

Coastal Dynamics 1

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10. Coastal Protection

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Coastal protection

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- A. Introduction
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- C. Types of coastal erosion processes
- D. Modification of longshore transports
- E. Hard' measures influencing littoral drift
- F. Hard' measures for storm induced erosion
- G. Artificial sand nourishment ('soft' measures)



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Sepetition

10-B Coastal protection strategies

Selection of coastal protection method

- Important to have good insight in coastal processes
- Two basic approaches:
 - 1. Try to solve the cause of the erosion problem (cure the cause)
 - 2. Mitigate the negative effects (cure the symptoms)
- Possible solutions to mitigate coastal erosion problems:
 - "soft" (natural) measures (beach and foreshore nourishment)
 - · "hard" measures (coastal structures)





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10-C Types of coastal erosion processes

Two types of erosion

1. Structural erosion

See Chapter 8

- long-term, gradual, due to 'normal' and slow processes
- e.g. 1 m/yr or 20 m³/m per year (if profile height is 20 m)
- 2. Episodic erosion, during severe storm (surge) events
 - i.e. dune erosion due to storm surge
 - fast process (hours)
 - e.g. $100 \text{ m}^3/\text{m}$ in 6 hours or even 200 to $300 \text{ m}^3/\text{m}$ (10 15 m) under design conditions

See Chapter 7

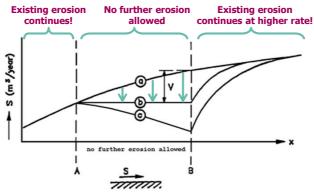
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Repetition

10-D Modification of longshore transports

Longshore transport curve along eroding coast



- Gradient in longshore sediment transport (S_B S_A) causes structural erosion in stretch A-B.
- Change curve of longshore transport such that gradient = 0 (from line a to line b).



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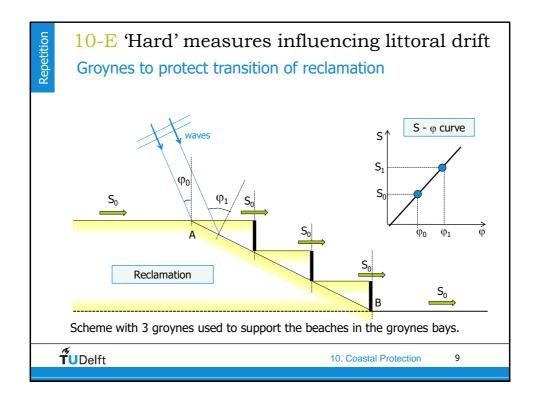
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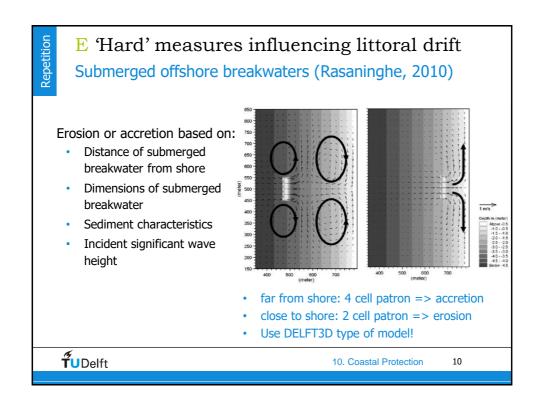
10-E 'Hard' measures influencing littoral drift Introduction

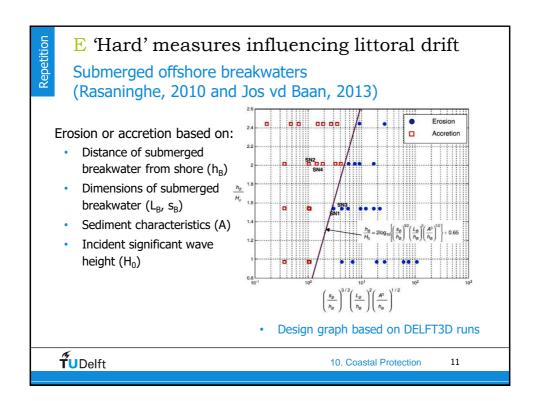
- In order to mitigate structural erosion with structures ('hard' measures), they must interfere in sediment transports processes.
- Possible structures are:
 - a) Jetties / shore-normal breakwaters;
 - b) Series of groynes;
 - c) Detached (offshore) breakwaters (emerged or submerged).
- Even for well-designed protection scheme, lee-side erosion is unavoidable.

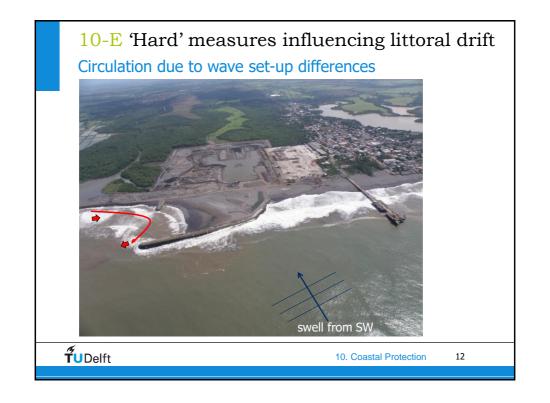
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Coastal protection

Chapter 10 of lecture notes

- A. Introduction
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- F. 'Hard' measures for storm induced erosion
- G. Artificial sand nourishment ('soft' measures)



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10-F 'Hard' measures for storm induced erosion

Introduction

- If storm-induced erosion is unacceptably large, structures may help to reduce erosion.
- Groynes do not reduce offshore directed sediment transport
- Detached emerged or submerged breakwaters only have limited effect on reduction of extreme wave height.
- Structures that may be effective against storm-induced erosion are:
 - Seawall
 - Revetment
 - · Sea-dike





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10-F 'Hard' measures for storm induced erosion Seawall at Lancashire, England, 1998



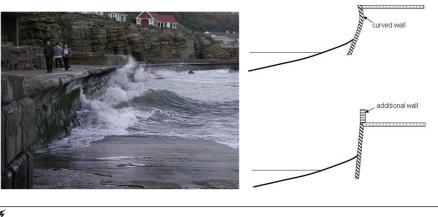
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10-F 'Hard' measures for storm induced erosion Seawall

- Seawall is vertical structure at transition between beach and dune of mainland.
- Generally smooth surface and impermeable, easy to build.
- Principle: storm erosion is prevented by cutting off supply of material from dune or mainland.
- Seawall tends to reflect incoming waves.
 Increased turbulence forms a trough along toe of seawall.
 This trough may endanger its foundation.
- This can be prevented by maintaining the sea bed in front of seawall by means of regular sand nourishment.

10-F 'Hard' measures for storm induced erosion Seawall

 Overtopping may be reduced by applying slightly curved sea wall or additional wall.



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10-F 'Hard' measures for storm induced erosion Revetment

- Revetment is similar to seawall. Revetment has gentle slope (e.g. 1:2 or 1:4) with either rough or smooth surface.
- Slope and type of surface of revetment influences the scour depth
 - Model tests at Deltares showed that steeper slope causes deeper scour hole than milder slopes.
 - · Scour depth is larger for smooth slope than for rough slope.
- Revetment for dune foot protection:
 The elevation of revetment determines volume of dune erosion and scour depth (higher revetment => less dune erosion => deeper scour hole)

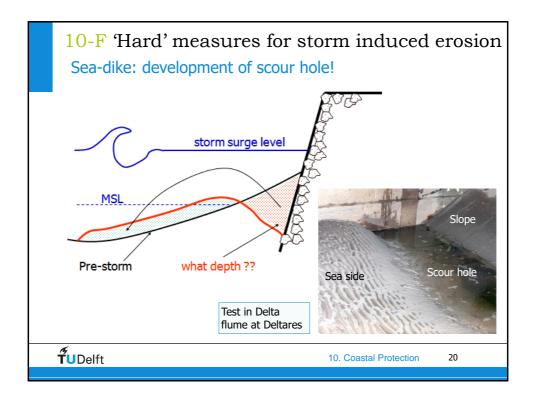
10-F 'Hard' measures for storm induced erosion Sea-dike

- Meant to prevent flooding (just like a river dike).
- Usually no beach in front of sea-dike.

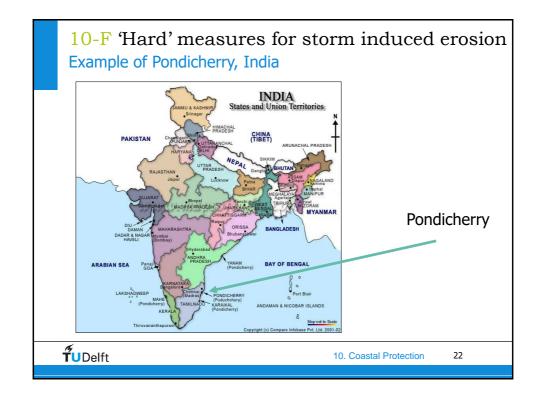
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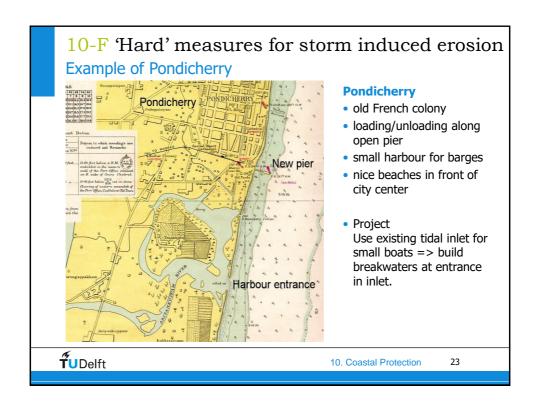


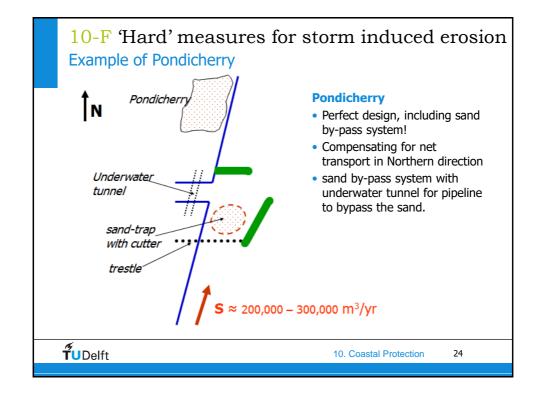
- Crest height must be high enough to prevent overtopping
- Flat and smooth slopes causes high wave run-up => high dike.
- Point of concern: erosion just in front of sea-dike (larger water depth results in higher wave at toe and higher wave run-up.



10-F 'Hard' measures for storm induced erosion Do shore-parallel structures prevent structural erosion? MSL Sxalong coast erosion due to gradient in longshore transport No! These structures do not influence longshore transport gradients! Structural erosion continues in the area below MSL.







10-F 'Hard' measures for storm induced erosion Cutter dredge for sand by-pass system at Pondicherry



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10-F 'Hard' measures for storm induced erosion Pondicherry's sand by-pass system **failed**



Effects:

- Accretion at the South side
- Structural erosion at the North side

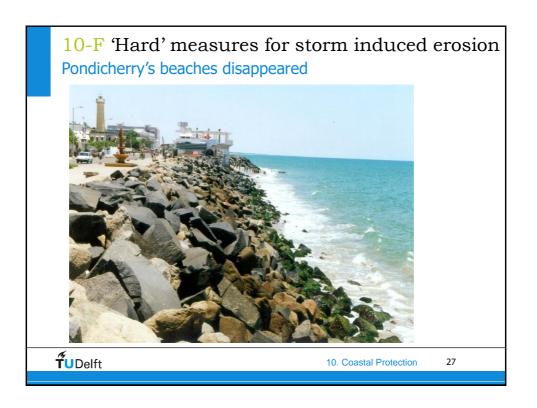
Applied solution:

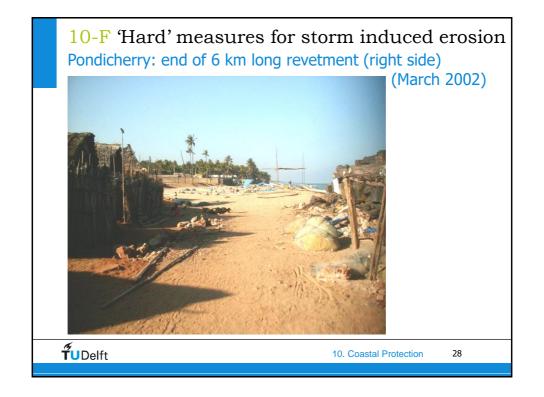
- <u>Revetment</u> along coast, 6 km long
- Is extended from time to time
- No beaches left

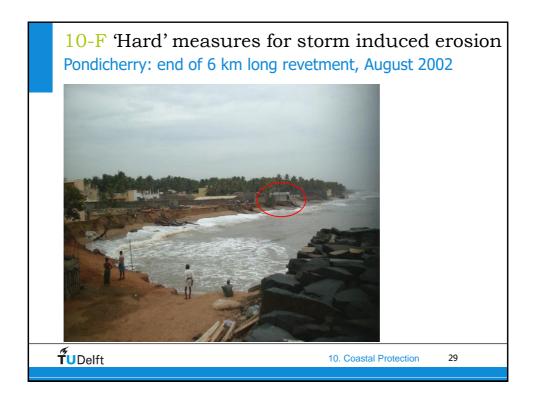
This revetment is good example of bad solution for structural erosion!

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10-F 'Hard' measures for storm induced erosion Pondicherry, 2007



- Revetment extended till last buildings of Pondicherry
- Erosion continues

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Coastal protection

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- **G.** Artificial sand nourishment ('soft' measures)

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Introduction

- Compensation for structural erosion
- Protection of beach and dunes against storm erosion.
- Creation of new beaches
- Reclamation of new land.
- Must be repeated regularly (only treating symptoms).
- Leaves coast in natural state, without lee-side erosion.
- Flexible: scheme can be modified if results are not as expected.
- Good for coastal system: sediment is added to it.



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10-G Artificial nourishment ('soft' measures) Introduction

- Nourishment can "never" go wrong (except for bad designed nourishment scheme)
- Nourishment is often an economic solution due to its cost structure (no capital cost, only maintenance cost).



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Design aspects - Origin of sand

Borrow sand

- Land sources (river beds or sand deposits)
- Marine sources (estuaries or sea bed)
- Maintenance dredging (Dutch: make work with work)

Borrow pit

- At sufficiently large distance from shore (in NL: 20 km) to prevent coastline erosion;
- Small and deep versus large and shallow?
 - Deep => stagnant water with poor quality
 - Large => environmental disturbance of surface layer
 - No clear recommendation. Perform EIA for each project



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10-G Artificial nourishment ('soft' measures)

Design aspects - Quality of sand

- Use preferably borrow sand that is similar to native material (same behavior)
- Sometimes coarser material is used to reduce losses (steeper slopes).
- Borrow material contains silt (2%-5% is normal), which may have negative impact on marine environment (during overflow).
- If necessary wash out silt before placing sand on beach.



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Design aspects - Location of nourishment

- Location of nourishment:
 - Landward, seaward or on top of dunes
 - Dry beach
 - Shoreface

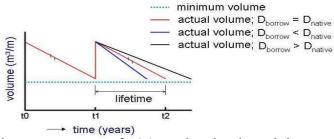


- Sand for (landward) dune nourishment is often from land sources, because marine sand may cause salt problems for dune vegetation.
- Placing sand on beach requires dredging equipment to cross breaker zone. Rainbowing is alternative solution (shallow water).
- Shoreface nourishment is placed at the seaward edge of the surf zone with sufficient depth for the hopper dredge to navigate.
- Effectiveness of shoreface nourishment is lower than that of beach nourishment. At the other hand unit cost are lower.



10-G Artificial nourishment ('soft' measures)

Re-nourishment for structural erosion



- Start re-nourishment at moment of minimum beach volume (t₁).
- Generally lifetime is 5 to 10 years (due to high mobilization costs).
- Borrow sand coarser => loss decreases => lifetime increases (black)
- Borrow sand finer => loss increases => lifetime decreases (blue)

Example nourishment project

- Assume coastline retreat is 2 m per year => sand loss over total profile height (20 m) is $\Delta V = 40 \text{ m}^3/\text{m}'$ per year.
- Assume period between nourishments is
 5 year => volume to be nourished is 200 m³/m′ every 5 years.
- Assume stretch of coast is 5 km long => total volume to be nourished is 1,000,000 m³ every 5 years.
- Volume is increased with 10% to 20% to account for additional losses of the fine fraction.
- Contract with dredging contractor:
 - Long term contract for regular nourishment of stretch of coastline
 - Combine foreshore with beach nourishment (flexibility contractor)
 - Control method (hopper volume measurements or as built survey)



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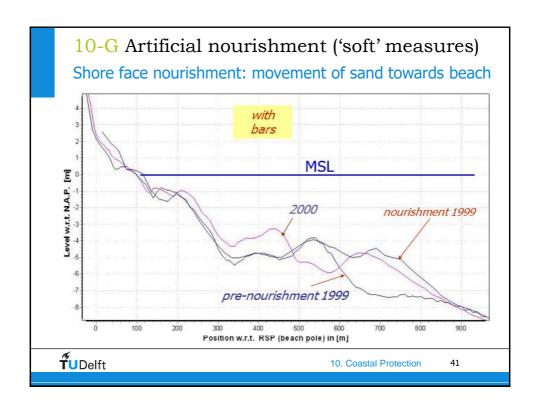
10-G Artificial nourishment ('soft' measures)

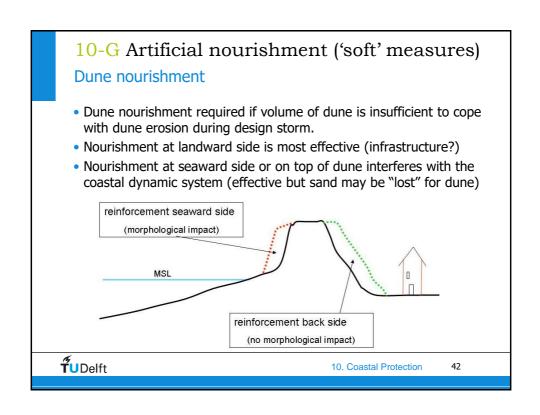
Shoreface nourishment

- Larger nourishment volumes are required as only 30% to 50% of nourishment volume will reach the beach.
- Costs per m³ for shoreface nourishment is 50% to 70% less than for beach nourishment
- Total cost in balance: 100 m³ x € 10/m³ = 200 m³ x € 5/m³

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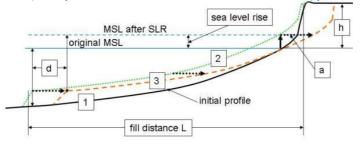
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10-G Artificial nourishment ('soft' measures)

Possible to counteract sea-level rise with nourishment?

How to determine equilibrium profile with sea level rise (profile 3)? (Chapter 7)

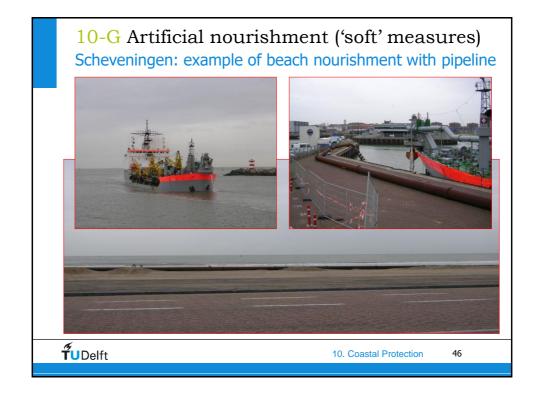
- Profile 2 is obtained by vertical shift of initial profile 1 over distance SLR.
- Profile 3 is obtained by horizontal shift of profile 2 over distance a. $(SLR \times L) = a \times (d+h) => a = (SLR \times L)/(d+h)$ $a = (1 \text{ m}/100 \text{ yr} \times 1000 \text{ m})/(10 \text{ m} + 10 \text{ m}) = 0.5 \text{ m/yr}$
- SLR of 1 m / 100 yr results in structural coastal erosion of 0.5 m/yr!



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10-G Artificial nourishment ('soft' measures) Possible to counteract sea-level rise with nourishment? • Nourishment can be used to compensate space between profile 1 and 2 created by the sea-level rise. Required volume = SLR x distance L • E.g. SLR = 1 m per century and L = 1000 m => volume is 10 m³/m per yr. • Nourishment of 100 m³/m per 10 year is a standard job to do! • Dutch coast: 200 km x 100 m³/m = 20 Mm³ per 10 yr sea level rise MSL after SLF original MSL initial profile fill distance L **TU**Delft 10. Coastal Protection 45



10-G Artificial nourishment ('soft' measures) Scheveningen: beach nourishment by pipeline



Much attention is given to informing the general public!

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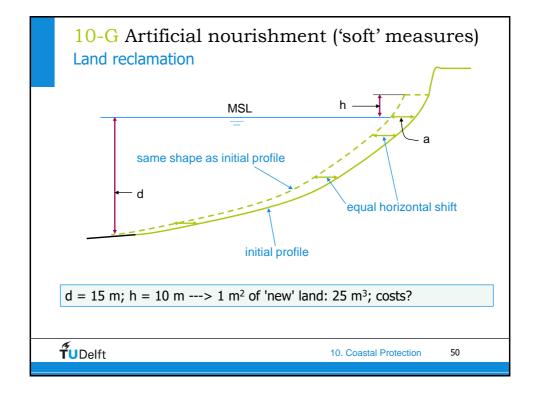


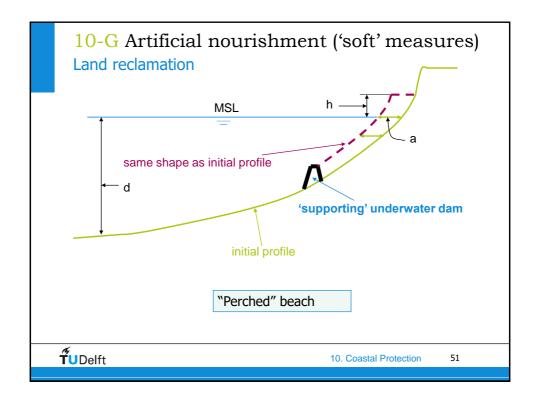


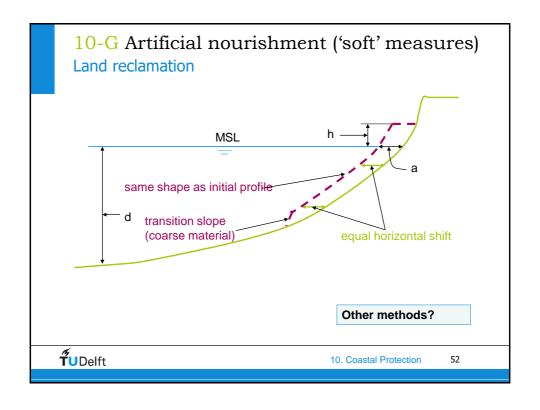
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10-G Artificial nourishment ('soft' measures) Land reclamation for new port at Maasvlakte 2



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10-G Artificial nourishment ('soft' measures)

Maasvlakte 2: necessary extension of Port of Rotterdam



- Extension 20% with 2,000 ha:
 - at water depth of NAP -18 m.
 - located seaward of existing port at Maasvlakte 1.
- In particular for container vessels of 12,500 TEU (nautical depth 20 m).
- PUMA (JV Van Oord and Boskalis) won DCM (design, construct and maintenance) contract, with maintenance till 2018.
- Total volume of sand is 240 million m³ (of which 30 million m³ obtained from deepening basins and channels).
- First vessel used the port in 2013.

Maasvlakte 2 with 11 km long sea defence



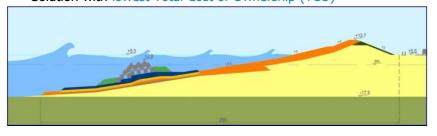
- 3.5 km hard sea defence with cobble beach and emerged reef
- 8 km soft sea defence with coarse sand

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10-G Artificial nourishment ('soft' measures)

Maasvlakte 2 - Sea Defence

- Thick layer of crushed cobbles (10 cm) over a sand core (0.35 mm) with foreshore reef of re-used concrete blocks (40 t).
- S-shaped cross-shore profile to absorb wave energy (deformation, porosity)
- Physical and numerical modelling was done to optimize design.
- Solution with lowest Total Cost of Ownership (TCO)



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Maasvlakte 2 - Program of requirements

System-oriented way of design, without reference design of client (design freedom).

Requirements:

- No sedimentation in Maasgeul, so northern section of the sea defence must be hard. Southern section may be soft. Position of boundary between hard and soft is free to define as long as all other requirements are fulfilled.
- Cross-currents should meet nautical criteria for safe navigation in the adjacent Maasgeul.
- Design storm for sea defence once every 10,000 years ($H_s = 7.9$ m, $T_p = 13.5$ s from NW and SSL = MSL +5.1 m)
- Lowest TCO (Total Cost of Ownership): sum of present value of construction costs and present value of 50 years of maintenance costs.



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10-G Artificial nourishment ('soft' measures)

Maasvlakte 2 - Model testing - Cross-shore dynamics

Movable bed model 2D at scale 6 (1:6) Old Delta flume (240 m x 5 m x 7 m):

- Cross-shore deformation
- Wave overtopping
- Check movement of sand into cobble layer!

Possible effect of intruded sand on cross-shore deformation and wave overtopping (less erosion of lower slope => less deposition near crest => more wave overtopping).



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10-G Artificial nourishment ('soft' measures) Maasvlakte 2 - Model testing - Longshore dynamics



Movable bed model 3D at scale 20 (HRW, 31 m x 24 m x 0.9 m)

- Shoreline deformation for annual maintenance and design condition
- Definition of alongshore buffer for cobble



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10-G Artificial nourishment ('soft' measures)

Maasvlakte 2 - Model testing - longshore dynamics

Numerical modelling DELFT3D (flow, wave, STONE)

- Cobble transport is estimated making use empirical bed-load formula of Bijker (originally developed for gravel rivers).
- Calibrated with results of scale model test (HRW)
- Validated for two cases (Shoreham, UK and Lake Coleridge, NZ).
- Alongshore cobble drift was calculated for design storm and for year-round conditions.
- Very much computational effort

Parametric 1D model (alongshore component of radiation stress)

- Relation S_{xv} alongshore cobble drift (similar to CERC formula).
- Including threshold (initiation of motion) of cobble.
- Calibrated using results of DELFT3D runs.
- Strong reduction of computational effort!

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10-G Artificial nourishment ('soft' measures) Maasvlakte 2: construction started in 2010

- Started with creation of small banana-shaped island at location of future sea defence
- Suction Hopper dredges of different size dredge coarse sand at distance between 7 and 20 km off the coast.
- After preparing coble profile 20,000 blocks of existing block dam have been placed at foreshore of hard sea defense.





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10-G Artificial nourishment ('soft' measures)

Mega nourishment: The sand motor at Delfland



Founder of the Sand Motor



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Mega nourishment: The sand motor at Delfland

- Delfland has vulnerable coastline.
- Dutch are continuously looking for new methods to protect The Netherlands against flooding and to gain land from the sea.
- Mega nourishment is unique in the world.
- Initially 21.5 Mm³ of sand was nourished from March 2011 to March 2012.



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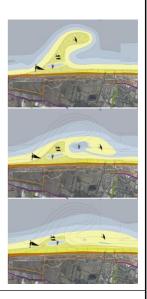
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10-G Artificial nourishment ('soft' measures)

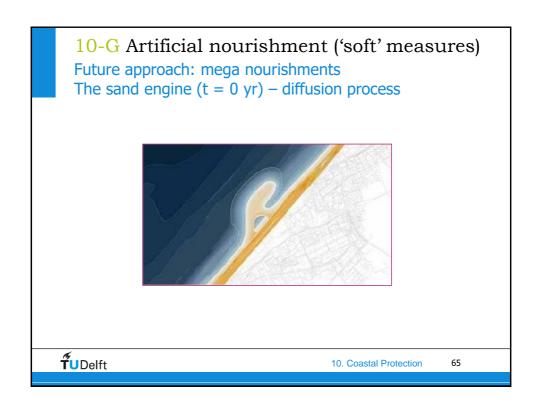
Future approach: mega nourishment The sand motor at Delfland (0 - 20 yrs)

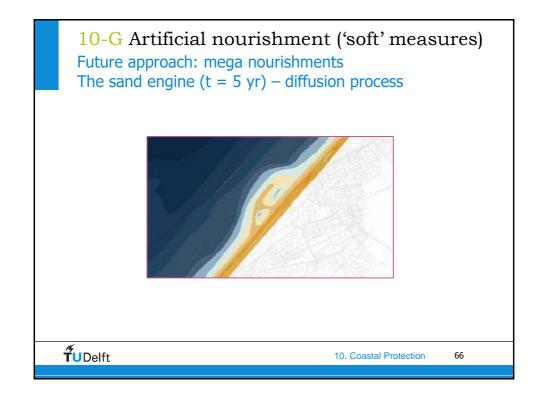
- Total surface area is 100 ha (1 km²) eventually resulting in 35 ha of new beach (65% moved alongshore)
- Morphological behaviour will be monitored to calibrate and improve our prediction models.
- Sea currents are constantly monitored for public safety.
- Project of Rijkswaterstaat and Province of Zuid-Holland.
- Executed by JV Van Oord and Boskalis

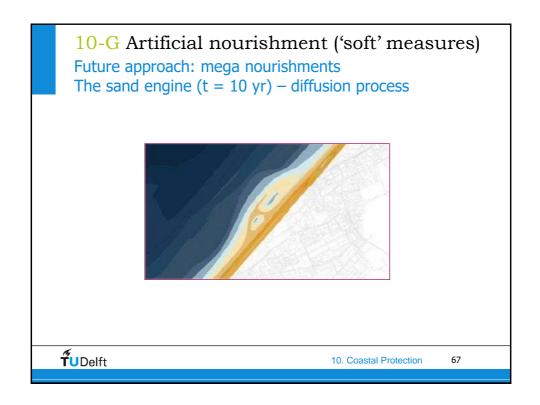


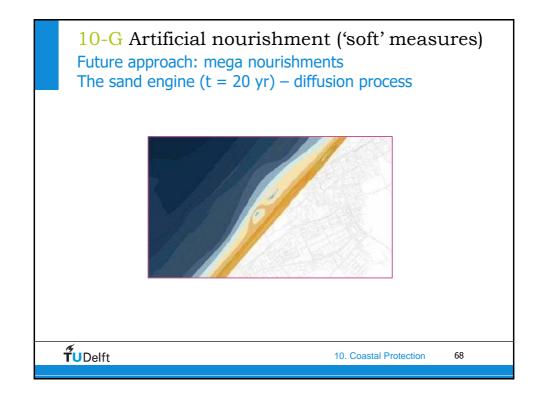
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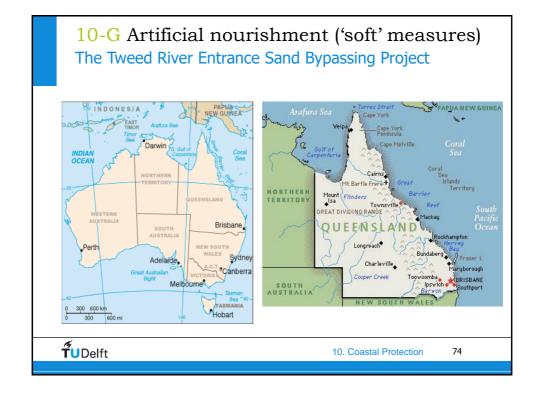




10-G Artificial nourishment ('soft' measures)
Movie "The sand motor" from VPRO (NL van Boven, 2011)

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10-G Artificial nourishment ('soft' measures) The Tweed River Entrance Sand Bypassing Project

- Joint initiative of New South Wales and Queensland State Governments.
- DCO contract to McConnell Dowell to operate project for 25 years.
- Operation started in 2001.

Background:

- Training walls at Tweed River from 1960's to improve navigation.
- Sediment transport is 500,000 m³/yr northwards.
- Training wall blocked transport causing accumulation at Letitia Spit South of entrance.
- Beaches at Gold Coast eroded due to cutting of natural sand supply.





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10-G Artificial nourishment ('soft' measures) The Tweed River Entrance Sand Bypassing Project

Aim of sand bypass system:

- To move sand past Tweed River entrance.
- To improve navigation conditions at the river mouth.
- To provide the Southern Gold Coast beaches with a constant supply of sand in order to bring them back to the state of before the 1960's.



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The Tweed River Entrance Sand Bypassing Project

The sand bypass system consists of:

- a 450 m long jetty south of the Tweed River entrance
- 10 submersible jet pumps at jetty located under the sea bed
- control building and deposition basin from where sand slurry is pumped northwards under river
- 3 km of underground pipeline to transport sand to the outlets at East Snapper Rocks, West Snapper Rocks and Kirra Point.



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