# Determining Drawdown From a Proposed Groundwater Pump

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1 INTRODUCTION 1

#### 1 Introduction

A new groundwater pump is proposed at a site near several ranches. The ranches' wells utilize the same aquifer that the new pump will. The aquifer is contained by impermeable boundaries on two sides (Figure 1). The other two boundaries of the aquifer are assumed constant (Figure 2). If the drawdown from the new pump is greater than 0.2 m at a point 200 m away from the proposed pump site the ranches' wells will go dry. The purpose of this report is to

- Determine whether the drawdown from the new pump will be greater than 0.2 m at any point 200m away from the well site.
- Investigate the sensitivity of the solution to changes in the parameters.

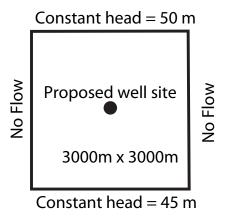


Figure 1: Aquifer plan view.

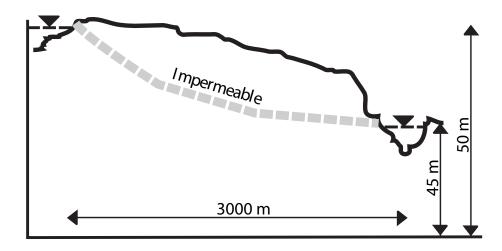


Figure 2: Aquifer cross section.

2 METHODOLOGY 2

#### 2 Methodology

The flow of groundwater in the aquifer is described by

$$\frac{\partial^2 P}{\partial x^2} + \frac{\partial^2 P}{\partial y^2} = -\frac{R(x,y)}{T} \tag{1}$$

where

P = Hydraulic head (m)

x, y = distance from defined point (0,0) (m)

 $T = \text{Transmissivity } (\text{m}^2/\text{day})$ 

R(x,y) = volume per unit area per unit time added to the aquifer

Transmissivity is a parameter associated with aquifer thickness and hydraulic conductivity of the soil. To determine the drawdown 200 m away from the pump site Equation 1 must be solved for P, the hydraulic head. No analytical solution exists for Equation 1, so a numerical method must be used. A finite difference approximation of P at node (i,j) is given by

$$\frac{P_{i-1,j} - 2P_{i,j} + P_{i+1,j}}{\Delta x^2} + \frac{P_{i-1,j} - 2P_{i,j} + P_{i+1,j}}{\Delta y^2} = \frac{-R_{i,j}}{T}$$
(2)

where

 $\Delta x$  = distance from node j to node j + 1 (m)  $\Delta y$  = distance from node i to node i + 1 (m)

Equation 2 generates a system of equations that must be solved for  $P_{i,j}$ . Liebman's method recognizes that the system of equations generated by Equation 2 can be represented as a tridiagonal matrix. The tridiagonal matrix can then represented in a successive overrelaxation (SOR) form. If  $\Delta x = \Delta y = h$ , then Equation 2 can be simplified and written in SOR form

$$P_{i,j}^{(k+1)} = P_{i,j}^{(k)} - \frac{\omega}{4} \left( P_{i-1,j}^{(k+1)} + P_{i+1,j}^{(k)} + P_{i,j-1}^{(k+1)} + P_{i,j+1}^{(k)} - 4P_{i,j} + \frac{Q_{i,j}}{T} \right)$$
(3)

where

k = iteration number  $\omega$  = relaxation factor  $0_i\omega_i1$   $Q_{i,j}$  = Flow rate into aquifer at node (i,j) (m<sup>3</sup>/day) =  $R_{i,j}h^2$ 

Since the pumping rate, Q, is a flow rate into the aquifer, any pumping rate out of the aquifer must be negative. The relaxation factor,  $\omega$ , does not affect the numerical solution of the SOR algorithm.  $\omega$  will affect the number of iterations necessary for the algorithm to converge on a solution. Program pde1 uses the SOR algorithm to iteratively solve for  $P_{i,j}$  (Figure 3).

3 APPLICATION 3

```
do
 psave=P(i,j)
  do i=2,nxgrd-1
                                 !dont include constant boundries
    do j=1,nxgrd
      psave=P(i,j)
      if(j==1)then
                                 !left edge no flow
        P(i,1)=P(i,1)+(w/4d0)*(P(i+1,1)+P(i-1,1)+2d0*P(i,2)-4d0*P(i,1))
      else if(j==nxgrd)then
                                !right edge no flow
        P(i,j)=P(i,j)+(w/4d0)*(P(i+1,j)+P(i-1,j)+2d0*P(i,j-1)-4d0*P(i,j))
                                 !pump site and everything else
        P(i,j)=P(i,j)+(w/4d0)*(P(i+1,j)+P(i-1,j)+P(i,j-1)+P(i,j+1)-4d0*P(i,j)+Q(i,j)/T)
      end if
      if (abs(psave-P(i,j)) <= eps) then
        exit=exit+1
                                 !node isnt changing
      end if
    end do
  end do
  if (stopping criteria met)exit
end do
```

Figure 3: SOR algorithm.

### 3 Application

Drawdown 200m from the pump site is dependent on various parameters that are known in the problem (Table 1).

Table 1. I arameters associated with determining drawdown.					
Parameter	Variable	Value			
Transmissivity (m <sup>2</sup> /day)	T	$200 \text{ m}^2/\text{day}$			
Relaxation factor	$\omega$	1.861 (dimensionless)			
Flow rate in to the aquifer at node (15,15)	$Q_{15,15}$	$-1000 \text{ m}^3/\text{day}$			

Table 1: Parameters associated with determining drawdown.

To test the sensitivity of the system, some parameters can be varied (Table 2).

	Variation o	it Paramatare t	or analyzzine mo	dol concitivity
TAUTE 4.	variation o	и гаганнетего в	or analyzing mo	dei sensilivitiv.

Run #	Variable		New value	Variation
1	T	$200 \text{ m}^2/\text{day}$	180	-10%
2			220	10%
3	ω	1.861	1.675	-10%
4			2.000	10%
5	$Q_{15,15}$	$-1000 \text{ m}^3/\text{day}$	900	-10%
6			1100	10%

4 RESULTS 4

#### 4 Results

Groundwater in the aquifer flows from the 50 m boundary to the 40 m boundary (Figure 4). Program pde1 gives the average hydraulic head with the proposed pump turned on as 47.93 m and with the pump turned off as 46.01 m (Figure 5) (Figure 6). The drawdown is therefore 1.92 m. This value is greater than the maximum allowable drawdown of 0.2 m.

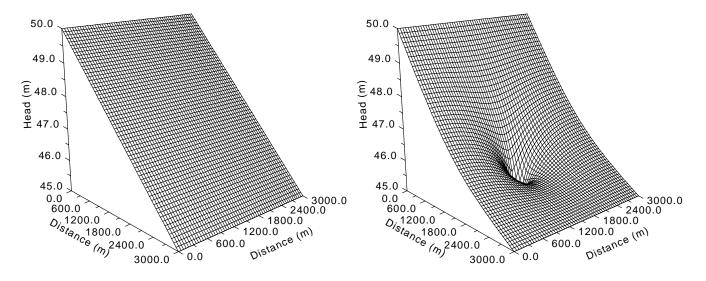


Figure 4: Hydraulic head with no pumping .

Figure 5: Hydraulic head with pump on.

The sensitivity analysis reveals how the drawdown varies when individual parameters are varied. Drawdown is most sensitive to increases in T, the transmissivity. Since T effects how water flows through the aquifer, this finding is reasonable. Varying  $\omega$ , the relaxation factor has a negligible effect on the drawdown but has an appreciable effect on the number of iterations (Table 4) (Figure 6).

Run #	Variable	Value	% Varied	pump on	pump off	Drawdown	Variation	Iterations
1	T	180	-10%	47.93	46.19	1.74 m	9.73%	130
2		220	10%	47.93	45.80	2.13 m	10.94%	130
3	$\omega$	1.675	-10%					>10,000
4		2.000	10%	47.93	46.01	1.92 m	0%	470
5	$Q_{15,15}$	-900	-10%	47.93	45.82	2.11 m	9.90%	130
6		-1100	10%	47.93	46.20	1.73 m	9.90%	130

Table 3: Parameters associated with determining drawdown.

4 RESULTS 5

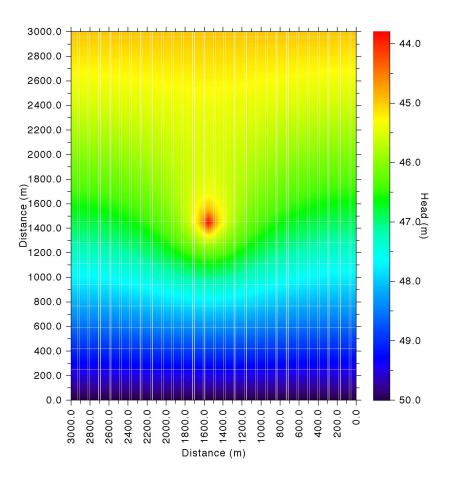


Figure 6: Hydraulic head levels with pump on (contour plot).

Table 4: Determining optimal relaxation factor.

Run #	$\omega$	Iterations
22	1	2182
23	1.1	1814
24	1.2	1501
25	1.3	1232
26	1.4	996
27	1.5	788
28	1.6	600
29	1.7	427
30	1.8	260
31	1.861	130
32	1.9	177
21	1.99	1720

To determine an acceptable pumping rate, different pumping rates were tested. A pumping rate of 104 m<sup>3</sup>/day will yield a drawdown of 0.199 m which is just below the .2m restriction. To fulfill the restriction, the proposed pumping rate must be decreased by 896 m<sup>3</sup>/day or 89.6%.

5 CONCLUSION 6

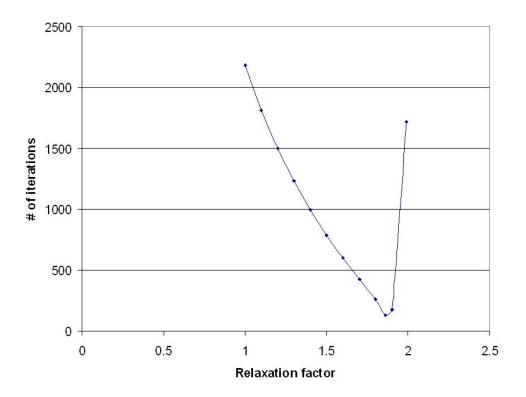


Figure 7: Relaxation factor variation.

#### 5 Conclusion

The following can be concluded from the analysis:

- Groundwater in the aquifer moves from the 50 m boundary to the 40 m boundary.
- When  $Q_{15,15}$ =-1000 m<sup>3</sup>/day, the drawdown will be 1.92 m.
- The model is most sensitive to increases in T.
- The model is least sensitive to changes in  $\omega$ .
- With the currently proposed pumping rate the restriction of 0.2 m drawdown will not be met.
- An 89.6% decrease in proposed pumping rate is needed to meet the restriction.

#### 6 References

Finney, Brad. PDE Lab 1 handout, Humboldt State University, Fall 2006.

## Appendix A

#### Source Code

APPENDIX A SOURCE CODE 7

```
implicit none
double precision,allocatable,dimension(:,:)::P,Q
double precision,allocatable,dimension(:)::x
double precision::ub,lb,eps,T,w,psave,prate
integer::nxgrd,prow,pcol,i,j,numit,maxit,exit,npump
!Variable list
             =Hydraulic head matrix
!Q
             =pumping rate matrix
!x
           =x and y array for shaded surface
           =upper and lower boundry conditions
!ub,lb
!eps
             =convergence tolerance
!T
             =transmisivity
             =saves value of current node for convergence check
!psave
!prate
             =pumping rate
             =number of nodes in x and y direction nxgrd^2 total nodes
!nxgrd
!prow,pcol =pump row and column number
             =number of iterations
!numit
!maxit
             =maximum number of iterations
!exit
             =convercence criteria exit==nxgrd**2-2*nxgrd
!npump
             =number of pumps
open(11,file="vars.dat")
open(12,file="head.out")
open(13,file="pump.dat")
read(11,*)nxgrd,ub,lb,eps,T,w,maxit
allocate(P(nxgrd,nxgrd),Q(nxgrd,nxgrd),x(nxgrd))
do i=1,nxgrd
 x(i)=x(i-1)+100d0
end do
Q=0
read(13,*)npump
do i=1,npump
  read(13,*)prow,pcol,prate
  !write(*,*)prow,pcol,prate
  Q(prow,pcol)=prate
end do
P=0
P(1,1:nxgrd)=ub
P(nxgrd,1:nxgrd)=lb
numit=0
 numit=numit+1
  exit=0
                                 !dont include
  do i=2,nxgrd-1
    do j=1,nxgrd
      psave=P(i,j)
      if(j==1)then
                                !left edge no flow
        P(i,1)=P(i,1)+(w/4d0)*(P(i+1,1)+P(i-1,1)+2d0*P(i,2)-4d0*P(i,1))
      else if(j==nxgrd)then
                             !right edge no flow
        P(i,nxgrd) = P(i,nxgrd) + (w/4d0) * (P(i+1,nxgrd) + P(i-1,nxgrd) + 2d0 * P(i,nxgrd-1) - 4d0 * P(i,j))
      else
                                  !pump site
        P(i,j)=P(i,j)+(w/4d0)*(P(i+1,j)+P(i-1,j)+P(i,j-1)+P(i,j+1)-4d0*P(i,j)+Q(i,j)/T)
      end if
      if (abs(psave-P(i,j)) <= eps) then
        exit=exit+1
                                !node isnt changing
      end if
    end do
  end do
```

APPENDIX A SOURCE CODE 8

```
if(numit>=maxit)then
     write(*,*)(nxgrd**2-2*nxgrd),exit,numit,maxit,"maxit"
     do i=1,nxgrd
       write(12,"(10000f10.5)")(P(i,j),j=1,nxgrd)
     end do
     stop
   else if(exit>=(nxgrd**2-2*nxgrd))then
     do i=1,nxgrd
       write(12,"(10000f10.5)")(P(i,j),j=1,nxgrd)
     end do
     write(*,*)numit
     write(*,*) "This is the number",P(13,15)
     call setpag("USAP") !"USAL" is US size A landscape, "USAP" is portrait
     call scrmod("REVERS") !sets black on white background
     call metafl("xwin") ! or "PS", "EPS", "PDF", "WMF" "BMP"
     call disini() !Initialize dislin
     call disalf
     call psfont("helvetica")
     call name("Distance (m)","XY") ! Set label for x-axis
     !call name("Phosphorus (mg/l)","Y") ! Set label for y-axis
     call name("Head (m)", "Z") ! Set label for z-axis
     call axis3d(2d0,2d0,2d0)
                                 !set up axis in absolute plot coordianates
     call view3d(5d0,-4.5d0,2d0,"abs") !set view point in absolute coordinates
     call vfoc3d(0d0,0d0,0d0,"abs") !set location to look at in absolute coordiates
      !these two for surface mesh
      call labl3d("HORIZONTAL") ! 'STANDARD', 'HORIZONTAL', 'PARALLEL' and 'OTHER'.
      call graf3d(0d0,3000d0,0d0,600d0,0d0,3000d0,0d0,600d0,45d0,50d0,45d0,1d0)
     call surmat(P,nxgrd,nxgrd,2,2)
      !these for contour plot
      !!!!!call nobar
      !call axspos(320,2400)
      !call labtyp("vert", "X")
      !CALL NAMDIS (5,"Y")
      !CALL NAMDIS (0,"z")
      !call graf3(3000d0,0d0,3000d0,-200d0,0d0,3000d0,0d0,200d0,50d0,43.8d0,50d0,-1d0)
                  !set up axis: label range, first label, step betwen labels, X-Y-Z
      !!!!call zaxis(45d0,50d0,45d0,1d0,1800,"Head",0,1,480,200)
                                                                 !custop color bAR
      !call colran(1,254)
                                               !color range
      !call crvmat(transpose(P),nxgrd,nxgrd,20,30)
                                                    !draw 2d color contour plot
      !these two for color surface plot
      !call lab13d("HORIZONTAL") ! 'STANDARD', 'HORIZONTAL', 'PARALLEL' and 'OTHER'.
      !call shdmod("smooth", "surface") !plot a "smooth" or "flat" interpolated graph
      !call surshd(x,nxgrd,x,nxgrd,P) !plot the surface
                    !draw on title
     call title
      call disfin
     stop
   end if
 end do
 rewind(12)
 rewind(11)
 stop
end program pde1
```