

Compute the y component of the reaction force at B

Data

$$l = 400 \text{ mm}$$

$$EA = 7.2 \times 10^7 \text{ N}$$

$$F = 5000 \text{ N}$$

unit for result: N

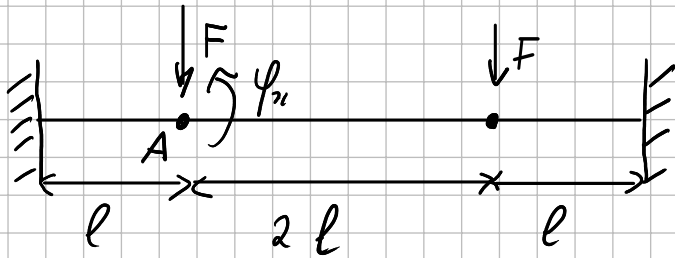
$$k_1 = \frac{EA}{l}$$

$$k_2 = \frac{EA}{2l}$$

$$(k_1 + k_2) w = -F$$

$$w = \frac{-F}{k_1 + k_2}$$

$$Y = -k_2 w = \frac{k_2 F}{k_1 + k_2} = 1666.7 \text{ N}$$



Compute the rotation  $\phi_A$  of point A

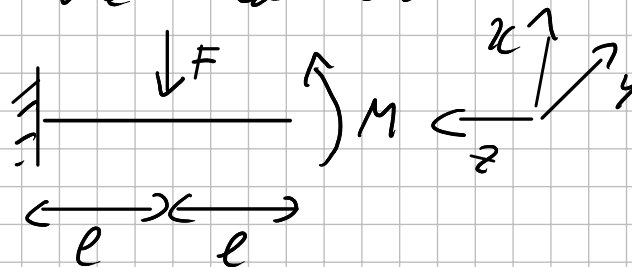
Data:  $l = 1400 \text{ mm}$

$F = 2500 \text{ N}$

$EI_{yy} = 6 \times 10^4 \text{ N mm}^2$

Unit for result: deg

Symmetric structure and load



$$0 < z < l \quad M_y = M$$

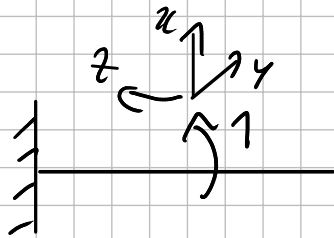
$$l < z < 2l \quad M_y = M + F(-z + l)$$

$$\delta M_y = 1$$

$$\int_0^l \delta M_y \frac{M_y}{EI_{yy}} dz + \int_l^{2l} \frac{\delta M_y M_y}{EI_{yy}} dz = 0$$

$$\frac{2 M l}{EI_{yy}} \left[ + \frac{3}{2} \frac{F l^2}{EI_{yy}} - \frac{F l^2}{EI_{yy}} \right] = 0$$

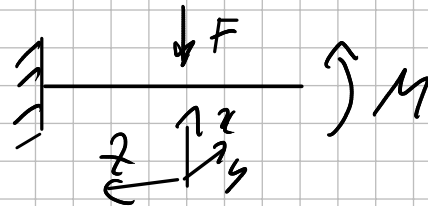
$$M = + \frac{1}{4} F l$$



$$0 < z < l \quad \delta M_y = 1$$

$$-l < z < 0 \quad \delta M_y = 0$$

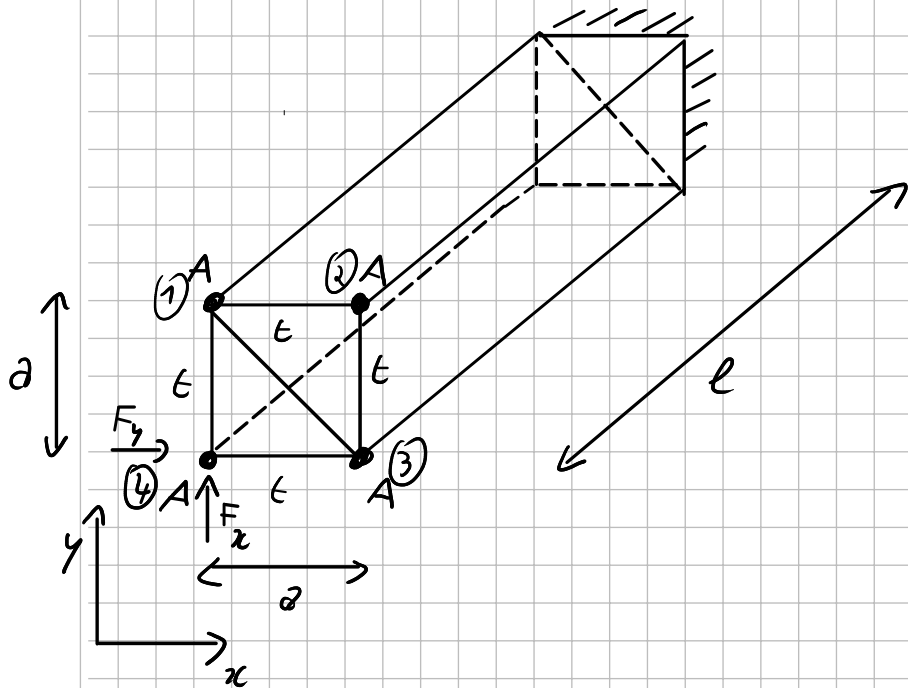
Cambia coordinate



$$0 < z < l \quad M_y = M - F z$$

$$\frac{1}{EI_{yy}} \int_0^l \left( + \frac{1}{4} F l - F z \right) dz = - \frac{1}{4} \frac{F l^2}{EI_{yy}}$$

$$\text{res in deg} = - \frac{1}{4} \frac{F l^2}{EI_{yy}} \cdot \frac{180}{\pi} = -1,1698^\circ$$

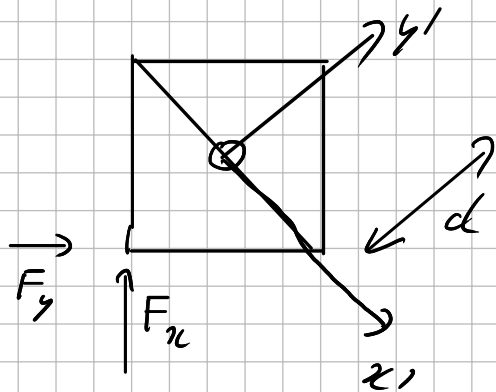


The semi-monologue structure in the figure is loaded at point (4) by the concentrated forces  $F_x$  and  $F_y$ . Compute the axial stress  $\sigma_{zz}$  of stringer (1) at a distance of  $l/2$  from the clamp.

Data

- $l = 4000 \text{ mm}$
- $F_x = 5000 \text{ N}$
- $F_y = 1000 \text{ N}$
- $A = 1000 \text{ mm}^2$
- $a = 200 \text{ mm}$
- $\epsilon = 1 \text{ mm}$
- $E = 72000 \text{ MPa}$
- $\nu = 0,3$

Unit for result:  $\text{MPa}$



$$F_{x'} = -\frac{F_x}{\sqrt{2}} + \frac{F_y}{\sqrt{2}}$$

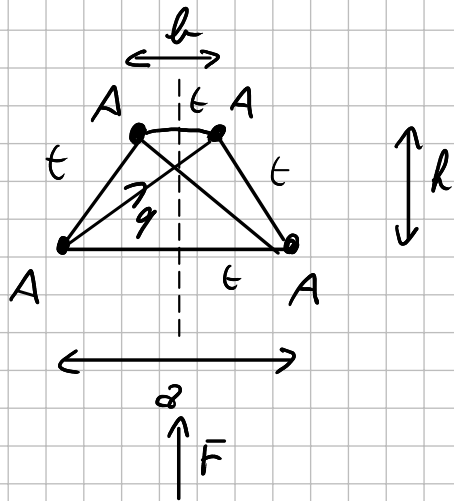
$$F_{y'} = \frac{(F_x + F_y)}{\sqrt{2}}$$

$$d = \frac{a}{\sqrt{2}}$$

$$\bar{I}_{y'y'} = 2A \cancel{d} d^2 = A a^2$$

$$M_{y'} = -F_{x'} \cdot \frac{a}{2}$$

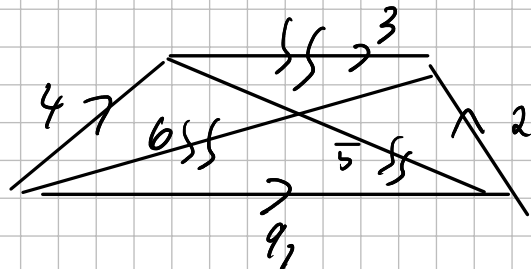
$$\sigma_{zz} = \frac{M_{y'}}{\bar{I}_{y'y'}} (-d) = \frac{(-F_x + F_y)}{4A} \frac{a}{2} = -20 \text{ MPa}$$



The symmetric three cells semi-monocoque cross section in the figure is loaded by the force  $F$  in the plane of symmetry. Compute the flux  $q$

Data

$A = 2000 \text{ mm}^2$
$a = 200 \text{ mm}$
$b = 100 \text{ mm}$
$h = 100 \text{ mm}$
$F = 5000 \text{ N}$
$t = 1 \text{ mm}$

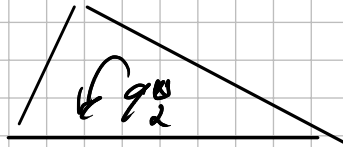


3 cells  $q^0$ :

$$q_1^0 = 0$$



for symmetry



$$q_2^* = q_3^*$$

$$q_1' = 0$$

$$q_2' = q_4' = \frac{F}{2h}$$

$$\dot{Q}_i = 0$$

for symmetry

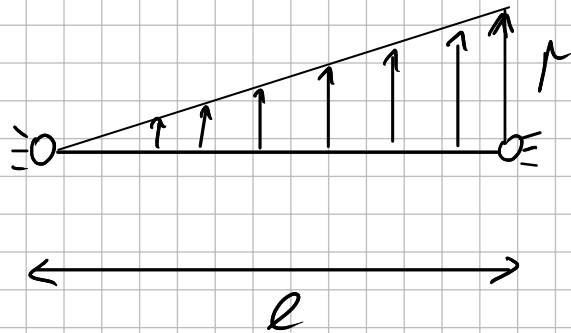
$$Q_2 = \frac{\partial h}{2}$$

$$\dot{Q}_2 = \frac{1}{2h} \left( \frac{-l_4 q_4'}{t} + \frac{(l_5 + l_4) q_2^*}{t} \right)$$

$$l_4 = \sqrt{\left(\frac{a-h}{2}\right)^2 + h^2}$$

$$l_5 = \sqrt{\left[a - \left(\frac{a-h}{2}\right)\right]^2 + h^2}$$

$$q_2^* = \frac{l_4 q_4'}{l_5 + l_4} = 9,5696 \text{ W/mm}$$



The beam in the figure is loaded by a linearly-varying force per unit of length. Resort to a displacement-based approach and using the simplest possible polynomial approximation in order to estimate the vertical displacement in the middle of the beam.

Data

$$l = 3000 \text{ mm}$$

$$p = 4 \text{ N/mm}$$

$$EI_{xx} = 6 \cdot 10^7 \text{ N mm}^2$$

Unit for result: mm

$$v = c \cdot z(z-l)$$

$$R = \frac{p}{l} \cdot z$$

$$\delta v = \delta c \cdot z(z-l)$$



$$v'' = 2c$$

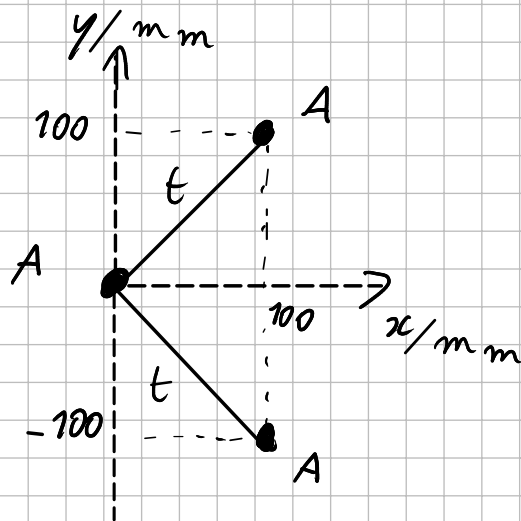
$$\int v'' = 2 \int c$$

$$\int_0^l \int c \quad 4 EI \quad c \quad dz = \int_0^l \int c \left( z^3 - z^2 l \right) \frac{1}{l} \quad dz$$

$$4 EI \int l \quad c = - \frac{1}{12} l^3 \quad r$$

$$c = - \frac{1}{48 EI} l^2 \quad r$$

$$v\left(\frac{l}{2}\right) = \frac{1}{192 EI} l^4 \quad r = 2,8125 \quad mm$$



Consider the open semi-monocoque cross section model sketched in the figure. Compute the  $x$  position of the shear center.

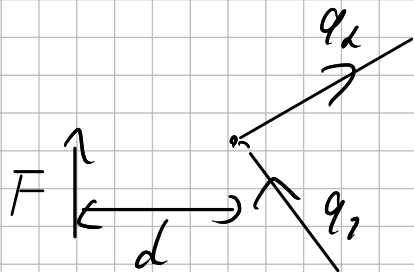
Data.  $t = 1 \text{ mm}$

$$A = 100 \text{ mm}$$

$$E = 72000 \text{ MPa}$$

$$\nu = 0,3$$

Unit for result : mm



$$q_1 = q_2 = \frac{F}{200}$$

$$M(0) = F \cdot d = 0$$

$$\Rightarrow d = 0 \text{ mm}$$

- The assumption of plane strain implies that a component of stress is null
  - False
- The essential boundary conditions are satisfied in a weak sense by the Principle of Virtual Work
  - False
- According to the semi-monocoque model, the axial stress  $\sigma_{zz}$  in the panels can be computed from the axial derivative of the shear stress
  - False
- The shear stress transmitted by a glued connection is
  - higher at the extremities
  - lower at the extremities
  - constant
  - described by a sin function
  - described by a cos function
  - described by a quadratic polynomial function
  - none of the above
- The bearing stress is related to
  - glued connections
  - riveted connections
  - the average shear stress in a semi-monocoque cross-section subject to constant torsional moment
  - the through-the-thickness shear stress in a Timoshenko shell model
  - the through-the-thickness shear stress in a Mindlin shell model
  - none of the above
- The transverse shear deformability for a thin-walled beam
  - is null
  - is generally larger with respect to a corresponding (same dimensions and bending stiffness) compact section
  - is generally smaller with respect to a corresponding (same dimensions and bending stiffness) compact section
  - is equal to that of a corresponding (same dimensions and bending stiffness) compact section
  - can be neglected
  - none of the above