PVD Beam-Column Element

- Review of truss element stiffness matrix using PVD
- 6 DOF flexural element
- Higher order beam-column element

REVIEW: PVD Truss element

$$u(x) = a_1 + a_2 x$$

$$u(x) = a_1 + a_2 x + a_3 x^2$$

$$\frac{du(x)}{dx} = a_2$$

$$u(x) = N_1(x) u_1 + N_2(x) u_2$$

$$\frac{du(x)}{dx} = N'_1(x) u_1 + N'_2(x) u_2$$

$$u(x) = N_1(x) u_1 + N_2(x) u_2 + N_3(x) u_3$$

$$\frac{du(x)}{dx} = N'_1(x) u_1 + N'_2(x) u_2 + N'_3(x) u_3$$

$$\frac{du(x)}{dx} = N'_1(x) u_1 + N'_2(x) u_2 + N'_3(x) u_3$$

$$N_1 = \left(1 - \frac{x}{L}\right) \qquad N_2 = \left(\frac{x}{L}\right)$$

$$N'_1 = -\frac{1}{L} \qquad N'_2 = \frac{1}{L}$$

The solution interpolated by the shape fine's must satisfy compatibility ideally close to satisfy equilibrium.

PND
$$\rightarrow \delta u_i \, F_{ij} = \delta u_i \int_L \, B_i \, (EA) \, B_j \, dx \, u_j$$
 Hence j $F_{ij} = \int_L \, B_i \, (EA) \, B_j \, dx \, u_j$

Flexural member calculations

$$\nu = v_2 \rightarrow a_1 = v_2$$

$$\nu' = \theta_3 \rightarrow a_2 = \theta_3$$

$$\nu(x) = N_2 v_2 + N_3 \theta_3 + N_5 v_5 + N_6 \theta_6$$

$$N_{2}(x) = 1 - 3\left(\frac{x}{L}\right)^{2} + 2\left(\frac{x}{L}\right)^{3} \qquad N_{3}(x) = x\left(1 - \frac{x}{L}\right)^{2}$$

$$N_{5}(x) = 3\left(\frac{x}{L}\right)^{2} - 2\left(\frac{x}{L}\right)^{3} \qquad N_{6}(x) = x\left(\left(\frac{x}{L}\right)^{2} - \frac{x}{L}\right)$$

$$\frac{\mathsf{x=L}}{\nu=v_5}$$
 $\nu'=\theta_6$

Solve for as & a, $\nu'=v_5$

$$f(x) = x \left(1 - \frac{x}{L}\right)^{2}$$

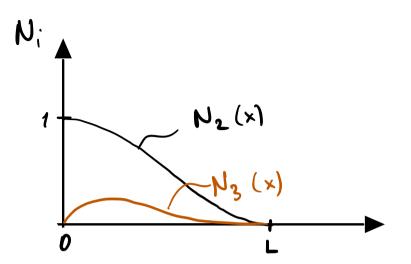
$$f(x) = x \left(\left(\frac{x}{L}\right)^{2} - \frac{x}{L}\right)$$
CUBIC HERMITIAN SHAPE FUNCTIONS

COMPATIBILITY OF SHAPE FUNCTIONS

$$\nu(x) = N_2 v_2 + N_3 \theta_3 + N_5 v_5 + N_6 \theta_6$$

$$N_2(x) = 1 - 3\left(\frac{x}{L}\right)^2 + 2\left(\frac{x}{L}\right)^3$$
 $N_3(x) = x\left(1 - \frac{x}{L}\right)^2$

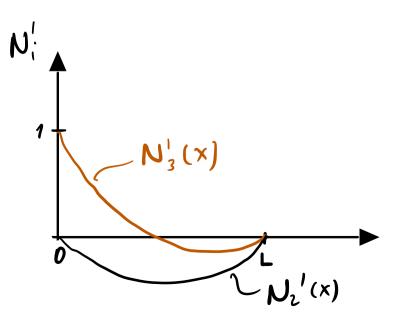
$$N_5(x) = 3\left(\frac{x}{L}\right)^2 - 2\left(\frac{x}{L}\right)^3 \qquad N_6(x) = x\left(\left(\frac{x}{L}\right)^2 - \frac{x}{L}\right)$$



$$\theta(x) = \nu'(x) = N_2'v_2 + N_3'\theta_3 + N_5'v_5 + N_6'\theta_6$$

$$N_2'(x) = -\frac{6x}{L^2} + \frac{6x^2}{L^3}$$
 $N_3'(x) = 1 - \frac{4x}{L} + \frac{3x^2}{L^2}$

$$N_5'(x) = \frac{6x}{L^2} - \frac{6x^2}{L^3}$$
 $N_6'(x) = \frac{3x^2}{L^2} - \frac{x^2}{L}$



EVALUATE EQUILIBRIUM: $M(x) = ET(x) \phi(x)$

$$\phi(x) \approx \theta'(x) = \nu''(x) = N_2'' v_2 + N_3'' \theta_3 + N_5'' v_5 + N_6'' \theta_6$$

$$N_{2}^{"} = B_{2} = -\frac{6}{L^{2}} + \frac{12 \cdot x}{L^{3}}$$

$$N_3'' = B_3 = -\frac{4}{L} + \frac{6x}{L^2}$$

$$N_5'' = B_5 = \frac{6}{L^2} - \frac{12 \times 12}{L^3}$$

$$N_b'' = B_b = \frac{6x}{L^2} - \frac{2}{L}$$

EVALUATE EQUILIBRIUM: cont.

$$\phi(x) \approx \theta'(x) = \nu''(x) = N_2''v_2 + N_3''\theta_3 + N_5''v_5 + N_6''\theta_6$$

FOR EXAMPLE:
$$U(x) = N_2 \cdot U_2$$

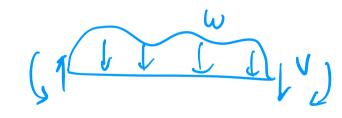
$$N = 1 - 3\left(\frac{x}{L}\right)^2 + 2\left(\frac{x}{L}\right)^3$$

$$M = EI\phi = EI\frac{d^2v}{dx^2}$$
 \longrightarrow $N'' = -\frac{6}{L^2} + \frac{12 \cdot x}{L^3}$

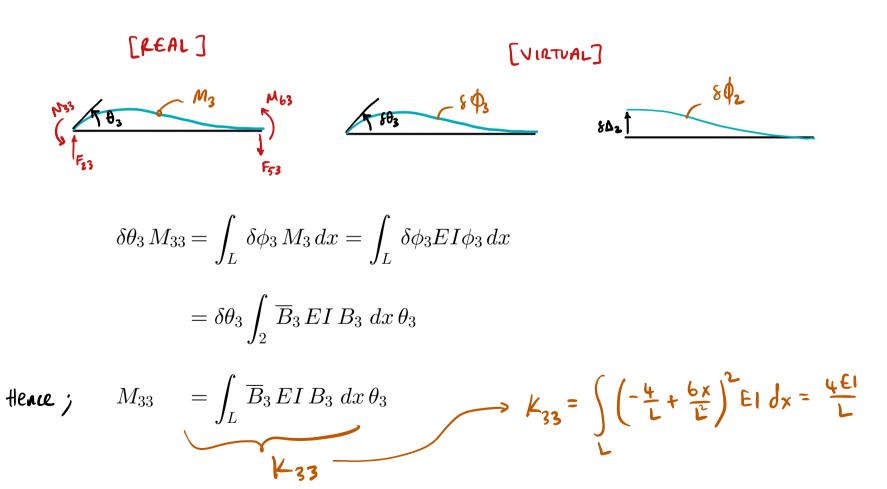
$$V = \frac{dM}{dx} = EI\frac{d^3v}{dx^3} \longrightarrow N'' = \frac{12}{13}$$

$$w = \frac{dV}{dx} = EI\frac{d^4v}{dx^4} \longrightarrow N'' = 0$$

$$w = \frac{dV}{dx} = EI\frac{d^4v}{dx^4} \longrightarrow N^{\prime\prime} = 0$$



Calculate K_33:



$$\begin{cases} F_1 \\ F_2 \\ M_3 \\ F_4 \\ F_5 \\ M_6 \end{cases} = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & \frac{12EI}{L^3} & \frac{6EI}{L^2} & 0 & -\frac{12EI}{L^3} & \frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{4EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & -\frac{12EI}{L^3} & -\frac{6EI}{L^2} & 0 & \frac{12EI}{L^3} & \frac{-6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{2EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} \begin{pmatrix} \Delta_1 \\ \Delta_2 \\ \theta_3 \\ \Delta_4 \\ \Delta_5 \\ \theta_6 \end{pmatrix}$$

THE ELEMENT STIFFNESS MATRIX [Ke']

• Let's try a different set of shape functions:

Check

$$\Theta(x) = x \left(1 - \left(\frac{x}{L}\right)^{2} \right)^{2} \Theta_{3}$$

$$\Theta'(x) = \left(1 - 6 x_{L^{2}}^{2} + \frac{5 x^{4}}{L^{4}} \right) \Theta_{3}$$

$$\Theta''(x) = \left(-\frac{12 x}{L^{2}} + 20 \frac{x^{2}}{L^{4}} \right) \Theta_{3}$$

$$\Theta'''(x) = \left(-\frac{12}{L^{2}} + 60 \frac{x^{2}}{L^{4}} \right) \Theta_{3}$$

$$\Theta^{11}(x) = \left(-\frac{12}{L^{2}} + 60 \frac{x^{2}}{L^{4}} \right) \Theta_{3}$$

$$\Theta^{11}(x) = \frac{120 x}{L^{4}} \Theta_{3}$$

Check Equil.:
$$M \rightarrow Q^{11}$$
, $N \rightarrow Q^{11}$, $W \rightarrow Q^{11}$, $W \rightarrow Q^{11}$

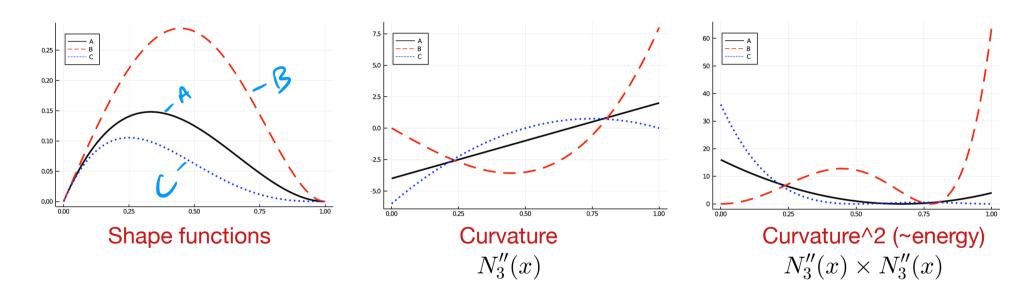
$$= \overline{B_3} : \text{Higher order } B_3 : \text{linear} \rightarrow K_{33} = \frac{4}{3}$$

BC's : 0(0) = 0(1) = 0 & (0) = 03 0 (1) = 0

· $K_{33} = \int \overline{B}_3 \ EI B_3 \ d_{\times} \longrightarrow \overline{B}_3 : \text{ Higher order } B_3 : \text{ linear } \longrightarrow K_{33} = \overline{L}$

COMPARE THREE DIFFERENT SHAPE FUNCTIONS

$$A \to N_3 = x \left(1 - \left(\frac{x}{L}\right)\right)^2$$
 $C \to N_3 = x \left(1 - \left(\frac{x}{2}\right)\right)^3$ $B \to N_3 = x \left(1 - \left(\frac{x}{L}\right)^2\right)^2$



OBSERVATION:

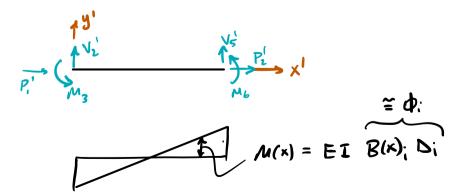
Using non-exact shape functions results in higher computed internal strain energy, leading to a stiffer predicted structural response and underestimation of displacements.

REMARKS:

1) The global solution for $\{b \in \}$ $\{F^e'\}$

$$\{k_{e_{i}}\}=\{k_{e_{i}}\}\{k_{e_{i$$

2) Calculation of internal forces: $P'_{(x)}$ $k M'_{(x)}$



TAPERED BEAM EXAMPLE:

$$h(x) = h_0 \left(1 + \frac{x}{L} \right)$$

$$I(x) = \frac{bh_0^3}{12} \left(1 + \frac{x}{L} \right)^3$$

$$= I_0 \left(1 + \frac{x}{L} \right)^3$$

$$\frac{1}{|\mathbf{k}|^{3}} \cdot \theta(\mathbf{x}) = N_3 \theta_3 = \mathbf{x} \left(1 - \frac{\mathbf{x}}{\mathbf{x}} \right)^2 \theta_3$$

$$\phi(x) = \mathcal{B}_3 \theta_3 = \left(6 \times_{L^2} - \frac{4}{L}\right) \theta_3$$

Equilibrium ?
$$\rightarrow M = EI \phi = EI_o \left(1 + \frac{x}{L}\right)^3 \left(\frac{6x}{L^2} - \frac{4}{L}\right) \theta_3$$

$$k_{33} = \int \overline{B}_3 \, \epsilon I(x) \, B_3 \, dx = \frac{9 \, \epsilon I}{L}$$
 vs. exact = 6.9 \(\exists I \)

HIGHER ORDER ELEMENTS

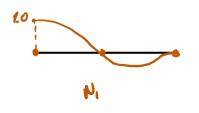
ER ORDER ELEMENTS
$$\begin{cases}
F_1 \\
M_2 \\
F_3 \\
M_4 \\
F_5
\end{cases} = \begin{bmatrix}
k^e^1 \\
M_4 \\
F_5
\end{bmatrix}$$

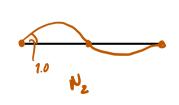
$$\begin{cases}
\theta_1 \\
\theta_2 \\
\theta_3 \\
\theta_4 \\
\theta_5
\end{cases}$$

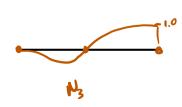
$$(9(x) = \alpha_1 + \alpha_2 \times + \alpha_3 \times^2 + \alpha_4 \times^3 + \alpha_5 \times^4$$

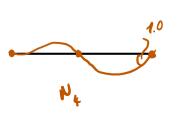
$$\phi(x) = \frac{d^2 \omega}{dx^2} = 2\alpha_3 + 6\alpha_4 \times + 12\alpha_5 \times^2$$

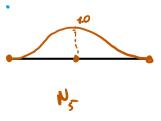
B(s:
$$\Theta(0) = \theta_1$$
 $\Theta(\frac{1}{2}) = \theta_2$ $\Theta(L) = \theta_3$ $\Theta'(L) = \theta_4$



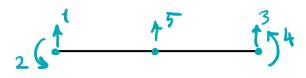


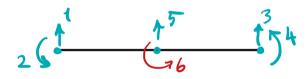


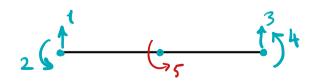




HIGHER ORDER ELEMENTS



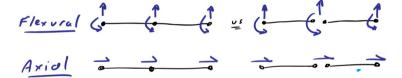




More elements vs. H.O. elements

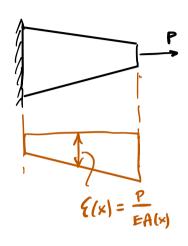
- More elements increases size of [K]
- H.O. elements increases HBW

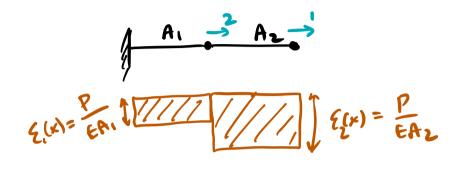
Which is more efficient?



For example: AXIAL DEFORMATION OF A TAPERED MEMBER

- Use two linear elements:





- Use one H.O. element:

