Preliminary Bridge Load Calculation

Analyzed wit Matlab Data Output Data to be filled in Discarded Data

Load Combo 1.2D +1.6L 1.4D

| | | Tandom Point Load 2 x 25 kip @ 4 ft per lane | | Live Load impact Factor one 25 kip must be distributed to each edge girder 0.55 for x = 3 lines | | | | | | | 2.496 | 1 | 106.6666667 | 17.5 | | | | | | | | | | | |
|------------------------------|--------------------------|---|--------------|---|----------------------|-----------------------------------|----------------|-------------------------|----------|--------|----------------|--|---------------------|----------------|----------------|-------------------------------|------------------|---------------|----------------|-----------------|--------|------------|---------------|------------|--|
| Calculation sheet correction | | | | 210 | | | | | | | | | | | | | | | Design Load | | | | | | |
| Section | Structural Element | | | Tributary Area | | | Distributed Lo | | | | | deflection | tion 0.000521049 18 | | | Service Load | | | | Moment | | | Shear | | |
| | | | Spacing (ft) | Tributary Width(ft) | Tributary Length(ft) | Tributary Area(sq. ft) Depth (ft) | Weight (plf | /Tribuary (| (ksf) (k | sf) | Load (ksf) (ki | Tandom Load Location | Distributed | Multiplication | Multiplication | Weight(kip) | Moment (k-ft) Sh | hear (kip) | (kip) | Capacity (k-ft) | Moment | Sufficent? | Capacity(kip) | Sufficent? | |
| Deck | 8" Slab | Concrete | - | 85 | 525 | 44625 0.6666666 | 67 150 | 4462.5 | 0.1000 | 0.0294 | 0.1670 - | | 14.1936 | 1 | | 4462.5 | 5 | | | | | | 4 / | 4 | |
| Deck | Pavement , needs data | | | | | | | | | | | | | | | | | | | | | | 4 / | 4 | |
| Deck | Springers | Steel W30x 391 | 1 | 4 14 | 25 | 350 section is use | 391 | 9.775 | 0.1279 | 0.0294 | 0.2005 | 81.25 middle of joist for maximum | 2.806969412 | 21 | | 1026.37 | 5 1234.92 | 116.34 | | 5026.3 | 6041.7 | Yes | 1280. | .3 Yes | |
| Note A on dwg | Girder Type 1 | flance, 6 x 48 | 3 | 0 30 | 76 | 2280 section is use | 1211.39 | 92.06564 | 0.1404 | 0.0294 | 0.2154 (75 | i,100,75)*0.65 4*25 = 100. @ 57 3*25 = | 6.463173882 | 1 | 17.5 | 1611.148 | 7 7445.16 | 330.91 | | 8700 | 20000 | Yes | 1280. | .3 Yes | |
| Note B on dwg | Girder Type 2 (Not Used) | Steel W30x261 | - 1 | 9 19 | 25 | 475 section is use | 261 | 6.525 | 0.1137 | 0.0294 | 0.1835 | | 3.485887059 | 21 | | 274.0 | 5 272.33 | 43.57 | | 3184 | 3929 | Yes | 82 | 31 Yes | |
| Deck | Bottom Deck (Not Used) | in x 2.5 in | 12. | 5 12.5 | 85 | 1062.5 section is use | 1276.041667 | 108.4635417 | 0.2699 | 0.0294 | 0.3779 | | 4.723891974 | 1 | 40 | 4555.4 | 7 3185.10 | 83.97 | | | 19804 | Yes | 4 - / | Yes | |
| Note C on dwg | Edge Girder | x 4.5 x 0.75 in | 9. | 5 9.5 | 30 | 285 | 574 | 17.2265625 | 0.1604 | 0.0294 | 0.2395 50 | 0.65 maximum moment, and at | 2.275406029 | 17.5 | | 602.9 | 3 -177.77 | 415.04 | 24162.80399 | 9 | 11907 | Yes | 4 - | Yes | |
| | | | | | | | | | | | | | | | | new moment | 255.9831783 | | | | | | | | |
| | | | | | | | | 20.35889605 610.7668816 | | | | | | (kip) | | 11506.09714 kip) 0.2578 = 104 | | | = 10415/525/85 | | | | | | |
| | | | | | | | | | | | | | | Total Loading | Live Load(kip) | 1310.4 | 4 Live Load | 0.0294 | =L20 | | | | | | |
| | | | | | 947 5609407 | | | | | | | | | (kin) | | 4 Total Load Comb | 0.3610 | Combo (ksf) = | | | | | | | |

"HSS 40inx40in 2.5 in wall thickness works Mu=19800 k-ft, Code has been Uploaded -Sam

Axial Axial Capacity (

263300 Yes