

# Vibration isolation by relative resonance perceived in combination of rigid bodies and elastic beams

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## Abstract

This paper develops a novel low-frequency vibration isolation device based on rigid body dynamics called a Rigid Elastic Vibration Isolator (REVI). The REVI model is realized by coupling four elastic beams with two rigid bodies monolithically made using 3D printing. The system operates based on the vibration of the intermediate rigid bodies, which leads to the anti-resonance phenomenon at low frequencies. The dynamic analysis of the proposed REVI system has been meticulously investigated through analytical methods and real-world experimentation. The analytical method uses the Spectral element method to obtain the dynamic response, which is also validated by the experimental findings. Furthermore, the REVI transmittance sensitivity analysis was conducted by adjusting the rigid mass and system load and exploring methods for generating wide low-frequency bandgaps. The inclination angle of the REVI system is also varied, revealing the bandgap characteristics regarding negative transmittance level. The parametric study varying the geometric properties of the REVI system enhances our understanding of the bandgap and attenuation characteristics within the attenuation band. The REVI mechanism is practical and easily implemented, allowing for accurate and repeatable modeling. Moreover, the analytical observations assist in refining the shape of the REVI mechanism to achieve the necessary bandgap for the desired transmittance.

**Keywords:** Vibration isolation, Anti-resonance, Spectral element method, Transmittance, Band-gap

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## 1. Introduction

The demand for vibration isolators is crucial in several real-life applications, such as automobiles [1, 2], helicopter rotors [3–5], machine foundations [6, 7], offshore structures [8, 9], high rise buildings [10, 11], space structures [12] etc. In general, vibration isolation can be easily achieved through passive vibration isolation devices [13] such as base isolation devices [14–17]. However, structures in most civil and mechanical engineering applications are subjected to lower-frequency vibrations [18–20]. This lower frequency vibration isolation can be achieved by designing resonators with lower natural frequency [21–24], i.e., having

heavier mass or lower stiffness. However, lower stiffness and heavier mass are difficult to achieve from a stability perspective. Therefore, researchers are inclined toward designing vibration isolators that can work well in lower frequency ranges without increasing their static mass [25–29].

The occurrence of the anti-resonance in the lower frequency range gives rise to a low-frequency stop band, which can be identified from the transmittance plot [30–32]. Recently, researchers have tried to obtain antiresonances using several methods; such as hydraulic leverage [33, 34], levered spring-mass system [35], beams connected by a spring mass system [36–38], inertial amplifier mechanism [14, 39–41], effective negative stiffness [42–44], effective negative mass [45, 46], multi resonating base isolation [16, 38, 47, 48], etc. Besides theoretical studies, several experimental studies have been performed on fabricated isolators such as flexible platforms with rigid mass [49, 50], shape memory alloy spring elements [51], negative stiffness vibration isolator device [52, 53], compliant lever-type passive vibration isolators [54] etc.

Fabricating a vibration isolating device with attached resonating units is always challenging. Few researchers have fabricated the inertial amplifier [55] and negative stiffness devices [56] for low-frequency vibration isolation. The fabrication of pinned connections and rigid mass-less bars are the two prime intricacies associated with the manufacturing process of inertial amplifier devices. Often, slender beams are used at the junction and the thick section at the middle part to replicate the inertial amplifier [57]. However, the stress concentration in slender beams is very high in these devices. Additionally, jointed parts may get dislocated due to high amplitude vibration in a resonating device that connects springs and masses. [58, 59].

Due to the intricate forms of many vibration isolation mechanisms, 3D printing is now widely used in manufacturing [60–62] due to repeatability and dimensional precision [55, 63, 64]. The monolithic model creation using 3D printing technology has reduced the number of joints in a structure and the geometrical uncertainties of physical modeling. In this paper, the rigid elastic vibration isolator (REVI) model has been developed using 3D printing technology. The proposed REVI could be confused with a negative stiffness [46, 65] or an inertial amplifier model due to geometric resemblance; however, the working mechanism of REVI and its intrinsic properties are very different. For the analytical study of this REVI model, its dynamic stiffness matrix has been developed using spectral element matrix and rigid body dynamics [66–68]. The natural frequency of the developed REVI model has been obtained by experimental investigation, which has been used to compute the rotational stiffness of the beam joint. Further incorporating the rotational stiffness, the modified dynamic stiffness matrix has been obtained. The transmittance of REVI has been found experimentally, validating the analytical model developed. The primary novelty of the paper lies in fabricating a monolithic vibration isolator that uses the concept of rigid body dynamics and anti-resonance for broadband vibration isolation. Finally, a parametric study by varying nodal masses has been performed to gain insight into the physics behind antiresonance occurrence in transmittance. Additionally, a detailed study of change in transmittance level due to variation in geometric design has been carried out, which illustrated that the proper geometric design of REVI can create a double antiresonance peak in transmittance level,

which results in a wider bandgap with required vibration transmittance.

This manuscript is organized as follows. Following this introduction, in section 2, a detailed procedure is provided for fabricating the REVI system and an analytical algorithm to formulate the dynamic stiffness matrix using the spectral element method. section 3 provides a detailed procedure to estimate the rotational stiffness of the semi-rigid joint of the REVI system experimentally. section 4 describes the complete analytical and experimental procedure for obtaining the transmittance. A comparison study of the transmittance parameters obtained from both analytical and experimental is shown in section 5. Additionally, a few parametric studies were conducted to evaluate the band-gap characteristics for different geometric parameters of the REVI system, such as mass ratio, inclination angle, etc. Finally, the conclusion from the findings is summarized in section 6.

## 2. Theoretical analysis and design of REVI

This section provides a detailed procedure to fabricate the REVI model and a complete analytical algorithm to formulate the dynamic stiffness matrix using the spectral element method.

Table 1: Material and Geometrical properties

Apparatus	Property	Symbols	Units	Values
PLA material	Density	$\rho$	kg/m <sup>3</sup>	778.89
	Young's modulus	$E$	GPa	2.9
Elastic beam	Length	$l$	m	0.0453
	Width	$b$	m	0.01
	Thickness	$d$	m	0.001
	Cross-sectional area	$A = bd$	m <sup>2</sup>	10 <sup>-5</sup>
	Second moment of area	$I = bd^3/12$	m <sup>4</sup>	8.33x10 <sup>-13</sup>
	Length	$l$	m	0.01
Rigid mass	Width	$b$	m	0.01
	Thickness	$d$	m	0.01
	Mass	$m_r = bdl\rho$	kg	7.889x10 <sup>-4</sup>
	Polar moment of inertia	$J_r = m_r d^2/6$	kg/m <sup>2</sup>	1.298x10 <sup>-8</sup>
Force transducer	Sensitivity	—	mV/kN	2248
	Mass	$M_f$	kg	0.02697
Two accelerometers	Sensitivity	—	mV/(m/s <sup>2</sup> )	4.98
	Mass	$M_a$	kg	0.0236

### 2.1. REVI model fabrication

The CAD model of the proposed REVI is made in AutoCAD software, which is then converted to a .gcode file using Ultimaker Cura software (Figure 1 (a)). The .gcode file is then printed in the 3D printing device (Ender) with the help of additive manufacturing technology (Figure 1 (b)). The monolithic 3D printed REVI model is demonstrated in Figure 1 (c). The material used for the REVI model is polylactic acid (PLA), whose density

<sup>96</sup> was calculated by finding the sample's ratio of mass and volume. The model and sensors'  
<sup>97</sup> overall material and geometric properties have been described in Table 1.

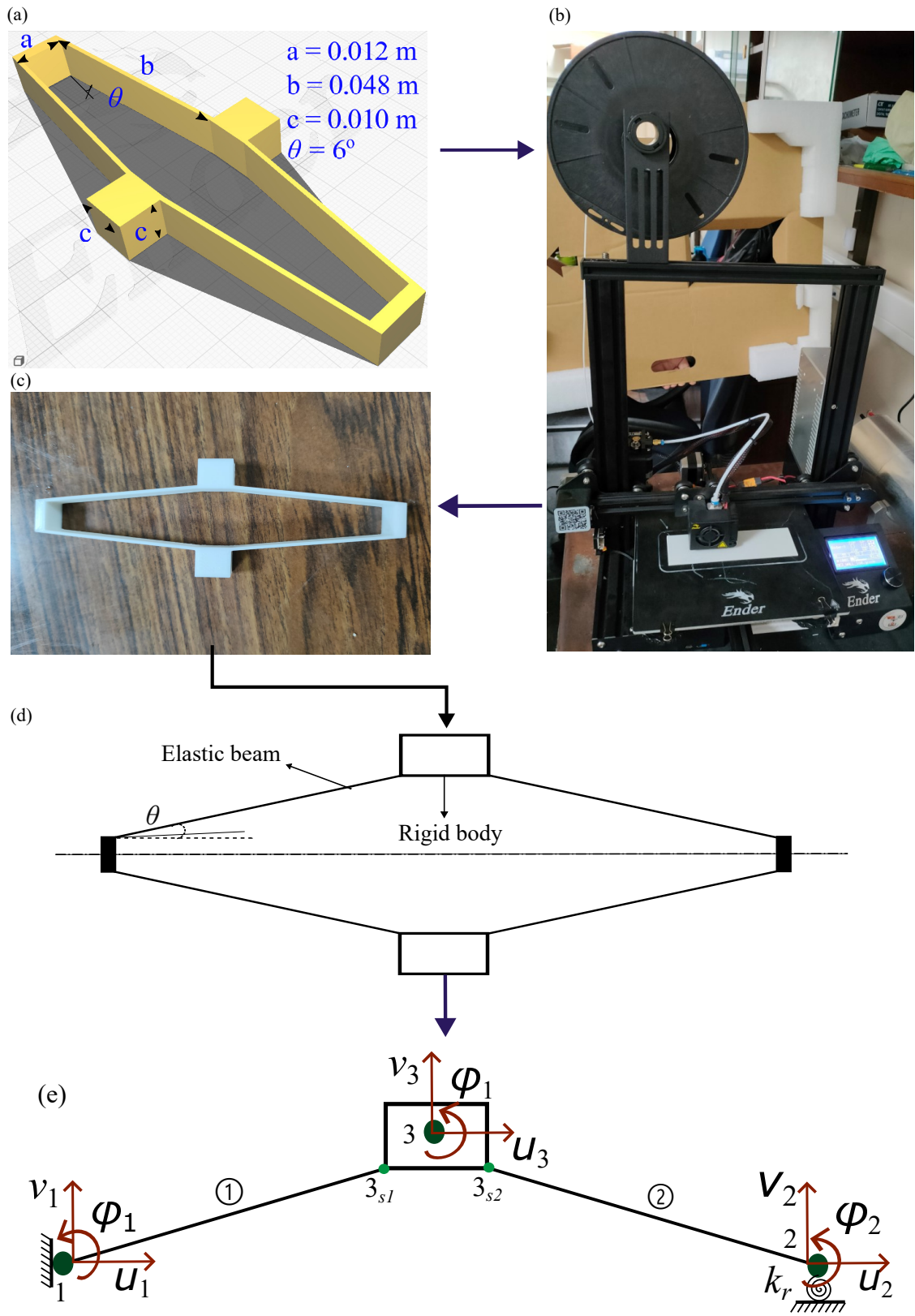


Figure 1: (a) CAD model of the REVI ; (b) 3D printing device; (c) 3D printed proposed REVI; (d) Analytically modeled REVI and (e) Degrees of freedom and boundary conditions at each node

## 2.2. Analytical model development

The REVI model is realized by coupling four elastic beams with two rigid bodies, as shown in Figure 1 (d). Exploiting the symmetry of this system, The analysis of only half part (shown in Figure 1 (e)) of the REVI has been carried out. Rigid body dynamics and spectral element techniques are used to obtain the global dynamic stiffness matrix [68]. The elastic beams are slender and assumed to follow the Euler-Bernoulli beam theory. The beams are modeled as frame elements, with three degrees of freedom in each node: axial, transverse, and rotational. To obtain the dynamic properties of the entire system, the REVI is subjected to input base excitation at one end. The output dynamic response at the other end is obtained regarding displacement. The details on the mathematical formulation of the dynamic stiffness matrix are provided in the subsequent sections.

### 2.2.1. Spectral element matrix in local coordinate system

The governing differential equation of motion (GDEM) for  $i^{th}$  beam element vibrating along the transverse direction can be expressed as follows,

$$EI \frac{\partial^4 v(x, t)}{\partial x^4} + \rho A \frac{\partial^2 v(x, t)}{\partial t^2} = 0 \quad (1)$$

and for axial direction, the GDEM can be defined as

$$E \frac{\partial^2 u(x, t)}{\partial x^2} - \rho \frac{\partial^2 u(x, t)}{\partial t^2} = 0 \quad (2)$$

Here,  $w$  and  $u$  represent the transverse and axial displacement of the  $i^{th}$  beam, which depends on the spatial coordinate ( $x$ ) and time ( $t$ ). The parameters  $EI$  and  $\rho A$  denote the beam's flexural rigidity and mass per unit length. A harmonic solution for the governing equation described above can be derived through the variable separable method for transverse direction, i.e.,

$$v(x, t) = V(x) e^{-i\omega t} \quad (3)$$

and, for axial direction, i.e.,

$$u(x, t) = U(x) e^{-i\omega t} \quad (4)$$

Here,  $V(x)$  and  $U(x)$  represent the amplitude of transverse and axial displacement of the  $i^{th}$  beam as a function of the spatial coordinate alone, and  $\omega$  is the frequency of the excitation. By substituting equation Eq. (3) and Eq. (4) into equation Eq. (1) and Eq. (2), the differential equations can be transformed into the spatial domain as follows:

$$EI \frac{\partial^4 V(x)}{\partial x^4} - \omega^2 \rho A V(x) = 0 \quad (5)$$

and,

$$E \frac{\partial^2 U(x)}{\partial x^2} + \omega^2 \rho U(x) = 0 \quad (6)$$

Under the assumption of a uniform section, where both  $EI$  and  $\rho A$  remain constant along the entire length of the beam, the transverse solution to the given equation can be expressed using hyperbolic trigonometric functions.

$$V(x) = A_1 \sin(\lambda x) + A_2 \cos(\lambda x) + A_3 \sinh(\lambda x) + A_4 \cosh(\lambda x) \quad (7)$$

and similarly, the solutions for the axial directions can be obtained as

$$U(x) = B_1 \sin(\mu x) + B_2 \cos(\mu x) \quad (8)$$

where,  $A_1$  through  $A_4$ ,  $B_1$  and  $B_2$  represent unknown arbitrary constants.  $\lambda$  and  $\mu$  represent the wave numbers for transverse and axial direction. By substituting equation Eq. (7) and Eq. (8) into equation Eq. (5) and Eq. (6), the wave numbers  $\lambda$  and  $\mu$  can be expressed as a function of the excitation frequency  $\omega$  as follows:

$$EI \lambda^4 V(x) - \omega^2 \rho A V(x) = 0 \Rightarrow \lambda = \sqrt[4]{\frac{\omega^2 \rho A}{EI}} \quad (9)$$

and

$$-EA \mu^2 U(x) + \omega^2 \rho A U(x) = 0 \Rightarrow \mu = \sqrt{\frac{\omega^2 \rho}{E}} \quad (10)$$

The state vectors at a single end of  $i^{th}$  beam element, specifically rotation ( $\phi$ ), axial force ( $P$ ), bending moment ( $M$ ), and shear force ( $V$ ), can be expressed in terms of the transverse displacement ( $w$ ) and axial displacement ( $u$ ) as

$$\begin{aligned} \phi(x, t) &= V^I(x) e^{-i\omega t}, F_x(x, t) = EA U^I(x) e^{-i\omega t}, \\ M(x, t) &= EIV^{II}(x) e^{-i\omega t}, F_y(x, t) = EIV^{III}(x) e^{-i\omega t} \end{aligned} \quad (11)$$

Substituting Eq. (7) and Eq. (8) in Eq. (11), the displacement state vectors for  $i^{th}$  beam element shown in Figure 1 (e) can be defined as

$$\mathbf{D} = \mathbf{H}\mathbf{\Theta} \Rightarrow \mathbf{\Theta} = \mathbf{H}^{-1}\mathbf{D} \quad (12)$$

where,

$$\mathbf{D}_j = \{ u_i \quad v_i \quad \phi_i \quad u_{i+1} \quad v_{i+1} \quad \phi_{i+1} \}^T, \mathbf{\Theta} = \{ B_1 \quad A_1 \quad A_2 \quad B_2 \quad A_3 \quad A_4 \}^T \text{ and}$$

$$\mathbf{H}_j = \begin{bmatrix} 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 1 \\ 0 & \lambda & 0 & 0 & \lambda & 0 \\ \sin(\mu l) & 0 & 0 & \cos(\mu l) & 0 & 0 \\ 0 & \sin(\lambda l) & \cos(\lambda l) & 0 & \sinh(\lambda l) & \cosh(\lambda l) \\ 0 & \lambda \cos(\lambda l) & -\lambda \sin(\lambda l) & 0 & \lambda \cosh(\lambda l) & \lambda \sinh(\lambda l) \end{bmatrix} \quad (13)$$

Here,  $l$  is the length of the  $i^{th}$  beam element. Similarly, the force state vectors for  $i^{th}$  beam element shown in Figure 1 (e) can be defined as

$$\mathbf{F}_j = \mathbf{X}_j \mathbf{K}_j \mathbf{\Theta} \quad (14)$$

where,  $\mathbf{F}_j = \{ F_{xi} \quad F_{yi} \quad M_i \quad F_{x(i+1)} \quad F_{y(i+1)} \quad M_{i+1} \}^T$   
 $= \{ F_{xi}(0) \quad F_{yi}(0) \quad M_i(0) \quad F_{x(i+1)}(L_j) \quad F_{y(i+1)}(L_j) \quad M_{i+1}(L_j) \}^T$ . Here,  $\mathbf{X}_i$   
and  $\mathbf{K}_i$  can be defined as

$$\mathbf{X}_j = \begin{bmatrix} -EA & 0 & 0 & 0 & 0 & 0 \\ 0 & EI & 0 & 0 & 0 & 0 \\ 0 & 0 & -EI & 0 & 0 & 0 \\ 0 & 0 & 0 & EA & 0 & 0 \\ 0 & 0 & 0 & 0 & -EI & 0 \\ 0 & 0 & 0 & 0 & 0 & EI \end{bmatrix} \quad (15)$$

and,

$$\mathbf{K}_j = \begin{bmatrix} \mu & 0 & 0 & 0 & 0 & 0 \\ 0 & -\lambda^3 & 0 & 0 & \lambda^3 & 0 \\ 0 & 0 & -\lambda^2 & 0 & 0 & \lambda^2 \\ \mu \cos(\mu L_j) & 0 & 0 & -\mu \sin(\mu L_j) & 0 & 0 \\ 0 & -\lambda^3 \cos(\lambda L_j) & \lambda^3 \sin(\lambda L_j) & 0 & \lambda^3 \cosh(\lambda L_j) & \lambda^3 \sinh(\lambda L_j) \\ 0 & -\lambda^2 \sin(\lambda L_j) & -\lambda^2 \cos(\lambda L_j) & 0 & \lambda^2 \sinh(\lambda L_j) & \lambda^2 \cosh(\lambda L_j) \end{bmatrix} \quad (16)$$

Substituting Eq. (12) in Eq. (14), the force-displacement relationship between the two ends of of the  $i^{th}$  beam element can be defined as

$$\mathbf{F}_j = \mathbf{X}_j \mathbf{K}_j \mathbf{\Theta} = \mathbf{X}_j \mathbf{K}_j \mathbf{H}_j^{-1} \mathbf{D}_j = \mathbf{S}_j \mathbf{D}_j \quad (17)$$



Here,  $\mathbf{S}_j$  is the dynamic stiffness matrix, also known as the Spectral element matrix of the  $i^{th}$  beam element, which can be expressed as

$$\mathbf{S}^i = \begin{bmatrix} s_{11} & 0 & 0 & s_{14} & 0 & 0 \\ 0 & s_{22} & s_{23} & 0 & s_{25} & s_{26} \\ 0 & s_{32} & s_{33} & 0 & s_{35} & s_{36} \\ s_{41} & 0 & 0 & s_{44} & 0 & 0 \\ 0 & s_{52} & s_{53} & 0 & s_{55} & s_{56} \\ 0 & s_{62} & s_{63} & 0 & s_{65} & s_{66} \end{bmatrix} \quad (18)$$

where,

$$\begin{aligned} s_{11} = s_{44} &= \frac{EA \mu \cos(\mu l_i)}{\sin(\mu l_i)} \\ s_{14} = s_{41} &= -\frac{EA \mu}{\sin(\mu l_i)} \\ s_{22} = s_{55} &= -\frac{EI \lambda^3 (\cosh(\lambda l_i) \sin(\lambda l_i) + \cos(\lambda l_i) \sinh(\lambda l_i))}{\cos(\lambda l_i) \cosh(\lambda l_i) - 1} \\ s_{23} = -s_{56} &= -\frac{EI \lambda^2 \sin(\lambda l_i) \sinh(\lambda l_i)}{\cos(\lambda l_i) \cosh(\lambda l_i) - 1} \\ s_{25} &= \frac{EI \lambda^3 (\sinh(\lambda l_i) + \sin(\lambda l_i))}{\cos(\lambda l_i) \cosh(\lambda l_i) - 1} \\ s_{26} = -s_{35} &= \frac{EI \lambda^2 (\cos(\lambda l_i) - \cosh(\lambda l_i))}{\cos(\lambda l_i) \cosh(\lambda l_i) - 1} \\ s_{33} = s_{66} &= \frac{EI \lambda (\cos(\lambda l_i) \sinh(\lambda l_i) - \cosh(\lambda l_i) \sin(\lambda l_i))}{\cos(\lambda l_i) \cosh(\lambda l_i) - 1} \\ s_{36} &= \frac{EI \lambda (\sin(\lambda l_i) - \sinh(\lambda l_i))}{\cos(\lambda l_i) \cosh(\lambda l_i) - 1} \end{aligned}$$

The displacement continuity equations and force equilibrium equations are used to derive the relation between primary node 3 (center of the rigid body) and secondary node  $3_{si}$  (beam's node connected to the rigid body). For beam  $i$ ,  $X_i$  and  $Y_i$  are distances between master node 3 and slave node  $3_{si}$  in global coordinates. Beam's local axis system makes an angle  $\theta_i$  with the global axis. The distances  $x_i$  and  $y_i$  in the local coordinate system are derived as

$$\begin{bmatrix} x_i \\ y_i \end{bmatrix} = \begin{bmatrix} \cos(\theta_i) & -\sin(\theta_i) \\ \sin(\theta_i) & \cos(\theta_i) \end{bmatrix} \begin{bmatrix} X_i \\ Y_i \end{bmatrix} \quad (19)$$

156 The dynamic stiffness matrix of  $i^{th}$  beam incorporating the second node at the center of  
 157 gravity of a rigid body can be defined as

$$\mathbf{R}^i = \mathbf{T}_{rd} \mathbf{S}^i \mathbf{T}_{rd}^T \quad (20)$$

158 where,

$$\mathbf{T}_{rd} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & y_i \\ 0 & 0 & 0 & 0 & 1 & -x_i \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \quad (21)$$

### 159 2.2.2. Spectral element matrix in global coordinate system

160 Further, the dynamic stiffness matrix in the local coordinate system  $\mathbf{R}_i$  is transformed  
 161 into a global coordinate system by coordinate transformation matrix  $\mathbf{T}$  as

$$\mathbf{K}^i = \mathbf{T} \mathbf{R}^i \mathbf{T}^T \quad (22)$$

162 where,

$$\mathbf{T} = \begin{bmatrix} \cos(\theta_i) & \sin(\theta_i) & 0 & 0 & 0 & 0 \\ -\sin(\theta_i) & \cos(\theta_i) & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \cos(\theta_i) & \sin(\theta_i) & 0 \\ 0 & 0 & 0 & -\sin(\theta_i) & \cos(\theta_i) & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \quad (23)$$

163 The force-displacement relation for beam 1 can be written as

$$\begin{Bmatrix} \mathbf{F}_1 \\ \mathbf{F}_3 \end{Bmatrix} = \begin{bmatrix} \mathbf{K}_{11}^1 & \mathbf{K}_{13}^1 \\ \mathbf{K}_{31}^1 & \mathbf{K}_{33}^1 \end{bmatrix} \begin{Bmatrix} \mathbf{U}_1 \\ \mathbf{U}_3 \end{Bmatrix} \quad (24)$$

164 Similarly, the force-displacement relation for beam 2 can be written as

$$\begin{Bmatrix} \mathbf{F}_2 \\ \mathbf{F}_3 \end{Bmatrix} = \begin{bmatrix} \mathbf{K}_{22}^2 & \mathbf{K}_{23}^2 \\ \mathbf{K}_{32}^2 & \mathbf{K}_{33}^2 \end{bmatrix} \begin{Bmatrix} \mathbf{U}_2 \\ \mathbf{U}_3 \end{Bmatrix} \quad (25)$$

165 where,  $\mathbf{U}_j$  is displacement vector and  $\mathbf{F}_j$  is force vector of  $j^{th}$  node as

$$\mathbf{U}_j = \begin{Bmatrix} u_j \\ v_j \\ \phi_j \end{Bmatrix} \quad \mathbf{F}_j = \begin{Bmatrix} F_{xj} \\ F_{yj} \\ M_j \end{Bmatrix} \quad (26)$$

### 2.2.3. Assembly

Due to inertial mass in the axial and transverse direction, the inertial force matrix and polar moment of inertia in the rotational degree of freedom must be added at corresponding nodes. Let  $m_j$  and  $J_j$  be the mass and polar moment of inertia at  $j^{th}$  node. The inertial force matrix is written as

$$\mathcal{I}_j = -\omega^2 \underbrace{\begin{bmatrix} m_j & 0 & 0 \\ 0 & m_j & 0 \\ 0 & 0 & J_j \end{bmatrix}}_{\mathbf{I}_j} \mathbf{U}_j \quad (27)$$

The final assembled dynamic stiffness matrix of the system in global coordinates can be written as

$$\begin{Bmatrix} \mathbf{F}_1 \\ \mathbf{F}_2 \\ \mathbf{F}_3 \end{Bmatrix} = \begin{bmatrix} \mathbf{K}_{11}^1 - \mathbf{I}_1 & 0 & \mathbf{K}_{13}^1 \\ 0 & \mathbf{K}_{22}^2 - \mathbf{I}_2 & \mathbf{K}_{23}^2 \\ \mathbf{K}_{31}^1 & \mathbf{K}_{32}^2 & \mathbf{K}_{33}^1 + \mathbf{K}_{33}^2 - \mathbf{I}_3 \end{bmatrix} \begin{Bmatrix} \mathbf{U}_1 \\ \mathbf{U}_2 \\ \mathbf{U}_3 \end{Bmatrix} \quad (28)$$

$\underbrace{\hspace{15em}}_{\bar{\mathbf{K}}}$

### 2.2.4. Boundary condition

The analytical model exploits the symmetricity of the system, which restrains the degree of freedom in the  $v$  direction for node 1 and 2, i.e.,  $v_1 = v_2 = 0$ . Node 1 will be fixed with the base plate, which can move only in the  $u$  direction, so the rotational degree of freedom at node 1 will be restrained, so  $\phi_1 = 0$ . However, at node 2, the rotational degree of freedom is not fully restrained. Therefore, the rotational spring of stiffness  $k_r$  is modeled at the node 2. Therefore, the rotational spring with stiffness  $k_r$  has been modeled at node 2. Therefore, the rotational spring stiffness  $k_r$  will be added in the global dynamic stiffness matrix ( $\bar{\mathbf{K}}$ ) at the stiffness coefficient corresponding to moment and rotation at node 2 as

$$\bar{\mathbf{K}}(6, 6) = \bar{\mathbf{K}}(6, 6) + k_r \quad (29)$$

The boundary conditions of this system are defined as

$$\begin{aligned} v_1 &= \phi_1 = 0 \\ v_2 &= 0 \\ M_2 &= k_r \phi_2 \end{aligned} \quad (30)$$

After applying the boundary conditions, the reduced force-displacement equation can be written as

$$\begin{bmatrix} F_{x1} \\ F_{x2} \\ M_2 \\ F_{x3} \\ F_{y3} \\ M_3 \end{bmatrix} = \bar{\mathbf{K}}_{bc} \begin{bmatrix} u_1 \\ u_2 \\ \phi_2 \\ u_3 \\ v_3 \\ \phi_3 \end{bmatrix} \quad (31)$$

where, the reduced stiffness matrix ( $\bar{\mathbf{K}}_{bc}$ ) has been derived in Appendix A.

### 3. Estimation of rotational stiffness $k_r$

While the rotational degree of freedom is fully restrained, the rotational stiffness  $k_r$  is infinite; however, the rotational stiffness value zero indicates free rotation. As node 2 is a semi-rigid joint, the value of its rotational stiffness  $k_r$  will be between 0 to  $\infty$ , which needs to be obtained experimentally. In this paper, the experiment has been carried out to find the system's natural frequency and equate it with the analytical natural frequency of the same mode to determine the rotational stiffness  $k_r$  of the joint.

#### 3.1. Experimental setup for determining natural frequency

The natural frequency of the REVI can be estimated by fixing it to a fixed base situated at a certain height, and an accelerometer was attached at the bottom of the sample. Small displacement was provided at the bottom in a vertical direction, allowing the model to vibrate freely (Figure 2 (a)). The vertical acceleration vs. time was recorded as shown in Figure 2 (b), which is then converted to the frequency domain using the Fast Fourier Transform (FFT) algorithm (Figure 2 (c)). The FFT graph's maximum amplitude is the REVI's natural frequency, i.e., 92 Hz ( $\omega_n = 578.05$  rad/s).

#### 3.2. Determining natural frequency analytically

Nodal masses and boundary conditions are similar to the experimental setup and have been applied in the Eq. (31) to determine the natural frequency. One side of the designed REVI has been fixed at the top, and the other is free to move. Therefore, The inertia matrix ( $\mathbf{I}_1$ ) at node 1 is zero as no additional mass is attached. Further, the attached accelerometer at the bottom is modeled as a point mass at node 2.  $m_2 = m_a/2$  ( $m_2$  is half of the accelerometer mass due to symmetric condition), and  $J_2 = 0$  has been used in the inertia matrix  $\mathbf{I}_2$ . The inertia matrix at node 3 will have mass ( $m_3 = m_r$ ) and polar moment of inertia ( $J_3 = J_r$ ) as calculated for a rigid body in Table 1.

The node 1 is fixed at the top, restraining the displacement in the  $u$  direction, thus incorporating the boundary condition  $u_1 = 0$  in Eq. (31). Further, by static condensation

the reduced matrix  $\bar{\mathbf{K}}_{bc1}(\omega, k_r)$  has been obtained Appendix B. The determinant of the dynamic stiffness matrix will be zero at its natural frequency.

$$|\bar{\mathbf{K}}_{bc1}(\omega_n, k_r)| = 0 \quad (32)$$

The analytical natural frequency for free rotation ( $k_r = 0$ ) and restrained rotation ( $k_r = \infty$ ) has been obtained as 73.21 Hz and 100.27 Hz, respectively. It validates that the experimental natural frequency lies within the natural frequency obtained analytically, 73.21 to 100.27 Hz. Therefore, applying this concept, the rotational spring stiffness  $k_r$  is obtained in Figure 3 by varying  $k_r$ . The  $|\bar{\mathbf{K}}_{bc}(\omega_n, k_r)|$  is found zero at rotational stiffness  $k_r = 0.3591$  Nm/rad.

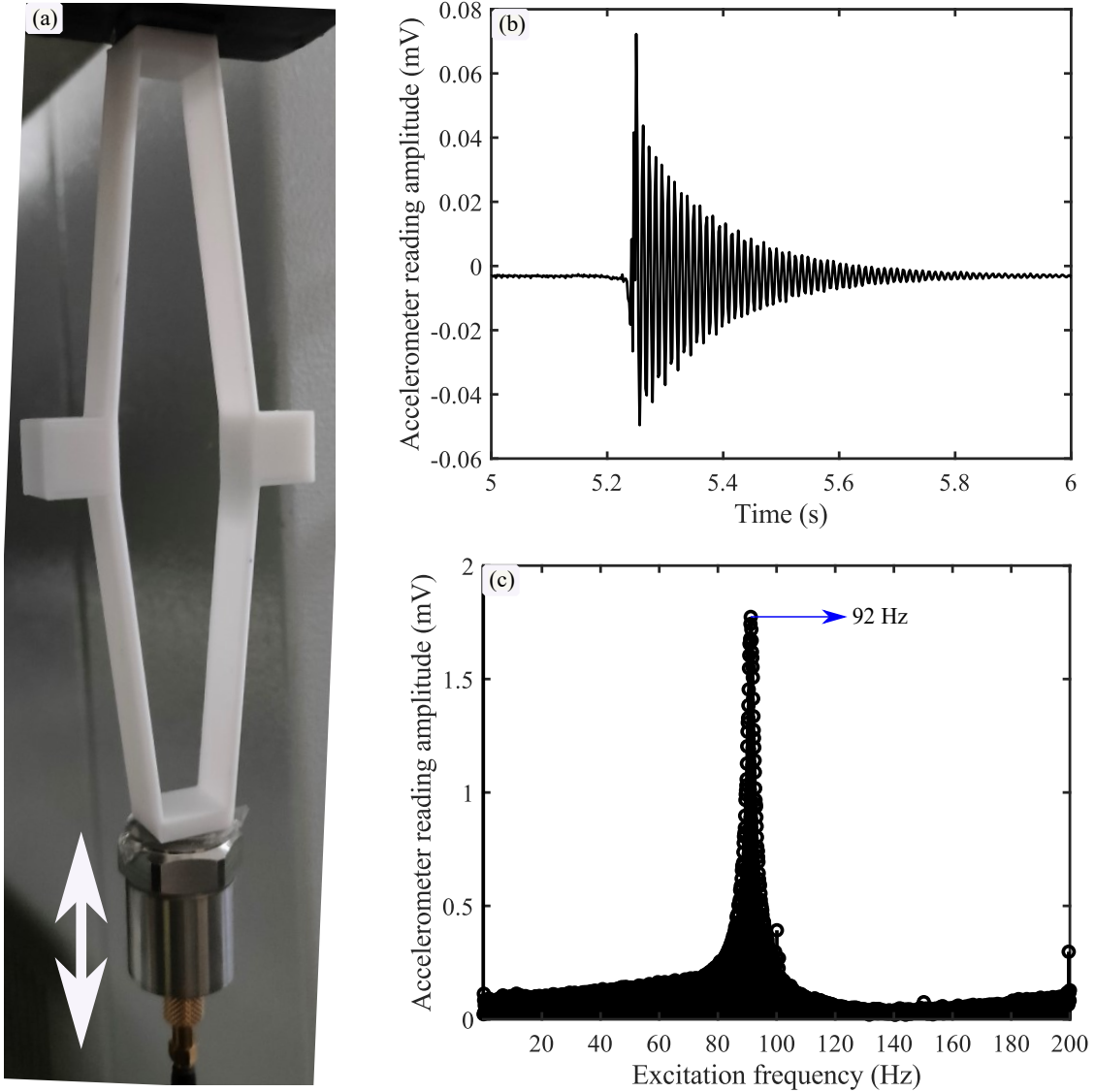


Figure 2: (a) Free vibration of REVI model for estimation of natural frequency; (b) Time acceleration plot of the free vibration analysis; (c) Frequency domain plots with the help of FFT algorithm (sensitivity of accelerometer =  $4.98 \text{ mV/ms}^{-2}$ )

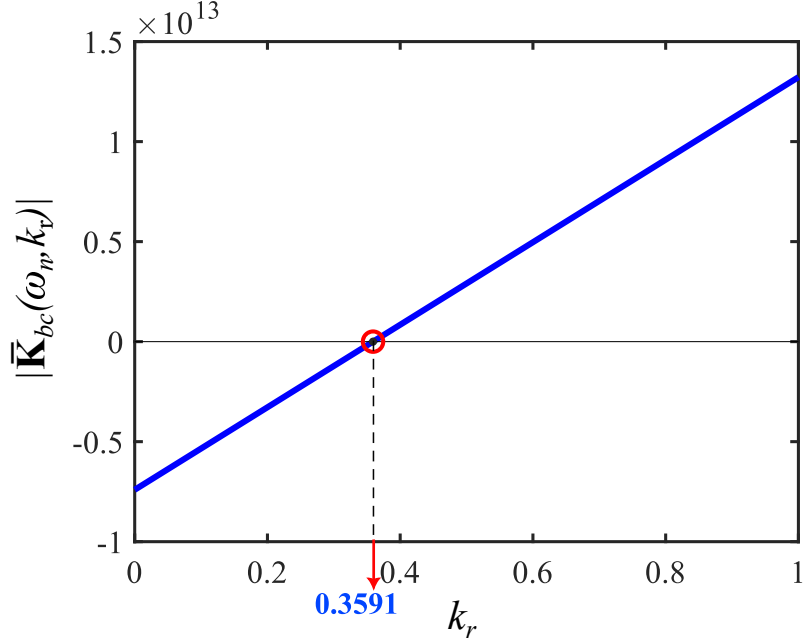


Figure 3: Plot of determinant of dynamic stiffness matrix for natural frequency  $\omega_n = 578.05$  rad/s with rotational spring stiffness  $k_r$ . The zero determinant is shown by a circle marker at the rotational stiffness  $k_r = 0.3591$  Nm/rad.

### 3.3. Natural Modal shapes

The REVI model has been simulated in COMSOL software, incorporating geometric and material properties outlined in Table 1 and utilizing the rotational stiffness ( $k_r$ ) determined in Figure 3. The model considered in the COMSOL software is the same as in the analytical model due to its symmetricity, as shown in Figure 1 (d). The results include the natural frequencies and their corresponding modal shapes, as illustrated in Figure 4 (a-c). Additionally, analytically obtained natural frequencies are presented in Figure 4 (d) through a plot of  $\log_{10}(|(\mathbf{K}_{bc})|)$  against frequency. This representation effectively highlights the occurrence of spikes at natural frequencies, where the determinant of the spectral element matrix becomes zero. The negligible difference between analytically derived and COMSOL-generated natural frequencies serves as a validation of the model. The modal shapes depicted in Figure 4 (a-c) provide insights into the vibration mechanism of the model.

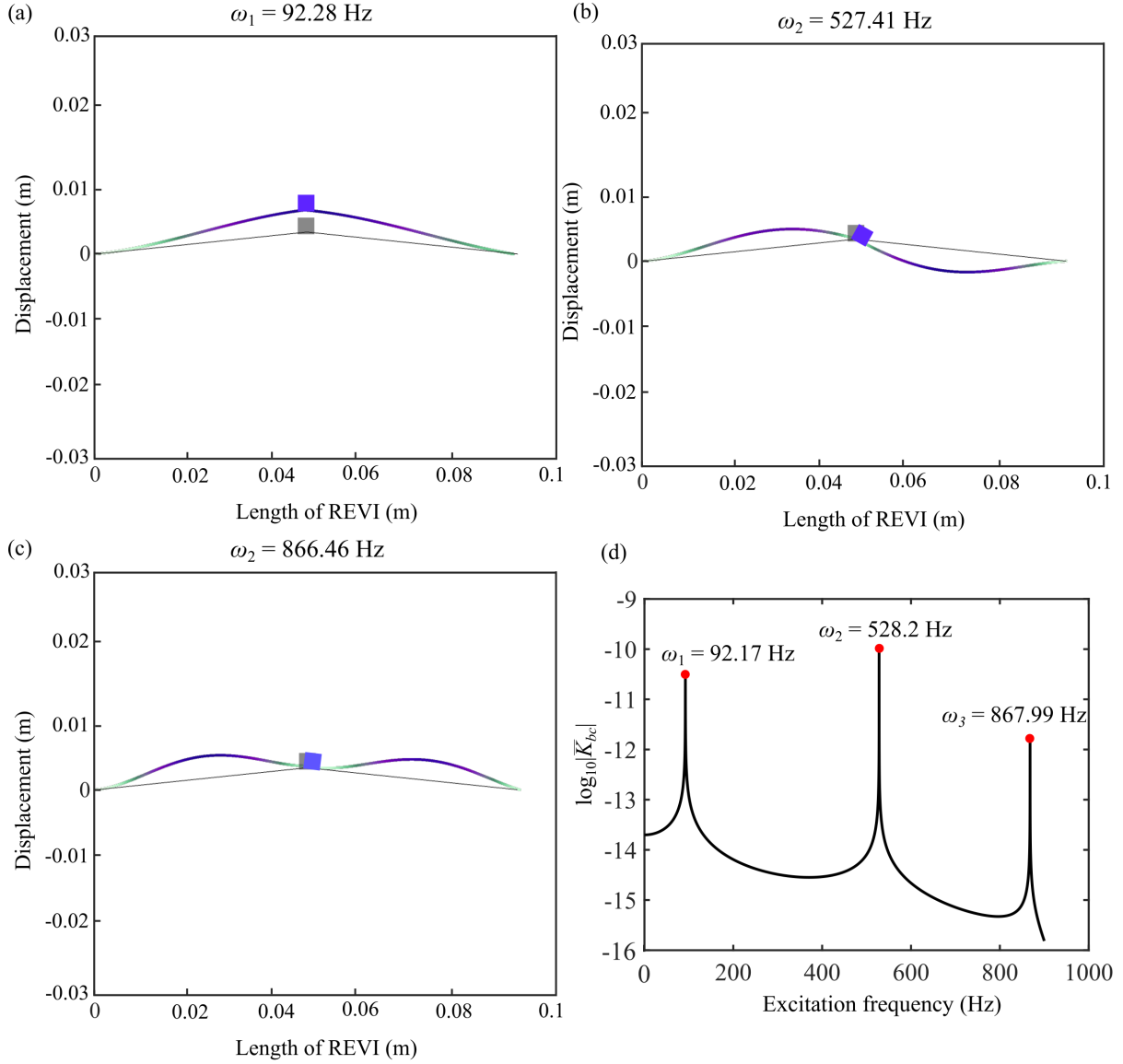


Figure 4: (a) First, (b) Second and (c) Third modal shapes of REVI and (d) Logarithm of the determinant of spectral element matrix ( $\mathbf{K}_{bc}$ ) of REVI.

## 4. Estimation of vibration isolation performance

The vibration isolation performance is estimated from the vibration transmittance from the base to the system. The transmittance in the  $u$  direction between node 1 and 2 has been obtained analytically and experimentally for validation.

### 4.1. Experimental procedure for transmittance computation

This section shows the detailed experimental procedure to compute the transmittance. Thus, this section is further subdivided into two parts, i.e., experimental setup, which shows

the complete procedure of setting up the REVI to the dynamic shaker, and post-processing, which elucidates the detailed procedure of obtaining the transmittance.

#### 4.1.1. Experimental setup

As shown in Figure 5 (b), the experimental setup consists of a PLA rigid beam whose one end is fixed with the base plate of the dynamic shaker, and the other is attached to the REVI model. A force transducer of sensitivity 2248 mV/kN is connected between the rigid beam and the REVI model (Figure 5 (b)) to record the amount of reaction force generated due to the vibration of REVI. Two accelerometers ( $A_1$  and  $A_2$ ) having a sensitivity of 4.98 mV/ms<sup>-2</sup> have been placed at the top and bottom of the REVI to measure the transmittance as shown in Figure 5 (b). The accelerometers and force transducer are connected in the corresponding channels of the data acquisition system (DAQ) as shown in Figure 5 (c). The DAQ is then connected to the PC interface, which converts the continuous analog signals to discrete analog signals, which can be simulated through LabView software (Figure 5 (d)). With the aid of a power amplifier (Figure 5 (a)), desired frequency values and voltage gain are provided to the dynamic shaker, which converts electrical energy to mechanical energy and produces a harmonic excitation at the base plate of the shaker system. The whole setup is configured to vibrate vertically.

#### 4.1.2. Post processing of the measured raw data

The dynamic shaker provides monochromatic harmonic sinusoidal base excitation to the REVI. The frequency range is varied from 20 Hz to 160 Hz. The LabView software obtains the time vs. amplitude graphs in millivolt (mV). The raw data for each frequency has been acquired after waiting a few seconds to allow the system to vibrate steadily. The time vs. force in Newton (N) and time vs. acceleration in m/s<sup>2</sup> are obtained from the force transducers and accelerometers using the sensitivity values given in Table 1 as shown in Figure 5 (e) to Figure 5 (g). The frequency domain plots of the corresponding time domain plots can be obtained through the FFT algorithm as shown in Figure 5 (h) to Figure 5 (j). The ratio of frequency amplitude of the bottom accelerometer ( $A_2$ ) to the top accelerometer ( $A_1$ ), also known as transmittance ( $T_r$ ) for the above frequency range, is compared with the corresponding analytical methods shown in Figure 6.

#### 4.2. Analytically obtaining transmittance

In this setup, the force transducer is attached to node 1, the accelerometer is attached to node 2, and the rigid body of PLA is at node 3. The nodal masses and polar moment of inertia at node 1, 2 and 3 due to force transducer, accelerometer, and rigid body will be accounted as  $m_1 = M_f/2$ ,  $J_1 = 0$ ,  $m_2/2 = m_a$ ,  $J_2 = 0$  and  $m_3 = m_r$ ,  $J_3 = J_r$  (Table 1).

Here, node 1 is fixed at the base plate of the beam connected to the shaker. The displacement of node 1 will be the same as the rigid beam end due to the vibrating shake base plate. The rigid beam is vibrating only in  $u$  directional degree of freedom. As the system is assumed to be in the linear range, the base displacement boundary condition  $u_1 = 1$  can be assumed for all the frequencies. Applying base excitation boundary condition by static



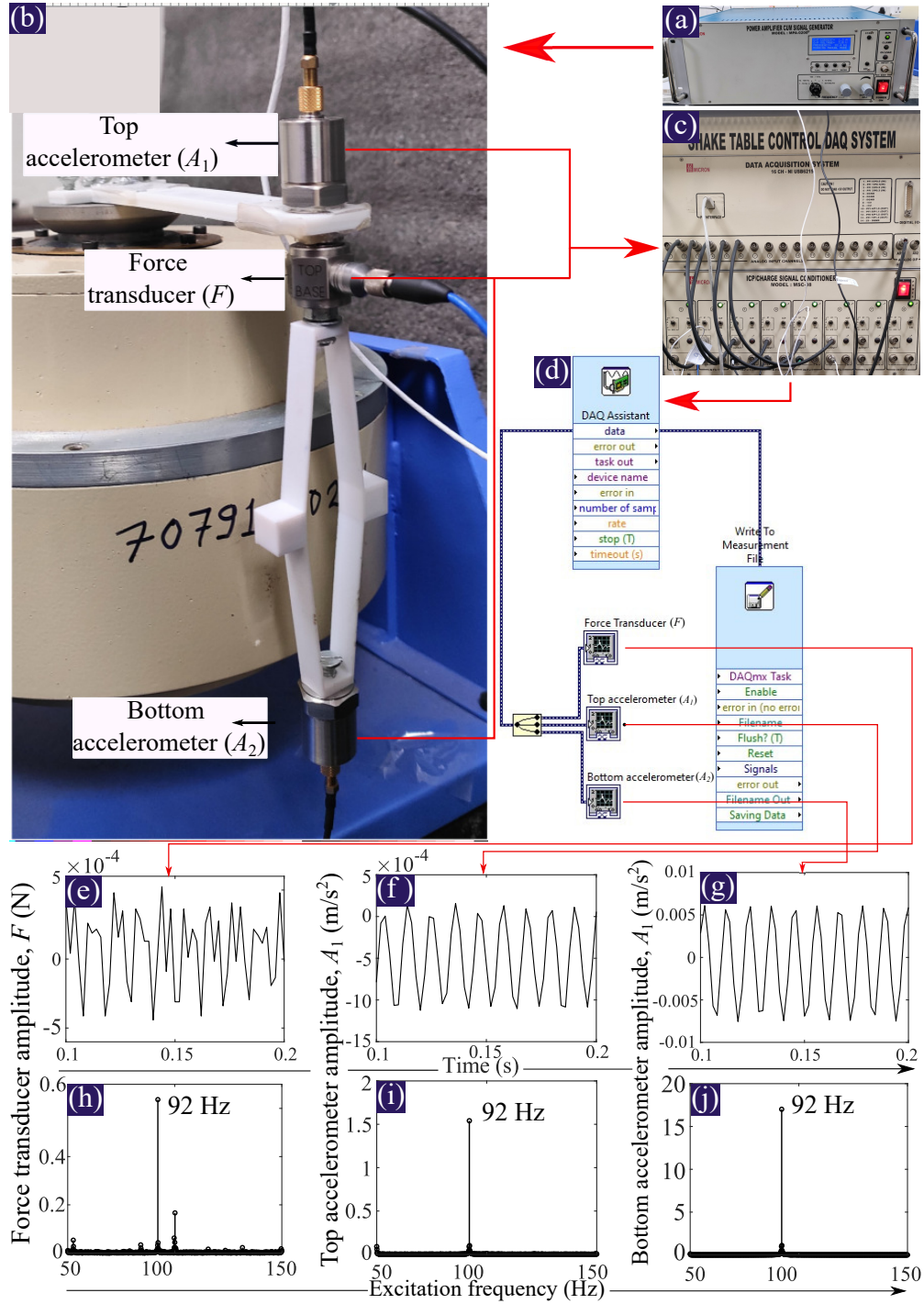


Figure 5: (a) Power amplifier to vibrate the shaker system at desired gain and frequency; (b) REVI connected to the dynamic shaker system; (c) DAQ device used to collect the response of accelerometers and force transducers; (d) Simulation of data acquisition system using LabView software; (e) to (j) Time and frequency domain response of force transducers and accelerometers at a frequency of 92 Hz.

condensation to Eq. (31), the reduced Force displacement equation can be determined as given in Appendix C.

The transmittance of vibration in  $u$  direction of base (node 1)  $u_1$ , to the accelerometer (node 2)  $u_2$  can be obtained by

$$T_r = \log_{10} \left( \frac{u_2}{u_1} \right) \quad (33)$$

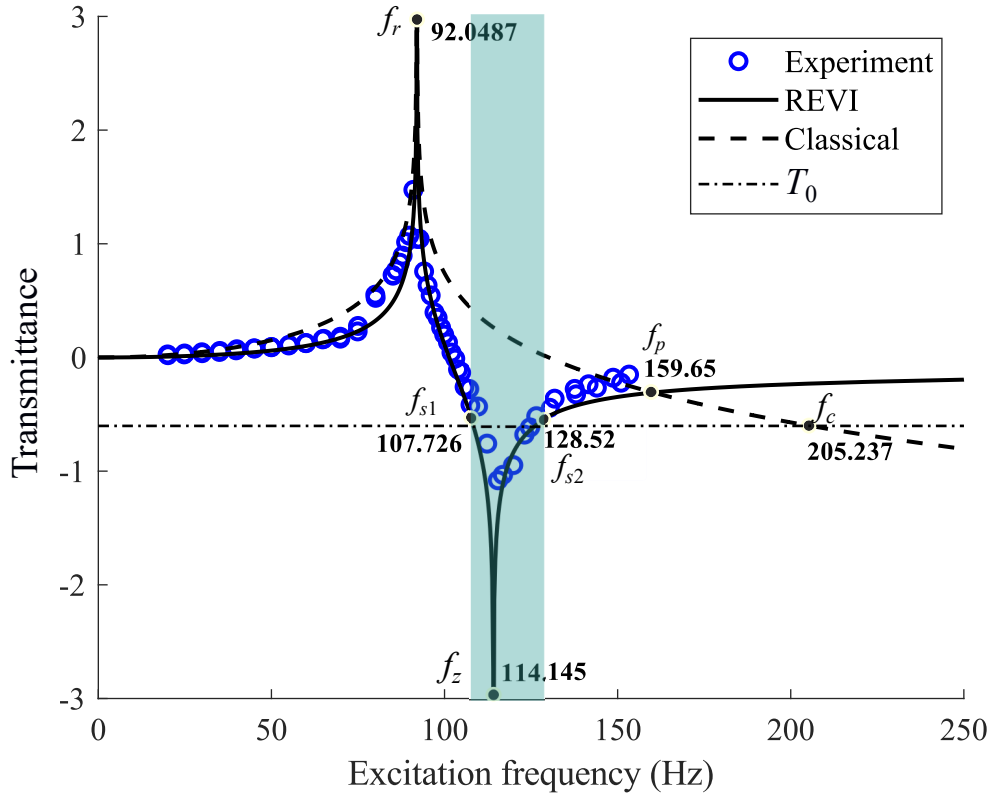


Figure 6: Transmittance of the proposed REVI system from experimental and analytical computation compared to an equivalent spring-mass system. The REVI system's bandwidth (shaded region) is 0.1767 for one-fourth amplitude transmittance.

## 5. Results and discussions

The transmittance of REVI has been compared with the classical spring mass resonator of a similar natural frequency (92 Hz). In equivalent classical resonator; keeping the mass ( $M_c = M_2$ ), the equivalent spring stiffness ( $K_c$ ) of classical resonator has been obtained as

$$K_c = \omega_n^2 M_c \quad (34)$$

285 The transmittance of the classical resonator can be obtained as

$$T_c = \log_{10} \left( \left| \frac{K_c}{K_c - \omega^2 M_c} \right| \right) = \log_{10} \left( \left| \frac{1}{1 - (\omega^2/\omega_r^2)} \right| \right) \quad (35)$$

286 The transmittance obtained for REVI using Eq. (33) and classical resonator using Eq. (35)  
 287 is plotted in Figure 6 in solid and dashed lines, respectively. The transmittance peak occurs  
 288 at resonance frequency ( $f_r = 92.0487$  Hz). However, an anti-resonance dip is obtained in  
 289 REVI at ( $f_z = 114.145$  Hz). The transmittance of REVI is less than the classical resonator  
 290 till ( $f_p = 159.65$  Hz) as shown in Figure 6. Generally, any system's allowable transmittance  
 291 ( $T_0$ ) is given. Let the allowable transmittance be ( $T_0 = \log_{10} 0.25$ ). Therefore the black  
 292 dashed line shown in Figure 6 shows logarithmic transmittance level, which cuts trans-  
 293 mittance of REVI at ( $f_{s1} = 107.726$  Hz) and ( $f_{s2} = 128.52$  Hz) and classical resonator at  
 294 ( $f_c = 205.237$  Hz). The frequency ranges  $f_{s1}$  to  $f_{s2}$  demonstrates the stop band for the  
 295 REVI system for maximum transmittance  $T_0$ , which is much less than the possible stopping  
 296 frequency we get from a classical resonator. The bandwidth of REVI can be calculated as  
 297 [35]

$$BW = \frac{\omega_{s2} - \omega_{s1}}{\sqrt{\omega_{s1} \omega_{s2}}} = \frac{f_{s2} - f_{s1}}{\sqrt{f_{s1} f_{s2}}} = 0.1767 \quad (36)$$

### 298 5.1. Experimental validation

299 A harmonic monochromatic sinusoidal base excitation of frequency range 20 Hz to 160  
 300 Hz is provided to the REVI through the dynamic shaker. The acceleration time histories  
 301 and their corresponding frequency domain plots can be obtained using the procedure defined  
 302 in subsection 4.1. The ratio of peak frequency amplitude of the bottom accelerometer to the  
 303 top accelerometer ( $\log_{10}(A_2/A_1)$ ), also known as transmittance, has been plotted in Figure 6  
 304 with blue circles. The transmittance calculated here is the ratio of acceleration in the  $u$   
 305 direction at nodes 2 and 1, which will be equal to velocity or displacement transmittance.  
 306 Further, this experimental transmittance for the above frequency range is compared with  
 307 the corresponding analytical displacement transmittance. From Figure 6, it is observed  
 308 that the experimental transmittance values are successfully validated with the analytical  
 309 transmittance of REVI, providing us essential confidence in our proposed methodology.

### 310 5.2. Parametric Study

311 The sensitivity of resonance and anti-resonance peaks in transmittance has been studied  
 312 by varying the masses at nodes 2 and 3. The transmittance  $T_r$  has been plotted (Figure 7)  
 313 for the following cases as

- 314 • The mass  $m_2 = 0$  and mass  $m_3 = 2^p m_r$ , where  $p$  varies from  $-1$  to  $3$  as shown in  
 315 Figure 7 (a). It has been observed that resonance and anti-resonance occur at the same  
 316 frequency. This frequency shifts to a lower frequency as the rigid mass  $m_3$  increases.  
 317 However, in this scenario, the width of the bandgap is too narrow.

- The mass  $m_2 = 2^q m_r$  and mass  $m_3 = m_r$ , where  $q$  varies from 2 to 6 as shown in Figure 7 (b). It can be observed that by increasing mass at node 2, the resonating frequency of transmittance shifts to a lower frequency. However, the anti-resonance frequency does not change. So, it can be concluded that anti-resonance depends on the rigid mass of the REVI and not on the system's resting mass. The higher the mass ratio  $\alpha = \frac{m_2}{m_3}$ , the wider will be the band gap. However, the mid-frequency  $f_z$  depends on the rigid mass  $m_2$ .
- The mass  $m_2 = 2^q m_r$  and mass  $m_3 = 2^p m_r$ , where  $p$  varies from  $-2$  to  $2$  and  $q$  varies from 2 to 6 as shown in Figure 7(c). Here, the mass ratio  $\alpha = 2^4$  is constant. It can be seen that the resonance and anti-resonance both peaks shift towards a lower frequency range as the system's total mass increases. Similarly, it can be observed that the bandgap also gets wider for higher system mass.

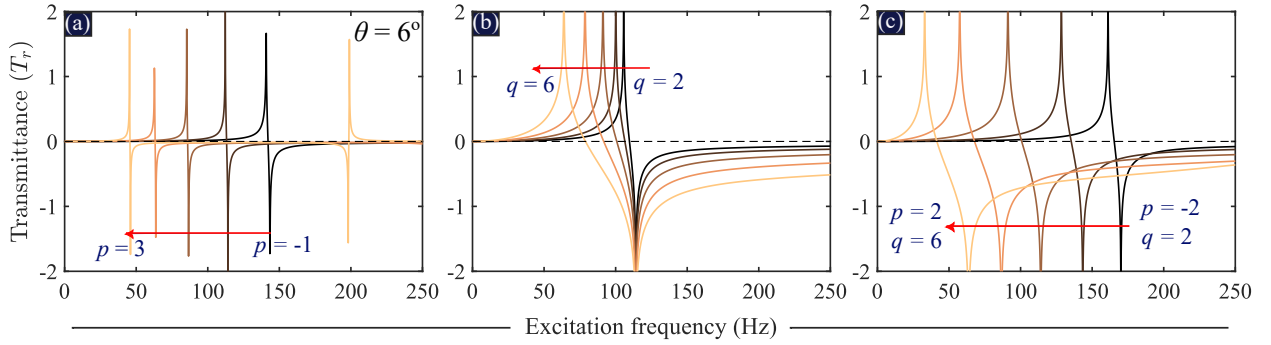


Figure 7: Variation of Transmittance ( $T_r$ ) with the excitation frequency for different mass ratios. (a) The mass  $m_2 = 0$  and mass  $m_3 = 2^p m_r$ , where  $p$  varies from  $-1$  to  $3$ , (b) The mass  $m_2 = 2^q m_r$  and mass  $m_3 = m_r$ , where  $q$  varies from  $2$  to  $6$ , (c) The mass  $m_2 = 2^q m_r$  and mass  $m_3 = 2^p m_r$ , where  $p$  varies from  $-2$  to  $2$  and  $q$  varies from  $2$  to  $6$

### 5.2.1. Relative transmittance

Anti-resonance frequency is the critical parameter for designing the base isolator, as it determines the mid-frequency of the band gap. The following plots have been obtained Figure 8 by keeping the mass  $m_3 = m_r$  constant and varying the mass  $m_2 = 2^q$ , where  $q$  increases from  $0$  to  $2$ , for different transmittance, to obtain the rationale behind this anti-resonance phenomenon as follows.

- First of all the transmittance  $T_r = \log_{10}(\frac{u_2}{u_1})$  has been plotted in Figure 8 (a). As discussed earlier, this demonstrates the constant  $f_z$  frequency with an increase in bandwidth as an increase in mass  $m_2$ .
- Further in Figure 8 (b), the transmittance of rigid mass  $m_3$  in  $T_{r1} = \log_{10}(\frac{u_3}{u_1})$  has been plotted. It can be noticed that the resonance frequencies are the same as that of

$T_r$ , but the anti-resonance frequency is changed. The precise observation has noticed that all three cases' transmittance plots have one common point, which appears at the anti-resonance frequency  $f_z$ .

- The observation of common point motivated to obtain the transmittance of node 3 for node 2, to get the knowledge of the relative motion of masses  $m_3$  and  $m_2$ . The Transmittance  $T_{r2} = \log_{10}(\frac{u_3}{u_2})$  and Transmittance  $T_{r3} = \log_{10}(\frac{v_3}{u_2})$  has been plotted in Figure 8 (c). In all three cases and both the transmittance  $T_{r2}$  and  $T_{r3}$ , the resonance frequency is the same as that of anti-resonance frequency  $f_z$ . So, it can be concluded that the anti-resonance of the system depends on the relative resonance of the base isolator concerning the load of the system resting on the isolator.

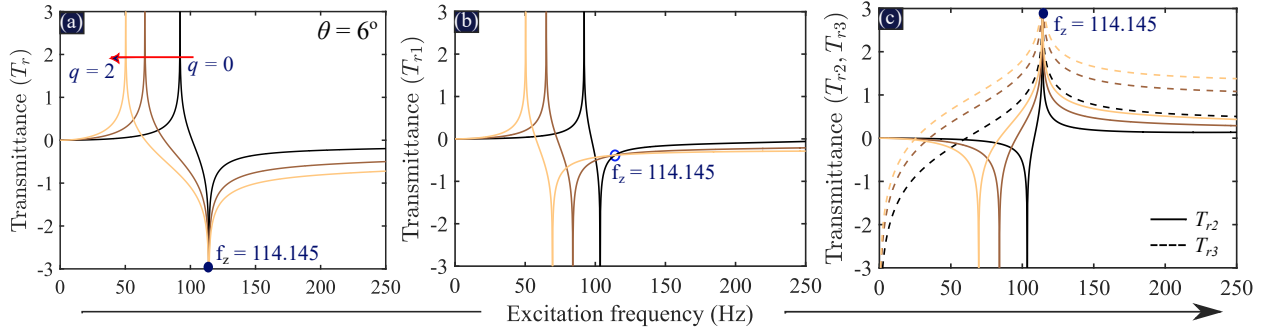


Figure 8: Variation of transmittance ( $T_r$ ) and relative transmittance's ( $T_{r1}$ ) and ( $T_{r2}$ ) with the excitation frequency for different mass ratios. The mass  $m_1 = 2^q m_a$  and mass  $m_2 = m_r$ , where  $q$  increases from 0 to 2 (a) Transmittance  $T_r = \log_{10}(\frac{u_2}{u_1})$ , (b) Transmittance  $T_{r1} = \log_{10}(\frac{u_3}{u_1})$ , (c) Transmittance  $T_{r2} = \log_{10}(\frac{u_3}{u_2})$  and transmittance  $T_{r3} = \log_{10}(\frac{v_3}{u_2})$ . The resonance in relative transmittance results in antiresonance in transmittance ( $T_r$ ).

### 5.2.2. Sensitivity to inclination angle

Further, a parametric study has been conducted showing the variation of transmittance as a function of excitation frequency for different values of inclination angle ( $\theta = 20^\circ, 30^\circ, 33.42^\circ$ , and  $40^\circ$ ) as illustrated in Figure 9 (a). Figure 9 (b) shows a contour plot about the negative transmittance profile (represented by negative values of transmittance plot in Figure 9 (a)) as a function of excitation frequency and inclination angle for enhanced comprehension of the bandgap and level of attenuation within the attenuation band. From Figure 9 (b), it can be observed that double anti-resonance peaks can be obtained in the attenuation band for inclination angles up to  $33.41^\circ$ . The presence of two neighboring anti-resonances without an in-between resonance peak is noteworthy since this increases the bandwidth of the attenuation effect significantly [69]. For  $\theta = 33.42^\circ$ , the two attenuation peaks merge to form a single attenuation peak. Further, an increment of the angle creates the resonance cancellation phenomenon, completely vanishing the attenuation peaks and further reducing the band gap.

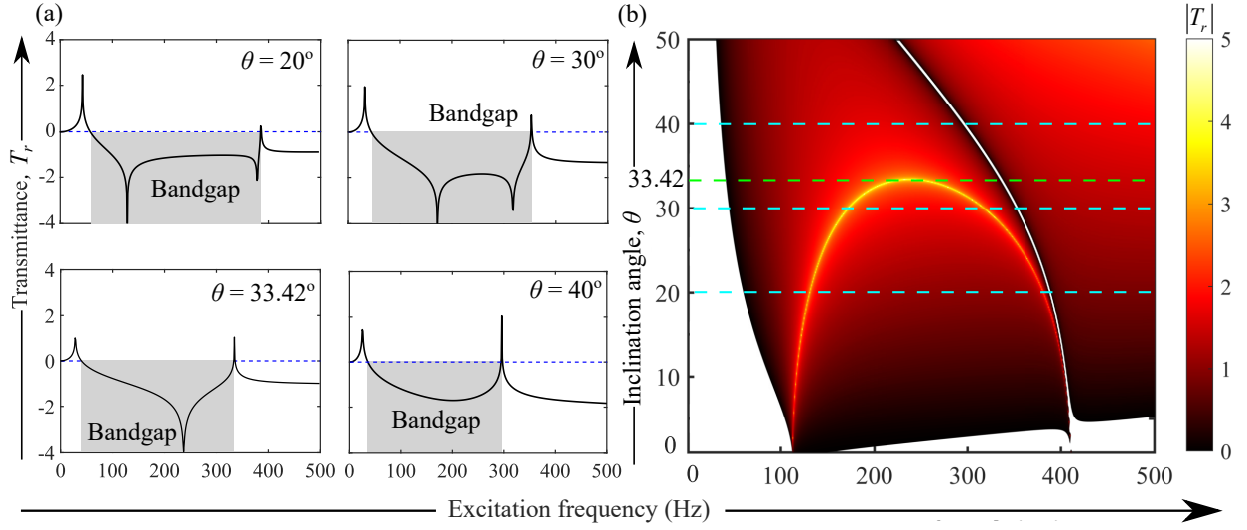


Figure 9: Effect of inclination angle on bandgap having node masses as  $7.889 \times 10^{-4}$  kg. (a) Transmittance plots for different values of inclination angle  $\theta$ . (b) Attenuation profile (represented by negative values of transmittance plot) as a function of excitation frequency and inclination angle. In the attenuation band, double attenuation peaks can be obtained for inclination angles up to  $33.41^\circ$ . For  $\theta = 33.42^\circ$ , the two attenuation peaks merge to form a single attenuation peak. Further, an increment of the angle creates the resonance cancellation phenomenon, completely vanishing the attenuation peaks and further reducing the band gap.

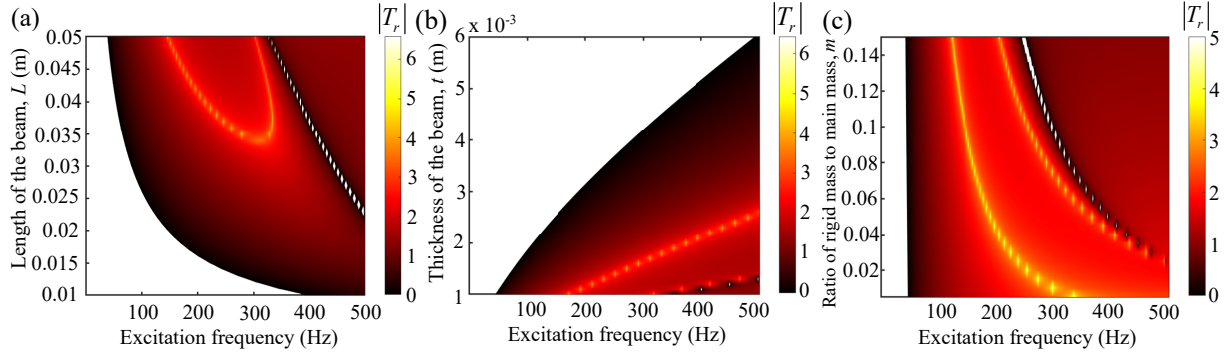


Figure 10: Attenuation profile (represented by negative values of transmittance plot) as a function of excitation frequency and several geometric parameters such as (a) Length of the beam. (b) The thickness of the beam. (c) The mass ratio (ratio of the rigid mass to the primary mass of the system).

### 5.2.3. Sensitivity of beam length, thickness, and nodal masses

The variation of the anti-resonance frequency of other geometric parameters of the REVI system has been observed using the contour plots shown in Figure 10. Figure 10 (a) to (c) shows the variation of negative transmittance profile (represented by negative values of transmittance) as a function of excitation frequency and three predominant geometric

parameters such as length of the beams, the thickness of the beams and the nodal mass ratio. Figure 10 (a) depicts the emergence of double anti-resonance dips in the first band of REVI as we increase the length of the beam. Further, Figure 10 (b) showcases the linear change in the position of antiresonance dips as the change in beam thickness. Moreover, Figure 10 (c) demonstrates the double anti-resonance peaks in the first bandgap coming closer as the mass ratio increases. The main purpose of the contour plots is to decide the range of parameters that can be considered to achieve the desired anti-resonant frequencies.

#### 5.2.4. Transmittance metrics and Relative bandgap

In continuation to subsubsection 5.2.3, to quantify the properties of the REVI band gap with double transmittance drops, three metrics are proposed as shown in Figure 11 (a) following [45]. The metric  $\mu_{\min}$  denotes the minimum level of attenuation achieved within the double anti-resonance drops within the frequency range  $\Omega_{\min}$  as illustrated in the figure. As for the metric  $\Omega_{\max} = f_2 - f_1$ , this represents the conventional band-gap width. In both frequency metrics, the bandwidth is normalized to its central frequency value,  $\Omega_{\text{mid}} = \frac{f_1+f_2}{2}$ . Upon normalization,  $\Omega_{\min}$  and  $\Omega_{\max}$  are denoted  $\Omega_{\min}^*$  and  $\Omega_{\max}^*$ , respectively. The variation of metrics as a function of inclination angle is plotted in Figure 11 (b). The variation in transmittance level (blue curve) can be achieved for an inclination angle from 4 to 33.41 degrees. At 33.42 degrees, the double transmittance drops merge into a single drop, thus vanishing the transmittance level. Additionally, for an inclination angle less than 4 degrees, the range between the double peaks coincides with zero, thus again vanishing the transmittance level. Similarly, the red curve shows the maximum relative bandgap, which increases from 120 percent to 160 percent with an increase in inclination level. It is also observed that the minimum relative bandgap (black curve) decreases with the inclination angle, which shows a trade-off between different relative bandgaps.

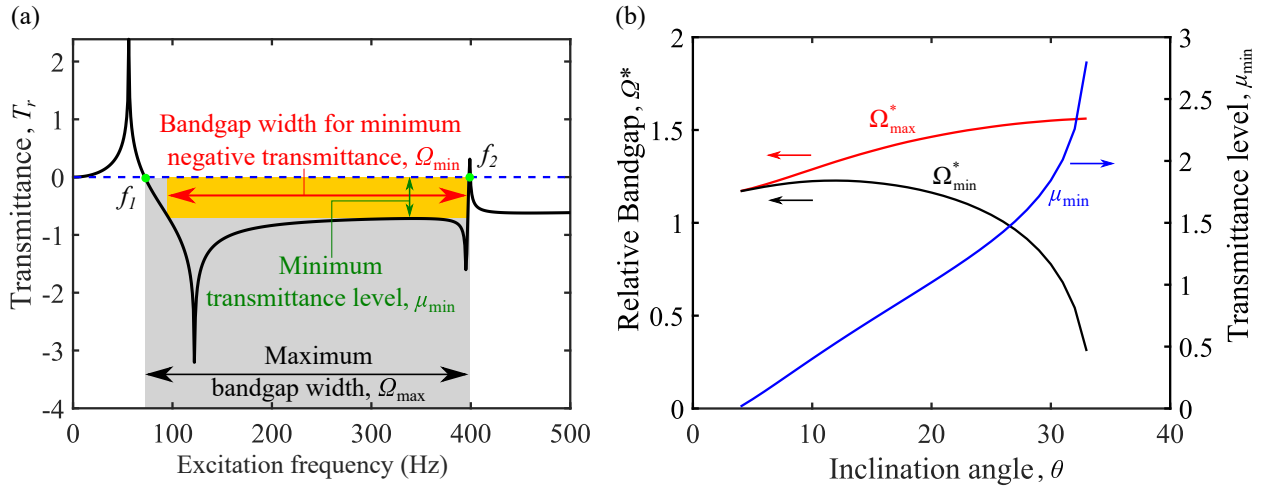


Figure 11: (a) Illustration of three metrics for the quantification of the relative band gap size, minimum transmittance level, and relative band gap size corresponding to the minimum transmittance level; (b) Variation of the band-gap and transmittance metrics to inclination angle.



## 6. Conclusion

The dynamic response analysis of a base-excited novel monolithic rigid elastic vibration isolator (REVI) system has been conducted using analytical and experimental methods. The REVI model has been fabricated using a 3D printer. An analytical formulation of the proposed model is done by applying the spectral element method and rigid body dynamics concept. The displacement transmittance between the fixed and free end of the REVI has been validated experimentally, and a stop band of bandwidth of 0.1767 has been obtained for a maximum allowable transmittance of 0.25. The designed REVI gives a stop band in a frequency range lower than the equivalent classical spring mass resonator. The antiresonance phenomenon in REVI facilitates this occurrence of a lower-frequency stop band.

Moreover, parametric studies have been conducted by varying the masses to obtain the rationale behind the anti-resonance peak in transmittance. The higher the mass ratio  $\alpha$ , widens the band gap, and the increase in the mass shifts the band towards the lower frequency range. The relative resonance of the rigid mass of REVI to the system's mass gives the antiresonance in the system transmittance. Additionally, an increment in the inclination angle  $\theta$  enables us to obtain two neighboring anti-resonance peaks, significantly increasing the attenuation effect. The variation of  $\theta$  also provides an insight into the bandwidth characteristics (horizontal and vertical) from which an optimum angle can be decided depending on the design requirement.

The primary novelty of the paper lies in fabricating a monolithic vibration isolator that uses the concept of rigid body dynamics and anti-resonance for broadband vibration isolation. The fabrication of the REVI is straightforward and without any manual connection. Therefore, this monolithic model has the benefits of repeatability and accuracy. Moreover, the analytical method is simple enough to apply in any modified version of the solved REVI system. Thus, the proposed REVI system can be a vibration isolator for the obtained stop band. In the future scope of this work, the proposed REVI type system can be applied in several real-life applications, such as automobiles and machine foundations where limited vibration transmittance is allowed.

## 7. Acknowledgements

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Additionally, the authors would like to acknowledge that a patent application related to the work described in this manuscript has been filed by the Indian Institute of Technology Delhi with the Indian Patent Office.



## 433 Appendix A. Reduced dynamic stiffness matrix ( $\bar{\mathbf{K}}_{bc}$ )

434 The global force-displacement relation derived can be written as per Eq. (28) as

$$\begin{bmatrix} F_{x1} \\ F_{y1} \\ M_1 \\ F_{x2} \\ F_{y2} \\ M_2 \\ F_{x3} \\ F_{y3} \\ M_3 \end{bmatrix} = \underbrace{\begin{bmatrix} \bar{K}_{11} & \bar{K}_{12} & \cdots & \bar{K}_{19} \\ \bar{K}_{21} & \bar{K}_{22} & & \\ & & \ddots & \\ \vdots & & & \vdots \\ & & & & \ddots & \\ & & & & & \ddots & \\ \bar{K}_{91} & \cdots & \bar{K}_{99} \end{bmatrix}}_{\bar{\mathbf{K}}} \begin{bmatrix} u_1 \\ v_1 \\ \phi_1 \\ u_2 \\ v_2 \\ \phi_2 \\ u_3 \\ v_3 \\ \phi_3 \end{bmatrix} \quad (\text{A.1})$$

435 Now, rearranging the global spectral element matrix ( $\bar{\mathbf{K}}_{bc1}$ ) by separating known degrees of  
 436 freedom (from boundary conditions given in Eq. (30)) as

$$\begin{bmatrix} F_{x1} \\ F_{x2} \\ M_2 \\ F_{x3} \\ F_{y3} \\ M_3 \\ \hline F_{y1} \\ M_1 \\ F_{y2} \end{bmatrix} = \begin{bmatrix} & | & \\ & | & \\ & | & \\ \bar{\mathbf{K}}_{rr} & | & \bar{\mathbf{K}}_{rc} \\ & | & \\ & | & \\ \hline & | & \\ \bar{\mathbf{K}}_{cr} & | & \bar{\mathbf{K}}_{cc} \\ & | & \\ & | & \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ \phi_2 \\ u_3 \\ v_3 \\ \phi_3 \\ \hline v_1 \\ \phi_1 \\ v_2 \end{bmatrix} \quad (\text{A.2})$$

437 Now, applying the static condensation to the Eq. (C.1), the condensed spectral element  
 438 matrix ( $\bar{\mathbf{K}}_{bc}$ ) can be obtained as

$$\bar{\mathbf{K}}_{bc} = \bar{\mathbf{K}}_{rr} \quad (\text{A.3})$$

### 439 Appendix B. Reduced dynamic stiffness matrix ( $\bar{\mathbf{K}}_{bc1}$ )

440 Now, rearranging the global spectral element matrix  $\bar{\mathbf{K}}$  by separating known degrees of  
 441 freedom as per boundary conditions ( $u_1 = v_1 = \phi_1 = v_2 = 0$ )

$$\begin{bmatrix} F_{x2} \\ M_2 \\ F_{x3} \\ F_{y3} \\ M_3 \\ F_{x1} \\ F_{y1} \\ M_1 \\ F_{y2} \end{bmatrix} = \begin{bmatrix} & & & & & & & & \\ & & & & & & & & \\ & & \bar{\mathbf{K}}_{rr} & & & \bar{\mathbf{K}}_{rc} & & & \\ & & & & & & & & \\ & & & & & & & & \\ & & & & & & & & \\ & & & & & & & & \\ & & & & & & & & \\ & & \bar{\mathbf{K}}_{cr} & & & \bar{\mathbf{K}}_{cc} & & & \\ & & & & & & & & \end{bmatrix} \begin{bmatrix} u_2 \\ \phi_2 \\ u_3 \\ v_3 \\ \phi_3 \\ u_1 \\ v_1 \\ \phi_1 \\ v_2 \end{bmatrix} \quad (\text{B.1})$$

442 Now, applying the static condensation to the Eq. (B.1), the condensed spectral element  
 443 matrix ( $\bar{\mathbf{K}}_{bc1}$ ) can be obtained as

$$\bar{\mathbf{K}}_{bc1} = \bar{\mathbf{K}}_{rr} \quad (\text{B.2})$$

### 444 Appendix C. Reduced force displacement equation for transmittance study

445 Applying the boundary conditions of zero displacements, the dynamic force-displacement  
 446 equation can be written as per Eq. (31). Further, the obtained dynamic stiffness matrix  
 447 Eq. (A.3) has been divided into four parts as per known and unknown degrees of freedom  
 448 as follows

$$\begin{bmatrix} F_{x1} \\ F_{x2} \\ M_2 \\ F_{x3} \\ F_{y3} \\ M_3 \end{bmatrix} = \begin{bmatrix} \bar{K}_{11} & \bar{\mathbf{K}}_{1r} \\ & \\ \bar{\mathbf{K}}_{r1} & \bar{\mathbf{K}}_{rr} \\ & \\ & \end{bmatrix} \begin{bmatrix} u_1 = 1 \\ u_2 \\ \phi_2 \\ u_3 \\ v_3 \\ \phi_3 \end{bmatrix} \quad (\text{C.1})$$

$$\longrightarrow \begin{bmatrix} F_{x1} \\ \mathbf{F}_r \end{bmatrix} = \begin{bmatrix} \bar{K}_{11} & \bar{\mathbf{K}}_{1r} \\ \bar{\mathbf{K}}_{r1} & \bar{\mathbf{K}}_{rr} \end{bmatrix} \begin{bmatrix} 1 \\ \mathbf{u}_r \end{bmatrix}$$

449 Finally, the reduced force-displacement equation can be written as

$$\mathbf{F}_r = \bar{\mathbf{K}}_{r1} + \bar{\mathbf{K}}_{rr} \mathbf{u}_r \quad (\text{C.2})$$

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