

11 SMRF

- Joint Shear
- Anchorage
- Column Shear
- $\sum M_{nc} \geq \frac{6}{5} \sum M_{nb}$ (Check one floor below roof)

Columns

SEE JOINT SECTION

Beams

Shear

$$V_u = f(M_p r)$$

$V_c = 0$ in plastic hinge regions.

12 Walls

δ_c Wall displacement capacity at top of wall

13.1

13.2

13.3

13.4 Behavioral Observations

Slender vs Squat

Flexural Response

Lateral stability

$$\lambda_c = \frac{c}{b} \frac{l_w}{b} > 40$$

$$b \geq \sqrt{0.025cl_w} \text{ for SBEs.}$$

Dynamic response

13.6 Resistance Factors

Axial/Moment:

$$\phi = 0.65 - 0.9$$

Shear:

$$\phi = 0.75, \text{ For } V_u \text{ amplified by } \Omega_v$$

$$\phi = 0.60 \text{ if } V_n < V(M_n)$$

for squat walls, take $\phi = 0.6$

Coupling beam shear:

$$\phi = 0.85 \text{ diagonally reinforced}$$

$$\phi = 0.75 \text{ otherwise}$$

13.7.1 Preliminary Proportioning for V_b

$$\text{assume } l_{be} = 0.2l_w$$

long. bar fracture:

$$A_s f_y \geq A_{be} f_r \therefore \rho_l \geq 6 \sqrt{f'_c} / f_y$$

13.8 Slender Walls w/ C. Section

Moment

SBE

Required if

$$c \geq \frac{l_w}{900(\delta_u/h_w)} \quad \text{or} \quad \sigma \geq \frac{f'_c}{5}$$

use given graph to determine c .

$$\text{for min. height of } h_{be} = \max\left(l_w, \frac{M_{u,cs}}{4V_{u,cs}}\right)$$

- $A_{sh} \geq 0.09 s b_c \frac{f'_c}{f_y} \geq 0.3 \left(\frac{A_g}{A_{ch}} - 1 \right) s b_c \frac{f'_c}{f_y}$
-

13.9 Walls w/o C. Section

13.10.1 Conventional Squat Walls

13.16 - Openings

Tie region: $A_s = T_u / \phi f_y$

Strut region: $P_u \leq \phi P_o$

$\phi = 0.65$ in wall piers

$\phi = 0.60$ for wall shear

$\phi = 0.75$ otherwise

13.12 Coupled Walls

Coupling beams

$$V_n = 2 A_{vd} f_y \sin \alpha \leq 10 \sqrt{f'_c} A_{cw}$$

Each group of diagonal bars shall consist of a minimum of four bars provided in two or more layers.

13 Gravity Framing

Columns

Confinement

if $P_u \geq 0.35 A_g f'_c$:

- Support all bars with 135° hook.
- $\frac{A_{sh}}{s b_c} \geq 0.3, 0.09, 0.2$

Shear

$$V_u = f(M_{pr}(P_u)), \text{ often } \frac{2M_{pr}}{l_w}$$

Beams

FIGURE FROM NOTES PAGE 200

Slabs

$$V_n = 4\lambda_s \sqrt{f'_c} b_o d$$

DRIFT CAPACITY

14 Diaphragms

Moment

$$M_u = \frac{wl^2}{8} \leq \phi M_n = 0.9(A_s f_y 0.9d)$$

$$T_u = C_u = \frac{M_u}{jd}$$

$$T_u \leq T_n = 0.9 A_s f_y$$

$$C_u \leq \phi \alpha P_{no} = (0.65)(0.8)(A_s f_y + 0.85 A_c f'_c)$$

Collectors

Shear friction

$$\Omega_o V_u \leq \underbrace{\phi}_{0.75} \underbrace{\mu}_{1.4} A_{sf} f_y$$

- Give A_{sf} as in^2/ft

Openings

Comp. Zone

- Confine if