SOIL ARCHITECTURE AND PHYSICAL PROPERTIES

And when that crop grew, and was harvested, no man had crumbled a hot clod in his fingers and let the earth sift past his fingertips.

—JOHN STEINBECK, THE GRAPES OF WRATH



Soil Texture (Particle size distribution)

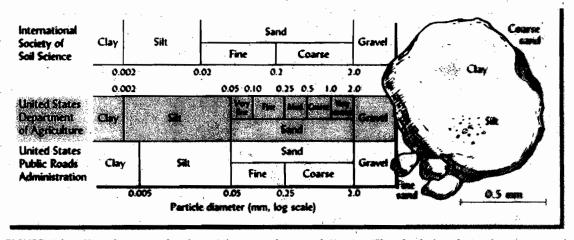


FIGURE 4.2 Classification of soil particles according to their size. The shaded scale in the center and the names on the drawings of particles follow the United States Department of Agriculture system, which is widely used throughout the world. The USDA system is also used in this book. The other two systems shown are also widely used by soil scientists and by highway construction engineers. The drawing illustrates the size of soil separates (note-scale).

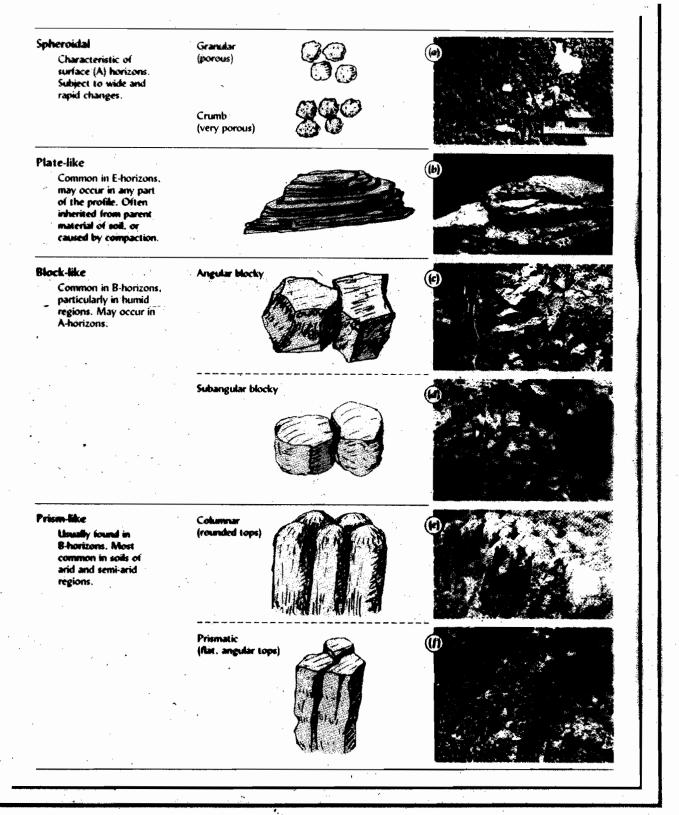
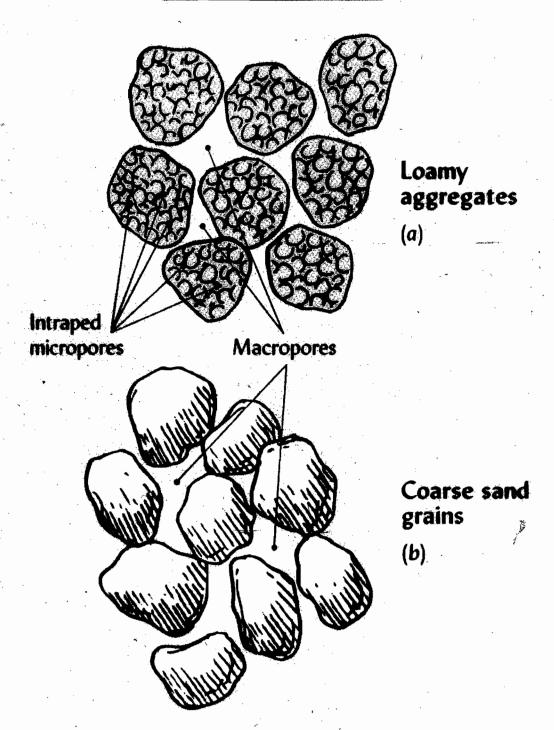


FIGURE 4.11 The various structure types (shapes) found, in mineral soils. Their typical location is suggested. The drawings illustrate their essential features and the photos indicate how they look in situ. For scale, note the 15-cm-long pencil in (e) and the 3-cm-wide knife blade in (d) and (f). (Photo (e) courtesy of J. L. Arndt, now with Petersen Environmental Consulting; North Dakota State University. Others courtesy of R. Weil)

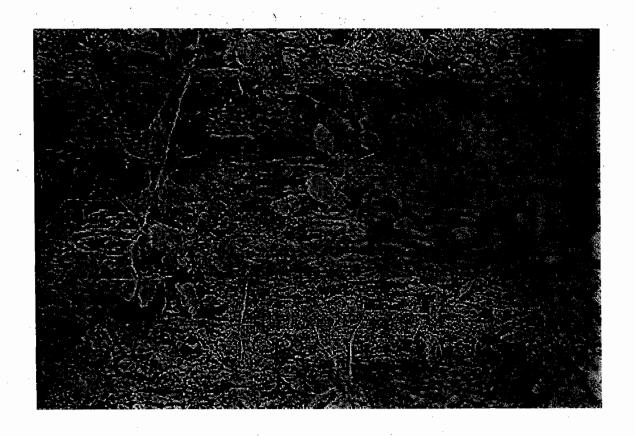


Granular



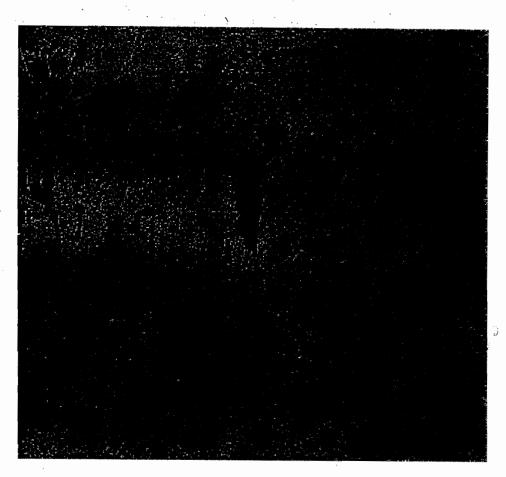
- surface soils, especially with high organic carbon. Grasslands and soils worked by earthworms

Platy



- Usually related to the structure of the parent material. Can also develop in clayey soils due to heavy machinery.

Prismatic



- Usually occur in subsoils in arid and semiarid regions. Also in poorly drained regions with swelling soil clays. Tops are angular and flat.

Blocky



- Usually in subsoils. Related to drainage, root penetration and aeration.

Columnar



- Like Prismatic except tops are rounded. Especially common in soils high in sodium.

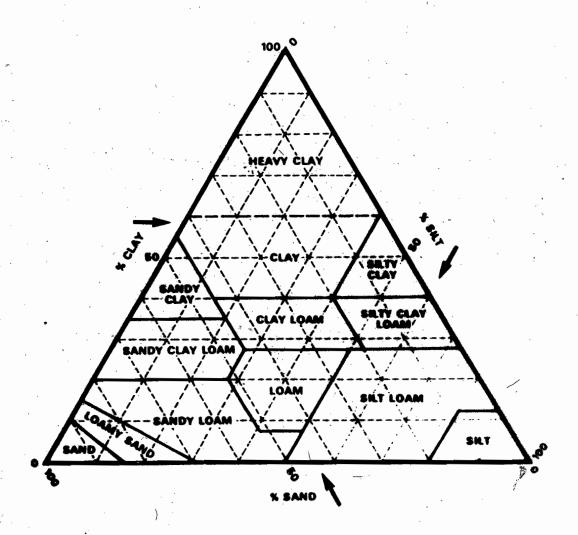


Figure 1.5 The textural triangle.

A Method for Determining Texture by Feel

The first, and most critical, step in the texture-by-feel method is to kneed a walkut-sized sample of moist soil into a uniform puttylike consistency, slowly adding water if necessary. This step may take a few minutes, but a premature determination is likely to be in error as hard clumps of clay and silt may feel like sand grains. The soil should be moist, but not quite glistening. Try to do this with only one hand so as to keep your other hand clean for writing in a field notebook (and shaking hands with your client).

While squeezing and kneeding the sample, note its maleability, stickiness, and stiffness, all properties associated with the clay content. A high silt content makes a sample feel smooth and silky, with little stickiness or resistance to deformation. A soil with a significant content of sand feels rough and gritty, and makes a grinding noise when rubbed near one's ear.

Get a feel for the amount of clay by attempting to squeeze a ball of properly moistaned soil between your thumb and the side of your forefinger, making a ribbon of soil. Make the ribbon as long as possible until it breaks from its own weight (see Figure 4.9).

Interpret your observations as follows:

- 1. Soil will not cohere into a ball, falls apart: sand
- 2. Soil forms a ball, but will not form a ribbon: loamy sand
- 3. Soil ribbon is dull and breaks off when less than 2.5 cm long and
 - a. Grinding noise is audible; grittiness is prominent feel: sandy loam
 - b. Smooth, floury feel prominent; no grinding audible: silt loam
 - c. Only slight grittiness and smoothness; grinding not clearly audible: loam
- 4. Soil cubibits moderate stickiness and femness, forms ribbons 2.5 to 5 cm long, and
 - a. Grinding moise is audible; grittiness is prominent feel: sandy clay loans
 - b. Smooth, floury feel prominent; no grinding audible: silty clay loans
 - c. Only slight grittiness and smoothness; grinding not clearly audible: clay learn
- 5. Soil exhibits dominant stickiness and firmness, forms shiny ribbons longer than 5 cm, and
 - a. Grinding noise is audible; grittiness is dominant feel: sandy clay
 - b. Smooth, floury feel prominent; no grinding audible: silty clay
 - c. Only slight grittiness and smoothness; grinding not clearly audible: clay

A more precise estimate of sand content (and hence more accurate placement in the horizontal dimension of the tentural class briangle) can be made by westing a pea-street champ of poli in this palm of your hand and omearing it around with your finger until it your palm becomes coated with a souplike suspension of soil. The sand grains will attend our visibly and their volume as compared to the original "pea" can be estimated, as can their relative size films, medium, coarse etc.).

It is best to learn the method using samples of known textural class. With practice, accurate textural class determinations can be made on the spot.

BOX 4.2 (Cont.) A METHOD FOR DETERMINING TEXTURE BY FEEL

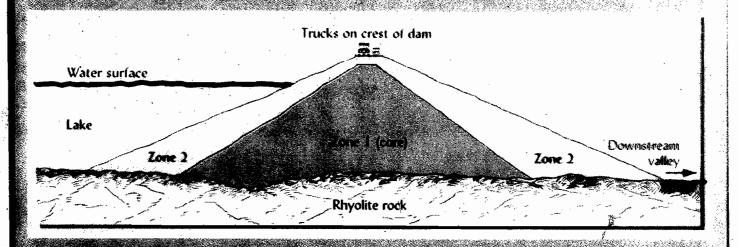


FIGURE 4.9 The "feel" method for determining soil-textural class. A moist soil sample is rubbed between the thumb and forefingers (insets) and squeezed out to make a "ribbon." (a) The gritty,"noncohesive appearance and short ribbon of a sandy loam with about 15% clay. (b) The srbooth, dull appearance and crumbly ribbon characteristic of a silt loam. (c) The smooth, shiny appearance and long, flexible ribbon of a clay. See Box 4.2 for details. (Large photos courtesy of R. Weil)

BOX 4.1 SILT AND THE FAILURE OF THE TETON DAM^a

The first track and costly engineering failures in American history occurred in southern Idaho on 5 June 1077. Lets than a year ofter construction was completed on a <u>large earth-fill dam across the Teton River. Eleven people</u> were failed and 25,000 made homeless in the five hours it took to empty the 28-km-long lake that had been held included by the dam. Some \$400 million (1977 dollars) worth of damages were caused as the massive wall of water and allowed by the collapsed dam and the valley below. The dam failed with little warning as small seepage leaks that held find profits that swept away a team of buildozers sent to make repairs.

All controls in the description of standard, time-tested design for coned earth-fill embarkments. Essentially, a core (some I) of tightly compacted soil assemble was our controls of the line line (form 2) of courses altered soil material to protect it from water and materials to be compacted into a materials of the circumstance of the compacted into a materials of the circumstance of the circums



And the structure of compacted all and crackers is settles because it backs do not be a compacted and the compacted and the compacted and the flowing server, or many the compacted and the flowing server, or many the compacted and the compacted an

Collect Albertabate the Importance of texture in determining soil behavior.

apor by the U.S. Department of menor Jeton Dam Fallure Group (1977).

BOX 4.3 STOKE'S LAW IN CALCULATING PARTICLE SIZE BY SEDIMENTATION METHODS

The complete expression of Stoke's Law tells us the velocity V of a particle falling through a fixed is directly proportional to the gravitational force g, the difference between the density of the particle and the density of the fluid $(D_1 - D_1)$ and the square of the effective' particle diameter (d^2) . The settling velocity is insersely proportional to the viscosity or "thickness" of the fluid η . Since velocity equals distance h divided by time t we can write Stoke's Law as:

$$V = \frac{h}{t} = \frac{d^3 g(O_s - O_s)}{18\eta}$$

Where: g = gravitational force = 9.81 Newtons per kilogram (9.81 N/kg)

 $\eta = \text{viscosity of water at } 20^{\circ}\text{C} = 1/1000 \text{ Newton-seconds per m}^{3} (10^{-1} \text{ Ns/m}^{3})$

 D_i = density of the solid particles, for most soils = $2.65 \times 10^9 \text{ kg/m}^3$

 $D_1 = \text{density of the fluid (i.e., water)} = 1.0 \times 10^3 \text{ kg/m}^3$

Substituting these values into the equation, we can write:

where $k = \frac{9 \times 10^{\circ}}{\text{sm}}$

Note that $V = kt^2$ is the same as the simplified formula given in the text.

Let's choose to sample a soil suspension at 0.1 m (10 cm) depth. We can calculate the seconds of unthing time we must allow if we want the smallest ofit particle to have just passed our sampling depth so our maple will contain only clay.

Chosen:

$$h = 0.1 \text{ m}$$

$$d = 2 * 10^{-6} \text{ m} (0.002 \text{ mm, smallest silt)}$$

Solving for t we can write:

$$\frac{h}{l} = d^2 k \implies \frac{l}{h} = \frac{l}{d^2 k} \implies l = \frac{h}{d^2 k}$$

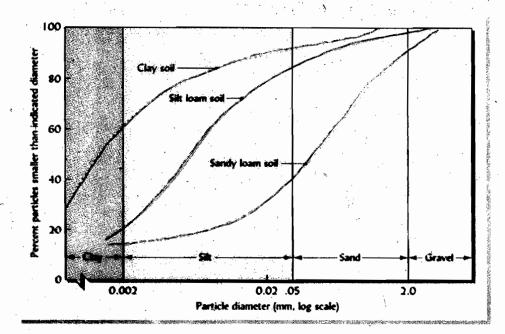
Therefore

$$t = \frac{0.1 \text{ m}}{(2 \cdot 10^{-3} \text{ m})^2 \cdot 9 \cdot 10^{9} \text{ c}^{-1}/\text{m}^{-1}}$$

1 = 27,777 seconds = 463 minutes = 7,72 hours

By comparison, the smallest sand particle (d = 0.05 sim) would make the same journey in prev 44 seconds

^{*} Stoke's Law applies to smooth round particles. Successort and particles are neither smooth not round, acdimentation techniques determine the effective diameters, not recessarily the actual diameter of the soil particle.



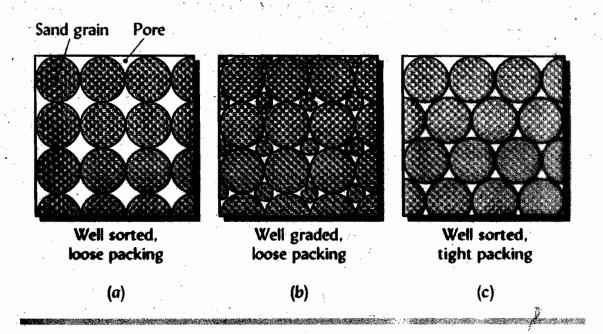
FROM 4 10 Particle-size distribution in three soils varying widely in their textures. Note that there is a gradual transition in the particle-size distribution in each of these soils,

Generalized Influence of Soil Separates on Some Properties and Behavior of Soils.^a

Rating associated with soil separates

Property/behavior	Sand	Silt	Clay
* Water-holding capacity	Low	Medium to High	High
Acration	Good	Medium	Poor
- Drainage rate	High	Slow to Medium	Very show
Soil organic matter level	Low	Medium to High	High to Medium
- Decomposition of organic matter	Rapid	Medium	Slow
Warm-up in spring	Rapid	Moderate	Show
Compactability	Low	Medium	High
Susceptibility to wind erosion	Moderate (high if fine sand)	High	Low
Susceptibility to water erosion	Low (unless fine sand)	High	Low if aggregated, high if not
Shrink-swell potential	Very Low	Low	Moderate to very high
Scaling of ponds, dams, and landfills	Poor	Poor	Good
- Suitability for tillage after rain	Good	Medium	Poor
Pollutant leaching potential	High	Medium	Low (unless cracked)
Ability to store plant mutrients	Poor	Medium to high	High
Resistance to pH change	Low	Medium	High
contract of the second	1	,	

Exceptions to these generalizations do occur, especially as a result of soil structure and clay minerology.



Soil Mass and Volume Relations

PARTICLE DENSITY (MEAN DENSITY OF SOLIDS) ρ_s

$$\rho_s = M_s/V_s$$

In most mineral soils, the mean density of the particles is about 2.6-2.7 gm/cm³, close to the density of quartz

DRY BULK DENSITY ρ_b

$$\rho_b = M_s / V_T = M_s / (V_s + V_a + V_w)$$

The dry bulk density expresses the ratio of the mass of dried soil to its total volume (solids and pores together).

In sandy soils, ρ_b can be as high as 1.6, whereas in aggregated loams and in clay soils, it can be as low as 1.1 gm/cm³.

The bulk density is affected by the structure of the soil, i.e., its looseness or degree of compaction, as well as by its swelling and shrinkage characteristics, which are dependent upon clay content and wetness.

POROSITY n

$$n = V_v / V_T = (V_a + V_w) / (V_s + V_a + V_w)$$

The porosity is an index of the relative pore volume in the soil. Its value generally lies in the range 0.3-0.6 (30-60%). Coarse-textured soils tend to be less porous than fine-textured soils, though the mean size of individual pores is greater in coarse-textured soils.

In clayey soils, the porosity is highly variable as the soil alternately swells, shrinks, aggregates, disperses, compacts, and cracks.

The total porosity, in any case, reveals nothing about the pore size distribution, which is itself an important property to be discussed in a later in the course.

VOID RATIO e

$$e = (V_a + V_w)/V_s = V_v/(V_T - V_v)$$

The void ratio is also an index of the fractional volume of soil pores, but it relates that volume to the volume of solids rather than to the total volume of soil.

Void ratio is the generally preferred index in soil engineering and mechanics, whereas porosity is the more frequently used index in agricultural soil physics. Generally, e varies between 0.3 and 2.0.

SOIL WETNESS

The wetness, or relative water content, of the soil can be expressed in various ways: relative to the mass of solids, relative to the total volume, and relative to the volume of pores. The various indexes are defined as follows:

Mass Water Content θ_m

$$\theta_m = M_w/M_s$$

This is the mass of water relative to the mass of dry soil particles, often referred to as the gravimetric water content.

The term dry soil is generally defined as a soil dried to equilibrium in an oven at 105 C. In a mineral soil that is saturated, θ_m can range between 25 and 60% depending on the bulk density.

The saturated water content is generally higher in clayey than in sandy soils. In the case of organic soils, such as peat or muck, the saturated water content on the mass basis may exceed 100%.

Volumetric Water Content θ_v

$$\theta_{v} = V_{w}/V_{T} = V_{w}/(V_{s} + V_{v})$$

In sandy soils, the value of θ_v at saturation is on the order of 40-50%; in medium-textured soils, it is approximately 50%; and in clayey soils, it can approach 60%.

In the latter, the relative volume of water at saturation can exceed the porosity of the dry soil, since clayey soils swell upon wetting.

The use of θ_v rather than of θ_m to express water content is often more convenient because it is more directly adaptable to the computation of fluxes and water quantities added to soil by irrigation or rain and to quantities subtracted from the soil by evapotranspiration or drainage. This is because θ_v also represents the depth ratio of soil water, i.e., the depth of water per unit depth of soil.

Degree of Saturation S

$$S = V_w/V_v$$

This index expresses the volume of water present in the soil relative to the volume of pores. The index S ranges from zero in dry soil to unity (or 100%) in a completely saturated soil.

However, complete saturation is seldom attained in soils, since some air is nearly always present and may become trapped in a very wet soil.

AIR-FILLED POROSITY (FRACTIONAL AIR CONTENT) n₂

$$n_a = V_a/V_T = V_a/(V_s + V_a + V_w)$$

This is a measure of the relative air content of the soil, and as such is an important criterion of soil aeration.

(modified from Hillel (1980))

Example calculation of bulk density, porosity, void ratio, mass water content, volumetric water content, degree of saturation and air-filled porosity.

A sample of moist soil having a wet mass of 1000 gm and a volume of 640cm³ was dried in an oven at 105 C and found to have a dry mass of 800 gm. Assuming the typical value of particle density for a mineral soil, calculate the bulk density, porosity, void ratio, mass water content, volumetric water content, degree of saturation, and air-filled porosity.

Void Ratio (e)

Mass water cost ent on

$$\frac{O_{m} = \frac{M_{w}}{m_{s}} = \frac{M_{T} - M_{s}}{m_{s}} = \frac{1000_{3} - 800_{3}}{800_{3}} = 0.25$$

Volumetric Water content Ou

$$O_{V} = \frac{V_{w}}{V_{\tau}} = \frac{200 \, \text{cm}^{3}}{640 \, \text{cm}^{3}} = 0.3125 \Rightarrow 31.25\%$$

degree of satuation (5)

$$S = \frac{V_w}{V_v} = \frac{V_w}{V_t - V_s} = \frac{200c^3}{640c^3 - 30.9c^3} = 0.592$$

Air-fillel porosity (na)

$$N_{a} = \frac{V_{a}}{V_{T}} = \frac{V_{T} - V_{w} - V_{s}}{V_{T}} = \frac{(640 - 200 - 301.9)_{c.7}}{640 co.7} = 0.216$$

Additional Mass and Volume Relations

1. Porosity (n) and Void Ratio (e)

$$e = \frac{n}{1-n} \qquad n = \frac{e}{1+e}$$

2. Volumetric Water Content (θ_n) and Saturation (S)

$$\theta_{\mathbf{v}} = nS$$
 $S = \frac{\theta_{\mathbf{v}}}{n}$

3. Porosity (n) and Bulk Density (ρ_s)

$$n = \frac{\left(\rho_s - \rho_b\right)}{\rho_s} = 1 - \frac{\rho_b}{\rho_s}$$

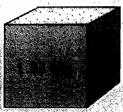
$$\rho_b = (1-n)\rho_s$$

4. Mass water Content (θ_m) and Volumetric Water Content (θ_v)

$$\theta_{v} = \theta_{m} \frac{\rho_{b}}{\rho_{w}} \qquad \theta_{m} = \theta_{v} \frac{\rho_{w}}{\rho_{b}}$$

In the field, one cubic meter of a certain soll appears as . . .

Solids and pore spaces



To calculate bulk density of the soil:

Weight = 1.33 Mg (solids only)

Bulk density =

Weight of oven dry soil

Volume of soil
(solids + pores)

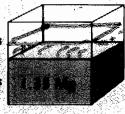
Therefore

Example 2 $D_{\rm b} = \frac{1.33}{1} = 1.33 \, \text{Mg/m}^3$

If all the solids were compressed to the bottom, the cube would look like...

1/2 pore spaces

1/2 solids



To calculate solid particle density:

Volume = 0.5 m^3 (solids only)

Weight = 1.33 Mg (solids only)

Weight of solids

Volume of solids

Solid particle density = -

Therefore

Solid particle density, $D_p = \frac{1.33}{0.5} = 2.66 \text{ Mg/m}^3$



PS

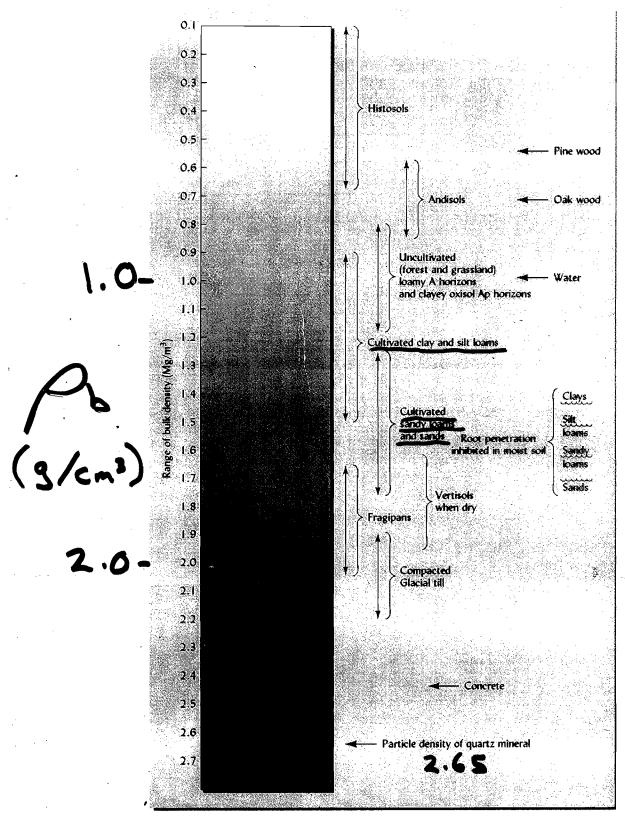


FIGURE 4.15 Bulk densities typical of a variety of soils and soil materials.

TABLE 4.5 A Size Chassification of Soil Pores and Some Functions of Each Size Class

Pore sizes are actually a continuum and the boundaries between classes given here are inexact and somewhat arbitrary. The term inscropore is often broadened to refer to all the pores smaller than macropores.

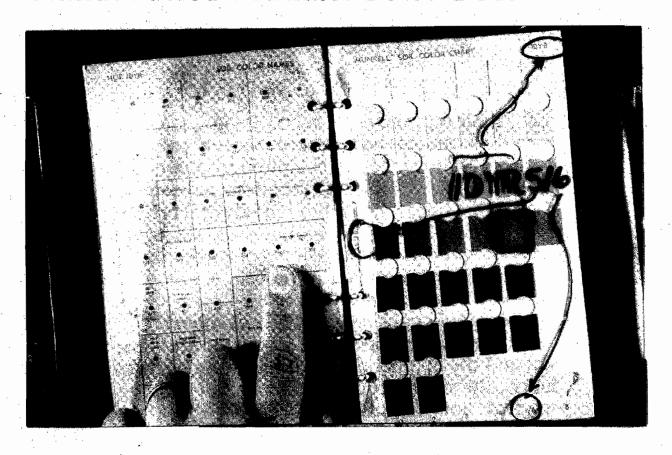
ě		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
Simplified class	Class	Especiave duameter range (nam)	Characteristics and functions
Macropores	Macropores	0.08-5+	Generally found between soil peds (interped); water drains by gravity; effectively transmit air; large enough to accommodate
Micropores	Mesopores	0.03-0.08	Retain tours, manual to certain soil spinning. Retain water after designing; transmit water by capillary action.
	Micropores	6.005-0.03	Generally found within peds (intraped); retain water that plants
	Ultramicropores	0.0001-0.005	Found largely with clay grouping; retain water that plants cannot
*	Cryptopores	<0.0001	unc, enclose more macrocognisment. Exclude all inscroonganisms, too small for large molecules to enter

^{*} The pore size classes and boundary diameters are those of Brewer, 1964 as cited in Soil Sci. Sec. Amer. (1996).

146 Soil Architecture and Physical Properties

Soil Colour

Standardised Munsell Color Book



See plates between p. 82-83 in Text