

# Coastal Dynamics 1 (CIE4305)

Judith Bosboom, Marcel J.F. Stive

Section of Hydraulic Engineering

# Coastal Dynamics 1

Maple-TA results Chapter 5-6

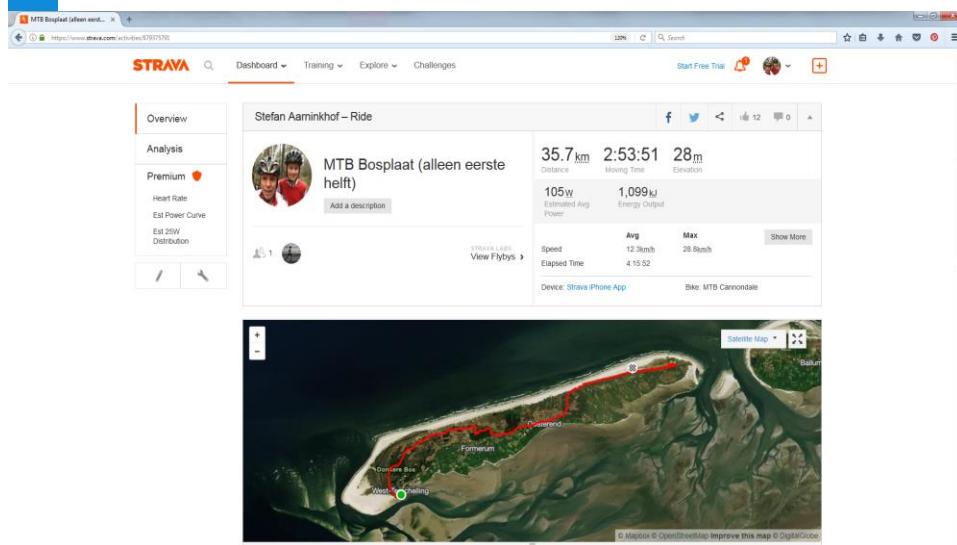
	Chapter 5 - Part II - Stage A	Chapter 5 - Part II - Stage B	Chapter 6 - Stage A	Chapter 6 - Stage B
#Attempts	188	186	183	180
Mean	91%	72%	91%	76%
Median	90%	77%	87%	83%
Total points	20.0	19.0	15.0	12.0

# 7.

## Cross-shore transport and profile development



## Coastal Dynamics 1



# Coastal Dynamics 1

## Terschelling – Boschplaat Tour



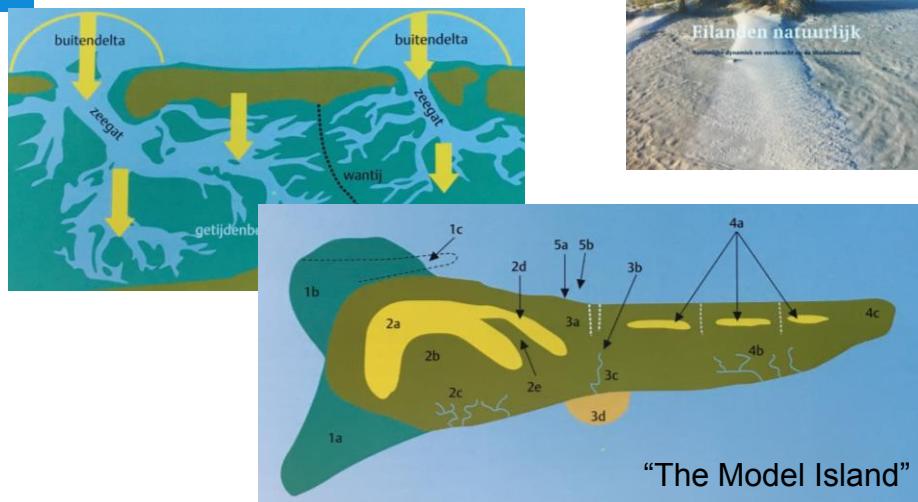
TU Delft

7. Cross-shore transport and profile development

5

# Coastal Dynamics 1

## 'Eilanden Natuurlijk' – Het tij geleerd



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7. Cross-shore transport and profile development

6

# Coastal Dynamics 1

## Terschelling – Washover complex



# Coastal Dynamics 1

## Terschelling – Beach erosion at tail



# Coastal Dynamics 1

## Terschelling – Salt marshes



# Coastal Dynamics 1

## Terschelling – Link to Contents CD-1

Ch.7: Cross-shore transport / development

Ch.8: Longshore transport / coastal changes

Ch.9: Coastal inlets and tidal basins



# Coastal Dynamics 1

## Contents

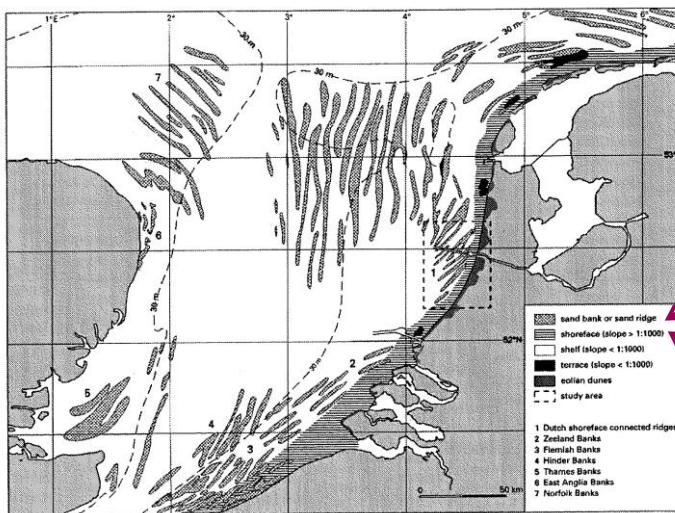
1. Introduction
2. Large-scale coastal variation
3. Oceanic wind waves and tide
4. Global wave and tidal environments
5. Coastal hydrodynamics
6. Sediment transport
- 7. Cross-shore transport and profile development (Chapter 7)**
8. Longshore transport and coastline changes
9. Coastal inlets and tidal basins
10. Coastal protection

## Cross-shore transport and profile development

### Chapter 7 of lecture notes

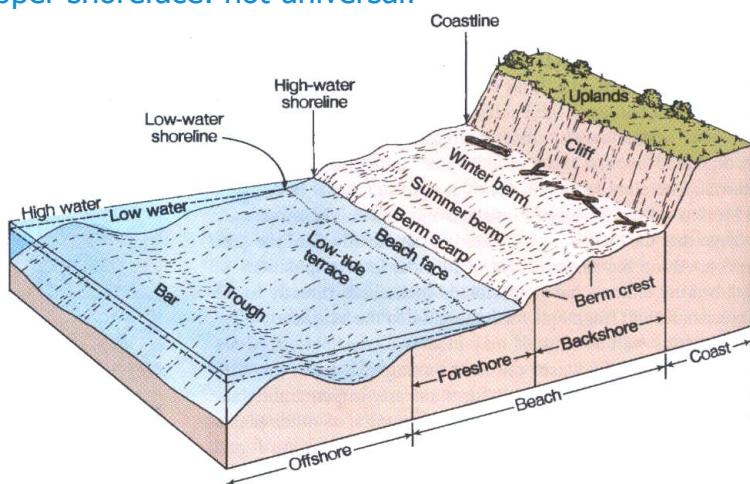
- A. Equilibrium shoreface profile**
- B. Beach states
- C. Cyclic profile and bar behaviour
- D. Episodic events: dune erosion
- E. Cross-shore transport mechanisms

## 7-A Equilibrium shoreface profile Shoreface and shelf of the North Sea



## 7-A Equilibrium shoreface profile

Upper shoreface: not universal!



## 7-A Equilibrium shoreface profile A beach scarp



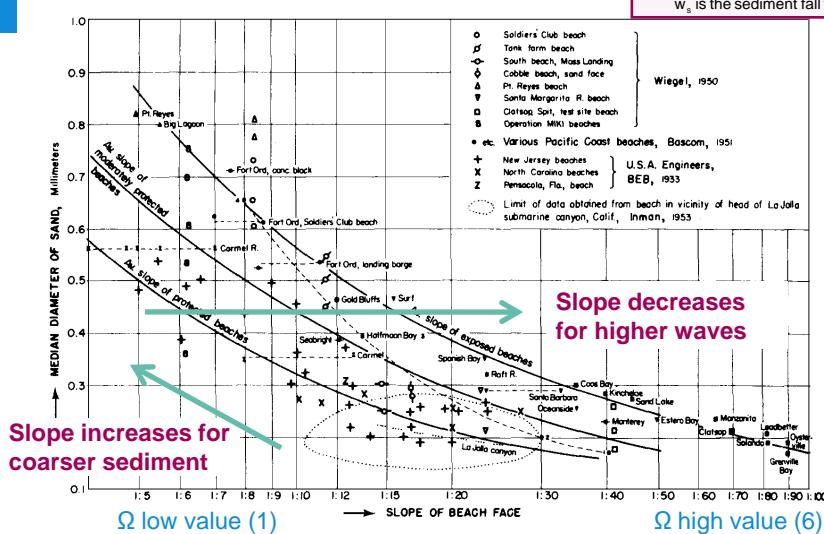
MSc. Cas van Bemmelen (TUD, 2018)

## 7-A Equilibrium shoreface profile Beach slope versus grain-size

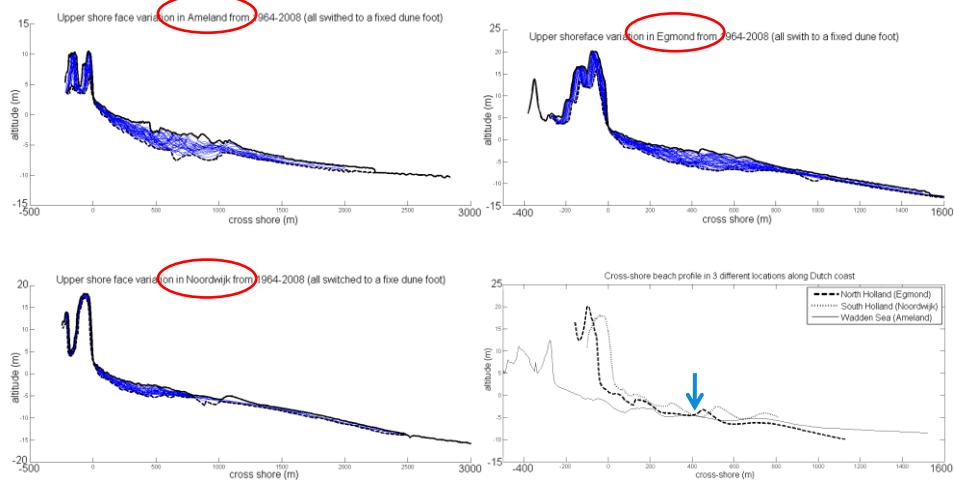
dimensionless fall velocity :

$$\Omega = \frac{H_b}{w_s T}$$

with:  $H_b$  is the wave height at breaking  
 $w_s$  is the sediment fall velocity



## 7-A Equilibrium shoreface profile Upper shoreface profile variation



## 7-A Equilibrium shoreface profile Semi-empirical derivations: Bruun (1954)

$$h = A(x')^m$$

the larger A the  
steeper the profile

**x'** => zero around the mean waterline and positive in offshore direction  
**m** =  $\frac{2}{3}$   
**A** = dimensional shape factor (order  $0.1 \text{ m}^{1/3}$ )

## 7-A Equilibrium shoreface profile

Semi-empirical derivations: Dean (1977) =>

for a certain grain-size nature strives towards a uniform energy dissipation (loss in wave power) per unit volume of water across the surf zone

$$\frac{d(Ec_g)}{dx} = -D = -h\varepsilon(D_{50})$$

Empirically:

$$A = 0.5w_s^{0.44}$$



$$\begin{aligned} \frac{d}{dx} \left( \frac{1}{8} \rho g \gamma^2 h^2 \sqrt{gh} \right) &= -h\varepsilon(D_{50}) \Rightarrow \\ \varepsilon(D_{50}) &= -\frac{1}{h} \frac{d}{dx} \left( \frac{1}{8} \rho g \gamma^2 h^{5/2} \sqrt{g} \right) = \\ &= \frac{5}{16} \rho g^{3/2} \gamma^2 \sqrt{h} \frac{dh}{dx} \end{aligned}$$

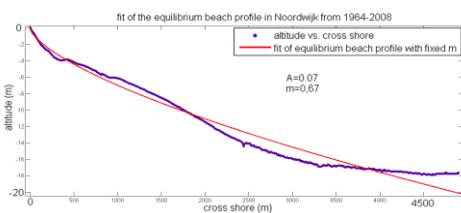
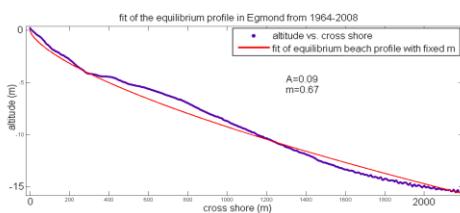
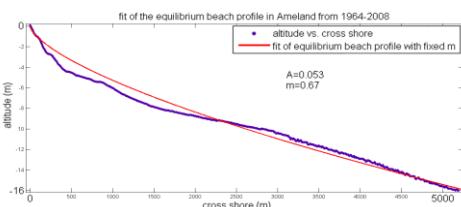
$$h = \left( \frac{24\varepsilon(D_{50})}{5\rho g^{3/2} \gamma^2} \right)^{2/3} (x')^{2/3}$$

## 7-A Equilibrium shoreface profile

Fits to long-term averaged profiles

### Values for A

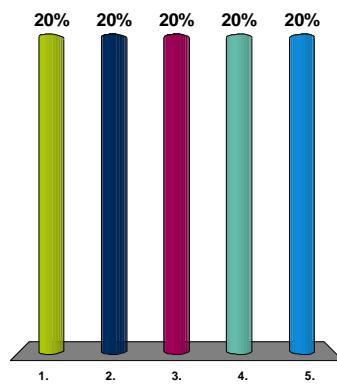
$A = 0.5w_s^{0.44}$	Ameland	Egmond	Noordwijk
0.10	0.053	0.09	0.07



## 7-A Equilibrium shoreface profile

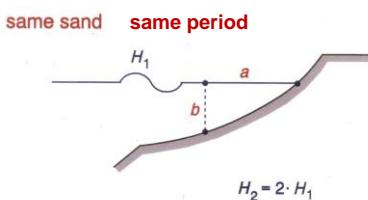
What is the most probable reason that A is different for the three regions?

1. The fall velocity of the sediment differs because of sea water temperature differences
2. The regions differ in wave exposure
3. The regions have different tidal ranges
4. A different reason
5. Abstain



## 7-A Equilibrium shoreface profile

Shape dependent on wave conditions and particle size

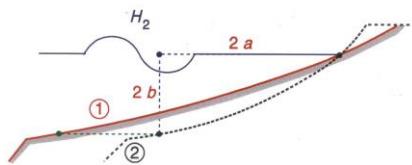


- Particle size:

- Fine sand – gentle slope
- Coarse sand – steep slope

- Wave height:

- Larger wave height – gentler slope
- Smaller wave height – steeper slope



dimensionless fall velocity :

$$\Omega = \frac{H_b}{w_s T}$$

with :  $H_b$  is the wave height at breaking  
 $w_s$  is the sediment fall velocity

## 7-A Equilibrium shoreface profile

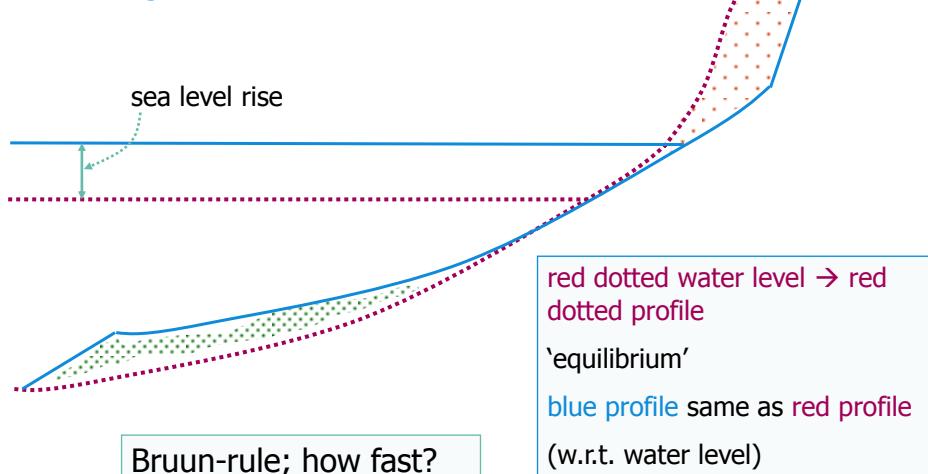
**Practical value:** what can an engineer do with this knowledge?

- Design a profile for a reclamation (see Chapter 10)
- Design nourishments with native and non-native nourishment material (see Chapter 10)
- Predict the impact of sea level rise
- Estimate dune erosion due to storm surge

However, try to calibrate and if possible validate A!

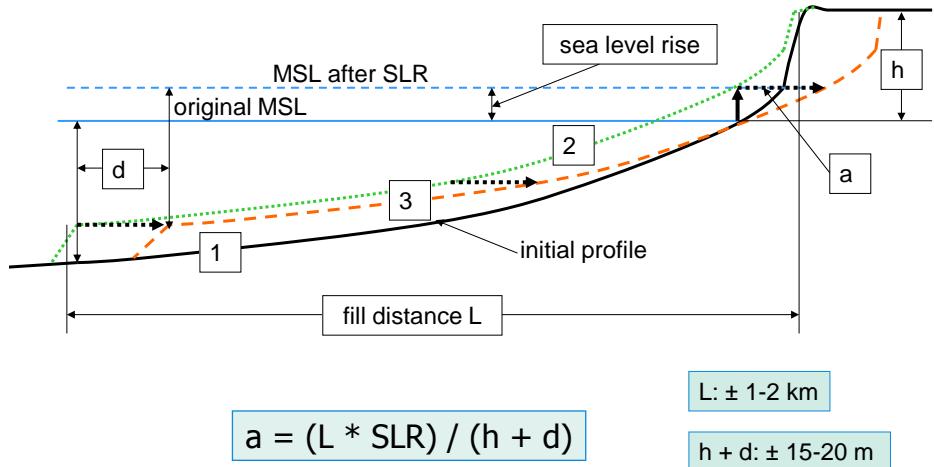
## 7-A Equilibrium shoreface profile

**Bruun-rule:** invariant profile shape in case of sea-level changes



## 7-A Equilibrium shoreface profile

**Bruun-rule: retreat distance**



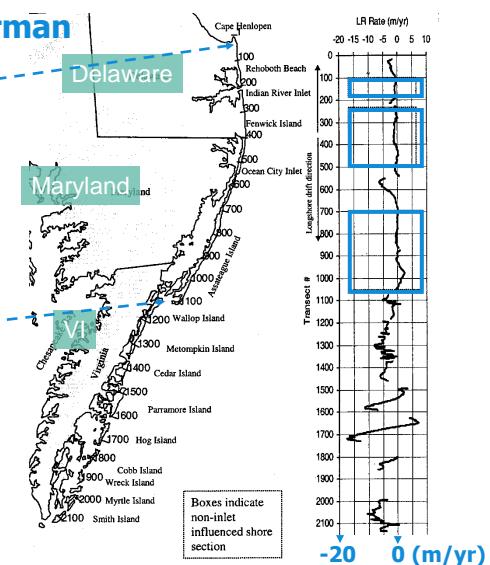
## 7-A Equilibrium shoreface profile

**Data analysis of Leatherman**



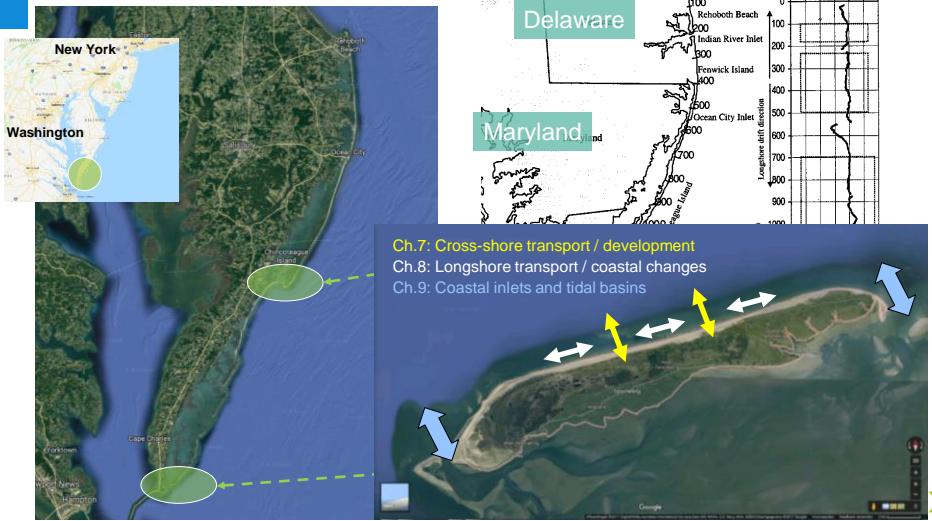
Coastal retreat due to SLR in non-inlet influenced sections:

- 50 to 120 times the SLR
- average = 78



## 7-A Equilibrium shoreface profile

### Data analysis of Leatherman



## 7-A Equilibrium shoreface profile

Why do we see much larger shoreline changes in sections influenced by tidal basins?

- ✓ 1. Tidal basins capture sediment, for instance under sea level rise
- 2. Density currents are able to transport sediment into the basin
- 3. The coastal grain-size is coarser near inlets
- ✓ 4. The ebb-tidal delta is dynamic
- 5. Abstain



# 7.

## Cross-shore transport and profile development



### 7-A Equilibrium shoreface profile Quick recap of yesterday

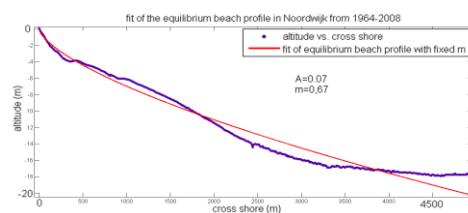
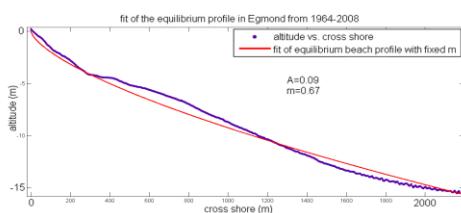
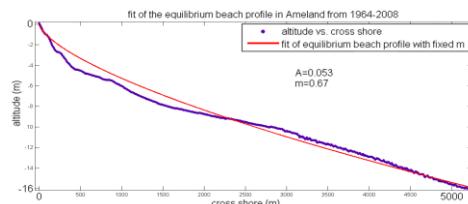
- Equilibrium profiles: Describe shoreface, long-term averaged profiles (decades and more)
- Theory (Bruun, Dean) relates profile shape to sediment characteristics only ( $w_s$ )
- BUT: Also affected by wave climate characteristics ( $H_b, T$ )
- Combined effect described by dimensionless fall velocity parameter  $\Omega$
- Useful concept for engineering applications (HPZ, MV2)

## 7-A Equilibrium shoreface profile (Bruun / Dean) Fits to long-term averaged profiles

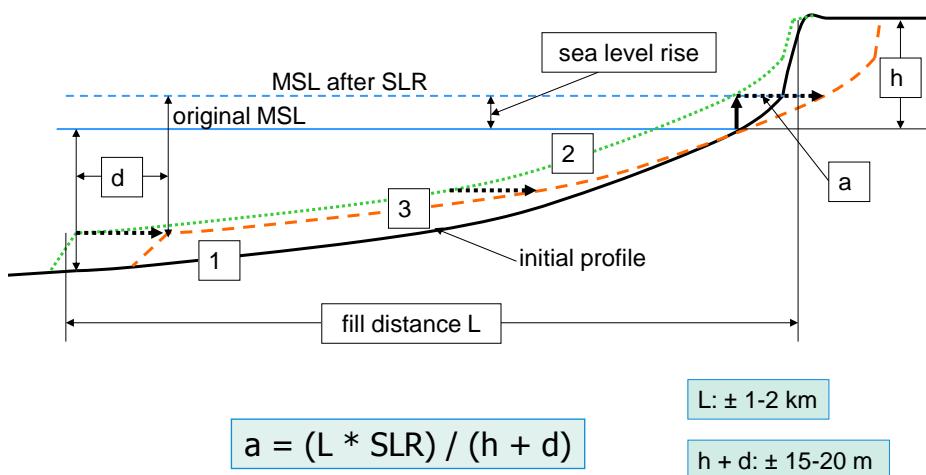
$$h = A(x')^m$$

Values for A

$A = 0.5w_s^{0.44}$	Ameland	Egmond	Noordwijk
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## 7-A Equilibrium shoreface profile Bruun-rule: retreat distance



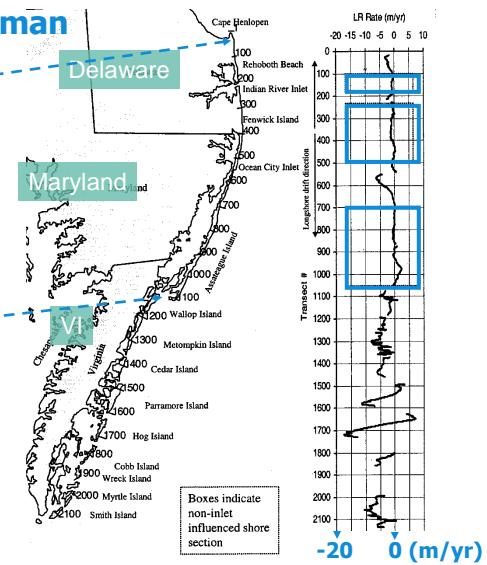
## 7-A Equilibrium shoreface profile

### Data analysis of Leatherman



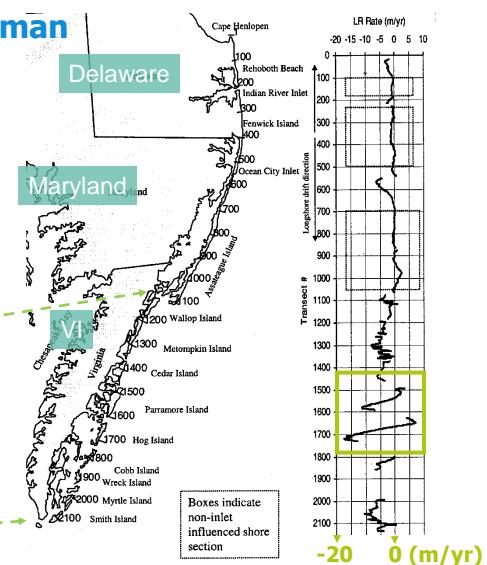
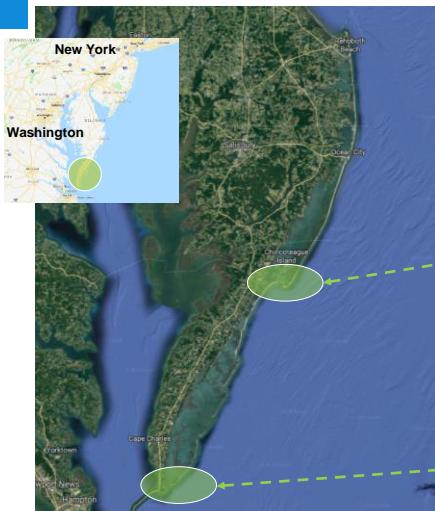
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## 7-A Equilibrium shoreface profile

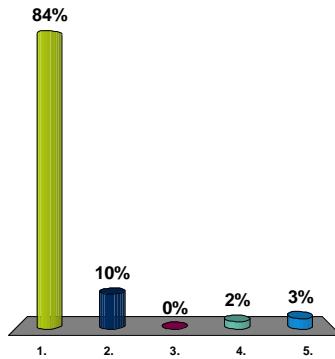
### Data analysis of Leatherman



## 7-A Equilibrium shoreface profile

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- 2. Density currents are able to transport sediment into the basin
- 3. The coastal grain-size is coarser near inlets
- ✓ 4. The ebb-tidal delta is dynamic
- 5. Abstain



## Cross-shore transport and profile development

Chapter 7 of lecture notes

- A. Equilibrium shoreface profile
- B. Beach states
- C. Cyclic profile and bar behaviour
- D. Episodic events: dune erosion
- E. Cross-shore transport mechanisms

## 7-B Beach states

**Storm wave climate:**  
high and short waves and  
highly variable ( $\Omega$  large)

dimensionless fall velocity :

$$\Omega = \frac{H_b}{w_s T}$$

with :  $H_b$  is the wave height at breaking  
 $w_s$  is the sediment fall velocity

- Higher and shorter waves give **wide and flat** sandy coastal zone with **multiple bars**, dunes and a wide beach:
  - High waves break far offshore
  - High waves tend to move sediment offshore
  - "Spilling" breakers
- Variability in wave heights results in:
  - **highly dynamic** coastal profile

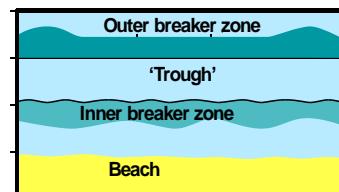
See Chapter 4

## 7-B Beach states

**At end of spectrum:**  
Dissipative beach ( $\Omega > 6$ )

See Chapter 4

### DISSIPATIVE

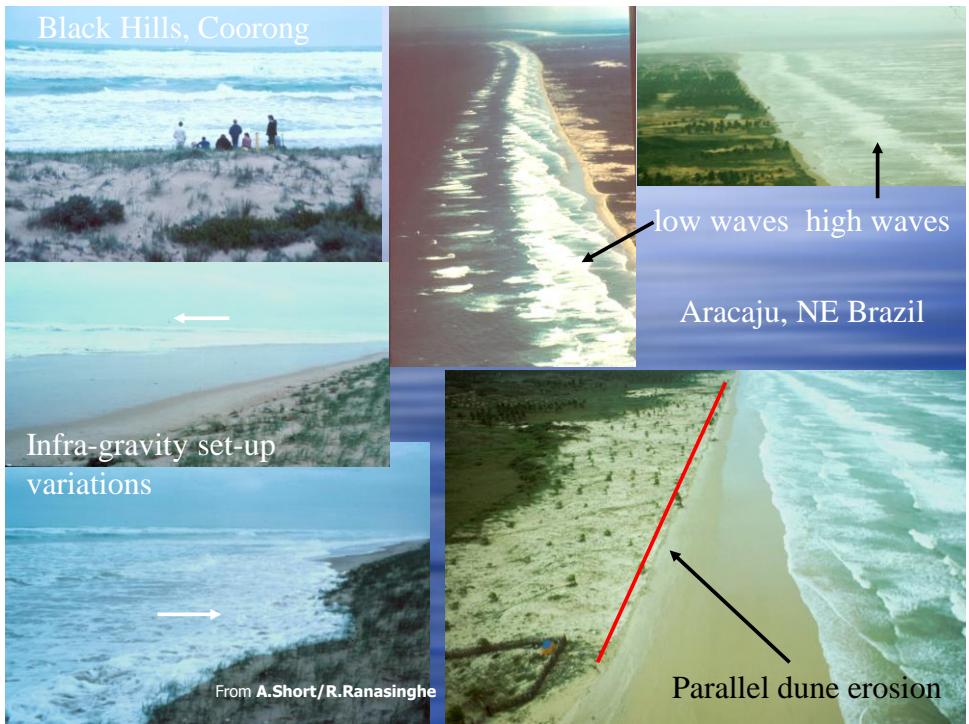


Muriwai, NZ



Aracaju, NE Brazil





## 7-B Beach states

See Chapter 4

Swell (and monsoon) wave climate: low, long waves  
(relatively constant in time):  $\Omega$  small

- Low and long waves break close to the shore and tend to move sediment onshore
  - Narrow sandy profile
  - Steep
  - Berms
- Less dynamic

Jibbon (Sydney)



At end of spectrum:  
reflective beach  $\Omega < 1$

From A.Short/R.Ranasinghe

\* BANEN  
 Nieuws Cultuur & Leven  
 Binnenland  
**Toch pogingen om te zwemmen bij Hoek van Holland**  
 14 juli 2007, 20:38  
 f t e-mail

**H**OEK VAN HOLLAND (ANP) - De strandwacht van Hoek van Holland heeft zaterdag tientallen mensen tegengehouden die een duik in de Noordzee wilden nemen. Dat meldde een woordvoerder van de strandwacht. Voor de zee bij Hoek van Holland geldt een zwemverbod. Door **zandsuppletie** is namelijk vlak voor de kust een gevaarlijke geul met los zand ontstaan. Op het strand is de situatie onveilig vanwege extra golfslag en risico op uitglijden in het water. De provincie Zuid-Holland besloot vrijdag **om een zwemverbod af te kondigen voor een stuk strand van drie kilometer vanaf de pier naar het noorden**. Volgens de strandwacht gaat het echter slechts om een stuk van anderhalve kilometer. Dit gedeelte is gemarkeerd met een rood lint. De reddingsbrigade heeft de rode vlag uithangen. **Matig strandweer**. Sommige mensen dachten ondanks de vele waarschuwingen toch een duik te kunnen nemen. We hebben ongeveer 75 mensen teruggezet", aldus de woordvoerder. Langs de toegangsweg naar Hoek van Holland staat een bord, waarop mensen worden gewaarschuwd niet het water in te gaan. Wie wel in de zee duikt, riskeert een boete van 150 euro. Door het matige strandweer viel het met de toelop naar het strand wel mee volgens de zegsman. „Het was bewolkt en winderig, dus niet lekker om even lekker naar het strand te gaan. Zondag wordt het waarschijnlijk warmer, dan krijgen wij het drukker. Er zullen verschillende strandwachters langs de kust lopen om de

Amerikanen boos om 'racistische' Wilders-tweet van politicus  
 13 maart 2017  
 Clingendaal-expert: 'Politieke schade van rel ligt vooral bij Nederland'  
 11 maart 2017  
 Huishoudchemicaliën veroorzaken mogelijk hersenstoornissen  
 8 maart 2017

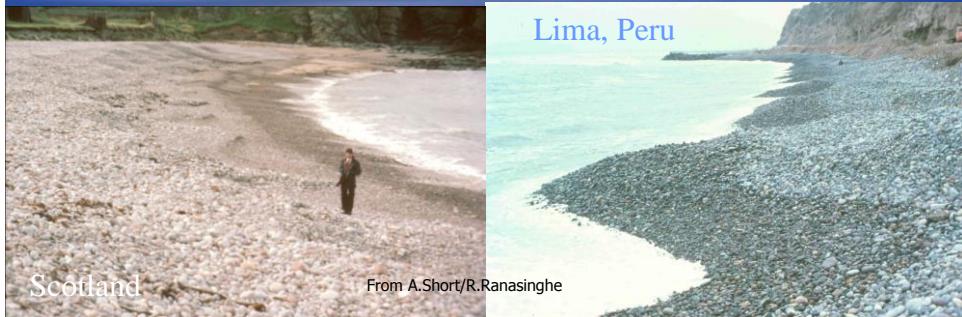
MEEST GELEZEN BINNENLAND

Carriewood

Carl Beach

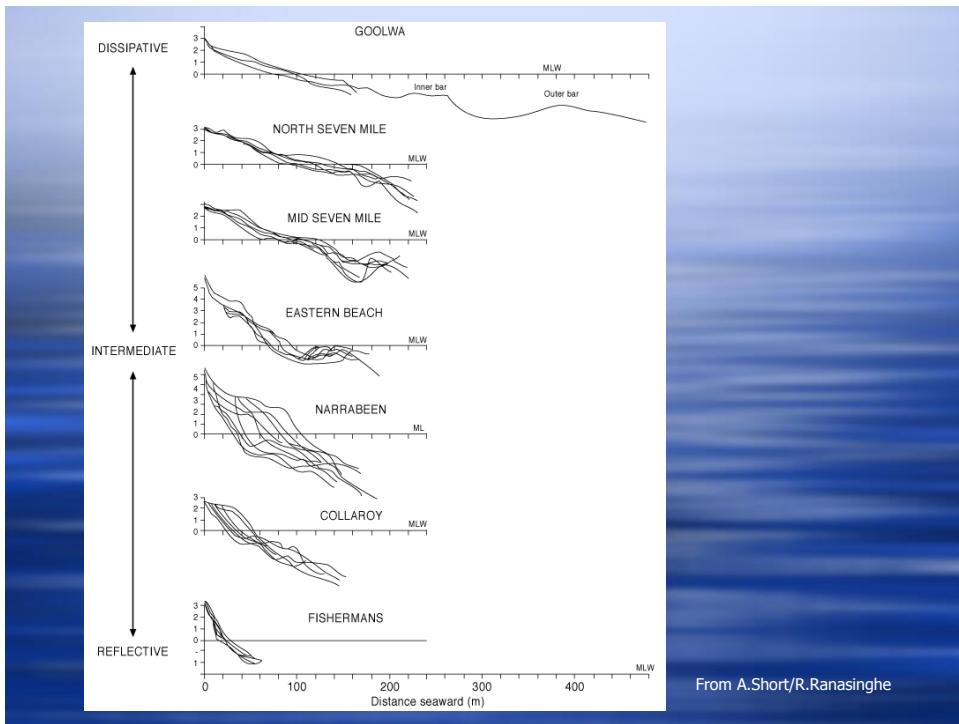


Reflective - higher energy gravel (cobble/shingle) beaches



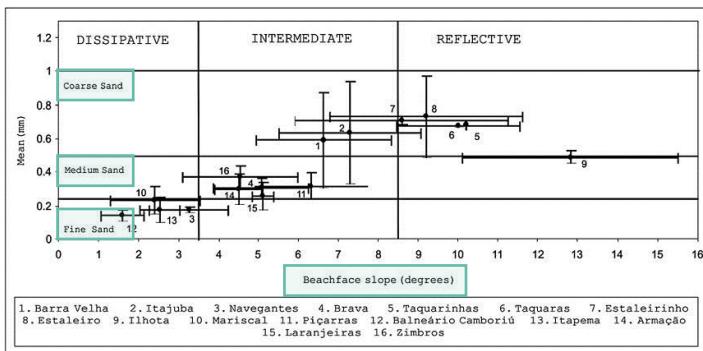
Scotland

From A.Short/R.Ranasinghe



## 7-B Beach states

Beach slope and material versus beach state (Klein et al)

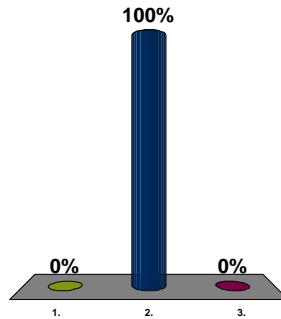


## 7-B Beach states

Dean's derivation - based on uniform energy dissipation per unit volume - leads to an equilibrium profile that to first order is a function of  $D_{50}$  only. The data of Klein et al seems to support this.

**Can we now draw the conclusion that the wave conditions do not play a role?**

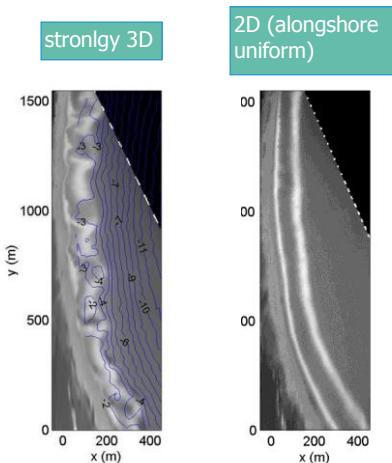
- 1. Yes
- 2. No
- 3. Abstain



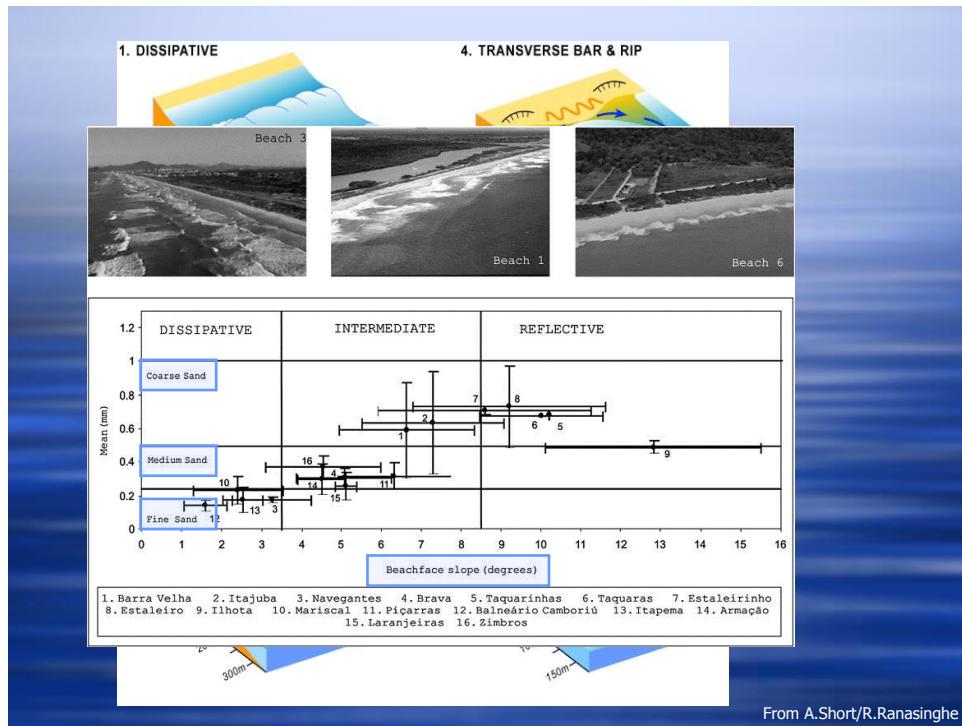
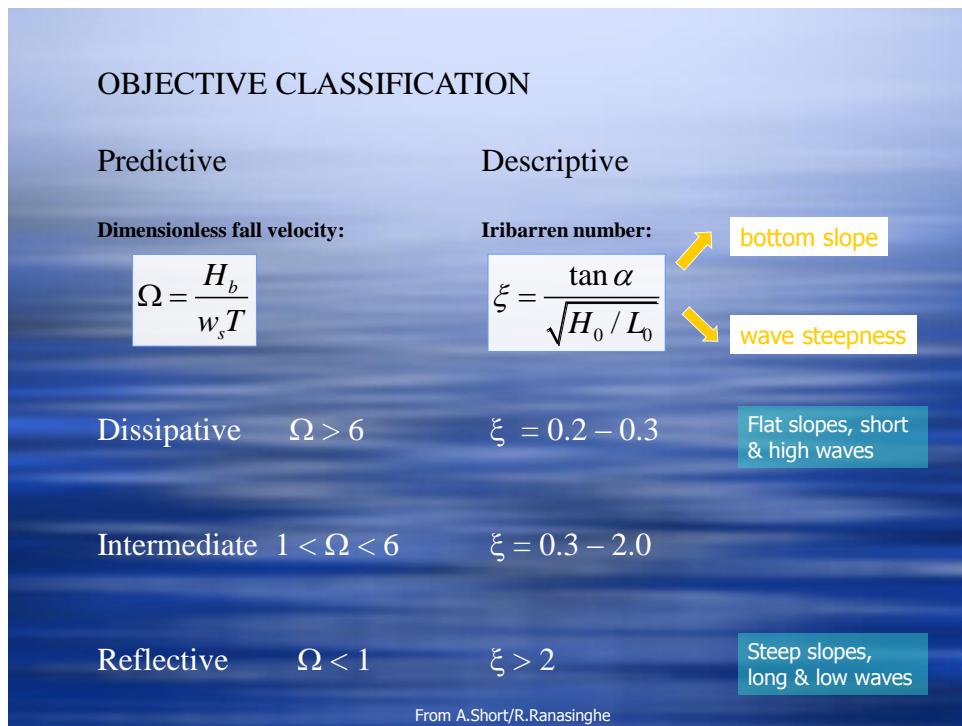
## 7-B Beach states

The beach state is not invariable and depends on amongst others the energy level and (a 2<sup>nd</sup> order effect) on the water level (tide)

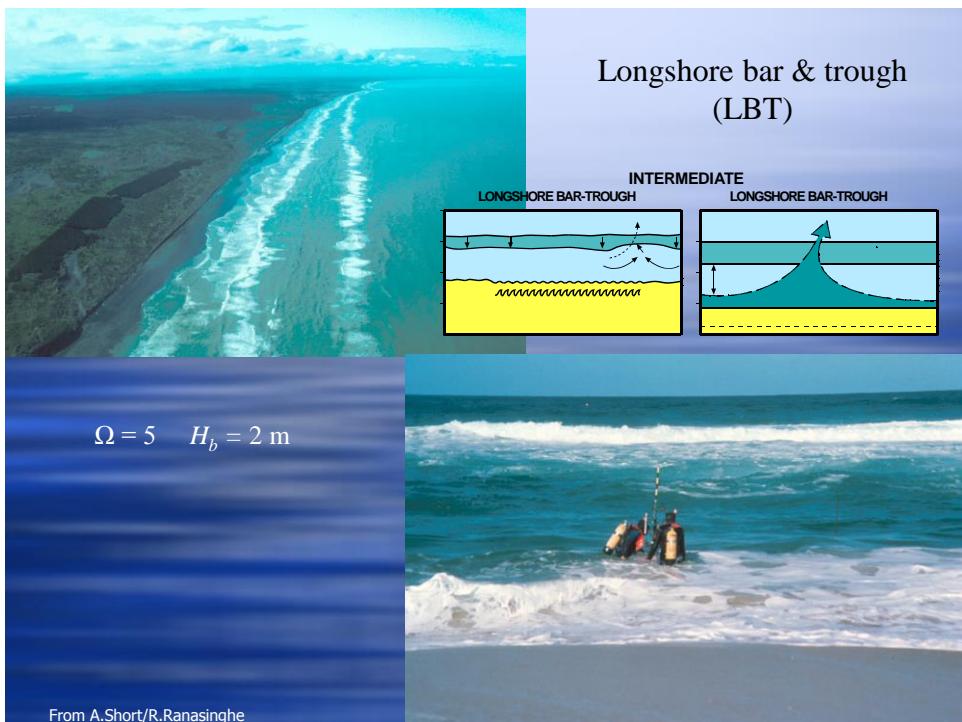
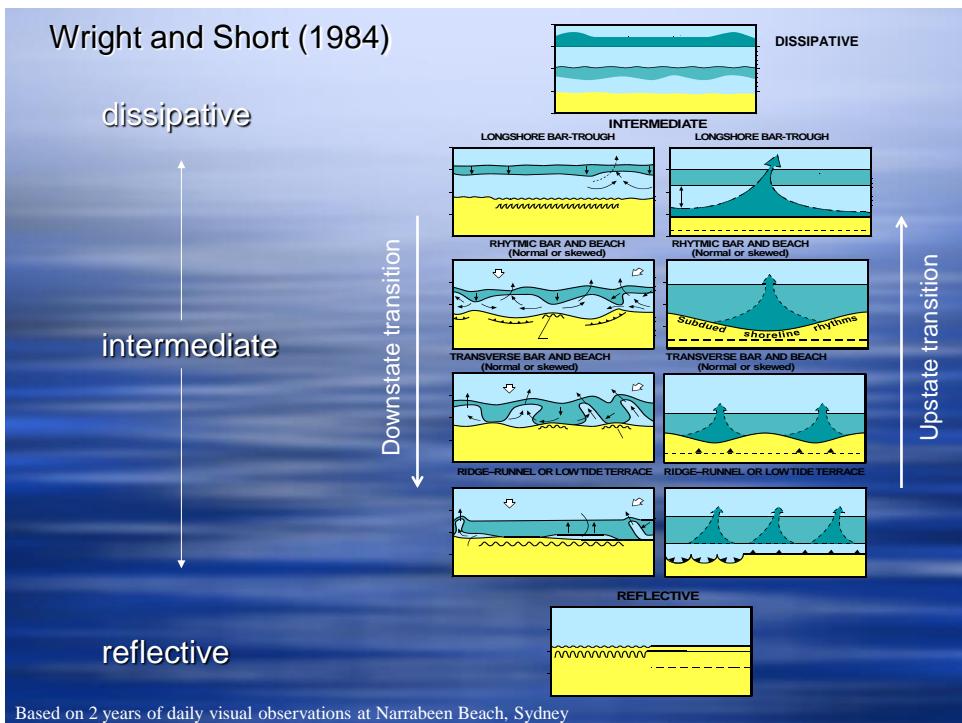
- Beaches can move through a series of beach states
- **Reset event:** 2D alongshore uniform morphology is restored
- Sequence of 6-8 beach states from dissipative to reflective
  - **down-state:** from reset under high energy to gradually less energy

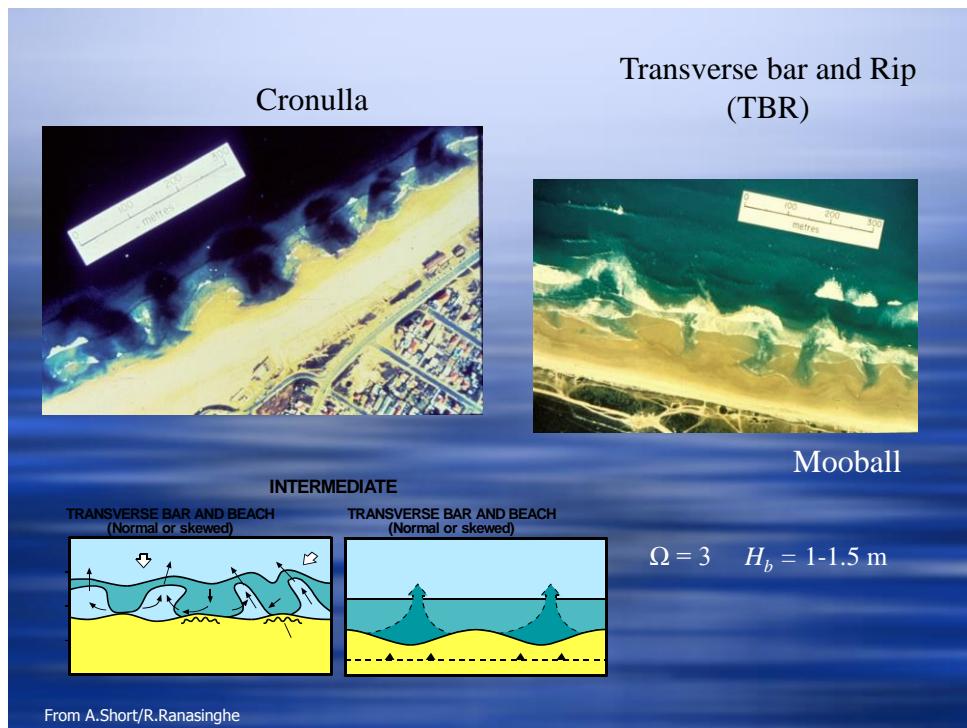
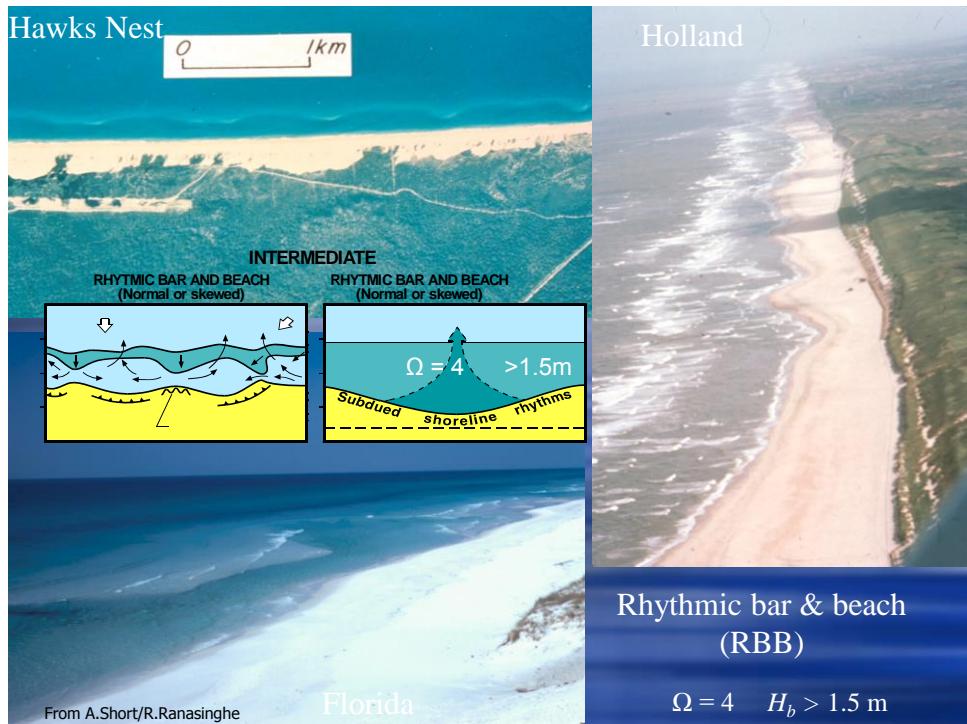


## OBJECTIVE CLASSIFICATION



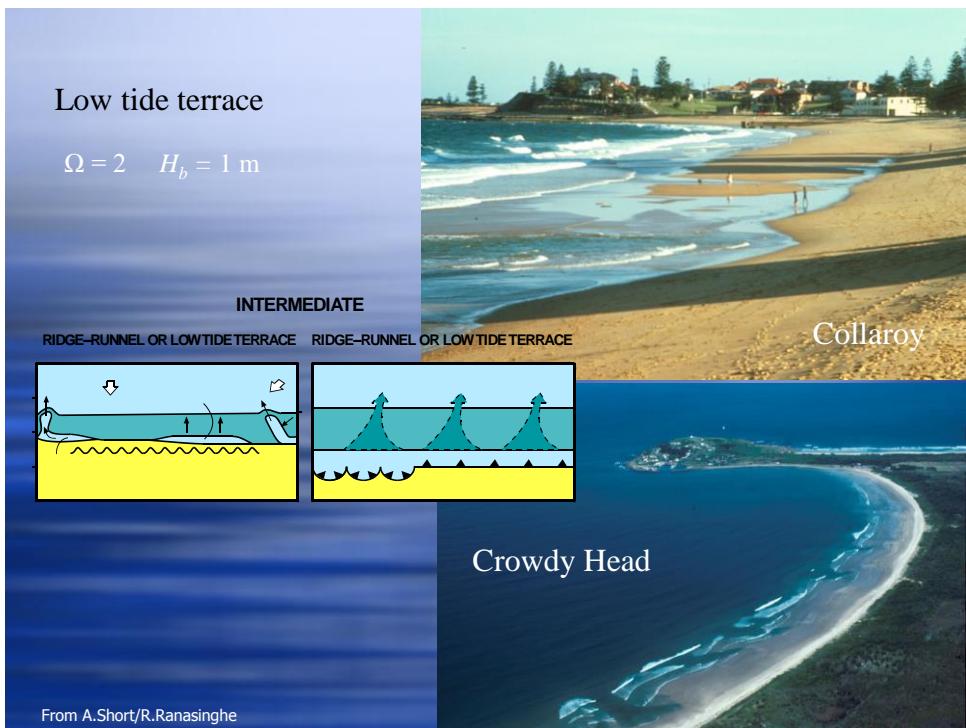
Wright and Short (1984)







From A.Short/R.Ranasinghe



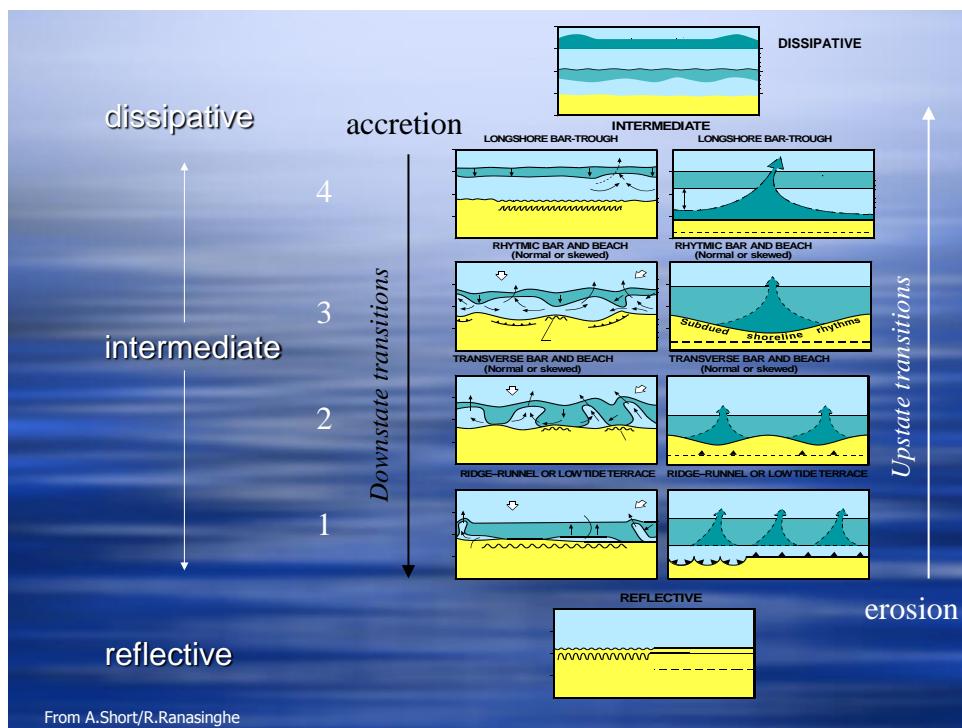
Spatial scales governed by external forcing or internal dynamics (self-organization)?

### Rip current types

*Rip current spacing, type and velocities*

- spacing approximately = surf zone width x 4
- on open swell coast rip spacing from 100 to 500 m, commonly 150 to 250 m apart
- on multi-bar beaches, spacing increases seaward.
- spacing is also a function of beach slope, the lower the slope (hence wider the surf zone) the wider the rip spacing.

From A.Short/R.Ranasinghe



From A.Short/R.Ranasinghe

# Cross-shore transport and profile development

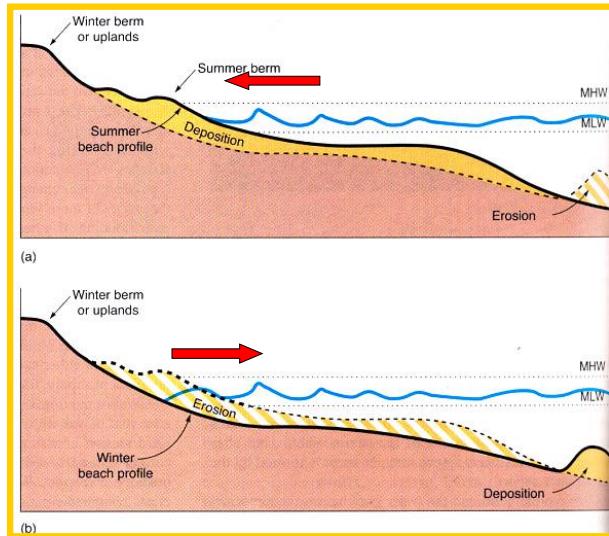
## Chapter 7 of lecture notes

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- C. Cyclic profile and bar behaviour**
- D. Episodic events: dune erosion
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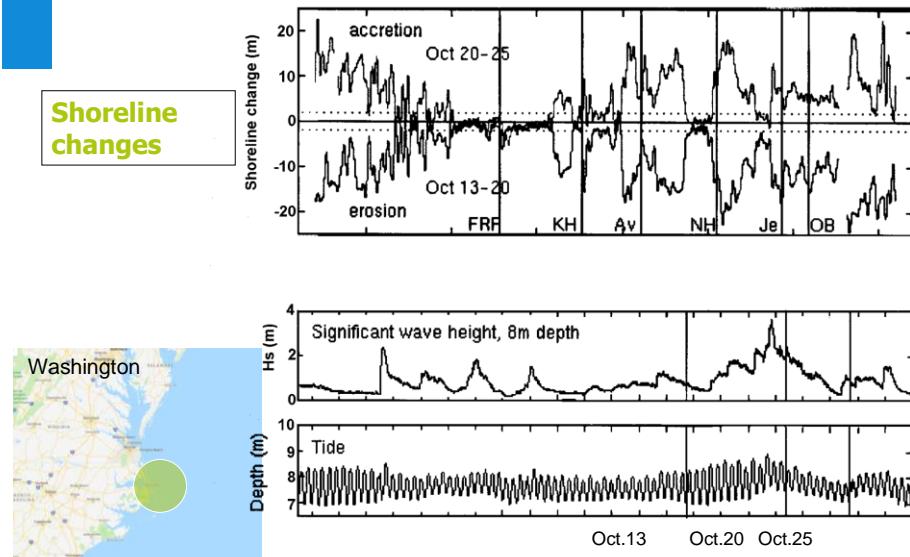
### 7-C Cyclic profile and bar behaviour Storm and seasonal changes

**Summer:** beach rich in sediment  
(resembling reflective beach)

**Winter and/or after storm:** beach poor in sediment  
(resembling dissipative beach)



## 7-C Cyclic bar and profile behaviour



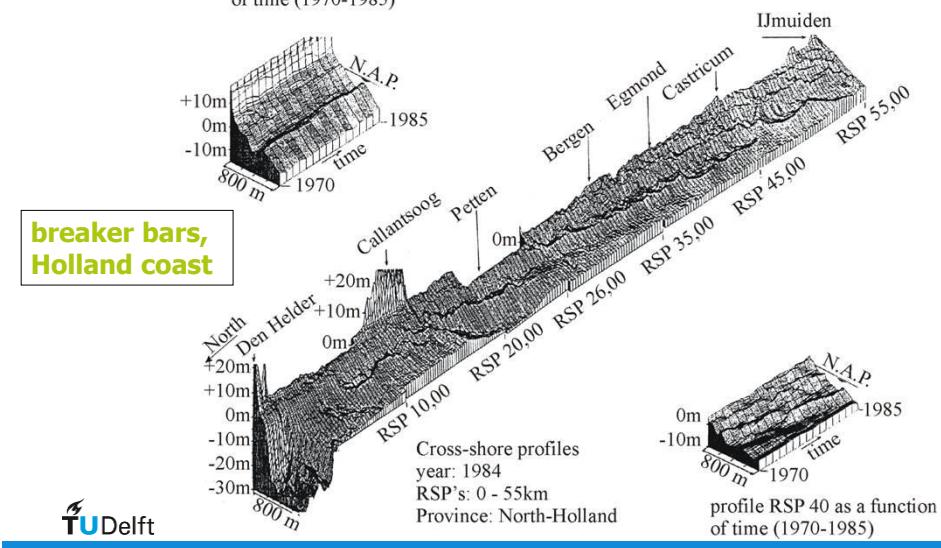
**TU Delft** List and Farris (1999)

7. Cross-shore transport and profile development

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## 7-C Cyclic bar and profile behaviour Bar cycles over years

profile RSP 20 as a function of time (1970-1985)



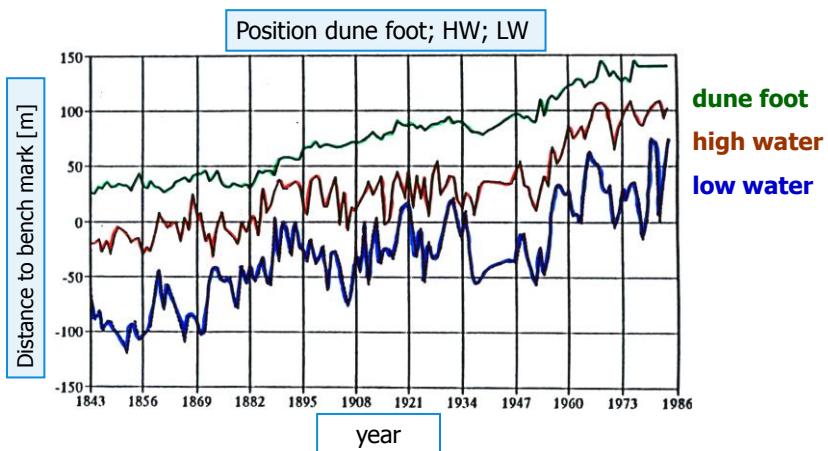
**TU Delft**



TUDelft: jet-ski as measuring tool  
 • GPS  
 • echo sounder

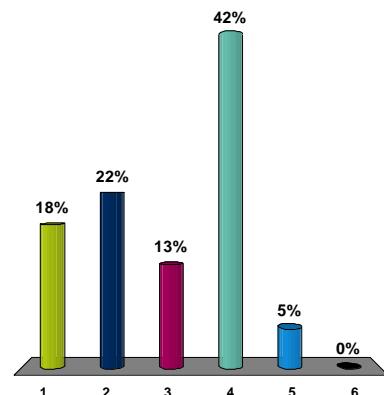


## 7-C Cyclic bar and profile behaviour Seasonal behaviour versus long term trend



The yearly variability of the LW position along the Dutch coast is larger than of the dunefoot position due to:

1. Wind-induced transport
2. Human interference such as vegetation maintenance
3. Rip-currents are more normal under intermediate energy situations
4. Stormy years impact the lower parts of the profile more
5. None of the above
6. Abstain



# 7.

## Cross-shore transport and profile development



# Cross-shore transport and profile development

## Chapter 7 of lecture notes

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### 7-D Episodic events: dune erosion

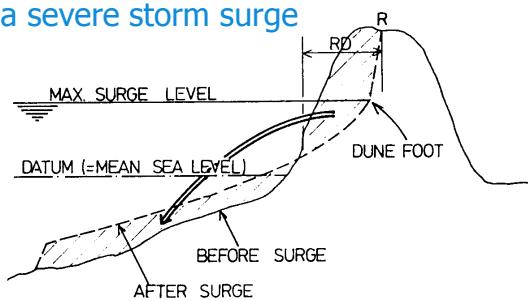
#### Damage (1)



## 7-D Episodic events: dune erosion Damage (2)



## 7-D Episodic events: dune erosion Dune erosion during a severe storm surge



- redistribution over cross-shore profile
- no loss out of control volume
- sooner or later return of sediments (in a stable case)

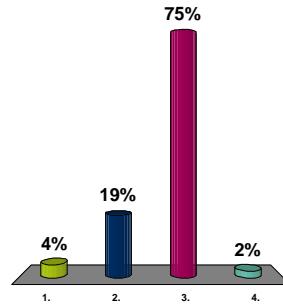


## 7-D Episodic events: dune erosion

### Which of the below statements is true?

- Nr 1. Dune erosion due to a storm surge creates an equilibrium profile relative to the storm surge level  
Nr 2. We can estimate the retreat due to dune erosion from a Bruun type rule

1. Nr 1 only  
2. Nr 2 only  
 3. Both  
4. Abstain

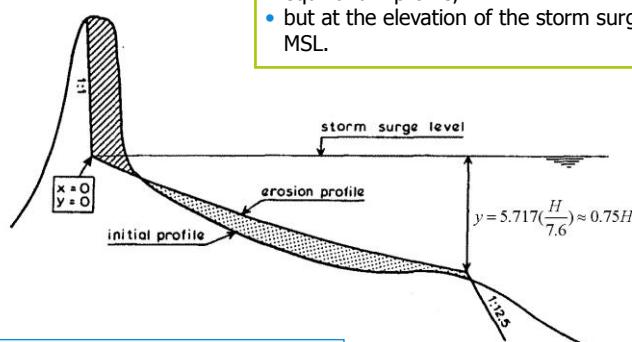


## 7-D Episodic events: dune erosion

### Extent of the sedimentation zone (Vellinga)

first-order magnitude estimate of dune retreat:

- using Bruun's rule;
- assume erosion profile is same as the pre-storm equilibrium profile;
- but at the elevation of the storm surge level instead of the MSL.



$$a = (L * SSL) / (h + d)$$

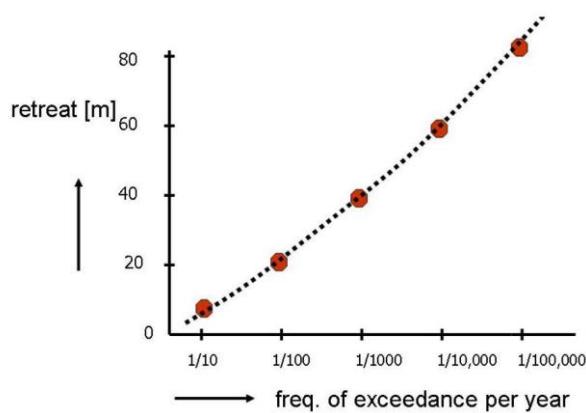
## 7-D Episodic events: dune erosion

### First order estimate of dune erosion

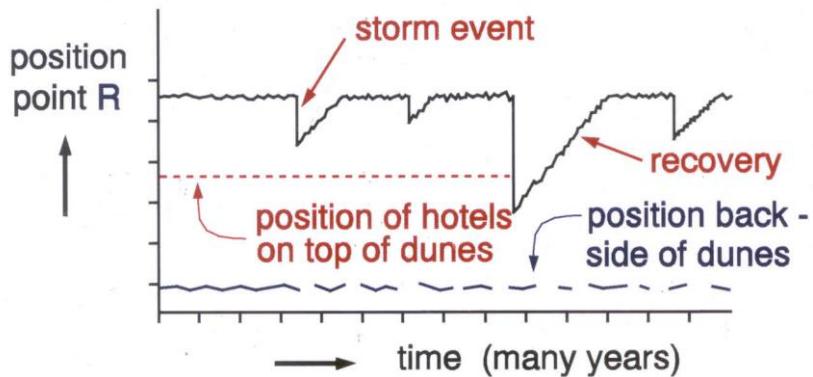
- Design storm conditions:
    - offshore water levels approximately 5 - 6 m above MSL
    - severe wave conditions  $H_s \approx 7 - 9$  m and  $T_p \approx 12 - 18$  s
    - $h+d = 10$  m (dune height above MSL)
    - $L = 200$  m (beach width between top of duneface to MSL)
- ⇒ Dune retreat  $\sim 100 - 120$  m (cf. design rule outcome of 80 to 100 m)

$$\text{retreat} = (L * \text{SSL}) / (h + d)$$

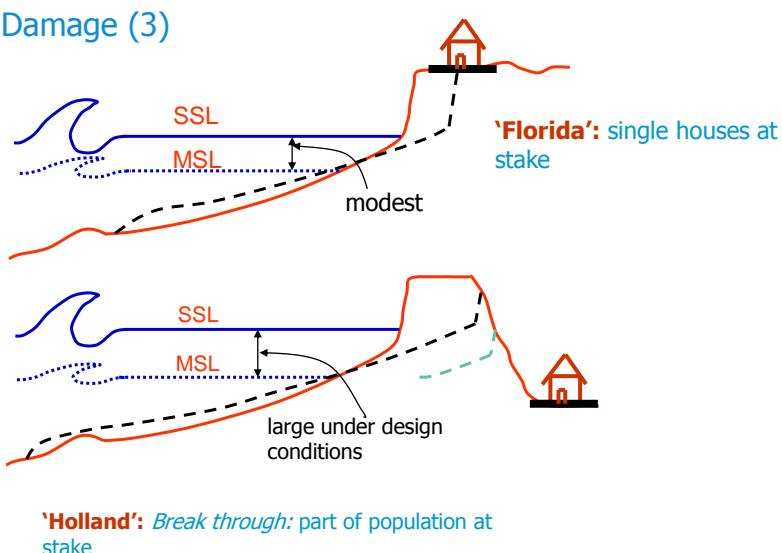
## 7-D Episodic events: dune erosion



## 7-D Episodic events: dune erosion Set-back lines



## 7-D Episodic events: dune erosion Damage (3)



# Cross-shore transport and profile development

## Chapter 7 of lecture notes

- A. Equilibrium shoreface profile
- B. Beach states
- C. Cyclic profile and bar behaviour
- D. Episodic events: dune erosion
- E. Cross-shore transport mechanisms**

### 7-E Cross-shore transport mechanisms

#### Order of magnitude of cross-shore transport

Typical cross-shore sediment transport into the surfzone Holland and Wadden coast	
Integral gross	$O(10^2)$ m <sup>3</sup> /m/year or per extreme event
Integrated net	$O(10^1)$ m <sup>3</sup> /m/year
Effective profile height	10 m
Net natural shoreline changes	$O(1)$ m/year
Net gross shoreline changes	$O(10)$ m/year or per extreme event

## 7-E Energetics approach

### Decomposition of the transport rate

$$\langle S_b \rangle \propto \langle u |u|^2 \rangle$$

Assuming that  $\bar{u} \ll u_{lo} \ll u_{hi}$

$$u = \bar{u} + u_{lo} + u_{hi}$$

time-mean component  
(streaming outside surf zone,  
undertow in surf zone)      low-frequency motion  
at wave-group scale      oscillatory motion at  
short wave scale

$$\langle u |u|^2 \rangle = \underbrace{3\langle \bar{u} |u_{hi}|^2 \rangle}_{1} + \underbrace{\langle u_{hi} |u_{hi}|^2 \rangle}_{2} + \underbrace{3\langle u_{lo} |u_{hi}|^2 \rangle}_{3} + \dots$$

mean flow as  
transporting velocity

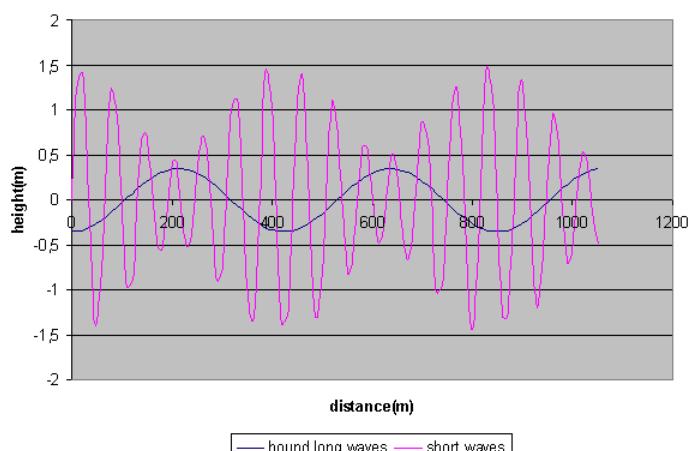
short wave  
skewness

correlation bound long waves  
and short wave envelope

## 7-E Cross-shore transport mechanisms

Intermezzo: bound long waves due to set-down  
variations on the wave group scale

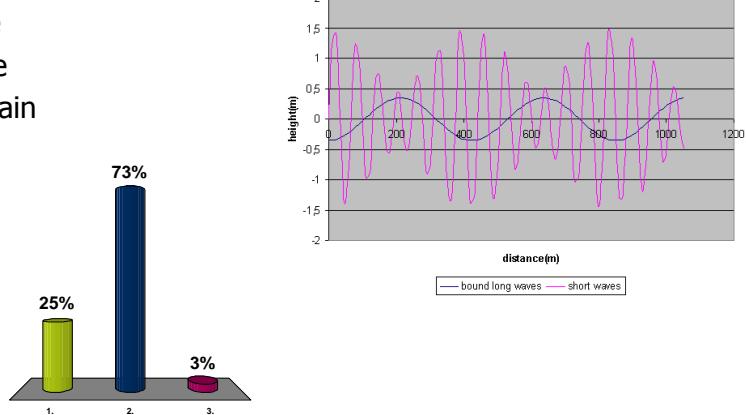
See  
Chapter 5



## 7-E Energetics approach

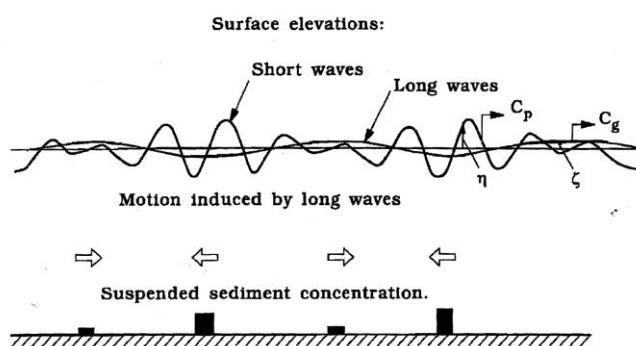
The correlation between high short waves and the bound long wave in the offshore results in a net onshore transport

1. True
2. False
3. Abstain



## 7-E Energetics approach

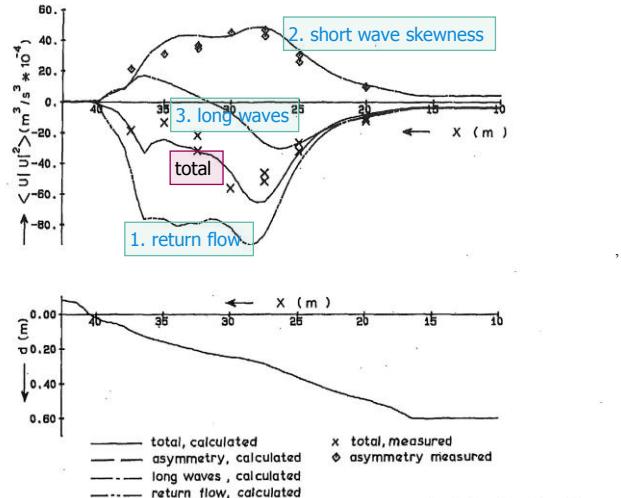
Offshore (suspended) sediment transport under bound long wave



## 7-E Energetics approach

### Measurements and model predictions

$$\langle u |u|^2 \rangle \approx \underbrace{3 \langle \bar{u} |u_{hi}|^2 \rangle}_{1} + \underbrace{\langle u_{hi} |u_{hi}|^2 \rangle}_{2} + \underbrace{3 \langle u_{lo} |u_{hi}|^2 \rangle}_{3}$$



## 7-E Cross-shore transport mechanisms

### On- and offshore transport mechanisms

- **Onshore directed**
  - LH streaming (outside surf zone)
  - Wave asymmetry
  - Free long waves (in surf zone)
- **Offshore directed**
  - Undertow (in surf zone)
  - (Bound) long waves (outside surf zone)
- **Down-hill (on average offshore)**
  - Gravity

## 7-E Cross-shore transport mechanisms

### Analytical solutions for the lower and middle shoreface

See Bowen (1980)!

#### Basic idea

An equilibrium profile (purely in suspended load) exists if:

$$\langle I_s \rangle = 0$$

Bagnold's transport description adapted for non-linear oscillatory motion:

$$I_s = \frac{\varepsilon_s c_f \rho}{w_s} \frac{u^3 |u|}{(1 - \gamma u)} \text{ with } \gamma = \frac{\tan \alpha}{w_s}$$

Eqs. 6.49 / 7.17

## 7-E Cross-shore transport mechanisms

### Velocity consist of symmetrical orbital velocity plus a perturbation

$$u = U_0 + U_1 = u_0 \cos(\omega t) + U_1, \text{ with generally } U_0 \gg U_1$$

#### Considered perturbations:

1. Mean flow (e.g. outside surf zone Longuet-Higgins streaming)

$$U_1 = u_1$$

2. Second harmonic of the primary wave:

$$U_1 = u_2 \cos(2\omega t)$$

## 7-E Cross-shore transport mechanisms

Taylor expansion to first order:

$$\langle I_s \rangle = \frac{\varepsilon_s c_f \rho}{w_s} \left[ \overline{4U_1 U_0^2 |U_0|} + \gamma \overline{U_0^4 |U_0|} + \dots \right]$$

With perturbation is the LH streaming:

$$\langle I_s \rangle = \frac{\varepsilon_s c_f \rho}{w_s} \left[ \frac{16}{3\pi} u_1 u_0^3 + \frac{16}{15\pi} \gamma u_0^5 \right]$$

$$\langle I_s \rangle = 0$$

$$U_1 = -u_0^2/c \quad \text{See Eq. 5.29}$$

$$\gamma = \frac{\tan \alpha}{w_s} = -\frac{5u_1}{u_0^2}$$

$$\tan \alpha \approx \frac{5w_s \omega}{c} = \frac{5w_s \omega}{g \tanh kh}$$



## 7-E Cross-shore transport mechanisms

Equilibrium profile implies that gravitational effects balance influence of streaming everywhere

$$\tan \alpha \approx \frac{5w_s}{c} = \frac{5w_s \omega}{g \tanh kh} \quad \rightarrow$$

$$\tan \alpha = \frac{dh}{dx} \approx 5w_s / \sqrt{gh} \Rightarrow h^3 \approx (7.5w_s x)^2 / g \quad \rightarrow$$

$$h \approx (7.5w_s)^{2/3} / g^{1/3} x^{2/3}$$

(Compare Bruun/Dean profile  
Eq. 7.1)

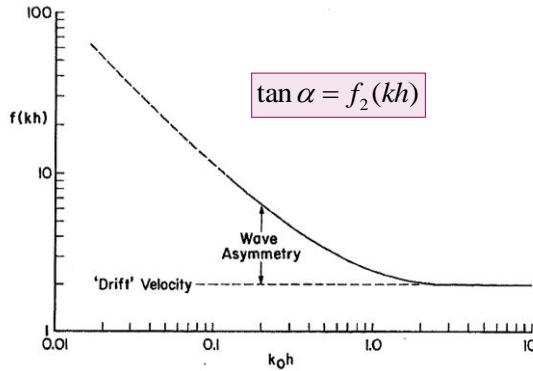
A

$$\text{for } w_s = 0.0256 \text{ m/s} \Rightarrow A = (7.5w_s)^{2/3} / g^{1/3} = 0.16$$

## 7-E Cross-shore transport mechanisms

Inclusion of effect higher harmonic  $u_2 \cos(2\omega t)$

$$\langle I_s \rangle = \frac{\varepsilon_s c_f \rho}{w_s} \frac{16}{15\pi} u_0^3 [5u_1 - 3u_2 + \gamma u_0^2] = 0 \rightarrow \gamma = \frac{\tan \alpha}{w_s} = -\frac{5u_1}{u_0^2} + \frac{3u_2}{u_0^2}$$



## Exam question april 2010

### 2. Transport mechanisms [14]

Consider normally incident waves at the shoreface (including the surf zone). The horizontal velocity  $u(t)$  at a certain distance from the bed consists of a time-averaged component  $\bar{u}$  and a short-wave oscillatory component  $u_{hs}(t)$ . Oscillations on the wave-group scale are neglected, hence  $u(t) = \bar{u} + u_{hs}$ . The onshore direction is taken as positive.

14(90)

It is often assumed that the sediment transport responds instantaneously to the shear stress or velocity to a certain power, e.g.  $S(t) \propto u(t)^3$ . For  $|\bar{u}| \ll |u_{hs}|$  this gives for the time-averaged transport (time-averaging now indicated by brackets):

$$\langle S \rangle \propto 3 \underbrace{\left\langle \bar{u} |u_{hs}|^2 \right\rangle}_{1} + \underbrace{\left\langle u_{hs} |u_{hs}|^2 \right\rangle}_{2} + \dots$$

- [3] Interpret the term  $|u_{hs}|^2$  in terms of sediment transport processes. (70 words).
- [3] Give a physical interpretation of the first term (indicated by 1) and explain the sign of this term at the upper shoreface (say surf zone) and lower shoreface. (70 words).
- [3] Give the simplest expression for  $u_{hs}$  for which the second term (indicated by 2) is non-zero and onshore directed and make a sketch of the signal. Do not forget an explanation. (60 words).
- [5] For a real-world situation, describe and sketch the distribution of the second term over the shoreface from deep water to the water line. Indicate the approximate extent of the surf zone in your sketch. (130 words).

## Exam topic june 2011

### Dynamic equilibrium profile:

- Definition dynamic versus stable equilibrium profile
- Dependency equilibrium profile on fall velocity
- Reasoning behind Bruun rule for sea-level rise
- Overview cross-shore transport mechanisms
  - Mechanism for onshore transport
  - Mechanism for offshore transport