```
%EGM3601 stoplight project
 2
3
    clc, clear all, close all;
5
    printf("EGM3601 Solid Mechanics stoplight Design Project\n")
    printf("Name: Caleb Gibson\nNID: ca727627")
7
    disp(date())
8
    printf("Due date: Friday April 22, 2022\n")
9
10
11
    %declare and define constants
12
    F q = 164.59; %Newton [N]
13
    rho w = 76518; %Newton per cubic meter [N/m<sup>3</sup>] material weight density
14
    ro = 0.177; %meters [mm]
    R y = 3*F g + w*L; %Newton
15
    E = 200e9; %Newtons per m^2 [N/m^2]
16
17
    L = 14; %meters [m]
18
    v = 0.1; %meters [m] deflection
19
    w = @(ri) rho w*pi*(ro^2 - ri^2);
20
    I = @(ri) (pi/2)*(ro^4 - ri^2);
21
22
    %declare and define functions
23
    v tip = Q(ri) (1/(E*I(ri)))*((-w(ri)*(14^4))/24+((14^3)*(-3*F q+w(ri)*L))/6 ...
24
                                            +((14^2)*(89*F g+98*w(ri)))/2) - 0.1;
25
26
    %solve for given params
27
    ri guess1 = 0.17;
28
    ri_guess2 = 0.18;
29
    tol = 1e-5;
30
    max iters = 20;
31
    ri actual = secant(v tip,ri guess1,ri guess2,max iters,tol);
    printf("The inner radius for the deflection to be 0.1m is %.6f.\n",ri actual)
33
34
35
36
    37
38
    %AFTER ATTENDING DR. YAVAS'S REVIEW SESSION: It has been discovered that the max
39
    % deflection for a point load at ANY POINT along the length "L" is easily given
    %by D MAX = ((P*a^2)/(6*E*I)) * (3*L - a), where "a" is the length from the
40
41
    %origin to the point of the application of the point load AND "P" is the POINT
42
    STOAD.
43
    % The max deflection for a distributed load ALONG THE WHOLE LENGTH of a
    %CANTILEVERED beam of length "L" and weight distribution "w" is given by:
44
45
    %D MAX = (w*(L^4))/(8*E*I). IN EACH CASE, "E" is the MODULUS OF ELASTICITY, "I"
46
    %is the MOMENT OF INERTIA ABOUT THE Z OR Y AXIS OF THE CROSS-SECTIONAL AREA OF
47
    %THE BEAM, and the MAX DEFLECTIONS CAN BE ADDED TOGETHER to get the TOTAL MAX
48
    %DEFLECTION!!!
49
    % THEREFORE, (**side note: "UDL" is UNIFORM DISTRIBUTED LOAD**)
50
    L = 14; %m
51
    E = 207.5e9; %N/m<sup>2</sup>; according to
    https://matmatch.com/materials/minfm52552-astm-a595-grade-a-carbon-steel [1], average
    Modulus of Elasticity of Grade A Structural A595 carbon steel. This material was chosen
    because it has been consistently seen as being used by different Departments of
    Transportation (DOTs) as one of their materials of choice for the construction of traffic
    light pole arms.
52
    P = 16.7829*9.8; %N; according to
    https://www.hillsboroughcounty.org/en/newsroom/2019/10/31/traffic-signals [2], average
     3-light aluminum traffic signals weigh 37 lbs, which equates to 16.7829 "kilograms"
     according to Google's built in unit converter (these are actually Newtons, as when we
     measure the weight of something we are NOT measuring its MASS -- we are measuring its
    WEIGHT, which is a FORCE, not a MASS, and therefore we must measure it in terms of
    NEWTONS, NOT KILOGRAMS)
53
    rho w = 7850*9.8; %N/m<sup>3</sup>; according to [1], average density of A525 steel in kilograms
    per cubic meter, multiplied by 9.81 \text{ m/s}^2 - the acceleration due to gravity - in order to
    convert it to Newtons per cubic meter.
54
    %FROM PAGE 6 OF THE NYDOT TRAFFIC DESIGN MANUAL [3], IT IS KNOWN THAT THE DIA-
55
    %-METER OF THE BASE OF THE MAST ARM IS 1.75 M FOR A STOPLIGHT OF THIS LENGTH (14
    % m =~ 45.9318 feet), SO THE TOTAL DIAMETER OF THE POLE ARM can be taken as
56
57
     %being HALF this base diameter, which is 1.75/2 m = *0.875 METERS.*
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59
               %[1]https://matmatch.com/materials/minfm52552-astm-a595-grade-a-carbon-steel
   60
               %[2] https://www.hillsboroughcounty.org/en/newsroom/2019/10/31/traffic-signals
   61
               %[3] https://www.dot.ny.gov/portal/pls/portal/mexis app.pa ei eb admin app.show pdf?id=13512
   62
   63
               d O = 0.875; %m
   64
               r0 = d 0/2; %m
   65
   66
   67
   68
               %SINCE we are calculating for the geometry such that the MAX DEFLECTION <= 100
               %MM, ALSO KNOWN AS 0.1 M, CAN MAKE the WHOLE DEFLECTION FUNCTION a FUNCTION of
   69
  70
               %INNER RADIUS "ri" and REMOVE THE EI FROM THE DENOMINATOR TO SIMPLIFY THE
               %EQUATION. (**side note: "rO" = outer radius)
  71
               % D MAX TOTAL = 0.1 = D MAX UDL + D MAX P1 + D MAX P2 + D MAX P3, THEREFORE,
  73
               % 0 = D MAX UDL + D MAX P1 + D MAX P2 + D MAX P3 - 0.1, THEREFORE
               ricalculatn = 0 = 3wL^4 + 4PL1^2(3L-L1) + 4PL2^2(3L-L2) + 4PL3^2(3L-L3) - 4PL3^2(3L-L3) + 4P
  74
  75
               \% 0.1EI , WHERE "I" = (PI/4)*(rO^4 - ri^4) and "w" = rho wpi(rO^2-ri^2)
  76
               %**SPECIAL NOTE TO SELF: here, "I" refers to the second PLANAR moment of area,
  77
               %NOT the second polar moment of area (also known as the PLANAR MOMENT OF
  78
               %INERTIA, not the polar moment of inertia). This can be confusing, since dif-
  79
               %-ferent professions and sub-specializations use "moment of inertia" and "second
  80
               % moment of area" to refer to different moments. However, THE MOMENT OF INERTIA
               %FOR CALCULATING DEFLECTION IS THE *PLANAR* MOMENT OF INERTIA, NOT the polar
  81
  82
               %moment of inertia. [2]
  83
  84
               응[2]
               https://www.engineeringtoolbox.com/area-moment-inertia-d 1328.html#:~:text=Area%20Moment%20o ₹
               f%20Inertia%20or,bending%20and%20stress%20in%20beams.
  85
  86
               %I y = @(ri) (pi/4)*(ro^4-ri^4);
   87
   88
               w = @(ri) rho w*pi*(ro^2-ri^2);
               ri calculatn = ((3*w(ri)*L^4) + ((4*P*(5^2))*(3*L-5)) + ((4*P*L*(9^2))*(3*P-9)) + ((4*P*L*(9^2
   89
                ((4*P*L*(13^2))*(3*P-13))) - (24*0.1*E*(pi/4)*(ro^4 - ri^4));
   90
   91
               rict{rict} = \frac{1}{2}(ri) (w(ri)*L^4)/(8*E*I y(ri)) + ((4*P*(5^2))*(3*L-5))/(6*E*I y(ri)) + ((4*P*(5^2)))*(3*L-5))/(6*E*I y(ri)) + ((4*P*(5^2)))*(3*L-5))/(6*E*I y(ri)) + ((4*P*(5^2)))/(6*E*I y(ri)) + ((4*P*(5^2))/(6*E*I y(ri)) + ((4*
                ((4^{-}P*L*(9^{2}))*(3*P-9))/(6*E*I y(ri)) + ((4*P*L*(13^{2}))*(3*P-13))/(6*E*I y(ri)) - 0.1;
   92
   93
               %(**special note: L1=5m, L2=9m, L3=13m**) **"24" is LCD of equatn.
               %NOW, HAVING the EQUATION WE NEED, we CAN USE OUR "secant" SOLVER FUNCTION TO
               %SOLVE FOR THE ROOTS OF THE "D MAX TOTAL" EQUATION SUCH THAT THE MAX DEFLECTION
   95
   96
               %IS <=100MM ALSO KNOWN AS 0.1M.
   97
   98
               tol = 1e-3;
  99
               max iters = 1000;
100
               ri guess=0.4; %m
101
               ri guess2=0.3; %m
102
               ri actual = secant(ri calculatn, ri guess, ri guess2, max iters, tol);
103
104
               105
               %NOW, we can CALCULATE *STEPS 4 AND 5 AND 6* of the project. Using what we know
106
107
               %now, we can calculate the force acting along on the beam due to a wind gust
               %with a velocity of 50 m/s. The total DRAG FORCE "D" due to the wind gust is the dynamic
108
               pressure
109
               % dynamic pressure due to the air being forced against it, given by
110
               p = CD*(0.5*rho air*v air^2), where "CD" is the drag coefficient of the shape
               %(in our case, a cylinder), "rho air" is air's density at standard temperature
111
               % and pressure (STP) which is 1.2\overline{2}5 \text{ kg/m}^3, using the International Standard
112
113
               %Atmosphere (ISA) values of temperature = 15 degrees Celsius and pressure
114
               %= 1 atm = 101325 Pa at sea level (according to macinstruments.com [^1]), all
115
               %that TIMES the PLATFORM AKA PROJECTED AREA of the SURFACE IT IS ACTING UPON
116
               % (in our case, the HORIZONTAL of the stoplight arm, MINUS THE END [so
117
               %OUTER DIAMETER TIMES TOTAL LENGTH, or "D OUTER" times "L"]).
118
119
               %[^3] https://macinstruments.com/blog/what-is-the-density-of-air-at-stp/
120
               %*Note to self: the reason we only use "outer diameter" instead of "pi*diamtr/2"
121
               % is because the "projected area" is not the area if we were to unwrap the
122
```

58

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124
      %from the singular direction (in this case) that the dynamic fluid is flowing
125
      %from, which in this case would be the same no matter which direction it came at
126
      % the cylinder at, as long as it is still normal to points on the wall - not the
127
      % bases - of the cylinder. Henceforth, OUTER DIAMETER WILL BE REFERRED TO AS
      %"CD" \sim= 0.5 for a cykinder.
129
      %THEREFORE, "D" = 0.5*(0.5*1.225*50^2)*(d0*L) = 0.25*1.225*2500*(0.875*14). The
130
      %DISTRIBUTED FORCE DUE TO THE WIND, "w DRAG", is this drag force "D" per unit
131
      %length, in essence "w DRAG" = "D" divided by the length "L", in another word,
132
      %"w DRAG" = D/L = (0.5*(0.5*1.225*50^2)*(d0*L))/L, so = 0.5*(0.5*1.225*50^2)*d0,
133
134
      % and since d0 = 2\text{rO} = 2*0.127 = 0.254 \text{ [m],"w DRAG"} = 0.25*1.225*2500*0.254
135
136
      w DRAG = 0.25*1.225*2500*d O; %(kg/m^3)(m^2/s^2)(m) = [kg/s^2], since [N] = [kgm/s^2], then
      [kq/s^2] = [(kqm/s^2)/m] = [(N)/m]
137
      printf("=======\n")
      printf("NOW APPLYING A WIND GUST OF 50 M/S PARALLEL TO THE Y-AXIS, NORMAL \n\
138
139
      TO THE Z AND X AXES ACTING ALONG THE ENTIRETY OF THE BEAM:\n")
140
      printf("The total drag force acting on the beam is %.4f N.\n",w DRAG*L)
141
      printf ("The drag force per unit length acting on the beam is %.4f N/m.\n", w DRAG)
142
      printf("The dynamic pressure due to drag is %.4f Pa.\n", (w DRAG/d 0))
143
144
      %the total moment acting in the "y" direction, normal to both the mast arm as
145
      %well as the vertical upright pole it is attached to (acts as the "z" axis), is
146
      %equivalent to the magnitude force times the magnitude (in other words the
147
      %length) of the moment which extends out to the point upon which it acts, which,
148
      % since it is a distributed force, can be modeled as having its total force -
149
      %which is "D" - as acting on the centroid of its (w_DRAG's) application to the
150
      %beam. THEREFORE, half the length of its (the UDL's) application distance, which
151
      % since it acts across the whole length of the beam is L over 2 \{L/2\}.
152
      %=> D(L/2). MOMENT ALONG THE *X* AXIS is given by the weight of the beam acting
153
      % on its centroid (the distance to which is the moment arm), plus each point
154
      % load times its respective moment arm.
155
156
      M z = -(w DRAG^*L)*L/2; % <- this is actually M in y, but the PowerPoint has it notated as
      M in z, so to be consistent I have notated it as M in z as well; but according to our
      reference system, it is actually {\tt M} in {\tt y}
      M y = -(w(ri actual)*L/2 + P*27); %since M y = w*L/2 + P*5 + P*9 + P*13, the "P" can just
157
      be factored out of the latter 3/4ths of the equation, to reduce math operations (MOPS),
      thereby reducing overall computational complexity (albeit only minimally)
158
159
      %it is worth noting that although traditionally, the sign of every moment in the M \times
      equation would be positive, since each one causes the arm to rotate counterclockwise, here
      we take clockwise to be our positive convention, since we denote to the left of "x" as
      positive (in another word, LEFT OF ORIGIN is our POSITIVE direction along the x axis by
      our convention, so we must use clockwise along x as our positive moment convention along
      x; our upwards and outwards [z and y] conventions are the same as traditional however, so
      moment need not be converted to negative counterclockwise positive clockwise for these 2
      cardinal directions)
      %now to find the combined stress-state at the critical cross-section of the
160
161
      %pole, i.e. where the moment is maximized, which is at the base of the beam
162
      %(just before the reaction forces act where it attaches to the rigid vertical light pole).
      Therefore, calculate the
163
      %bending moment at different points (which are essentially infinitesimally small
      %areas on the cross-section) of the cross-section. In order to calculate the max
164
165
      % stresses experienced at these points due to the bending moments (which are dif
166
      %-ferent along the y and z axes, since the bending moment in z is only due to the wind
      force blowing against the mast arm whereas the benfding moment in x is due to the weight
      of the mast arm acting along its centrodi of its applicatioin as well as the weights of
      each stoplight acting at a moment arm of the length from the base at which they act).
167
      %Therefore, the bending stresses and shear stresses at the outermost points along the wall
      (of the mast arm) must be calculated, and each one respectively must be treated as most
      critical or not so by comparing its stresses with the others and then seeing which one is
      the mallest (since this is the most critical stress, since it will be able to resist only
      this amount of stress, which is smaller than all the other stresses). Then we have to
      calculate which stress is the greatest and whether or not it will break the material
      (because it exceeds the ultimate tensile strength of the material, which in our case is
      Structural A992 steel, which has an yield strength of 345 MPa and an ultimate strength of
168
      %equation for max normal bending stress in y:bsy=-(M z*c)/Iz; in z:bsz=-(M y*c)/Iz
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%surface the dynamic fluid is blowing against, but rather simply our view of it

123

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169
           \label{eq:continuous} \mbox{\ensuremath{\$e}} \mbox{quation for max shear stress in } \mbox{\ensuremath{\$y}} : \mbox{tauy} = (\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$Q$}}\mbox{\ensuremath{$y$}}) \, / \, (\mbox{\ensuremath{$I$}}\mbox{\ensuremath{$Y^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$Q$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{$V^*$}}\mbox{\ensuremath{
170
           IZ = (pi/2)*(rO^4-ri^4) = Iy = I; Qz = Qy = A*distance to centroid normal to axis; the
                                                                                                                                                                                   \leq
           geometric center (centroid) for a half-circle is
           (pi*(1/3)*(ro^3-ro^2)/(pi*(ro^2-ri^2)/2)=(2/3)*((ro^3-ri^3)/(ro^2-ri^2)), therefore since
                                                                                                                                                                                   \leq
           Q max = Q for the whole half-circle area, then the centroid will be:
           half circ centroid = (2/3)*((r0^3-ri actual^3)/(r0^2-ri actual^2));
171
           %And therefore, Q will be this times the area of the half circle, (pi/2)*(r0^2-ri^2), => Q
172
                                                                                                                                                                                  <
           = (pi/3)*(r0^3-ri actual^3)
173
           Q = (2/3)*(r0^3-ri_actual^3);
174
175
           %shear force in z direction is wL+3P; shear force in y direction is total drag force;
                                                                                                                                                                                  \leq
           thickness "t" is r0-ri; so t = r0-ri_actual:
176
177
           t = 2*(rO-ri actual);
178
179
           %Therefore, we have
180
181
           I = (pi/4) * (r0^4-ri actual^4);
182
           bsy = (M_z*r0)/I;
183
           bsz = (M y*r0)/I;
184
           total flexure = abs(bsy) + abs(bsz);
185
           tauy = ((w(ri actual)*L+3*P)*Q)/(I*t);
           tauz = ((w DRAG*L)*Q)/(I*t);
186
187
           printf("=======\n")
188
           printf("The maximum bending stress in the y direction of the cross-sectional \
189
           area is %.8f Pa.\n",bsy)
190
           printf("The maximum bending stress in the z direction of the cross-sectional \
191
           area is %.8f Pa.\n",bsz)
192
           printf("The magnitude of the total flexure of the cross-sectional area is %.8f \
193
           Pa.\n", total flexure)
194
           printf("The maximum shear stress in the y direction of the cross-sectional area\
195
           is %.8f Pa.\n", tauy)
           printf("The maximum shear stress in the z direction of the cross-sectional area\
196
197
            is %.8f Pa.\n", tauz)
198
199
           stresses = [bsy;bsz;total_flexure;tauy;tauz];
200
           max_stress=0;
201
202
           for i = 1:length(stresses)
203
              if abs(stresses(i)) > abs(max stress)
204
                  max stress=stresses(i);
205
              else
206
                  continue
207
              endif
208
           endfor
209
           min stress = max stress;
210
           for j=1:length(stresses)
211
              if abs(stresses(j)) < abs(min stress)</pre>
212
                  min stress = stresses(j);
213
              else
214
                  continue
215
              endif
216
           endfor
217
218
           printf("\nThe MOST CRITICAL STRESS is %.8f Pa.\n", max stress)
219
220
           %printf("The most critical of these stresses is %s.\n", min stress)
221
222
           %Now, it must be determined whether this most critical stress exceeds the ultimate thesile
           strength of the material selected, Structural A992 steel.
223
224
           uts_A595_grade a = 450e6; %N/m<sup>2</sup> aka Pa
225
           ys_A595_grade_a = 380e6; %N/m^2 aka Pa
226
           if abs(uts A595 grade a) > abs(max stress)
227
              printf("The material will NOT FAIL due to the stress.\n")
228
229
           else
230
              printf("FAILURE!! The material will fail at this stress load.\n")
231
              failure = 1;
232
           endif
```

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