

Course of Spacecraft Structures

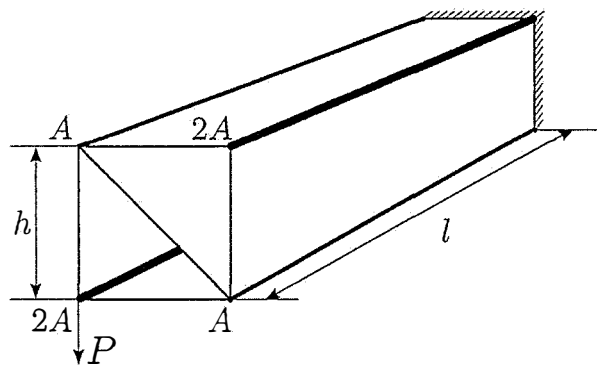
Written test, February 11th, 2019

Exercise 1

The two-cell beam in the figure is composed of five panels of thickness t , and four stringers of areas A and $2A$ (note: the area is the lumped one). The section is square and has dimension h .

The material is a light aluminum alloy, whose elastic properties are provided in terms of Young's modulus and Poisson's coefficient, denoted with E and ν , respectively. The beam is fixed at one end and free at the other one, and is loaded with a concentrated forces of magnitude P .

Determine the internal stresses at a distance $l/2$ from the free end.



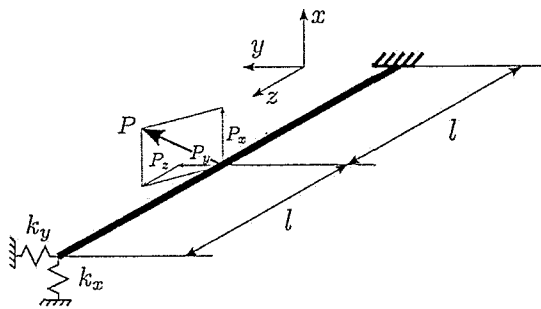
Data

$h = 100 \text{ mm}$; $l = 1500 \text{ mm}$;
 $t = 1.5 \text{ mm}$; $A = 150 \text{ mm}^2$;
 $E = 72000 \text{ MPa}$; $\nu = 0.3$;
 $P = 15 \text{ kN}$;

Exercise 2

The slender beam in the figure is fixed at one end, and is elastically supported by two springs of stiffness k_x and k_y at the other end. The beam axial stiffness is EA , while the bending stiffnesses are EJ_{xx} and EJ_{yy} (note: $EJ_{xy} = 0$). A concentrated force P , whose components are P_x , P_y and P_z , is applied at the middle.

Determine the unknown reactions forces and the strain energy stored in the structure (including the springs).



Data

$l = 1000 \text{ mm}$;
 $EA = 7 \times 10^7 \text{ N}$;
 $EJ_{xx} = 5 \times 10^9 \text{ Nmm}^2$;
 $EJ_{yy} = 8 \times 10^9 \text{ Nmm}^2$;
 $k_x = 5 \text{ N/mm}$; $k_y = 2 \text{ N/mm}$;
 $P_x = 150 \text{ N}$; $P_y = 100 \text{ N}$; $P_z = 10000 \text{ N}$;

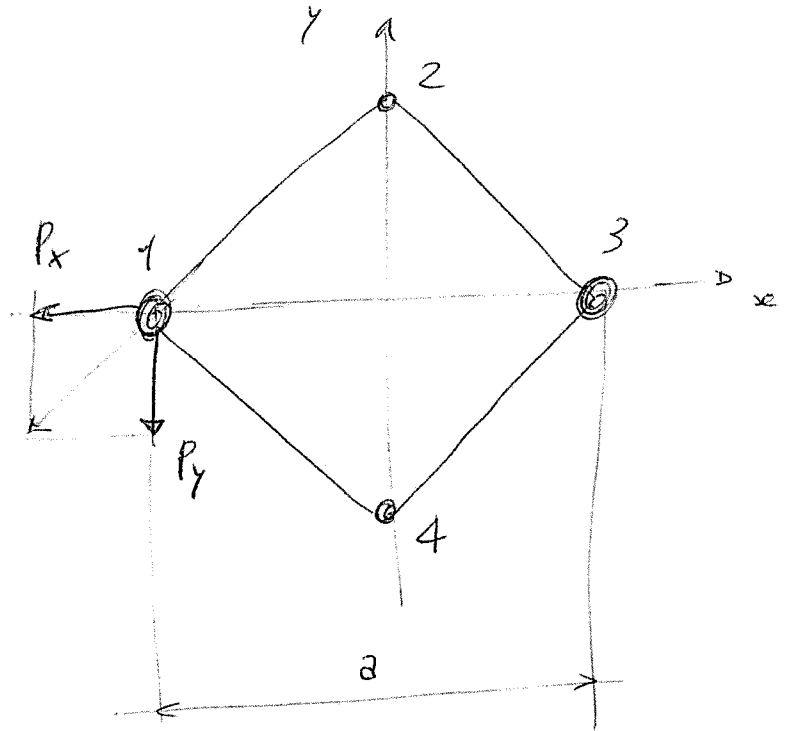
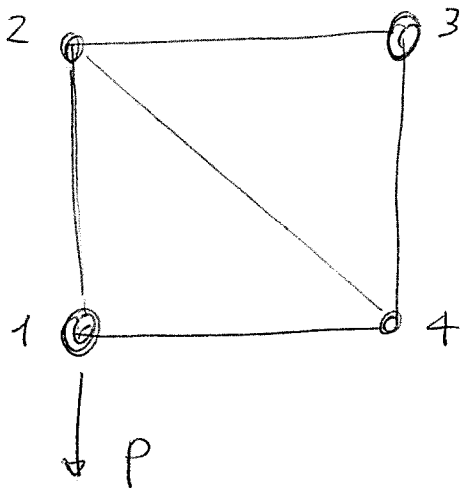
Question 1

Illustrate how to determine the torsional and transverse shear stiffnesses for a thin-walled beam in the context of the semi-monocoque approximation.

Exercise 1

Section properties

The principal axis are available from the symmetry of the section



$$a = h\sqrt{2} = 141.42 \text{ mm}$$

$$J_{xx} = 2A \left(\frac{a}{2} \right)^2 = 1.5 \cdot 10^6 \text{ mm}^4$$

$$J_{yy} = 4A \left(\frac{a}{2} \right)^2 = 3.0 \cdot 10^6 \text{ mm}^4$$

$$S_{x1} = 0$$

$$S_{x2} = A \frac{a}{2}$$

$$S_{x3} = A \frac{a}{2}$$

$$S_{x4} = 0$$

$$S_{y1} = -Aa$$

$$S_{y2} = -Aa$$

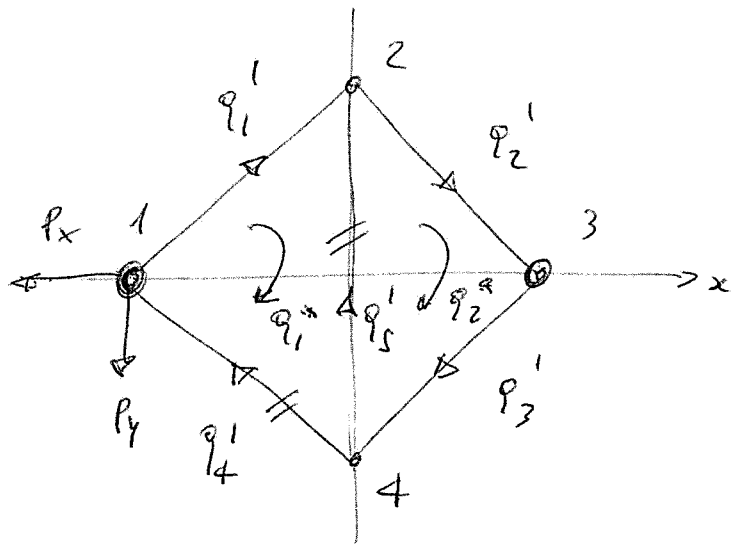
$$S_{y3} = 0$$

$$S_{y4} = 0$$

Shear flows

$$q_1' = + \frac{P_x S_{y1}}{J_{yy}} + \frac{P_y S_{x1}}{J_{xx}}$$

$$= - \frac{P_x}{a} = -75 \text{ N/mm}$$



$$q_2' = 0 \text{ N/mm}$$

$$q_3' = \frac{P_y}{a} = 75 \text{ N/mm}$$

$$q_4' = q_s' = 0$$

- Equivalence (wrt 1)

$$2\Omega_2 q_2' + 2\Omega_3 q_3' + 2\Omega_{c1} q_1^* + 2\Omega_{c2} q_2^* = 0$$

$$\Omega_2 = \Omega_3 = \Omega_{c1} = \Omega_{c2} = a^2/2, \text{ So:}$$

$$q_2' + q_3' + q_1^* + q_2^* = 0$$

$$\boxed{q_1^* + q_2^* = -75}$$

- Compatibility

$$q_1^* - q_2^* = \frac{(q_3' - q_1')h}{2(h+a)}$$

$$\boxed{q_1^* - q_2^* = 31.07}$$

The q^* flows are then:

$$\begin{cases} q_1^* + q_2^* = -75 \\ q_1^* - q_2^* = 31.07 \end{cases} \Rightarrow \begin{aligned} q_1^* &= -21.97 \text{ N/mm} \\ q_2^* &= -53.04 \text{ N/mm} \end{aligned}$$

And so:

$$q_1 = q_1^l + q_1^* = -96.97 \text{ N/mm}$$

$$\tau_1 = 64.65 \text{ MPa}$$

$$q_2 = q_2^* = -53.04 \text{ N/mm}$$

$$\tau_2 = 35.36 \text{ MPa}$$

$$q_3 = q_3^l + q_2^* = 21.96 \text{ N/mm}$$

$$\tau_3 = 14.64 \text{ MPa}$$

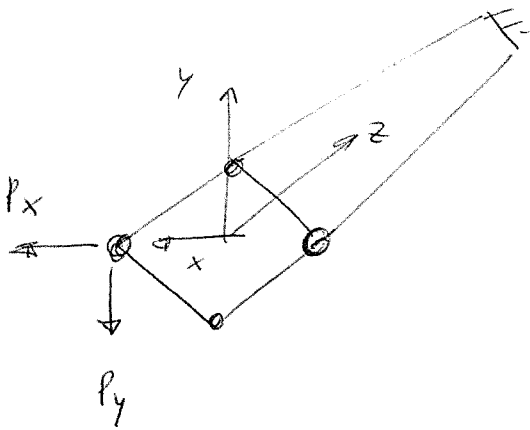
$$q_4 = q_1^* = -21.97 \text{ N/mm}$$

$$\tau_4 = 14.65 \text{ MPa}$$

$$q_5 = q_2^* - q_1^* = -31.08 \text{ N/mm}$$

$$\tau_5 = 20.72 \text{ MPa}$$

Stresses σ_{zz}



Referring to the principal axis.

$$M_x = P_y l/2$$

$$M_y = P_x l/2$$

The stresses are:

$$\sigma_{zz} = \frac{M_x}{J_{xx}} y - \frac{M_y}{J_{yy}} x$$

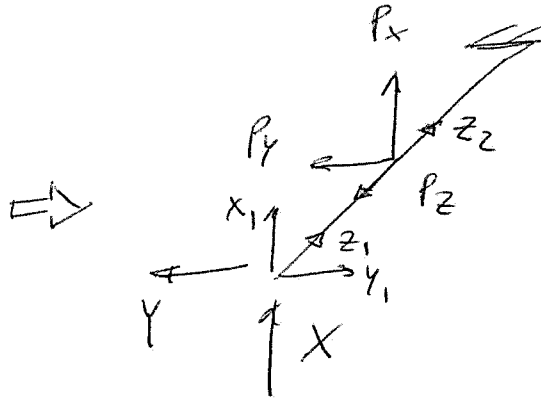
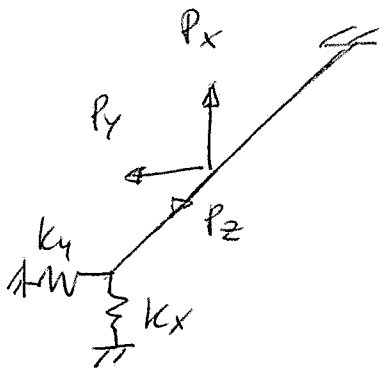
Stringers 2,4: $x=0, y=\pm a/2$

$$\sigma_{zz} = \frac{M_x}{J_{xx}} \left(\pm \frac{a}{2} \right) = \pm 375 \text{ MPa}$$

Stringers 1,3: $x=\pm a/2, y=0$

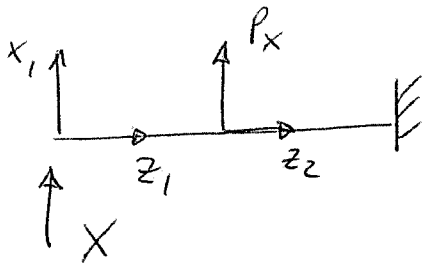
$$\sigma_{zz} = - \frac{M_y}{J_{yy}} \left(\pm \frac{a}{2} \right) = \mp 187.5 \text{ MPa}$$

Exercise 2

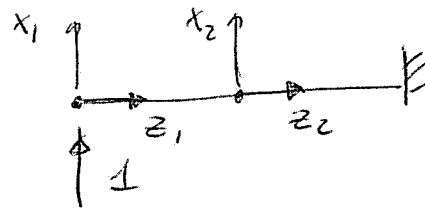


(2 times statically indetermined)

XZ - Plane



Real



Dummy

$$M_y^1 = X z_1$$

$$\delta M_y^1 = z_1$$

$$M_y^2 = X l + (X + P_x) z_2$$

$$\delta M_y^2 = z_2 + l$$

• PÖRV

$$\int_0^l \frac{M_y^1 \delta M_y^1}{E J_{yy}} dz_1 + \int_0^l \frac{M_y^2 \delta M_y^2}{E J_{yy}} dz_2 + \frac{X}{k_x} = 0$$

$$X \left(\frac{8}{3} l^3 + \frac{E J_{yy}}{k_x} \right) + \frac{5}{6} P_x l^3 = 0$$

yz-plane

By analogy with the xz-plane, it is immediate to obtain:

$$Y \left(\frac{8}{3} l^3 + \frac{EI_{yy}}{k_y} \right) + \frac{5}{6} P_y l^3 = 0$$

Solution: $X = -29.27 \text{ N}$

$$Y = -16.13 \text{ N}$$

Internal energy

$$U = U_{b_{xz}} + U_{b_{yz}} + U_{ax} + U_{\text{springs}}$$

L axial

L bending in
yz plane
L bending
in xz plane

$$U_{b_{xz}} = \frac{1}{2} \int_0^l \frac{M_y^2}{EI_{yy}} dz_1$$

$$= \frac{1}{EI_{yy}} \left(\frac{2}{3} X^2 l^3 + \frac{1}{6} (X + P_x)^2 l^3 + \frac{1}{2} X (X + P_x) l^3 \right)$$

$$= 0.154 \text{ J}$$

$$U_{b_{yz}} = \frac{1}{EI_{xx}} \left(\frac{2}{3} Y^2 l^3 + \frac{1}{6} (Y + P_y)^2 l^3 + \frac{1}{2} Y (Y + P_y) l^3 \right)$$

$$= 0.133 \text{ J}$$

$$U_{ax} = \frac{1}{2} \int_0^l \frac{P_z P_z}{EA} dz_z = \frac{1}{2} \frac{P_z^2}{EA} l = 0.714 \text{ J}$$

$$U_{springs} = \frac{1}{2} \frac{x^2}{k_x} + \frac{1}{2} \frac{y^2}{k_y} = 0.150 \text{ J}$$