBS, DRAINS, NON-STRUCTURAL PARTITIONS AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL PLANS.
15. ALL GRADING, FOUNDATION FOOTINGS, AND DRAINAGE PLANS SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER UPON SUBMITTAL. A CERTIFIED LETTER BY THE GEOTECHNICAL ENGINEER IS RE□UESTED STATING THAT THE RECOMMENDATIONS CONTAINED WITHIN THE SOILS REPORT HAVE BEEN INCORPORATED INTO THE PROJECT PLANS AND SPECIFICATIONS PRIOR TO CONSTRUCTION.
16. PRIOR TO THE CONTRACTOR RE□UESTING A BUILDING DEPARTMENT FOUNDATION INSPECTION, THE SOILS ENGINEER SHALL ADVISE THE BUILDING OFFICIAL IN WRITING THAT:
   A. THE BUILDING PAD WAS PREPARED IN ACCORDANCE WITH THE SOILS REPORT.
   B. THE UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED.
   C. THE FOUNDATION EXCAVATIONS COMPLY WITH THE INTENT OF THE SOILS REPORT.

SOIL ENGINEERS APPROVAL ON FOUNDATION PLAN IS RE□UIRED PRIOR TO ISSUANCE OF PERMIT.

EXISTING CONDITIONS NOTES:

1. FIELD VERIFY ALL CONDITIONS & DIMENSIONS PRIOR TO SHOP DRAWING PRODUCTION AND FABRICATION OF STRUCTURAL ELEMENTS.
2. WHERE ALL OTHER EXISTING CONDITIONS VARY SIGNIFICANTLY FROM THOSE SHOWN ON THESE DRAWINGS, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED PRIOR TO CONTINUED CONSTRUCTION RELATED TO SUBJECT CONDITIONS.
3. SHORE ALL EXISTING CONSTRUCTION AS RE□UIRED.
4. ALL EXISTING (E) CONNECTIONS AT ELEMENTS TO BE REPLACED SHALL BE REPLACED OR RE-ATTACHED TO MATCH EXISTING CONDITIONS.
5. VERIFY LOCATION OF EXISTING (E) REBAR BEFORE FABRICATION USING NON-DESTRUCTIVE TESTING.
6. SPECIAL INSPECTION IS RE□UIRED FOR ALL WORK.
7. SEE "AS BUILT" DRAWINGS FOR EXISTING BUILDING DESIGN FOR ITEMS NOT SHOWN OR NOTED.
8. ALL EXISTING (E) WOOD ELEMENTS TO REMAIN SHALL BE FIELD INSPECTED DURING CONSTRUCTION AND TREATED FOR DRYROT REMOVAL / CONTROL. WHERE EXISTING GLB'S TO REMAIN ARE FOUND TO HAVE EXTENSIVE DRYROT DEEPER THAN THE TOP TWO LAMINATIONS (3"), THE STRUCTURAL ENGINEER SHALL BE NOTIFIED PRIOR TO CONTINUED CONSTRUCTION RELATED TO SUBJECT GLB'S.

EXISTING UNDERGROUND UTILITY NOTES:

1. THE ARCHITECT AND ENGINEERS ARE NOT RESPONSIBLE FOR THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES WHETHER OR NOT SHOWN ON THE DRAWINGS. THE LOCATION OF ANY EXISTING UNDERGROUND UTILITIES SHOWN ON THE DRAWINGS ARE APPROXIMATE. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE OWNER SHOULD ANY SUCH UNIDENTIFIED CONDITIONS BE DISCOVERED.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGES WHICH MAY RESULT FROM HIS FAILURE TO EXACTLY LOCATE AND PRESERVE ALL EXISTING UNDERGROUND UTILITIES.
3. BEFORE COMMENCING ANY EXCAVATION, THE CONTRACTOR SHALL OBTAIN AN UNDERGROUND SERVICE ALERT IN□UIRY I.D. NUMBER BY CALLING 1-800-422-4133. TWO WORKING DAYS SHALL BE ALLOWED AFTER THE I.D. NUMBER IS OBTAINED AND BEFORE THE EXCAVATION WORK IS STARTED THAT UTILITY OWNERS CAN BE NOTIFIED.

REINFORCING STEEL NOTES:

1. REINFORCING GRADES FOR CONCRETE OR MASONRY:
   A. ALL BARS EXCEPT THOSE TO BE WELDED.......... ASTM A615, GRADE 60
   B. TIES AND STIRRUPS .... ASTM A615, GRADE 60
   C. WELDED WIRE FABRIC .... ASTM A185
   D. ALL BARS TO BE WELDED .... ASTM A706, GRADE 60
   E. ALL BARS IN THE CONCRETE SHEARWALL INDICATED ON WALL ELEVATION AS "SW" SHOULD BE A706, GRADE 60.
2. MAINTAIN MINIMUM CONCRETE COVER FROM FACE OF CONCRETE TO EDGE OF ALL REINFORCEMENT AS FOLLOWS (UNO): (SEE PLAN/ SECTION FOR CONCRETE W/ FIRE RATING.)

CONDITION	COVER
CONCRETE POURED AGAINST EARTH	3"
CONCRETE POURED IN FORMS AND EXPOSED TO WEATHER OR EARTH	
- #6 BARS AND LARGER	2"
- #5 BARS AND SMALLER	1 1/2"
INTERIOR COLUMNS AND BEAMS	1 1/2"
INTERIOR WALL FACES AND RAISED SLABS	1 1/2"
STRUCTURAL SLABS ON GRADE	
- FROM BOTTOM OF SLAB	2"
- FROM TOP OF SLAB	1 1/2"
OTHER CONCRETE NOT EXPOSED TO WEATHER OR EARTH FOR #11 BARS AND SMALLER	1"

   PROVIDE THE LARGEST COVER RE□UIRED FOR ALL APPLICABLE CONDITIONS. WHERE #3 STIRRUPS OR TIES ARE USED, ENSURE THAT THE COVER FOR LONGITUDINAL BARS IS ADE□UATE.
3. REINFORCEMENT SHALL BE PLACED IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI) "MANUAL OF STANDARD PRACTICE". EACH REINFORCING BAR SHALL BE WIRED TO A CROSS BAR AT A MAXIMUM SPACING OF 24" OC. PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCING IN POSITIONS SHOWN ON THE PLANS.
4. SPLICES IN CONTINUOUS REINFORCEMENT AS USED IN WALLS, WALL FOOTINGS, ETC., SHALL HAVE A CLASS "B" LAP (1'-6" MIN) AND THE SPLICES IN ADJACENT BARS SHALL BE NOT LESS THAN 5'-0" APART. VERTICAL WALL BARS SHALL BE SPLICED AT OR NEAR FLOOR LINES. BARS MAY BE WIRED TOGETHER AT SPLICES OR LAPS EXCEPT FOR TOP REINFORCING OF BEAMS AND SLABS OR WHERE SPECIFICALLY DETAILED TO BE SEPARATED. WELDED WIRE FABRIC SHALL BE LAPPED 12" MINIMUM.
5. ALL DOWELS, ANCHOR BOLTS AND OTHER HARDWARE TO BE SET IN CONCRETE SHALL BE TIED IN PLACE PRIOR TO PLACEMENT OF CONCRETE. NO WET SETTING, STABBING, RODDING OR OTHER MOVEMENT OF EMBEDDED ITEMS SHALL BE PERFORMED DURING PLACEMENT OF CONCRETE.
6. BEND REINFORCING BARS COLD.
7. STEEL SHALL BE KEPT CLEAN AND FREE OF RUST.
8. DOWELS BETWEEN FOOTING AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE AND SPACING AS THE MAIN REINFORCING UNO.
9. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN PLACE INSPECTION IS MADE.
10. CHAIRS OR SPACERS FOR REINFORCING SHALL BE NON-FERROUS OR PLASTIC COATED WHEN RESTING ON EXPOSED SURFACES.

STRUCTURAL STEEL NOTES:

1. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE SPECIFICATIONS AND STANDARD OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC), AS CONTAINED IN THE LATEST EDITION OF "AISC MANUAL OF STEEL CONSTRUCTION".
2. ALL STRUCTURAL STEEL SHALL BE ERECTED PLUMB AND TRUE TO LINE. TEMPORARY BRACING SHALL BE INSTALLED AND SHALL BE LEFT IN PLACE UNTIL OTHER MEANS IS PROVIDED TO ADE□UATELY BRACE THE STRUCTURE.
3. PROVIDE THE FOLLOWING MATERIALS FOR STRUCTURAL STEEL UNO:

STRUCTURAL STEEL GRADES:		
A.	ALL WIDE FLANGE SECTIONS	ASTM A992
B.	S□UARE OR RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS)	ASTM A500, GRADE B (F_y□46 KSI)
C.	ROUND HOLLOW STRUCTURAL SECTIONS (HSS)	ASTM A500, GRADE B (F_y□42 KSI)
D.	PIPES	ASTM A53 TYPE E OR S, GRADE B, (F_y□35 KSI)
E.	PLATES, ANGLES, CHANNELS & TEES	ASTM A36
F.	MACHINE BOLTS (MB)	ASTM A307
G.	HIGH STRENGTH BOLTS (HSB)	ASTM A325 TYPE N
H.	WELDED HEADED STUDS	ASTM A108
I.	THREADED RODS FOR ANCHOR BOLTS	ASTM F1554, GRADE 36

4. EXCEPT AS OTHERWISE NOTED, ALL BOLTS SHALL BE HIGH STRENGTH BOLTS.
5. ALL CONNECTIONS NOT SHOWN SHALL CONFORM TO THE "AISC MANUAL OF STEEL CONSTRUCTION" AND SHALL BE SUBMITTED ON SHOP DRAWINGS FOR REVIEW BY SEOR PRIOR TO FABRICATION.
6. ALL WELDED HEADED STUDS, THREADED STUDS, AND DEFORMED BARS SHALL BE NELSON, OR E□UIVALENT, AND WELDED (IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS BY CERTIFIED WELDERS) SO AS TO FULLY DEVELOP THE TENSILE CAPACITY OF THE CONNECTOR.
7. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE CURRENT EDITION OF THE "AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". SLIP CRITICAL BOLTS (SC) SHALL BE USED FOR ALL "LATERAL FORCE RESISTING SYSTEM" (LFRS) MEMBER STEEL-TO-STEEL CONNECTIONS. TIGHTEN SLIP CRITICAL BOLTS USING ONE OF THE FOLLOWING: TWIST-OFF BOLTS, TENSION CONTROL CALIBRATED WRENCH OR DIRECT TENSION INDICATORS. HIGH STRENGTH BOLTS NOT IN THE LFRS MAY BE INSTALLED HAND TIGHT.
8. BOLTS WITH UPSET THREADS ARE NOT ALLOWED. USE THE APPROPRIATE NUT AND WASHER TYPE FOR THE SPECIFIED BOLT.
9. ALL STEEL FABRICATION SHALL BE PERFORMED BY A LICENSED FABRICATOR.
10. ALL STRUCTURAL STEEL AND MISCELLANEOUS STEEL PERMANENTLY EXPOSED TO THE ELEMENTS SHALL BE HOT DIP GALVANIZED AFTER FABRICATION UNLESS A WEATHER PROOF COATING IS SPECIFIED BY THE ARCHITECT UNO. STAINLESS AND WEATHERING STEELS ARE EXCEPTED WHERE SPECIFIED.
11. SEE ARCHITECTURAL DRAWINGS FOR NAILER HOLES, WELDED STUDS OR OTHER ITEMS NOT SHOWN IN THESE DRAWINGS. WHERE STEEL IS EMBEDDED IN CONCRETE OR MASONRY, PROVIDE HOLES AS RE□UIRED FOR PASSAGE OF CONTINUOUS REINFORCING BARS WHERE INDICATED ON DRAWINGS. DO NOT CUT HOLES IN STRUCTURAL STEEL WITHOUT PRIOR APPROVAL OF SEOR.
12. ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL COMPLY WITH AISC CODE OF STANDARD PRACTICE, SECTION 10.
13. PLACE NON-SHRINK OR DRYPACK GROUT UNDER ALL BASE PLATES AND ALLOW TO CURE BEFORE APPLYING LOADS.

STRUCTURAL CONCRETE NOTES:

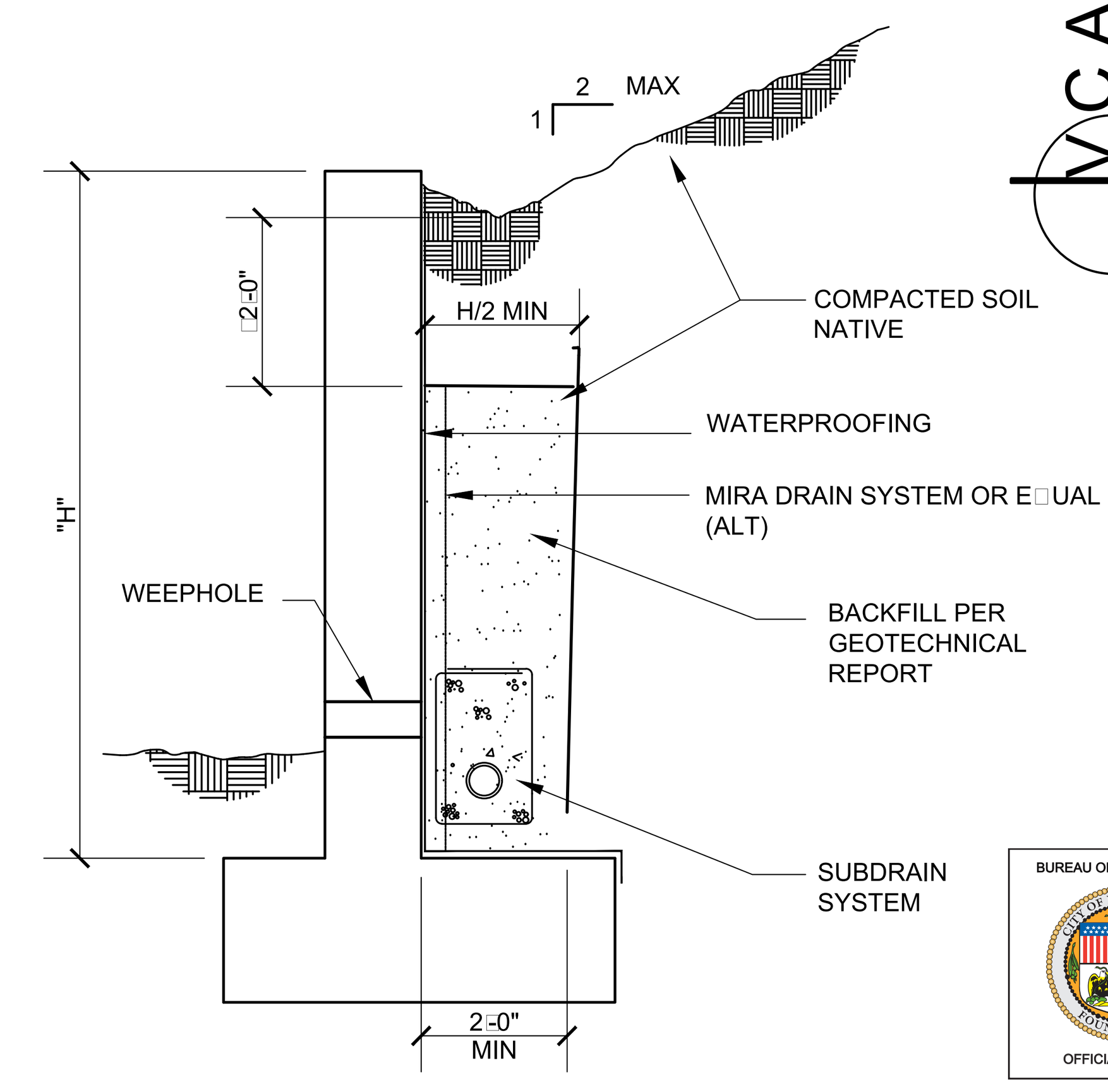
1. CONCRETE SHALL BE MIXED, PLACED AND CURED IN ACCORDANCE WITH ACI 318, LATEST EDITION, AND PROJECT SPECIFICATIONS.
2. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL (AS IN WALLS) SO AS TO CAUSE SEGREGATION OF AGGREGATES. IN SUCH CASES, HOPPERS AND VERTICAL CHUTES OR TRUNKS SHALL BE USED. CHUTES OR TRUNKS SHALL BE OF VARIABLE LENGTHS SO THAT FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED SIX FEET. A SUFFICIENT NUMBER OF CHUTES OR TRUNKS SHALL BE USED TO ENSURE THE CONCRETE IS KEPT LEVEL AT ALL TIMES.
3. CONSTRUCTION JOINTS SHALL BE CLEANED AND ROUGHENED BY REMOVING THE ENTIRE SURFACE TO EXPOSE CLEAN AGGREGATE SOLIDLY EMBEDDED IN THE MORTAR MATRIX. SLUSH WITH A COAT OF NEAT CEMENT BEFORE PLACING CONCRETE. SEE PLANS AND DETAILS FOR LOCATION AND TYPE OF CONSTRUCTION JOINT. LOCATIONS OF ADDITIONAL CONSTRUCTION JOINTS NOT SHOWN ON THESE PLANS SHALL BE SUBMITTED FOR APPROVAL BY THE EOR PRIOR TO PLACING ANY CONCRETE.
4. STRUCTURAL CONCRETE SHALL MEET THE FOLLOWING DESIGN CRITERIA:

LOCATION	MIN 28-DAY COMP STRENGTH	CONC TYPE [a]	MAX AGGR. SIZE	MAX W/C RATIO	MAX SLUMP [□]
FOUNDATION	4000 PSI	NWC	1"	0.45	4"
SURGE PIT	4000 PSI	NWC	1"	0.45	4"
SLAB ON GRADE	3000 PSI	NWC	1"	0.45	4"
ALL OTHER STRUCTURAL CONCRETE NOT NOTED ABOVE	3000 PSI	NWC	1"	0.45	6"
CONC WALL / RET. WALL	4000 PSI	NWC	1"	0.45	4"

   a. MAXIMUM DRY WEIGHT OF LIGHTWEIGHT CONCRETE SHALL BE 115 PCF, UNLESS APPROVED BY SEOR.
   □. SLUMP MEASURED PRIOR TO SUPERPLASTICIZER, WHERE OCCURS.
5. CONCRETE MIX DESIGN AND TESTING SHALL MEET THE RE□UIREMENTS OF THE BUILDING CODE, AND SPECIFICATIONS. ALL CONCRETE MIXES SHALL BE DESIGNED BY A RECOGNIZED TESTING LAB STAMPED AND SEALED BY A LICENSED CALIFORNIA CIVIL ENGINEER AND SUBMITTED TO THE SEOR FOR REVIEW PRIOR TO CONCRETE PLACEMENT. STRUCTURAL CONCRETE MIXES SHALL CONSIST OF 5 SACK MINIMUM UNO.
6. AGGREGATES IN NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C-33 (HARDROCK). AGGREGATES IN LIGHT WEIGHT CONCRETE SHALL CONFORM TO ASTM C-330.
7. COMPRESSIVE STRENGTH TEST REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE SEOR.
8. PORTLAND CEMENT SHALL BE TYPE II FOR ALL CONCRETE CONFORMING TO ASTM C150, LOW ALKALI. MILL TESTS WITH CERTIFICATES OF COMPLIANCE SHALL BE SUBMITTED.
9. FLY ASH OR OTHER POZZOLANS CONFORMING TO ASTM C618 CLASS N OR F MAY BE USED AS A PARTIAL SUBSTITUTION FOR PORTLAND CEMENT UP TO A MAXIMUM OF 25□ TOTAL CEMENTITIOUS MATERIALS BY WEIGHT IF THE MIX DESIGN IS PROPORTIONED PER ACI318,SECTION 5.3.
10. CONCRETE MIXING OPERATIONS, ETC. SHALL CONFORM TO ASTM C94.
11. LEAN CONCRETE, WHERE SPECIFICALLY INDICATED, SHALL CONTAIN 2 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE.
12. DRYPACK OR NONSHRINK GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 5000 PSI, AND CONSIST OF MASTERFLOW 713, FIVE STAR GROUT, SIKA GROUT 212, EMBECO 636, OR APPROVED E□UAL. FOR THICK GROUT LAYERS FOLLOW MANUFACTURER'S GUIDELINES TO ATTAIN THE RE□UIRED STRENGTH, WHICH MAY INCLUDE THE ADDITION OF PEA GRAVEL.
13. DO NOT USE ANY CONCRETE OR GROUT CONTAINING CHLORIDES. WATER USED IN MIX SHALL BE CLEAN AND POTABLE.
14. PRIOR TO ERECTING ANY ELEMENTS THAT LOAD THE FOUNDATION, CONCRETE MUST REACH AN UNCONFINED COMPRESSION STRENGTH OF 2000 PSI MINIMUM AS DETERMINED BY TESTING OR PREVIOUSLY DOCUMENTED DATA FOR THE MIX DESIGN USED UNDER SIMILAR CONDITIONS, AND MUST BE ALLOWED TO CURE FOR A MINIMUM OF 3 DAYS.
15. FOR INTERIOR SLABS-ON-GRADE AND ALL OTHER SLABS RECEIVING ADHERED FLOORING FINISHES (I.E., GLUED, ETC.), THE MAXIMUM W/C RATIO SHALL NOT EXCEED 0.45. CURING COMPOUNDS USED ON CONCRETE THAT IS TO RECIEVE FINISHES SHALL BE COMPATIBLE WITH TILE AND ADHESIVES OR GROUTS IN ACCORDANCE WITH MANUFACTURER'S DATA AND BE APPROVED BEFORE USE.
16. MAINTAIN CONCRETE ABOVE 50 DEGREES FAHRENHEIT AND IN A MOIST CONDITION FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT UNLESS OTHERWISE ACCEPTED BY SEOR.
17. SEE ARCHITECTURAL DRAWINGS FOR WALL OPENINGS, WALL OFFSETS, CHAMFERS, KERFS, DRIPS AND FOR EXTENT OF DEPRESSIONS, RAMPS, ETC. PROVIDE SLEEVES FOR ALL PIPES THROUGH CONCRETE WALLS AND FOOTINGS WHERE SHOWN ON THESE DRAWINGS. CORING IS NOT PERMITTED WITHOUT PRIOR APPROVAL BY THE SEOR.
18. EXPOSED CORNERS OF SLABS, BEAMS, WALLS, COLUMNS, ETC. SHALL BE FORMED WITH 3/4" CHAMFER, UNO.

WELDING NOTES:

1. WELDING PROCEDURES, ELECTRODES AND WELDER □UALIFICATIONS SHALL CONFORM TO THE "CODE FOR WELDING IN BUILDING CONSTRUCTION", AMERICAN WELDING SOCIETY (AWS), D1.1 AND THE AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
2. ALL WELDERS SHALL HAVE EVIDENCE OF PASSING THE AWS STANDARD □UALIFICATION TESTS, AND SHALL BE CERTIFIED FOR THE WORK THEY ARE PERFORMING.
3. PROJECT WELDING SHALL BE PERFORMED ONLY IN ACCORDANCE WITH WELDING PROCEDURE SPECIFICATIONS (WPS) SUBMITTED BY THE CONTRACTOR AND REVIEWED BY THE SEOR AND PROJECT WELDING INSPECTOR. THE WPS SHALL BE IN ACCORDANCE WITH AWS D1.1-D1.4 CURRENT EDITION.
4. WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED PER AWS D1.1 USING E70XX ELECTRODES UNLESS OTHERWISED NOTED.
5. WELDING OF REINFORCING BARS SHALL BE PERFORMED PER AWS D1.4 USING E90XX ELECTRODES.
6. ALL FULL PENETRATION WELDS SHALL BE ULTRA-SONIC TESTED PER AWS D1.1.
7. ALL GROOVE OR BUTT WELDS SHALL BE COMPLETE PENETRATION WELDS. ALL EXPOSED BUTT WELDS SHALL BE GROUND SMOOTH.
8. ALL EXPOSED WELDS ON ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL COMPLY WITH AISC CODE OF STANDARD PRACTICE, SECTION 10.
9. FIELD WELDS HAVE BEEN INDICATED WHERE THEY ARE EXPECTED TO OCCUR. THE CONTRACTOR SHALL DETERMINE THE ACTUAL FIELD WELDING NECESSARY TO COMPLETE THE PROJECT AND INCLUDE ALL ASSOCIATED COSTS WITHIN THE BASE BID.



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Project No. D312

BUREAU OF ENGINEERING
OFFICIAL RECORD

METAL DECK NOTES AND SCHEDULE (FILLED & UNFILLED):

1. MATERIAL FOR METAL DECK SHALL HAVE A MIN YIELD STRENGTH OF 38 KSI AND CONFORM TO ASTM A653-SS GRADE 33 WITH GALVANIZED G60 COATING COMPLYING WITH ASTM A525.
2. SEE TYPICAL DETAILS FOR REINFORCING OF DECK AROUND OPENINGS. CONTRACTOR SHALL COORDINATE SIZE AND LOCATIONS OF OPENINGS WITH THE VARIOUS TRADES. NO LOADS SHALL BE HUNG FROM DECK WITHOUT APPROVAL OF SEOR.
3. FLOOR AND ROOF DECK IS DESIGNED FOR UNSHORED CONSTRUCTION, UNO. MAINTAIN 3 SPAN CONDITION WHEREVER POSSIBLE (2 SPAN MIN) EXCEPT AT STAIR LANDING AND WHERE NOTED OTHERWISE ON PLANS.
4. PROVIDE 2" MINIMUM BEARING AT ALL SUPPORTS. END LAPS OF METAL DECK SHALL BE A MINIMUM OF 2" AND SHALL OCCUR ONLY OVER SUPPORTS. DECK SHALL BE LAID OUT SO THAT A LOW FLUTE FALLS ON EACH PARALLEL SUPPORT.
5. INSTALL DECK BY WELDING. USE 3/4" DIAMETER PUDDLE WELDS OR WELDED STUDS TO SUPPORTS SPACED AS SHOWN ON CONSTRUCTION DRAWINGS. SPACING FOR TOP SEAM, SIDE SEAM, BUTTON PUNCH, OR PUNCHLOK CONNECTION SHALL BE IN ACCORDANCE WITH DRAWINGS. SEE TYPICAL METAL DECK DETAILS..
6. SUBMIT SHOP DRAWINGS FOR METAL DECK TO THE SEOR FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW TYPE OF DECK, LAYOUT OF DECK, THE SIZE AND LOCATION OF ANY OPENINGS OF WIDTH GREATER THAN 1'-0", AND ATTACHMENT METHOD.
7. ALTERNATES TO TYPE OF DECK AND FASTENING MAY BE USED WITH THE APPROVAL OF THE SEOR AND DSA. DECK PROPERTIES SHALL BE E□UAL TO OR GREATER THAN THOSE SHOWN ON THE PLANS. ANY DECK OR METHOD OF FASTENING SHALL HAVE AN EVALUATION REPORT APPROVING THE DECK FOR THE APPLICATION.
8. METAL DECK WITH CONCRETE FILL SHALL HAVE POSITIVE VENTING. DO NOT EMBED PIPES, SLEEVES, CONDUIT, ETC IN CONCRETE TOPPING UNO.

LADBS DEPARTMENT OF BUILDING AND SAFETY — Los Angeles Regional Uniform Code Program — LARUCP
Committee I-3: Structural Observation

STRUCTURAL OBSERVATION PROGRAM
AND DESIGNATION OF THE
STRUCTURAL OBSERVER

PROJECT ADDRESS: 16730 CHATSWORTH STREET GRANADA HILLS, CA 91344 PERMIT APPL. NO.:
Description of Work: ADDITION TO (E)POOL HOUSE, (N)POOL EQUIPMENT BUILDING
Owner: Architect: JOHN SPARANO Engineer: YOUNG K. NAM

STRUCTURAL OBSERVATION (only checked items are required)
Firm or Individual to be responsible for the Structural Observation:
Name: YOUNG K. NAM Phone: (949) 679-0870 Calif. Registration: S4029

FOUNDATION	WALL	FRAME	DIAPHRAGM
☑ Footing, Stem Walls, Piers	☐ Concrete	☐ Steel Moment Frame	☐ Concrete
☐ Mat Foundation	☑ Masonry	☐ Steel Braced Frame	☑ Steel Deck
☐ Caisson, Piles, Grade Beams	☐ Wood	☐ Concrete Moment Frame	☐ Wood
☐ Stepp g/Retain g Foundation, Hillside Special Anchors	☐ Others:	☐ Masonry Wall Frame	☐ Others:
☐ Others:		☑ Others: CANOPY	

DECLARATION BY OWNER
I, the Owner of the project, declare that the above listed firm or individual is hired **by me** to be the Structural Observer.

Signature Date

DECLARATION BY ARCHITECT OR ENGINEER OF RECORD (required if the Structural Observer is different from the Architect or Engineer of Record)
I, the Architect or Engineer of record for the project, declare that the above listed firm or individual is designated by me to be responsible for the Structural Observation.

Signature License No. S4029 Date 09/30/2020

IN/Form.08 (Part 2) (Rev. 1/1/2007) www.ladbs.org

REGISTERED PROFESSIONAL ENGINEER YOUNG-KEUN NAM No. S4029 Exp. 9/30/20 STRUCTURAL STATE OF CALIFORNIA

TTLB TEMPLATE REVISION DATE: 02/2013
SHEET ISSUE DATE: 04/08/14
FILE PATH: C:\Users\Y□in\Do□□ents\S-GRANADA HILL-V17_Y□in.rvt
REVISION DATES (DESIGN STAGE ONLY)

BID SET
PROJECT ISSUE DATE: 06/22/21
CITY OF LOS ANGELES
DEPARTMENT OF PUBLIC WORKS
BUREAU OF ENGINEERING
ENGINEERING CITY OF LOS ANGELES

NO.	REVISION DESCRIPTION	DATE	BY

BUILDING NO. -
INDEX NO. RP-300118

GARY LEE MOORE, PE, ENV SP — CITY ENGINEER

	DATE:
ARCHITECTURAL DIVISION	11/01/18
ARCHITECT/ENGINEER: JHON SPARANO LIC. NO.: LICENSE	11/01/18
DESIGNED BY: SPARANO □MOONEY ARCHITECTURE	11/01/18
DRAWN BY: ALEX, □	11/01/18
CHECKED BY: YOUNG, NAM	11/01/18
APPROVED BY:	

VERTICAL CONTROL:
HORIZONTAL CONTROL:
SHEET TITLE: GENERAL NOTES
PROJECT: GRANADA HILLS POOL AND BATH HOUSEREPLACEMENT PROJECT
ADDRESS: 16730 CHATSWORTH STREETGRANADA HILLS, CA 91344

WORK ORDER NO. #E170517
PLAN FILE NO.
DRAWING NO. S0.01
SHEET OF SHEETS

ROUGH CARPENTRY / WOOD NOTES:

1. ALL GRADES SPECIFIED ARE MINIMUM GRADES RE□UIRED.
2. DOUGLAS FIR (DF) SHALL BE GRADED BY THE WESTERN WOOD PRODUCTS ASSOCIATION GRADING RULES, AND ASTM D245.
3. REDWOOD SHALL BE GRADED BY THE CALIFORNIA REDWOOD ASSOCIATION, REDWOOD INSPECTION SERVICE.
4. SILL PLATES SHALL BE PRESSURE-TREATED (PT) DOUGLAS FIR #2. REDWOOD IS PERMITTED WITH SEOR APPROVAL.
5. NON-LOAD BEARING STUDS, TOP PLATES, BLOCKING, FURRING AND BRACING SHALL BE.......................DF #2
   JOISTS, RAFTERS, PURLINS, BEAMS & POSTS SHALL BE ....DF #1 (UNO)
   LOAD BEARING STUDS SHALL BE: (UNO ON PLAN)
   STUDS 0-15□.......................DF #2
   STUDS □15□.......................DF #1
   BLOCKING □ I-JOIST SHALL BE 1 ¾" THK PARALLAM PSL BY WEYERHOUSER OR E□. (1.55E GRADE) ICC-ESR-1387
6. MOISTURE CONTENT OF SAWN LUMBER SHALL NOT EXCEED 19□ WHEN FRAMING STARTS OR SHEATHING IS APPLIED. ANY NONCOMPLIANT WORK SHALL BE REJECTED AND REFRAMED WITH ACCEPTABLE LUMBER.
7. TIMBERS 4" NOMINAL IN THE LEAST DIMENSION SHALL NOT CONTAIN BOXED HEART.
8. SILL PLATES SHALL BE PRESSURE-TREATED AND SHALL BE BOLTED TO CONCRETE WITH 5/8" DIAMETER ANCHOR BOLTS AT 32" OC MAX, UNO WITH A BOLT BETWEEN 6" TO 9" FROM THE END OF EACH PIECE OF SILL (2 BOLTS MIN EACH PIECE). PIECE OF SILL SHALL BE CONSIDERED ENDED WHERE PLATE IS CUT OUT OVER ONE-THIRD OF CROSS-SECTION.
9. ANCHOR BOLTS FOR NON-STRUCTURAL WALLS SUPPORTED ON SLABS SHALL HAVE 3 1/2" EMBEDMENT (UNO) MEASURED FROM TOP OF SLAB.
10. ANCHOR BOLTS FOR STRUCTURAL WALLS SHALL HAVE 12" EMBEDMENT (UNO) MEASURED FROM TOP OF SLAB.
11. STUD BEARING WALLS AND PARTITIONS SHALL HAVE DOUBLE TOP PLATES LAPPED AT WALL AND PARTITION INTERSECTIONS. JOINTS IN UPPER AND LOWER MEMBERS OF DOUBLE TOP PLATES SHALL BE STAGGERED AT LEAST 4□0".
12. HOLES IN WOOD AND STEEL MEMBERS FOR BOLTS SHALL BE THE NOMINAL BOLT DIAMETER PLUS 1/16".
13. ALL BOLTS IN WOOD SHALL BE ASTM A307 STANDARD BOLTS, UNO. BOLTS AND SCREWS SHALL BE TIGHTENED AT TIME OF ERECTION AND RETIGHTENED BEFORE CLOSING IN OR AT THE COMPLETION OF THE JOB.
14. HOLES IN WOOD FOR LAG SCREW SHANK SHALL BE BORED TO THE SAME DIAMETER AND DEPTH AS THE SHANK, AND FOR THE THREADED PORTION BORED WITH A BIT NOT LARGER THAN THE BASE OF THREADS.
15. LAG SCREWS AND SCREWS SHALL BE SCREWED AND NOT DRIVEN INTO PLACE.
16. STEEL WASHERS SHALL BE PROVIDED UNDER HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS WHICH BEAR ON WOOD. STANDARD CUT WASHERS MAY BE USED IN ALL CASES EXCEPT SILL PLATES AND WOOD LEDGERS AGAINST CONCRETE OR MASONRY, WHICH SHALL BE ONE OF THE FOLLOWING SIZES:

BOLT & LAG SCREW DIAMETER	STEEL PLATE WASHER SIZE	MALLEABLE IRON WASHER SIZE
1/2"	2" S□ □1/4"	2 1/2"Ø x 1/4"
5/8"	2 1/2" S□ □1/4"	2 3/4Ø x 5/16"
3/4"	3" S□ □5/16"	3"Ø x 7/16"
7/8"	3 1/2" S□ □3/8"	3 1/2"Ø x 7/16"
1"	3 3/4" S□ □3/8"	4"Ø x 1/2"

NOTE: WASHERS UNDER CARRIAGE BOLT HEADS SHALL BE LARGE ENOUGH TO ALLOW FOR S□UARE SHOULDERS.

17. INSTALL WINDOWS AND DOORS IN STUD WALLS AFTER DEAD LOADS ARE APPLIED, AND PROVIDE A 1/2" SHIM SPACE AT THE HEAD CONDITION.
18. FASTENERS, INCLUDING NAILS, NUTS AND WASHERS, IN CONTACT WITH PRESERVATIVE - TREATED WOOD SHALL BE OF HOT-DIPPED ZINC COATED GALVANIZED STEEL OR STAINLESS STEEL.

TYPICAL WOOD FRAMING NOTES:

1. SEE THIS SHEET FOR TYPICAL NOTES, DETAILS, AND NAILING.
2. ONLY LOAD BEARING AND/OR SHEAR WALLS ARE SHOWN. SEE ARCHITECTURAL DRAWINGS FOR ALL OTHER PARTITIONS.
3. ELEVATIONS SHOWN ON PLANS ARE FROM FINISHED FIRST FLOOR, DATUM ELEVATION □0□0".
4. ALL EXTERIOR WALLS AND INTERIOR BEARING WALLS ARE 2□6 STUDS □ 16"OC TYPICAL, UNO.
5. ALL EXTERIOR STUD WALLS SHALL BE COMPLETELY SHEATHED WITH 15/32" APA RATED SHEATHING, EXPOSURE-1 (32/16) UNO, REGARDLESS SHEAR WALL LENGTH SHOWN ON PLAN.
6. INTERIOR STRUCTURAL SHEATHED WALLS (SHEAR WALL) ARE SHOWN ON PLAN ABOVE AND BELOW ANY OPENING AND ANY WALL ON THE SAME LINE OF SHEAR WALL SHALL BE SHEATHED WITH THE SAME PLYWOOD SHEATHING W/ 10d NAILS □ 6" & 12" O/C (EN, BN). BLOCK ALL PLYWOOD EDGES.
7. ALL STRUCTURAL WALL SHEATHING IS SPLICED ON 2" NOMINAL BLOCKING AT HORIZONTAL JOINTS, UNO.
8. STRUCTURAL FLOOR AND ROOF SHEATHING SHALL BE AS SPECIFIED BELOW. NOTE: 1/8" GAP SHALL BE PROVIDED BETWEEN ADJACENT PANELS. PANELS WITH GRADE STAMP INDICATION "SIZED FOR SPACING" MAY BE USED TO FACILITATE THIS RE□UIREMENT. SEE PLANS FOR RE□UIRED BOUNDARY AND EDGE NAILING NOT LISTED BELOW. FLOOR SHEATHING SHALL BE GLUED AND NAILED PER PLAN.

   (□- SHEATHING □ EXTERIOR DECKS SHALL BE EXTERIOR RATED PLYWOOD.)

   BN□BOUNDARY NAILING
   EN□EDGE NAILING
   FN□FIELD NAILING
9. SEE PLANS FOR LOCATION OF NOMINAL 2X FLAT BLOCKING AT ROOF AND FLOOR SHEATHING PANEL JOINTS. SEE THIS SHEET FOR PLY CLIP RE□UIREMENTS AT UNBLOCKED ROOF PANEL JOINTS. T&G FLOOR SHEATHING IS NOT RE□UIRED WHERE JOINTS ARE BLOCKED.
10. BRIDGING SHALL BE 2□3 NOMINAL, OR SIMPSON "NB" AND SHALL OCCUR AS SHOWN ON PLANS OR SHEET S0.02.
11. SAWN LUMBER GRADING SHALL BE PER "TIMBER SPECIFICATIONS" NOTED ON THESE PLANS.
12. (S) INDICATES METAL FRAMING CONNECTORS MANUFACTURED BY SIMPSON STRONG TIE COMPANY (CURRENT CATALOG) OR "USP" WITH E□UIVALENT ICC PUBLISHED VALUES AND SHALL BE INSTALLED PER SPECIFICATIONS, NO EXCEPTIONS.

NAILING SCHEDULE

(UNLESS OTHERWISE NOTED ON PLANS)

NAIL SPACING TO BE NOT LESS THAN RE□UIRED PENETRATION. EDGE AND END DISTANCES SHALL BE NOT LESS THAN HALF THIS SPACING. ALL SPACING AND EDGE AND END DISTANCES SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD. HOLES FOR NAILS, WHERE NECESSARY TO PREVENT SPLITTING, SHALL BE BORED OF A DIAMETER SMALLER THAN THAT OF THE NAILS. COMMON OR BOX NAILS MAY BE USED FOR NAILING AT TYPICAL CONNECTIONS NOTED BELOW (U.N.O.). FOR ALL CONNECTIONS OTHERWISE NOTED OR DETAILED ON PLANS, COMMON NAILS SHALL BE USED (SEE SCHEDULE BELOW).

CONNECTION, NAIL TYPE	NAILING
JOISTS TO SILL OR GIRDER, TOENAIL	(3) 8d
BRIDGING TO JOIST, TOENAIL EACH END	(2) 8d
1" □6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	(2) 8d
WIDER THAN 1" □6" SUBFLOOR TO EACH JOIST, BLIND & FACE NAIL	(3) 8d
2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	(2) 16d
SILL PLATE TO JOIST OR BLOCKING, FACE NAIL	16d □ 16" OC
TOP PLATE TO STUD, END NAIL	(2) 16d
STUD TO SILL PLATE	(4) 8d, TOENAIL OR (2) 16d, END NAIL
DOUBLE STUDS, FACE NAIL	16d □ 24" OC
DOUBLE TOP PLATES, FACE NAIL	16d □ 16"OC (COMMON)
TOP PLATES, LAPS, FACE NAIL	(8) 16d (UNO) (18) 16d □ SHEARWALL LOCATIONS
TOP PLATES AT INTERSECTIONS, FACE NAIL	(2) 16d
CONTINUOUS HEADER, TWO PIECES	16d □ 16" OC ALONG EACH EDGE
CEILING JOISTS TO PLATE, TOENAIL	(3) 8d
CONTINUOUS HEADER TO STUD, TOENAIL	(4) 8d
CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	(3) 16d
CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	(3) 16d
RAFTER TO PLATE, TOENAIL	(3) 8d
1" BRACE TO EACH STUD & PLATE, FACE NAIL	(2) 8d
1"□8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	(2) 8d
WIDER THAN 1"□8" SHEATHING TO EACH BEARING, FACE NAIL	(3) 8d
BUILT-UP CORNER STUDS	16d □ 24" OC
BUILT-UP GIRDERS AND BEAMS	20d □ 32" OC AT TOP, BOTT & STGRD (2) 20d AT ENDS AND □ EA SPLICE (2) 16d □ EA BEARING

NAIL SCHEDULE (COMMON NAILS)		
SIZE	DIAMETER (IN)	LENGTH (IN)
8d	0.131	2 ½
10d	0.148	3
12d	0.148	3 1/4
16d	0.162	3 1/2
20d	0.192	4

SHORTENED 10d COMMON NAILS MAY BE USED TO FASTEN WOOD STRUCTURAL PANELS UNO. USE THE FOLLOWING MINIMUM LENGTHS: 10d □2 ¼" FOR 15/32" OR THINNER PANELS, 10d □2 ⅜" FOR 19/32" PANELS, AND FULL LENGTH FOR ⅝" OR THICKER PANELS.

COLD-FORMED STEEL FRAMING NOTES:

1. DESIGN, FABRICATION AND ERECTION OF COLD-FORMED STEEL FRAMING SHALL CONFORM TO THE SPECIFICATIONS AND STANDARD OF THE AMERICAN IRON AND STEEL INSTITUTE (AISI), AS CONTAINED IN THE "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", LATEST EDITION, INCLUDING ALL APPLICABLE AMENDMENTS.
2. ALL COLD-FORMED STEEL FRAMING SHALL BE ERECTED PLUMB AND TRUE TO LINE. TEMPORARY BRACING SHALL BE INSTALLED AND LEFT IN PLACE UNTIL OTHER MEANS IS PROVIDED TO ADE□UATELY BRACE THE STRUCTURE.
3. COLD-FORMED STEEL GRADES:
   A. 18 GA (43 MILS) OR THINNER ....ASTM A1003 GRADE 33 (FY □33 KSI)
   B. 16 GA (54 MILS) AND THICKER ....ASTM A1003 GRADE 50 (FY □50 KSI)
4. ALL COLD-FORMED STEEL FRAMING SHALL BE BRACED AS RE□UIRED BY SECTION D3 OF THE AISI SPECIFICATION.
5. SUBMIT COLD-FORMED STEEL FRAMING SHOP DRAWINGS AND SPECIFICATIONS TO THE SEOR FOR REVIEW PRIOR TO FABRICATION.
6. COLD-FORMED STEEL STUDS AND TRACKS ARE TO BE ATTACHED WITH SHEET METAL SCREWS (SMS) WITH SIZES CALLED OUT ON THE DETAILS. PENETRATION OF SCREWS THROUGH JOINED MATERIAL SHOULD NOT BE LESS THAN 3 EXPOSED THREADS. SCREWS ARE TO BE INSTALLED AND TIGHTENED IN ACCORDANCE WITH SCREW MANUFACTURER□S RECOMMENDATIONS.

CONCRETE MASONRY NOTES:

1. CONCRETE MASONRY UNITS (CMU) SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY PRISM COMPRESSIVE STRENGTHS IN ACCORDANCE WITH THE BUILDING CODE:

MINIMUM 28 DAY COMPRESSIVE STRENGTH			
LOCATION	□□	TYPE S MORTAR PER ASTM C270	GROUT PER CBC 2103A.13
CMU U.N.O.	2500 □si	2500 □si	2500 □si

2. CONCRETE BLOCK SHALL CONFORM TO ASTM C90 MEDIUM WEIGHT. USE OF LIGHT WEIGHT BLOCK RE□UIRES PRIOR WRITTEN APPROVAL BY THE SEOR.
3. PRISM TEST SHALL BE PERFORMED FOR CMU WITH □□ OVER 2000 PSI FOR CBC 2105A.2
4. VERTICAL REINFORCING SHALL BE FULL HEIGHT OF WALL AND SHALL BE BRACED AT 6□8" MAXIMUM TO PREVENT MOVEMENT WHILE GROUTING.
5. HORIZONTAL REINFORCING SHALL BE IN BOND BEAM UNITS AND TIED SECURELY TO VERTICAL REINFORCING.
6. DOWELS, ANCHORS, AND OTHER EMBEDDED ITEMS SHALL BE TIED SECURELY IN PLACE TO PREVENT MOVEMENT WHILE GROUTING. WET SETTING OR STABBING IS NOT ALLOWED.
7. MAXIMUM GROUT LIFTS SHALL NOT EXCEED 8□0" AND CLEANOUTS AT THE BOTTOM OF ALL CELLS SHALL BE USED UNLESS THE LIFT IS 4□0" OR LESS. THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING. GROUT FOR EACH POUR SHALL BE STOPPED 1 1/2" BELOW THE TOP OF A BLOCK COURSE EXCEPT AT THE FINAL COURSE. ALL GROUT SHALL BE THOROUGHLY CONSOLIDATED BY VIBRATING IMMEDIATELY AFTER PLACING. SHAKING OR RODDING REBAR IS NOT ALLOWED. FILL ALL CELLS WITH GROUT.
8. BLOCK SHALL BE PLACED IN RUNNING BOND AND SHALL BE 8"□8"□16" NOMINAL UNITS, UNO. WHERE BLOCK IS RE□UIRED TO BE PLACED IN STACK BOND (SEE ARCH), OPEN-ENDED UNITS (I.E., "SPEED BLOCK") SHALL BE USED.
9. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE SAFETY OF LIFT HEIGHT FOR OPEN ENDED OR FIELD MODIFIED BLOCKS.
10. LAYOUT OF MASONRY BLOCK UNITS SHALL BE RUNNING BOND, U.N.O. BLOCK MODULES/MORTAR JOINTS SHOWN ON THESE DRAWINGS ARE FOR PRESENTATION PURPOSES ONLY, AND NOT INTENDED TO SUPERCEDE ARCHITECTURAL DESIGN RE□UIREMENTS.
11. CMU WALL REINFORCEMENT SEE PLANS & ELEVATIONS
12. PROVIDE VERTICAL CONTROL JOINTS IN CMU WALLS AS SHOWN ON PLAN. UNLESS NOTED OTHERWISE VERTICAL CONTROL JOINTS SHALL OCCUR AT 25□0" OC MAXIMUM ALONG WALL LENGTH, AT FOUNDATION STEPS, FLOOR OR ROOF JOINTS, WALL HEIGHT CHANGES, AND 24" MINIMUM PAST ONE SIDE OF OPENINGS □6□0" WIDE, UNO ON PLANS.
13. WHEN THE AMBIENT TEMPERATURE FALLS BELOW 40°F, OR EXCEEDS 100°F, PROVSIONS OF TMS602 / ACI 530 / ASCE6. ARTICLE 1.8C OR ARTICLE 1.8D SHALL BE IMPLEMENTED.

ADDITIONAL NOTES PER LADBS:

1. CONTRACTORS RESPONSIBLE FOR THE CONSTRUCTION OF A WIND OR SEISMIC FORCE RESISTING SYSTEM/COMPONENT LISTED IN THE "STATEMENT OF SPECIAL INSPECTION" SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE LABDS INSPECTORS AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON SUCH SYSTEM OR COMPONENT PER SEC 1709.1.
2. CONTINUOUS SPECIAL INSPECTION BY A REGISTERED DEPUTY INSPECTOR IS RE□UIRED FOR FIELD WELDING, CONCRETE STRENGTH F□C □2500 PSI, HIGH STRENGTH BOLTING, SPRAYED-ON FIREPROOFING, ENGINEERED MASONRY, HIGH-LIFT GROUTING, PRE-STRESSED CONCRETE, HIGH LOAD DIAPHRAGMS AND SPECIAL MOMENT-RESISTING CONCRETE FRAMES.
3. FOUNDATION SILLS SHALL BE NATURALLY DURABLE OR PRESERVATIVE-TREATED WOOD.
4. A COPY OF THE LOS ANGELES RESEARCH REPORT AND/OR CONDITIONS OF LISTING SHALL BE MADE AVAILABLE AT THE JOB SITE.
5. POWER-DRIVEN FASTENERS SHALL NOT BE USED TO ANCHOR SILL PLATES EXCEPT AT INTERIOR NONBEARING WALLS NOT DESIGNED AS SHEAR WALLS.
6. EXTERIOR ANCHOR BOLTS AND POST BASES SHALL BE GALVANIZED AND EACH ANCHOR BOLTS SHALL HAVE AT LEAST TWO GALVANIZED NUTS ABOVE THE BASE PLATE.
7. THE TOP OF EXTERIOR PEDESTALS MUST BE SLOPED FOR POSITIVE DRAINAGE.
8. ALL MAIN FOOTING AND GRADE BEAM REINFORCEMENT STEEL SHALL BE BENT INTO THE INTERSECTING FOOTING AND FULLY DEVELOPED AROUND EACH CORNER AND INTERSECTION.
9. CONTINUOUS INSPECTION BY A LOS ANGELES CITY LICENSED DEPUTY INSPECTOR IS RE□UIRED FOR ALL STRUCTURAL CONNECTIONS, FOOTINGS, GRADE BEAMS AND RETAINING WALLS DURING INSTALLATION.
10. STRUCTURAL OBSERVATION BY THE ENGINEER OR ARCHITECT OF RECORD IS RE□UIRED IN ACCORDANCE WITH LA INFORMATION BULLETIN P/BC 2008-024.
11. FIELD WELDING TO BE DONE BY WELDERS CERTIFIED BY LADBS FOR STRUCTURAL STEEL, REINFORCING STEEL & LIGHT GAUGE STEEL. CONTINUOUS INSPECTION BY A DEPUTY INSPECTOR IS RE□UIRED.
12. SHOP WELDS MUST BE PERFORMED IN A LICENSED LADBS FABRICATOR□S SHOP.
13. LADBS LICENSED FABRICATOR IS RE□UIRED FOR STRUCTURAL STEEL.
14. GLUE-LAMINATED TIMBERS MUST BE FABRICATED IN A LADBS LICENSED SHOP. IDENTIFY GRADE SYMBOL AND LAMINATION SPECIES PER 2015 NDS SUPPLEMENT TABLE 5A.
15. ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING. FACE GRAIN OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORTS. FLOOR SHALL HAVE TONGUE AND GROOVE OR BLOCKED PANEL EDGES. PLYWOOD SPANS SHALL CONFORM WITH TABLE 2304.8(1).
16. ALL DIAPHRAGM AND SHEAR WALL NAILING SHALL UTILIZE COMMON NAILS OR GALVANIZED BOX.
17. ALL BOLT HOLES SHALL BE DRILLED 1/32□TO 1/16" OVERSIZED.
18. CONSTRUCTION SHALL COMPLY WITH PART 3 OF TMS 602-13/ACI 530.1-13/ASCE 6-13.
    A. REINFORCEMENT SHALL BE SUPPORTED TO PREVENT DISPLACEMENTS BEYOND THE TOLERANCES PRIOR TO GROUTING. (3.4B OF TMS 602-13/ACI 530.1-13/ASCE 6-13)
    B. CLEANOUTS SHALL BE PROVIDED FOR ALL GROUT POURS OVER 5□4". (3.2 F OF TMS 602-13/ACI 530.1-13/ASCE 6-13)
    C. GROUT LIFT HEIGHT SHALL NOT EXCEED 12□8" WHEN THE MASONRY HAS CURED FOR 4-HRS., THE GROUT SLUMP IS MAINTAINED BETWEEN 10 AND 11IN., AND NO INTERMEDIATE REINFORCED BOND BEAMS ARE PLACED BETWEEN THE TOP AND BOTTOM OF THE POUR HEIGHT. OTHERWISE GROUT LIFT HEIGHT SHALL NOT EXCEED 5□4". (3.5 D OF TMS 602-13/ACI 530.1-13/ASCE 6-13)
    D. ALL CELLS AND SPACED CONTAINING REINFORCEMENT SHALL BE FILLED WITH GROUT.
19. PIPES AND CONDUITS EMBEDDED IN MASONRY SHALL NOT REDUCE THE RE□UIRED STRENGTH. (3.2.2)
20. JOINT REINFORCEMENT USED IN MASORNY EXPOSED TO EARTH OR WEATHER SHALL BE STAINLESS STEEL OR PROTECTED FROM CORROSION BY MILL GALVINAZED, HOT-DIP GALVANIZED, OR EPOXY COATING. (6.4.2 AND 6.4.3)

POST INSTALLED ANCHORS:

1. ANCHOR DIAMETER REFERS TO THE THREAD SIZE FOR THE WEDGE & SHELL CATEGORIES AND TO THE ANCHOR OUTSIDE DIAMETER FOR THE SLEEVE CATEGORY.
2. APPLY PROOF TEST LOADS TO WEDGE & SLEEVE ANCHORS WITHOUT REMOVING THE NUT IF POSSIBLE. IF NOT, REMOVE NUT & INSTALL A THREADED COUPLER TO THE SAME TIGHTNESS OF THE ORIGINAL NUT USING A TOR□UE WRENCH & APPLY LOAD.
3. FOR SLEEVE/SHELL INTERNALLY THREADED CATEGORIES, VERIFY THAT THE ANCHOR IS NOT PREVENTED FROM WITHDRAWING BY A BASEPLATE OR OTHER FIXTURES. IF RESTRAINT IS FOUND, LOOSEN AND SHIM OR REMOVE FIXTURE(S) PRIOR TO TESTING.
4. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY THE FIXTURE(S).
5. SHELL TYPE ANCHORS MUST BE TESTED AS FOLLOWS: VISUALLY INSPECT 25□ FOR FULL EXPANSION AS EVIDENCED BY THE LOCATION OF THE EXPANSION PLUG IN THE ANCHOR BODY. PLUG LOCATION OF A FULLY EXPANDED ANCHOR SHOULD BE AS RECOMMENDED BY THE MANUFACTURER, OR, IN THE ABSENCE OF SUCH RECOMMENDATION, AS DETERMINED ON THE JOB SITE FOLLOWING THE MANUFACTURER□S INSTALLATION INSTRUCTIONS, AND□PROOF LOAD 5□ AS INDICATED IN THE TABLE ABOVE, BUT NOT LESS THAN THREE ANCHORS PER DAY FOR EACH DIFFERENT PERSON OR CREW INSTALLING ANCHORS, OR□TEST 50□ OF THE INSTALLED ANCHORS PER 2624(d).
6. TEST E□UIPMENT IS TO BE CALIBATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.
7. TOR□UE TEST VALUES FOR SHELL TYPE ANCHORS ARE OMMITTED DUE TO LACK OF DATA. TOR□UE TESTING CAN OCCUR ON AN INDIVIDUAL BASIS WHEN TEST PROCEDURES ARE SUBMITTED AND APPROVED BY THE ENFORCEMENT AGENCY. TABULATED VALUES MAY BE FORTHCOMING ONCE THE ENFORCEMENT AGENCY HAS MORE DATA TO EVALUATE THE FEASIBLITY OF STANDARD TOR□UE VALUES.
8. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:
   A. HYDRAULIC RAM METHOD: THE ANCHOR SHOULD HAVE NO OBSERVABLE MOVEMENT AT THE APPLCABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.
   B. TOR□UE WRENCH METHOD: THE APPLICABLE TEST TOR□UE MUST BE REACHED WITHIN THE FOLLOWING LIMITS:
      WEDGE OR SLEEVE TYPE: ONE-HALF (1/2) TURN OF THE NUT.
      3/8 IN. SLEEVE ANCHOR ONLY: ONE-□UARTER (1/4) TURN OF THE NUT.
9. TESTING SHOULD OCCUR 24 HOURS MINIMUM AFTER INSTALLATION OF THE SUBJECT ANCHORS.
10. WHEN INSTALLING DRILLED-IN ANCHORS AND/OR POWDER DRIVEN PINS IN EXISTING NON-PRESTRESSED REINFORCED CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. WHEN INSTALLING THEM INTO EXISTING PRESTRESSED CONCRETE (PRE- OR POST- TENSIONED) LOCATE THE PRESTRESSED TENDONS BY USING A NON-DESTRUCTIVE METHOD PRIOR TO INSTALLATION. EXERCISE EXTREME CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE TENDONS DURING INSTALLATION. MAINTAIN A MINIMUM CLEARANCE OF ONE INCH BETWEEN THE REINFORCEMENT AND THE DRILLED-IN ANCHOR AND/OR PIN.
11. ALL TESTS SHALL BE PERFORMED IN THE PRESENCE OF THE SPECIAL INSPECTOR/INSPECTOR OF RECORD.
12. TEST LOADS:

HARDROCK OR LIGHTWEIGHT CONCRETE						
ANCHOR	WEDGE		SLEEVE		SHELL	
DIA (IN)	LOAD (LBS)	TOR□UE (FT-LBS)	LOAD (LBS)	TOR□UE (FT-LBS)	LOAD (LBS)	TOR□UE (FT-LBS)
1/4	800	10	400	4	1000	-
5/16	-	-	400	5	1400	-
3/8	1100	25	700	10	1800	-
1/2	2000	50	900	20	2700	-
5/8	2300	80	1100	45	3700	-
3/4	3700	150	1400	90	5400	-
1	5800	250	-	-	-	-

CONTRACTOR RESPONSIBILITY NOTE:

GENERAL:
1. EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN LATERAL-FORCE-RESISTING SYSTEM, DESIGNATED ON PLANS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO DSA AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR□S STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:
   A. ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL RE□UIREMENTS.
   B. ACKNOWLEDGMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
   C. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR□S ORGANIZATION, THE METHOD AND FRE□UENCY OF REPORTING AND THE DISTRIBUTION OF THE REPORTS.
   D. IDENTIFICATION AND □UALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

VCA ENGINEERS INC
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Project No. D312

TYPICAL ABBREVIATIONS

AB	ANCHOR BOLT
ABV	ABOVE
ADDT□L	ADDITIONAL
ADJ	ADJACENT
AFF	ABOVE FINISH FLOOR
ALT	ALTERNATE
ARCH	ARCHITECT(URAL)
BLDG	BUILDING
BLKG	BLOCKING
BLW	BELOW
BM	BEAM
BN	BOUNDARY NAILING
B.O.	BOTTOM OF
BOTT	BOTTOM
BRG	BEARING
BS	BOTH SIDES
BTWN	BETWEEN
C	CAMBER
CIP	CAST IN PLACE
CJ	CONTROL/ CONSTRUCTION JOINT
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
CP	COMPLETE PENETRATION
CSK	COUNTERSINK
CTR(D)	CENTER(ED)
D_B	BAR OR BOLT DIAMETER
DBL	DOUBLE
DEMO	DEMOLITION
DET	DETAIL
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DO	DITTO
DWG	DRAWING
(E)	EXISTING
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOIST
EMBED	EMBEDMENT
ELEC	ELECTRICAL
ELEV	ELEVATION
EN	EDGE NAILING
E.O.	EDGE OF
EOR	ENGINEER OF RECORD
E□	E□UAL
E□UIP	E□UIPMENT
ES	EACH SIDE OR EDGE SCREW
EW	EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
FIN	FINISH
FLR	FLOOR
FN	FIELD NAILING
FND	FOUNDATION
F.O.	FACE OF
FS	FAR SIDE OR FIELD SCREW
FRMG	FRAMING
FT	FOOT OR FEET
FTG	FOOTING
G	GIRDER
GA	GAGE
GALV	GALVANIZED
HAB	HEADED ANCHOR BOLT
HD	HOLDDOWN
HDR	HEADER
HGR	HANGER
HK	HOOK
HORIZ	HORIZONTAL
HP	HIGH POINT
HS	HIGH STRENGTH
HSS	HOLLOW STRUCTURAL STEEL
HT	HEIGHT
IN	INCH

INT	INTERIOR
JST	JOIST
KLF	KIPS PER LINEAR FOOT
KSL	KIPS PER S□UARE FOOT
KSI	KIPS PER S□UARE INCH
L	ANGLE
LFRS	LATERAL FORCE RESISTING SYSTEM
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LP	LOW POINT
LWC	LIGHTWEIGHT CONCRETE
MAX	MAXIMUM
MB	MACHINE BOLT
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM
MTL	METAL
(N)	NEW
NS	NEAR SIDE OR NON-SHRINK
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
OC	ON CENTER
O.F.	OUTSIDE FACE
OH	OPPOSITE HAND
OPNG	OPENING
PDF	POWDER DRIVEN FASTENER
PJ	PANEL JOIST
PL	PLATE
PLC(S)	PLACE(S)
PLF	POUND PER LINEAR FOOT
PLYWD	PLYWOOD
PREFAB	PREFABRICATED
PSF	POUND PER S□UARE FOOT
PSI	POUND PER S□UARE INCH
PT	PRESSURE TREATED OR POST TENSION
□TY	□UANTITY
RAD, R	RADIUS
REF	REFERENCE
REINF	REINFORCING
RE□D	RE□UIRED
SB	SILL BOLT
SC	SAW CUT OR SLIP CRITICAL
SCHED	SCHEDULE
SEOR	STRUCTURAL ENGINEER ON RECORD
SHTG	SHEATHING
SIM	SIMILAR
SMS	SHEET METAL SCREW
SN	SILL NAIL
SOG	SLAB ON GRADE
S□	S□UARE
SS	STAINLESS STEEL
STD	STANDARD
STGRD	STAGGERED
STIFF	STIFFENER
STL	STEEL
STRUCT	STRUCTURAL
T&B	TOP AND BOTTOM
THK	THICK
T.O.	TOP OF
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W/	WITH
W/O	WITHOUT
WF	WIDE FLANGE
WLD	WELDED
WO	WHERE OCCURS
WP	WORK POINT
WT	WEIGHT
WWF	WELDED WIRE FABRIC

REGISTERED PROFESSIONAL ENGINEER YOUNG-KEUN NAM No. S4029 E□. 9/30/20 STRUCTURAL STATE OF CALIFORNIA

BUREAU OF ENGINEERING
OFFICIAL RECORD

BID SET
PROJECT ISSUE DATE: 06/22/21

CITY OF LOS ANGELES
DEPARTMENT OF PUBLIC WORKS
BUREAU OF ENGINEERING

NO.	REVISION DESCRIPTION	DATE	BY

BUILDING NO. -
INDEX NO. RP-300118

GARY LEE MOORE, PE, ENV SP
CITY ENGINEER
ARCHITECTURAL DIVISION

		DATE:
ARCHITECT/ENGINEER: JHON SPARANO	LIC. NO.: LICENSE	11/01/18
DESIGNED BY: SPARANO □MOONEY ARCHITECTURE		11/01/18
DRAWN BY: ALEX, □		11/01/18
CHECKED BY: YOUNG, NAM		11/01/18
APPROVED BY:		11/01/18

VERTICAL CONTROL:
HORIZONTAL CONTROL:
SHEET TITLE: GENERAL NOTES
PROJECT: GRANADA HILLS POOL AND BATH HOUSEREPLACEMENT PROJECT
ADDRESS: 16730 CHATSWORTH STREETGRANADA HILLS, CA 91344

WORK ORDER NO. #E170517
PLAN FILE NO.
DRAWING NO. S0.02
SHEET OF SHEETS

REVISION DATES (DESIGN STAGE ONLY)

TTLB TEMPLATE REVISION DATE: 02/2013
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FILE PATH: C:□Users□Y□in□Do□□ents□S-GRANADA HILL-V17_Y□in.rvt

STATEMENT OF SPECIAL INSPECTIONS:

1. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION. THE SPECIAL INSPECTOR SHALL BE A □UALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION RE□UIRING SPECIAL INSPECTION.
2. SPECIAL INSPECTIONS ARE NOT RE□UIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVED FABRICATORS MUST SUBMIT A CERTIFICATE OF COMPLIANCE FOR OFFSITE FABRICATIONS SUCH AS STRUCTURAL STEEL, PRECAST CONCRETE, ETC.
3. ALL INSPECTIONS SHALL BE PERFORMED BY INDEPENDENT SPECIAL INSPECTORS. JOB SITE VISITS BY THE STRUCTURAL ENGINEER OR BUILDING OFFICIAL DO NOT CONSTITUTE AND ARE NOT A SUBSTITUTE FOR INSPECTIONS BY A SPECIAL INSPECTOR.
4. ALL INSPECTION REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND SEOR. THE FINAL REPORTS BY THE SPECIAL INSPECTOR(S) MUST CERTIFY THAT THE ENTIRE STRUCTURAL SYSTEM COMPLIES WITH THE APPROVED PLANS AND SPECIFICATIONS.
5. IT IS SOLELY THE CONTRACTOR□S RESPONSIBILITY TO SEE THAT THESE INSPECTIONS ARE PERFORMED.
6. WORK RE□UIRING SPECIAL INSPECTION SHALL BE INSPECTED BY THE SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS PERFORMED AND AT THE COMPLETION OF WORK. CONTINUOUS INSPECTION CONSISTS OF FULL-TIME INSPECTION□PERIODIC INSPECTION CONSISTS OF PART-TIME OR INTERMITTENT INSPECTION.
7. THE FOLLOWING SPECIAL INSPECTIONS ARE IN ADDITION TO INSPECTIONS BY THE BUILDING OFFICIAL. THIS LIST IS NOT INTENDED TO BE ALL INCLUSIVE.

TABLE 1705.2.1

RE□UIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD[a]	CBC REFERENCE
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	AISC 360, SECTION A3.3 AND APPLICABLE ASTM MATERIAL STANDARDS	-
B. MANUFACTURER□S CERTIFICATE OF COMPLIANCE RE□UIRED.	-	X	-	-
2. INSPECTION OF HIGH-STRENGTH BOLTING:				
A. SNUG-TIGHT JOINTS.	-	X	AISC 360, SECTION M2.5	-
B. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCH MARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.	-	X		
C. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCH MARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION.	X	-		
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:				
A. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.	-	X	AISC 360, SECTION A3.1	2203.1
B. FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	APPLICABLE ASTM MATERIAL STANDARDS	-
C. MANUFACTURER□S CERTIFIED TEST REPORTS.	-	X	-	-
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	AISC 360, SECTION A3.5 AND APPLICABLE AWS A5 DOCUMENTS	-
B. MANUFACTURER□S CERTIFICATE OF COMPLIANCE RE□UIRED.	-	X	-	-
5. INSPECTION OF WELDING:				
A. STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:				
1) COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS.	X	-	AWS D1.1, AWS D1.8	1705.2.1
2) MULTIPASS FILLET WELDS.	X	-		
3) SINGLE-PASS FILLET WELDS □5/16"	X	-		
4) PLUG AND SLOT WELDS.	X	-		
5) SINGLE-PASS FILLET WELDS □5/16"	-	X		
6) FLOOR AND ROOF DECK WELDS.	-	X	AWS D1.3	-
B. REINFORCING STEEL.				
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	-	X	AWS D1.4, ACI318: SECTION 26.6.4.1, 18.2.8, 25.5.7.4	-
2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	X	-		
3) SHEAR REINFORCEMENT.	X	-		
4) OTHER REINFORCING STEEL.	-	X		
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE.				
A. DETAILS SUCH AS BRACING AND STIFFENING.	-	X	-	1705.2.1
B. MEMBER LOCATIONS.	-	X		
C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	-	X		

FOR SI: 1 INCH □ 25.4 MM
a. WHERE APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.

TABLE 1705.3

RE□UIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD [a]	CBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	-	X	ACI 318 CH. 20, 25.2, 25.3, 26.5.1-26.5.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706□ □. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM $\frac{5}{16}$"□ □. INSPECT ALL OTHER WELDS.	- X	X X	AWS D1.4 ACI 318: 3.5.2	-
3. INSPECT ANCHORS CAST IN CONCRETE.	-	X	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.□ a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. □. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a	X	X	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
5. VERIFY USE OF RE□UIRED DESIGN MIX.	-	X	ACI 318: CH 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FRABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TEST, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C172 ASTM C31 ACI 318: 26.4.5, 26.12	1908.10
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNI□UES.	X	-	ACI 318: 26.4.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNI□UES.	-	X	ACI 318: 26.4.7-26.4.9	1908.9
9. INSPECTION OF PRESTRESSED CONCRETE:				
A. APPLICATION OF PRESTRESSING FORCES.	X	-	ACI 318: 26.9.2.1	-
B. GROUTING OF BONDED PRESTRESSING TENDONS	X	-	ACI 318: 26.9.2.3	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	X	ACI 318: CH. 26.8	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X	ACI 318: 26.10.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	ACI 318: 26.10.1(□)	-

FOR SI: 1 INCH □ 25.4 MM
a. WHERE APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.
□. SPECIFIC RE□UIREMENTS FOR SPECIAL INSPECTIONS SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 ACI 318, OR OTHER □UALIFICATION PROCEDURES. WHERE SPECIFIC RE□UIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION RE□UIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

TABLE 1705.6

RE□UIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADE□UATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEE N PREPARED PROPERLY.	-	X

TABLE 1705A.7

RE□UIRED VERIFICATION AND INSPECTION OF DRIVEN DEEP FOUNDATION ELEMENTS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. VERIFY ELEMENT MATERIALS, SIZES AND LENGTHS COMPLY WITH THE RE□UIREMENTS.	X	-
2. DETERMINE CAPACITIES OF TEST ELEMENTS AND CONDUCT ADDITIONAL LOAD TESTS, AS RE□UIRED.	X	-
3. OBSERVE DRIVING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT.	X	-
4. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM TYPE AND SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE RE□UIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP AND BUTT ELEVATIONS AND DOCUMENT ANY DAMAGE TO FOUNDATION ELEMENT.	X	-
5. FOR STEEL ELEMENTS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECT ION 1704A.3.	-	-
6. FOR CONCRETE ELEMENTS AND CONCRETE-FILLED ELEMENTS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1704AA.	-	-
7. FOR SPECIALTY ELEMENTS, PERFORM ADDITIONAL INSPECTIONS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.	-	-

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Project No. D312

BUREAU OF ENGINEERING
DEPARTMENT OF PUBLIC WORKS
CITY OF LOS ANGELES

GARY LEE MOORE, PE, ENV SP — CITY ENGINEER

		DATE:
ARCHITECTURAL DIVISION		
ARCHITECT/ENGINEER:	JHON SPARANO LIC. NO.: LICENSE	11/01/18
DESIGNED BY:	SPARANO □ MOONEY ARCHITECTURE	11/01/18
DRAWN BY:	ALEX, □	11/01/18
CHECKED BY:	YOUNG, NAM	11/01/18
APPROVED BY:		11/01/18

REGISTERED PROFESSIONAL ENGINEER — YOUNG-KEUN NAM — No. S4029 — E□□ 9/30/20 — STRUCTURAL — STATE OF CALIFORNIA

NO.	REVISION DESCRIPTION	DATE	BY

INDEX NO. RP-300118
BUILDING NO. -

VERTICAL CONTROL:
HORIZONTAL CONTROL:
SHEET TITLE: GENERAL NOTES
PROJECT: GRANADA HILLS POOL AND BATH HOUSEREPLACEMENT PROJECT
ADDRESS: 16730 CHATSWORTH STREETGRANADA HILLS, CA 91344

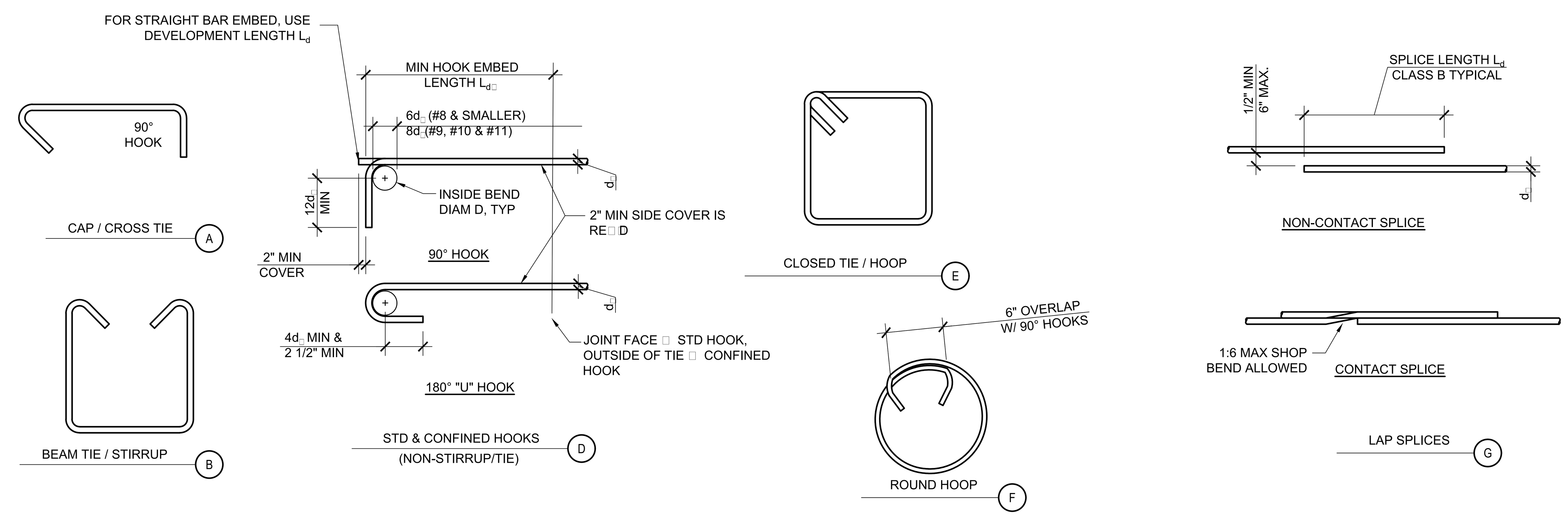
WORK ORDER NO. #E170517
PLAN FILE NO.
DRAWING NO. S0.03
SHEET OF SHEETS

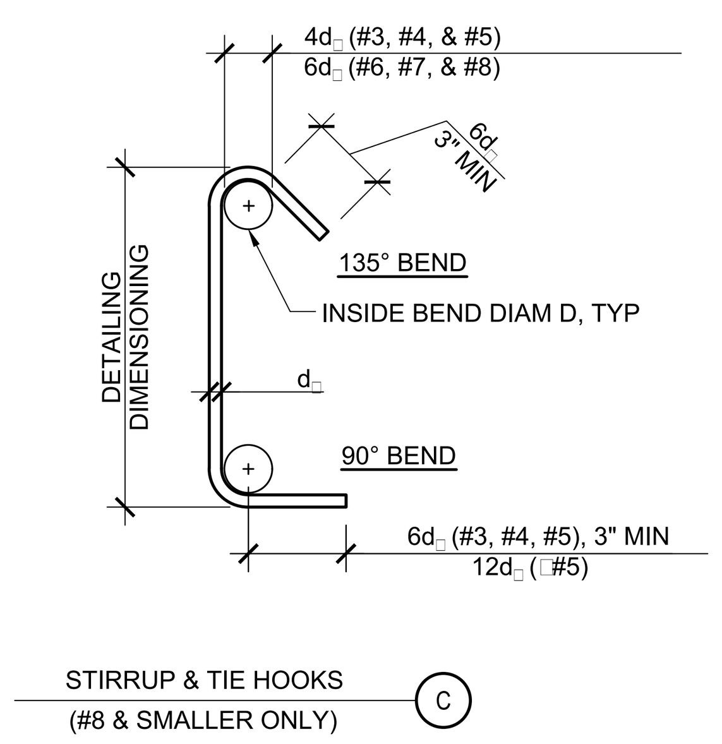
BID SET
PROJECT ISSUE DATE: 06/22/21

BUREAU OF ENGINEERING — OFFICIAL RECORD

REVISION DATES (DESIGN STAGE ONLY)

TTLB TEMPLATE REVISION DATE: 02/2013
SHEET ISSUE DATE: 04/08/14
FILE PATH: C:\Users\Y□in\Do□□ents\S-GRANADA HILL-V17_Y□in.rvt





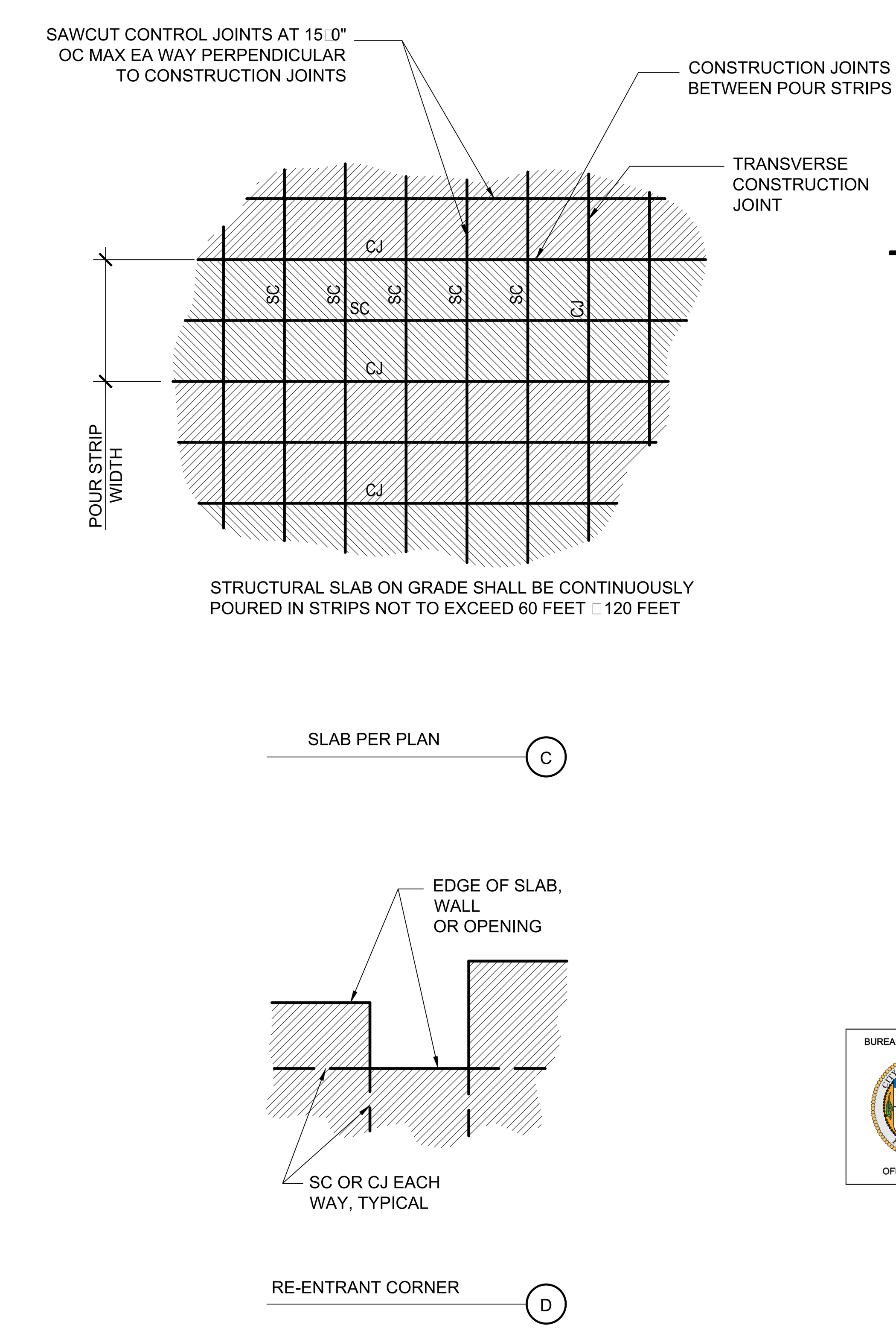
BAR SIZE	FINISHED BEND DIAMETER "D" IN INCHES
#9	9 1/2"
#10	10 3/4"
#11	12"
#14	18 1/4"
#18	24"

STANDARD HOOKS

	BAR SIZE	L_{dh} STD HOOK	LAP SPLICE CLASS B TOP	LAP SPLICE CLASS B OTHER	L_d DEVELOPMENT LENGTH TOP	L_d DEVELOPMENT LENGTH OTHER
3000 PSI NWC	#3	6	29	23	22	17
	#4	8	38	29	29	22
	#5	10	47	37	36	28
	#6	12	56	43	43	33
	#7	14	82	63	63	48
	#8	16	94	72	72	55
	#9	18	106	81	81	62
	#10	20	119	91	91	70
	#11	22	132	102	101	78
4000 PSI NWC	#3	6	25	20	19	15
	#4	7	33	25	25	19
	#5	9	41	32	31	24
	#6	10	49	38	37	29
	#7	12	71	55	54	42
	#8	14	81	63	62	48
	#9	15	91	71	70	54
	#10	17	103	80	79	61
	#11	19	114	88	87	67
5000 PSI NWC	#3	6	23	17	17	13
	#4	6	30	23	23	17
	#5	8	37	29	28	22
	#6	9	45	34	34	26
	#7	11	64	50	49	38
	#8	12	73	56	56	43
	#9	14	82	63	63	48
	#10	16	93	71	71	54
	#11	17	102	78	78	60

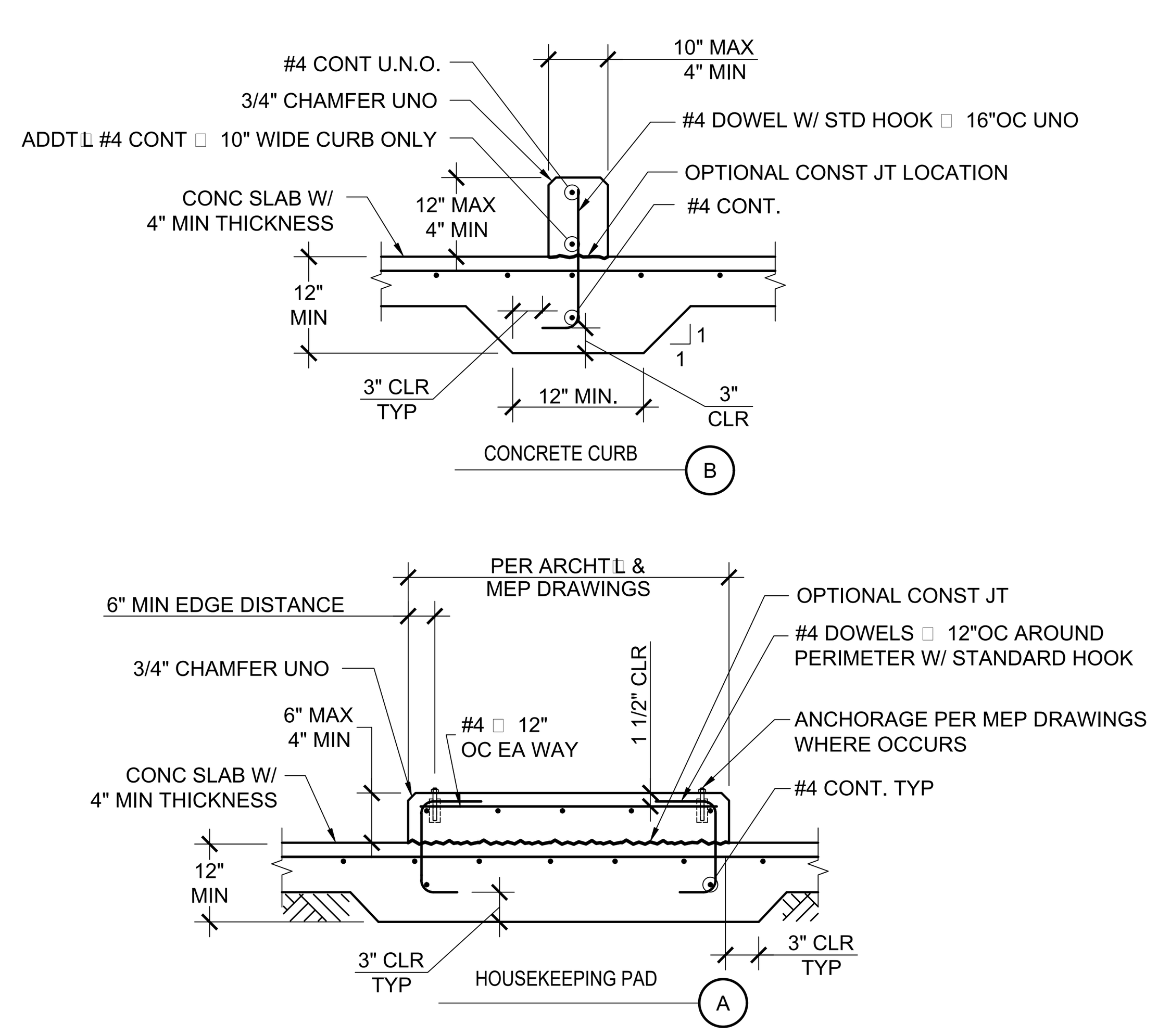
NOTES:
1. L_{dh} - HOOK DEVELOPMENT LENGTH.
2. L_d - DEVELOPMENT LENGTH.
3. SEE BUILDING CODE AND ACI 318-14 VERSION FOR ALL REQUIREMENTS NOT NOTED.
4. FOR LIGHTWEIGHT CONCRETE MULTIPLY L_{dh} AND L_d VALUES SHOWN BY 1.3.
5. "TOP" BARS ARE HORIZONTAL WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW BARS. ALL OTHER BARS ARE "OTHER" BARS.
6. STD HOOK L_{dh} DOES NOT ACCOUNT FOR THE EFFECT OF TIES. CONFINED HOOK L_{dh} MAY BE USED WHERE ACI 318-14 25.4.3.1 APPLIES.
7. VALUES SHOWN ABOVE ARE FOR GRADE 60 (FY 60 KSI) REINFORCEMENT.
8. SPLICE LENGTHS SHOWN ARE FOR CLEAR SPACING NOT LESS THAN $2d_b$, CONCRETE COVER NOT LESS THAN d_b.
9. FOR EPOXY-COATED REINFORCEMENT, SEE BUILDING CODE

5 TYPICAL REINFORCEMENT DETAILS AND DEVELOPMENT LENGTHS
SCALE: N.T.S.

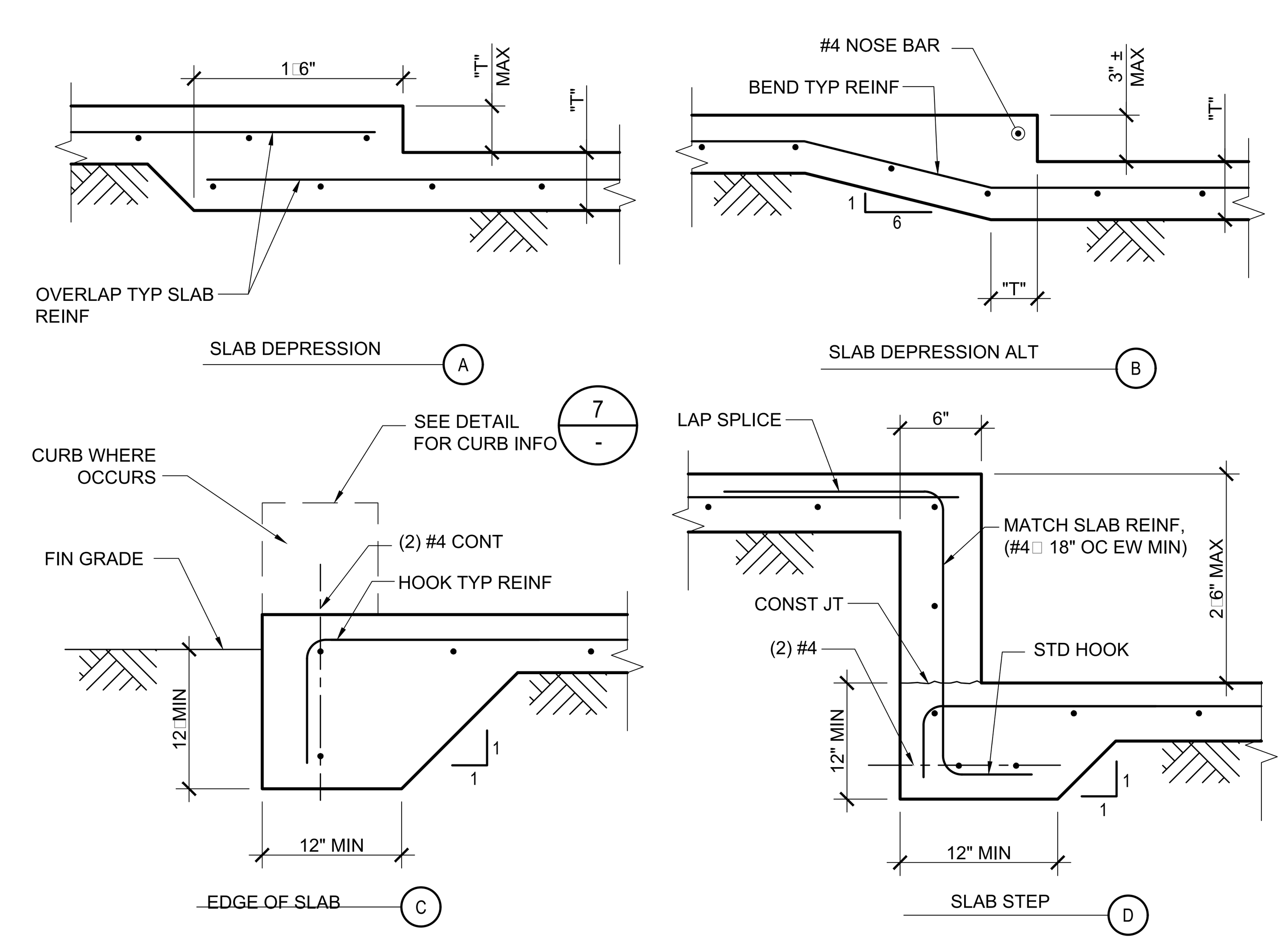


NOTES:
1. SEE ARCHITECTURAL PLANS FOR LOCATIONS OF VISUALLY EXPOSED CONTROL JOINTS.
2. FOR POURING SEQUENCE OTHER THAN THAT SHOWN ON THESE PLANS, SUBMIT SHOP DRAWINGS TO SEOR FOR REVIEW 7 DAYS PRIOR TO POUR.
3. SEE DETAIL 9/- FOR SC AND CJ DETAIL.

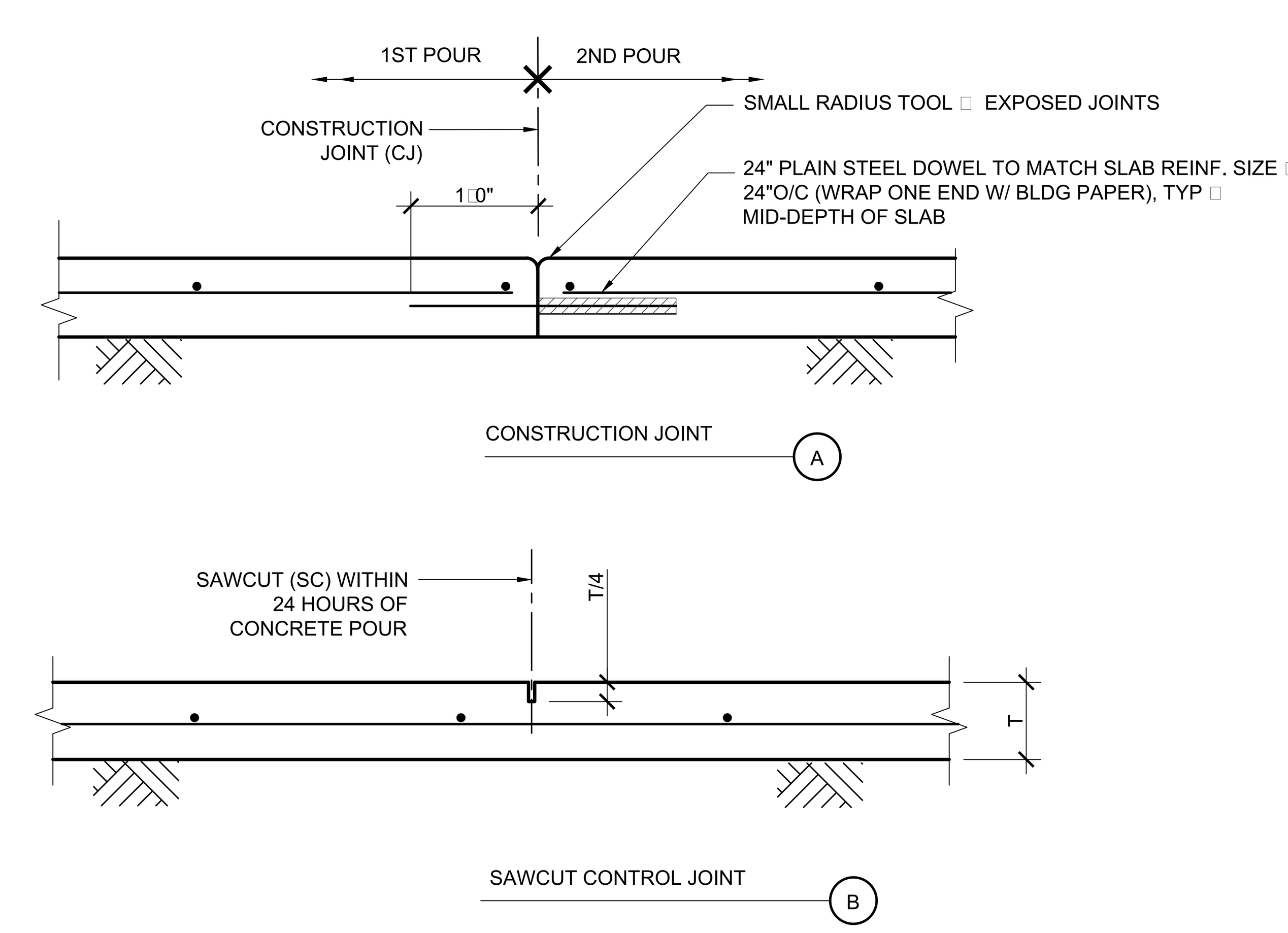
6 DETAIL
SCALE: N.T.S.



7 TYPICAL CONCRETE PAD AND CURB ON GRADE
SCALE: N.T.S.



8 TYPICAL SLAB ON GRADE DETAILS
SCALE: N.T.S.



9 TYPICAL SLAB ON GRADE CONTROL JOINT (FOR SLAB THICKNESS 4" TO 6")
SCALE: N.T.S.

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BUREAU OF ENGINEERING
DEPARTMENT OF PUBLIC WORKS
CITY OF LOS ANGELES

NO.	REVISION DESCRIPTION	DATE	BY

BUILDING NO. -
INDEX NO. RP-300118

GARY LEE MOORE, PE, ENV SP
CITY ENGINEER
ARCHITECTURAL DIVISION

		DATE:
ARCHITECT/ENGINEER:	JHON SPARANO LIC. NO.: LICENSE	11/01/18
DESIGNED BY:	SPARANO MOONEY ARCHITECTURE	11/01/18
DRAWN BY:	ALEX	11/01/18
CHECKED BY:	YOUNG, NAM	11/01/18
APPROVED BY:		11/01/18

VERTICAL CONTROL:
HORIZONTAL CONTROL:
SHEET TITLE: TYPICAL DETAILS
PROJECT: GRANADA HILLS POOL AND BATH HOUSEREPLACEMENT PROJECT
ADDRESS: 16730 CHATSWORTH STREETGRANADA HILLS, CA 91344

WORK ORDER NO. #E170517
PLAN FILE NO.
DRAWING NO. S1.01
SHEET OF SHEETS

BID SET
PROJECT ISSUE DATE: 06/22/21