

## Example 2 of welded eccentric connection in tension and shear

### Bracket connection

Determine the size of fillet weld required for the bracket connection shown in Figure 10.28. The web welds are to be taken as one half the leg length of the flange welds. All dimensions and loads are shown in the figure.

Design assuming rotation about  $XX$  axis

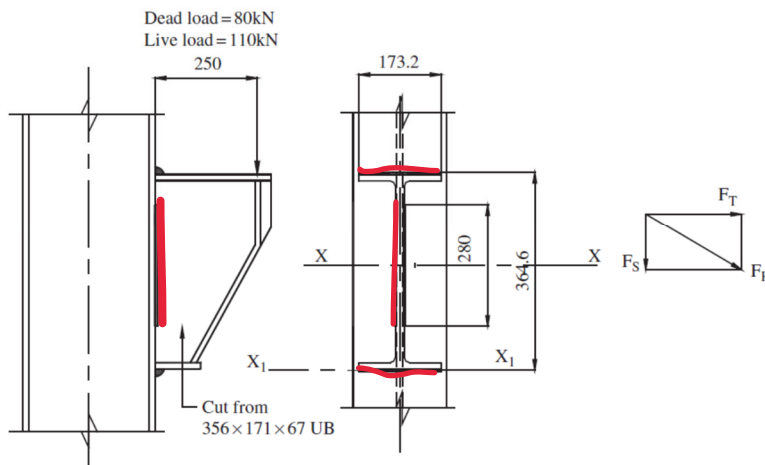
Direct shear	$F_s = P/L,$	Equations used
Load due to moment	$F_T = Ped/2I_x,$	
Resultant load	$F_R = (F_T^2 + F_s^2)^{0.5}.$	

$$\text{Factored load} = (1.4 \times 80) + (1.6 \times 110) = 288 \text{ kN},$$

$$\text{Length } L = (2 \times 173.2) + 280 = 626.4 \text{ mm},$$

$$\text{Inertia } I_x = (2 \times 173.2 \times 182^2) + 280^3/12 = 13.3 \times 10^6 \text{ mm}^3,$$

$$\text{Direct shear } F_s = 288/626.4 = 0.46 \text{ kN/mm},$$



Bracket connection

$$d/2 = 364.6/2 = 182 \text{ mm}$$

1-rotation around  $-X-X$

$$\text{Shear from moment } F_T = \frac{288 \times 250 \times 182}{13.3 \times 10^6} = 0.985 \text{ kN/mm},$$

$$\text{Resultant shear } F_R = [0.462^2 + 0.985^2]^{0.5} = 1.09 \text{ kN/mm}.$$

Provide 8-mm fillet welds for the flanges, strength 1.23 kN/mm. For the web welds provide 6-mm fillets (the minimum size recommended).

2 rotation around  $X_1-X_1$

Design assuming rotation about  $X_1X_1$  axis

The flange weld resists the moment  $-288 \times 250$

$$F_T = \frac{288 \times 250}{364 \times 173.2} = 1.14 \text{ kN/mm}.$$

Provide 8-mm fillet welds, strength 1.23 kN/mm. The web welds resist the shear:

$$F_s = 288/(2 \times 280) = 0.514 \text{ kN/mm}.$$

Provide 6-mm fillet welds. The methods give the same results.

how it been calculated for flange  
use electrode 35 then strength = 220 kN/mm  
leg 8mm then throat =  $0.7 \times 8 = 5.6 \text{ mm}$   
then strength of weld  
 $= 5.6 \times 220 / 1000 = 1.23 \text{ kN/mm}$  Greater than  $F_T = 0.985 \text{ kN/mm}$

how it been calculated for the web  
leg = 6mm then throat =  $0.7 \times 6 = 4.2 \text{ mm}$   
strength for of weld in web  
 $= 4.2 \times 220 / 1000 = 0.924 \text{ kN/mm}$   
greater than  $F_s = 0.46 \text{ kN/mm}$

### SECOND METHOD OF ANALYSIS

In a second assumption rotation takes place about the bottom flange  $X_1X_1$ . The flange welds resist moment and web welds shear. In this case:

$$F_T = Pe/db$$

$$F_s = P/2a.$$