DOI	https://doi.org/10.1061/(ASCE)0733-9445(1997)123:11(1454)
Title	Period Formulas for Moment-Resisting Frame Buildings
	Background
Why this paper? How'd you find i	It is one of a <u>pair</u> of articles cited by the building code for estimating building periods based on their heights. I wanted to see what methods were used to "measure" periods based on data. Found by mention of Prof. it? Mosalam.
Study Objective	Improve the period formulas used in the building code.
Intended gaps to fill	Lack of empirical data for "measured" periods from earthquake shaking.
Authors	Rakesh K. Goel and Anil K. Chopra
Affiliations	Syracuse University, Cal Poly SLO, UC Berkeley
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Journal / Field	ASCE Journal of Structural Engineering
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	See ASCE-7 Section 12.8.2, part of the ELF (equivalent lateral force) procedure for estimating a building's shear demand during design.  12.8.2.1 Approximate Fundamental Period. Eq. (12.8-7) is an empirical relationship determined through statistical analysis of the measured response of building structures in small-to moderate-sized earthquakes, including response to wind effects (Goel and Chopra 1997, 1998). Fig. C12.8-2 illustrates such data
	fundamental period $(T_n)$ , in seconds, shall be determined from the following equation: $T_a = C_t h_n^* \qquad (12.8-7)$ where $h_n$ is the structural height as defined in Section 11.2 and the Previous couer iori irium: $T = C_t H_0^{34} \qquad Was derived with assumption of linearly distributed static lateral forces, seismic base shear proportional to T^*(-2/3), and deflections controlled by drift limitations. Ct = 0.030 for RC and 0.035 for steel were derived from "measured" periods of buildings during 1971 Fernando earthquake.$
Historical Context Relationship to SEMM	CE 225, CE 227
Relationship to SEMIM	Methods
Civen	2000
Given:	Recorded vibrations during earthquakes
Find:	An empirical formula for computing moment frame building fundamental periods based on height  1. 90 vibrations recorded from instrumented moment frame buildings during the 1994 Northridge and 1971 San Fernando earthquakes.  2. System identification (see appx B) using (a) transfer function, (b) modal minimization method, and (c)
Experimental Design	autoregressive modeling to get "measured periods".  3. Regression analysis to get an equation of the form $T = \alpha H^{\beta}$ elating height to building period.
Experimental Design Test Subjects	autoregressive modeling to get "measured periods".
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Test Subjects  Baseline for comparison	autoregressive modeling to get "measured periods".  3. Regression analysis to get an equation of the form $T = \alpha H^{\beta}$ alating height to building period.  27 reinforced concrete and 42 steel moment frame buildings  Results  Previous code formula $T = C_t H^{3/4}$
Test Subjects	<ul> <li>autoregressive modeling to get "measured periods".</li> <li>3. Regression analysis to get an equation of the form T = αH<sup>β</sup> slating height to building period.</li> <li>27 reinforced concrete and 42 steel moment frame buildings</li> </ul> Results
Test Subjects  Baseline for comparison  Metric for comparison	autoregressive modeling to get "measured periods".  3. Regression analysis to get an equation of the form $T = \alpha H^{\beta}$ ≥ lating height to building period.  27 reinforced concrete and 42 steel moment frame buildings  Results  Previous code formula $T = C_t H^{3/4}$ Visual comparison of period and resulting seismic coefficient vs height  "Measured" periods are longer than the code computed periods, especially for tall buildings, and the resulting
Test Subjects  Baseline for comparison  Metric for comparison  Difference from baseline	autoregressive modeling to get "measured periods".  3. Regression analysis to get an equation of the form $T = \alpha H^{\beta}$ elating height to building period.  27 reinforced concrete and 42 steel moment frame buildings  Results  Previous code formula $T = C_t H^{3/4}$ Visual comparison of period and resulting seismic coefficient vs height  "Measured" periods are longer than the code computed periods, especially for tall buildings, and the resulting seismic coefficient is generally smaller.  Conclusions $T_L = 0.028 H^{0.80}$ for steel frames $T_L = 0.016 H^{0.90}$ for RC frames
Test Subjects  Baseline for comparison  Metric for comparison	autoregressive modeling to get "measured periods".  3. Regression analysis to get an equation of the form $T = \alpha H^{\beta}$ ≥lating height to building period.  27 reinforced concrete and 42 steel moment frame buildings  Results  Previous code formula $T = C_t H^{3/4}$ Visual comparison of period and resulting seismic coefficient vs height  "Measured" periods are longer than the code computed periods, especially for tall buildings, and the resulting seismic coefficient is generally smaller.  Conclusions  T. = 0.028 H^{0.80} for steel frames