

# Solution: Doubly Reinforced Beam (Practice Question 3)

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October 29, 2025

## Problem Statement

A doubly reinforced concrete beam has a cross-section of 300 mm (width)  $\times$  550 mm (overall depth).

- The tension reinforcement consists of 4 bars of 25 mm diameter.
- The compression reinforcement consists of 2 bars of 16 mm diameter.

The clear cover to all reinforcement is 30 mm. Use M25 grade concrete ( $f_{ck} = 25 \text{ N/mm}^2$ ) and Fe415 grade steel ( $f_y = 415 \text{ N/mm}^2$ ). Find the ultimate Moment of Resistance of the section.

## Solution

### Step 1: Given Data and Properties

- Width,  $b = 300 \text{ mm}$
- Overall Depth,  $D = 550 \text{ mm}$
- Concrete, M25:  $f_{ck} = 25 \text{ N/mm}^2$
- Steel, Fe415:  $f_y = 415 \text{ N/mm}^2$
- Area of Tension Steel ( $A_{st}$ ):

$$A_{st} = 4 \times \frac{\pi}{4} \times (25)^2 = 1963.5 \text{ mm}^2$$

- Area of Compression Steel ( $A_{sc}$ ):

$$A_{sc} = 2 \times \frac{\pi}{4} \times (16)^2 = 402.1 \text{ mm}^2$$

- Effective Depth (Tension,  $d$ ):

$$d = D - \text{clear\_cover} - \frac{\text{dia}_{\text{tension}}}{2} = 550 - 30 - \frac{25}{2} = 507.5 \text{ mm}$$

- Effective Cover (Compression,  $d'$ ):

$$d' = \text{clear\_cover} + \frac{\text{dia}_{\text{comp}}}{2} = 30 + \frac{16}{2} = 38 \text{ mm}$$

### Step 2: Find Limiting Neutral Axis ( $x_{u,max}$ )

For Fe415 grade steel, the maximum allowed neutral axis depth is:

$$x_{u,max} = 0.48 \times d$$

$$x_{u,max} = 0.48 \times 507.5 = \mathbf{243.6 \text{ mm}}$$

### Step 3: Diagnose the Beam (Find Actual $x_u$ )

We find the actual neutral axis,  $x_u$ , by equating the total compressive force ( $C_u$ ) and total tensile force ( $T_u$ ).

$$C_u = T_u$$

- **Total Tension ( $T_u$ ):** (Assuming tension steel yields)

$$T_u = 0.87 f_y A_{st} = 0.87 \times 415 \times 1963.5 = \mathbf{708,575 \text{ N}}$$

- **Total Compression ( $C_u$ ):** (By trial and error)

$$C_u = C_{concrete} + C_{steel} = (0.36 f_{ck} b x_u) + (f_{sc} - 0.45 f_{ck}) A_{sc}$$

Let's try  $x_u = 211.5 \text{ mm}$ :

First, find the strain in compression steel ( $\epsilon_{sc}$ ) at this  $x_u$ :

$$\epsilon_{sc} = \frac{0.0035(x_u - d')}{x_u} = \frac{0.0035(211.5 - 38)}{211.5} = 0.00288$$

From the IS 456 stress-strain curve for Fe415, the yield strain is  $\approx 0.00276$ . Since  $\epsilon_{sc} > 0.00276$ , the steel yields. For  $\epsilon_{sc} = 0.00288$ ,  $f_{sc} \approx \mathbf{353 \text{ N/mm}^2}$ .

Now, calculate  $C_u$ :

$$C_u = (0.36 \times 25 \times 300 \times 211.5) + (353 - 0.45 \times 25) \times 402.1$$

$$C_u = 571,050 + (353 - 11.25) \times 402.1$$

$$C_u = 571,050 + (341.75) \times 402.1$$

$$C_u = 571,050 + 137,420 = \mathbf{708,470 \text{ N}}$$

**Diagnosis:** Since  $C_u(708,470 \text{ N}) \approx T_u(708,575 \text{ N})$ , our trial  $x_u = 211.5 \text{ mm}$  is correct. We compare  $x_u$  to  $x_{u,max}$ :

$$x_u(211.5 \text{ mm}) < x_{u,max}(243.6 \text{ mm})$$

The section is **Under-Reinforced**, which is a valid ductile design.

### Step 4: Calculate Ultimate Moment of Resistance ( $M_u$ )

Since the section is under-reinforced, we use  $x_u = 211.5 \text{ mm}$ . We find  $M_u$  by taking moments of the compression forces about the tension steel.

$$M_u = M_{u,c} + M_{u,s}$$

- **Moment from Concrete ( $M_{u,c}$ ):**

$$\begin{aligned} M_{u,c} &= 0.36 f_{ck} b x_u (d - 0.42 x_u) \\ &= (0.36 \times 25 \times 300 \times 211.5) \times (507.5 - 0.42 \times 211.5) \\ &= 571,050 \times (507.5 - 88.83) \\ &= 571,050 \times 418.67 \\ &= 239,076,818 \text{ N-mm} \\ M_{u,c} &= \mathbf{239.08 \text{ kNm}} \end{aligned}$$

- **Moment from Compression Steel ( $M_{u,s}$ ):**

$$\begin{aligned} M_{u,s} &= (f_{sc} - 0.45 f_{ck}) A_{sc} (d - d') \\ &= (353 - 0.45 \times 25) \times 402.1 \times (507.5 - 38) \\ &= (341.75) \times 402.1 \times 469.5 \\ &= 64,484,720 \text{ N-mm} \\ M_{u,s} &= \mathbf{64.48 \text{ kNm}} \end{aligned}$$

- **Total Moment ( $M_u$ ):**

$$\begin{aligned} M_u &= M_{u,c} + M_{u,s} \\ M_u &= 239.08 \text{ kNm} + 64.48 \text{ kNm} \\ M_u &= \mathbf{303.56 \text{ kNm}} \end{aligned}$$

### **Final Answer**

The ultimate Moment of Resistance ( $M_u$ ) of the section is **303.56 kNm**.