

# CVEN5313: Environmental Fluid Mechanics

## Open Channel Flow: Uniform and Gradually Varied Flow Due Friday, 10/1/10

1. Use the Buckingham-Pi theorem to formally show that if

$$\tau_0 = \phi(V, \rho, g, \nu, R_h, k)$$

where  $\phi$  is some nondimensional function of :

- $V$  is the flow velocity [ $L T^{-1}$ ]
- $\rho$  is the fluid density [ $M L^{-3}$ ]
- $g$  is the gravitational constant [ $L T^{-2}$ ]
- $\nu$  is the fluid viscosity [ $L^2 T^{-1}$ ]
- $R_h$  is the channel hydraulic radius [ $L$ ]
- $k$  is the channel roughness scale [ $L$ ]

then

$$\frac{V}{\sqrt{\tau_0/\rho}} = \phi\left(\frac{VR_h}{\nu}, \frac{V}{\sqrt{R_h g}}, \frac{k}{R_h}\right)$$

2. Consider the flow shown in Fig. 1. Flow passes under a gate, and then undergoes a hydraulic jump. You may assume that no energy is lost as the flow passes under the gate from station 1 to station 2. The volumetric discharge is  $Q = 0.125 \text{ m}^3/\text{s}$ , the width of the channel is  $B = 125 \text{ cm}$ , and the depth of the flow upstream of the gate is  $d_1 = 50 \text{ cm}$ .
  - (a) Determine the depth, velocity, and Froude number for all three sections of the flow.
  - (b) Classify the jump based on its Froude number. What percentage energy loss would you expect for this type of jump?
  - (c) Calculate the energy loss in the jump, and express it as a percentage of the upstream head.
  - (d) Plot a specific energy diagram showing all three points of the flow.

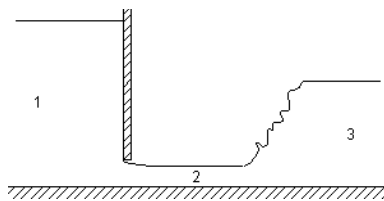


Figure 1: Hydraulic jump.

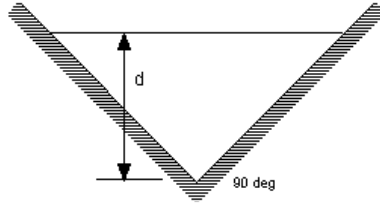


Figure 2: Concrete channel section.

3. Consider the triangular concrete channel section shown in Fig. 2. Assume  $Q = 20 \text{ m}^3/\text{s}$ , and  $S_0 = 0.001$ .

- (a) Calculate the normal depth  $d_N$ .
- (b) Calculate the critical depth  $d_C$ .
- (c) Classify the channel slope (e.g. Mild, Critical, or Steep).
- (d) Calculate the Critical Slope.

4. Consider the following cases:

(a) Case I:

Trapezoidal Section

$$B_0 = 5 \text{ m}$$

$$s = 1$$

$$n = 0.013$$

$$Q = 50 \text{ m}^3/\text{s}$$

$$S_0 = 0.0004$$

A dam at  $x = 0$  backs up the water to a depth of 6 m.

$\Rightarrow$  Compute the upstream profile. Note that this is the case shown in the notes.

(b) Case II:

Rectangular Section

$$B = 2 \text{ m}$$

$$n = 0.010$$

$$Q = 10 \text{ m}^3/\text{s}$$

$$S_0 = 0.005$$

At  $x = 0$  the flow passes under a gate, resulting in a depth of 0.5 m.

$\Rightarrow$  Compute the downstream profile.

For each case, your solution should contain the following:

- i. Determine  $d_C$ ,  $d_N$ , and the free-surface classification (e.g., M1, S2, etc.)
- ii. Plot depth vs.  $x$ . You should compute the profiles far enough in  $x$  so that the depth is within 1% of the normal depth.
- iii. Plot the free-surface elevation vs.  $x$ . The plot should also show the bed location, CDL, and NDL as a function of  $x$ . This plot should resemble the figure in the notes.
- iv. Determine the distance required for the flow to asymptote to within 1% of the normal depth.