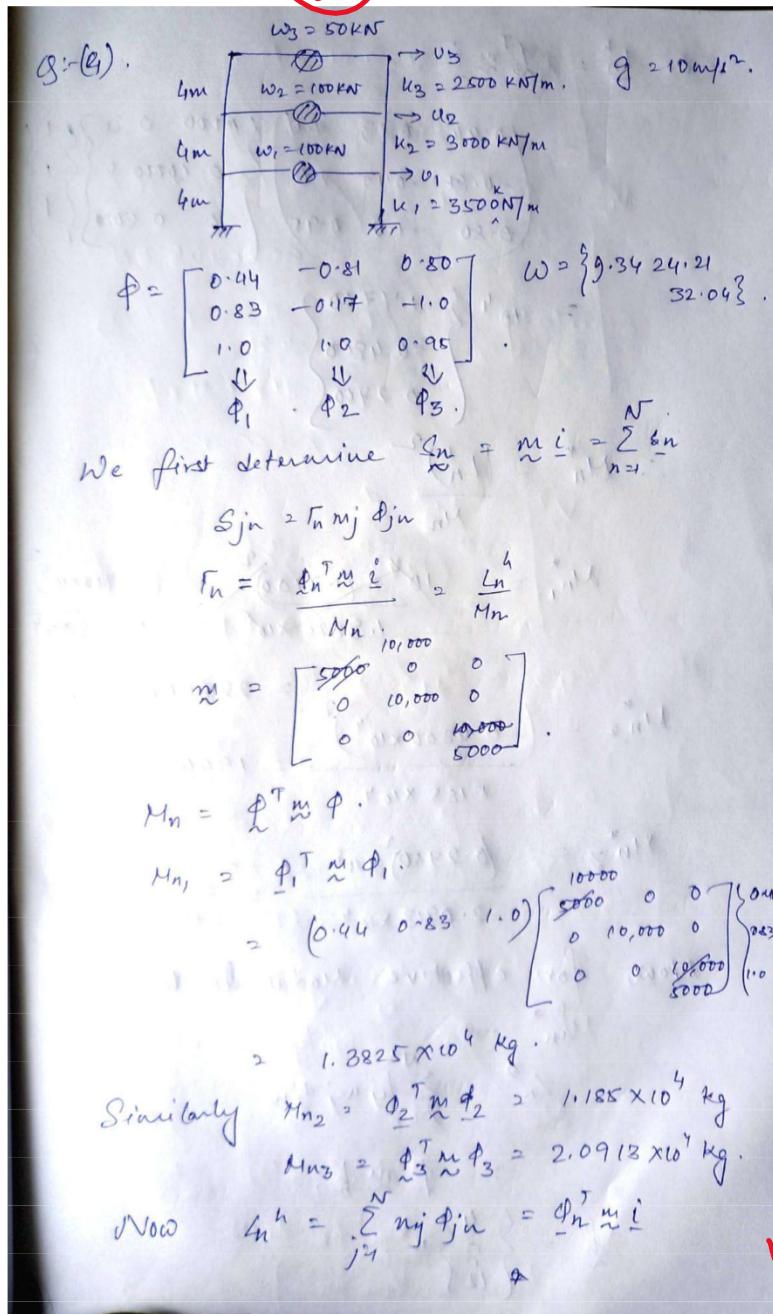


Question 4

24 February 2022 15:28

(17)
20



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Or $\underline{\underline{f}}_n = \underline{\underline{\phi}}_n^T \underline{\underline{m}} \underline{i}$.

$$= \begin{pmatrix} 0.44 & 0.83 & 1.0 \\ -0.81 & -0.17 & 1.0 \\ 0.80 & -1.0 & 0.95 \end{pmatrix} \begin{Bmatrix} 10000 & 0 & 0 \\ 0 & 10000 & 0 \\ 0 & 0 & 10000 \end{Bmatrix} \begin{Bmatrix} 1 \\ 1 \\ 1 \end{Bmatrix}$$

$$= 10^4 \left\{ \begin{array}{l} 1.7700 \\ -0.4800 \\ 0.2800 \end{array} \right\}$$

What?

$$\therefore M_n^* = \frac{(L_n)^2}{M_n} \quad (1)$$

$$M_{n_1}^* = \frac{10^4 \times (1.7700)^2}{1.3825 \times 10^9} = \frac{(1.7700 \times 10^4)^2}{1.3825 \times 10^9}$$

$$M_{n_2}^* = \frac{(-0.4800 \times 10^4)^2}{1.185 \times 10^9} = 22661.12 \text{ kg.}$$

$$M_{n_3}^* = \frac{(0.2950)^2 \times 10^8}{2.0913 \times 10^4} = 361.62 \text{ kg.}$$

Now, for effective modal height

$$L_n^0 \rightarrow$$

$$L_n^0 = \sum_{j=1}^N h_j w_j \varphi_j n$$

$$= \sum_{j=1}^N h_j w_j$$

$$\bar{h} = \left\{ \begin{array}{l} 4 \\ 8 \\ 12 \end{array} \right\} \text{ m.}$$

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$$L_n^0 = \begin{pmatrix} 0.44 & 0.83 & 1.0 \\ -0.81 & -0.19 & 1.0 \\ 0.20 & -1.0 & 0.95 \end{pmatrix} \begin{pmatrix} 10000 & 0 & 0 \\ 0 & 10000 & 0 \\ 0 & 0 & 5000 \end{pmatrix} \begin{Bmatrix} 9 \\ 8 \\ 12 \end{Bmatrix},$$

$$= 10^5 \times \begin{Bmatrix} 1.44 \\ 0.14 \\ 0.09 \end{Bmatrix}$$

$$\therefore h_n^* = L_n^0 / L_n^0$$

$$h_{n_1}^* = \frac{(10^5 \times 1.44)}{10^4 \times 1.77} = 8.13 \text{ m.}$$

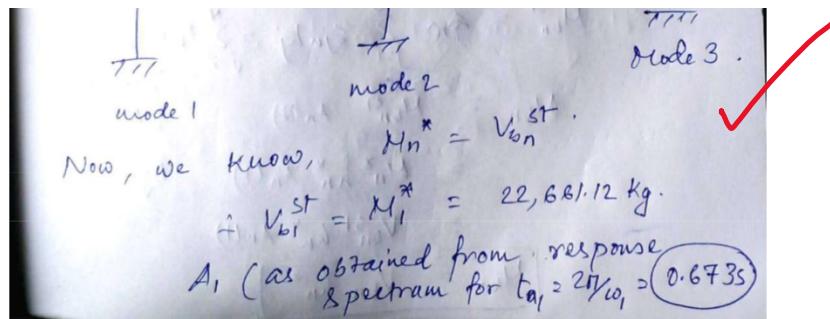
$$h_{n_2}^* = \frac{10^5 \times 0.14}{10^4 \times (-0.48)} = -2.91 \text{ m.}$$

$$h_{n_3}^* = \frac{10^5 \times 0.09}{10^4 \times 0.2950} = 3.27 \text{ m.}$$

$$M_{n_1}^* = 22661.12 \text{ kg.}$$

$$M_{n_2}^* = 1944.3 \text{ kg.}$$

$$M_{n_3}^* = 361.62 \text{ kg.}$$



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$$\begin{aligned}
 A_1 &= 2.71 \times 9.81 \text{ m/s}^2 \\
 &= 26.585 \text{ m/s}^2 \\
 V_b^1 &= V_{bi}^{st} \cdot A_1 = 22,661.12 \times 26.585 \\
 &= 602.446 \text{ KN.} \\
 \text{Similarly } A_2 \cdot \left(t_2 = \frac{2H}{24.21} = 0.259 \text{ s} \right) \\
 &= 2.71 \times 9.81 \text{ m/s}^2 \\
 &= 26.585 \text{ m/s}^2. \quad V_b(t) = ? \\
 V_b^2 &= M_2^* \cdot A_2 \\
 &= (1944.3 \times 26.585) \text{ kg m/s}^2 \\
 &= 51.68 \text{ KN.} \\
 V_b^3 &= M_3^* \cdot A_3 \\
 A_3 &= \text{for } T_3 = \frac{2H}{32.04} = 0.2 \\
 A_3 &= 2.71 \times 9.81 = 26.585 \text{ m/s}^2. \quad (2) \\
 V_b^3 &= (361.62 \times 26.585) \\
 &= 9.613 \text{ KN.} \\
 \text{For base overturning moment } M_b(t) = ? \\
 M_{bn}^{st} &= \sum_{j=1}^N S_{bj} h_j = \sum_{j=1}^N T_{bj} g_j h_j = T_b L_b g \\
 M_{bn} &= M_{bn}^{st} \cdot A_n(t) \Rightarrow M_n^* L_n \\
 &= M_n^* h_n^* A_n(t) \quad \text{By def.} \\
 &= V_{bn}^{st} h_n^* A_n(t) \\
 \boxed{M_{bn} = V_{bn}^{st} h_n^*}.
 \end{aligned}$$

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(c) Yes it is possible to determine

the peak modal base shear
and base overturning moments
without determining time histories of
 $\Delta u(t)$. We can use the response
spectrum for this purpose. We only need the
peak pseudo acceleration response values.

$$\text{equivalent static force} = f_n(t) = S_a \Delta u(t)$$

$$f_n = S_a \Delta u$$

$\Rightarrow f_n = S_a \Delta u$

we can do any analysis
for this force now.
instead of S_a in RHA.



Question 5 :-

Roof slab dimensions = 7m x 5m.

$$\text{Weight} = 5 \text{ kN/m}^2$$

Total weight = $(5 \times 7 \times 5) \text{ kN}$.

$$= 175 \text{ kN} = \frac{175 \times 10^3}{10} \text{ kg}$$

$$K_A = K_B = 500 \text{ kN/m.} \quad \text{Total weight} = 17500 \text{ kg}$$

$$K_C = 250 \text{ kN/m}$$

$$K_D = 400 \text{ kN/m.}$$

$$P_{GT} = 0.3g.$$

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