

AALBORG UNIVERSITY

**Identification of Randers Water Distribution
Network for Optimal Control**

Electronic & IT:
Control & Automation

Group:
CA9-938

STUDENT REPORT

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Fredrik Bajers Vej 7
DK-9220 Aalborg East, Denmark
<http://www.es.aau.dk>

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STUDENT REPORT

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Participants:

Krisztian Mark Balla

Supervisors:

Tom Nørgaard Jensen
Jan Dimon Bendsten
Carsten Skovmose Kallesøe

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Krisztian Mark Balla
kballa16@student.aau.dk

Nomenclature

Acronyms

EPA	Environmental Protection Agency
FCV	Flow Control Valve
GT	Graph Theory
MPC	Model Predictive Control
PRV	Pressure Regulating Valve
WSS	Water Supply System

Symbols

Symbol	Description	Unit
a	Description	[.]
b	Description	[.]

Graph theory

Symbol	Description	Unit
a	Description	[.]
b	Description	[.]

Glossary of mathematical notation

Description of the mathematical notation and terminology used in the report.

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Due to the fast-paced technological development all over the world, the demand for industrial growth and energy resources has seen a rapid increase. Along with the industrial growth, the sudden rise in population has made the world realize that this shortage of energy sources is an actual and universally anticipated problem [1]. In order to cope with such shortage issues and to make the rapid development possible and less expensive, the world is moving towards more efficient use of resources and optimization of infrastructure. Therefore, technological development is also moving the focus on green energy, resulting in more and more renewable energy sources added to the grid [2].

Water Supply Systems(WSSs) are among the sectors which make the industrial growth possible. On top of this, WSSs are one of the most vital infrastructures of modern societies in the world. In Denmark typically, such networks are operating by making pumps transport water from reservoirs through the pipe network, to the end-users. In most cases, elevated reservoirs are exploited in these WSSs, such that they can even out the demand differences for the consumers. Although elevated reservoirs are usually an integrated part of these systems, providing drinking water is a highly energy-intensive activity. For instance, in the United States alone, the drinking water and waste water systems are typically the largest energy consumers, accounting for 25 to 40 percent of a municipality's total public expenditure. [3].

Since fresh water is limited, and due to the presence of global changes such as climate change and urbanization, new trends are emerging in the water supply sector. In the past few decades, several research and case study showed that WSSs and other energy distribution networks need to be improved due to the leakages in the system, high cost of maintenance and due to high energy consumption. Companies also realized that by using proper pressure management in their networks, the effect of leakages can be reduced, thereby huge amount of fresh water can be saved [4].

In Denmark recently, the larger water suppliers have been focusing on making the water supply sector more effective through introducing a benchmarking system focusing on the environment, the security of supply and the efficiency based on user demands. Since 1980, these efficiency activities has been an important issue [5]. It has been proved that by utilizing advanced, energy- or cost-optimizing control schemes and utilizing renewable energy sources, such as elevated reservoirs, the life of the existing infrastructure can be extended and money or energy can be saved [1]. Therefore there is a growing demand in industry for developing methods, leading towards more efficient WSSs.

The presented project is executed in collaboration with the company, Verdo A/S. It is in the interest of Verdo A/S to utilize an advanced model-based optimal control scheme on the WSS with several storages in Randers, Denmark. For a large municipality such as Randers, the water distribution network is complex and consists of thousands of elements. Since the control algorithm itself is complex and model-based, the computational effort is also high. Furthermore, the offline optimisation of a large-scale WSS means that any changes to the network may require significant changes in the optimisation method, which leads to high costs of the system maintenance [6]. Therefore typically a model reduction is required in such networks to make the online execution of the control algorithm possible.

The long-term goal of this project is to find a solution for implementing Model Predictive Control(MPC) on the Randers WSS. However, before the implementation of any control scheme would be possible, a proper and identified model is required. Therefore, as the

first part of the project, the following problem statement can be formulated:

How can the WSS in Randers be simplified and identified, with storages included in the system, such that the reduced model preserves the original nonlinear behaviour and remains suitable for a plug-and-play commissionable Model Predictive Control scheme.

Part I

System Analysis

Description of Water Supply Systems 2

This chapter gives a general overview of hydraulic systems and an introduction to the WSS in Randers. The basic topology and structures of water supply networks are explained. Furthermore, the basic components of hydraulic systems are discussed and the unit called head, as an alternative measure of pressure, is introduced.

2.1 Hydraulic system overview

WSSs are designed to deliver water to consumers in terms of sufficient pressure and appropriate chemical composition. Distribution systems as such are typically transport water from one geographical place to another. In practice, there are different methods existing to achieve this water transport. One example is to make use of natural advantages such as the water stored in mountains, and thereby use the potential energy of the water to provide pressure in the network. Examples for this are countries like Norway and Sweden where the advantages of the landscape are being exploited. However, in this project the source of the water is considered as groundwater, considering that in Denmark all reservoirs in the network are tapping water from the ground. After tapping the water, it goes through a cleaning process at the waterworks and afterwards the pure water is pumped into the network [7]. In WSSs, pumps and valves are the elements that enable the delivery of water to the consumers or to elevated reservoirs, storing water for later use. Such a network is illustrated in the figure below:



Figure 2.1: Illustration of a WSS.

The delivered water needs to fulfil a certain pressure criteria in order to reach consumers at higher levels. For example, in some cases the pressure has to be high enough to make it to the fourth floor of a building and still provide appropriate pressure in the water taps. Generally, in such cases booster pumps are placed in the area, helping to supply the pressure. Too large pressure values, however increase water losses due to pipe waste [8].

Another criteria is that the flow through particular pipes need to stay within acceptable limits. A low flow rate can lead to water quality problems due to the undesirable microorganisms in the water and due to the metal and salt accumulation on the wall of the pipes [8].

As can be seen in *Figure 2.6*, typically WSSs consist of pipe, valve, reservoir, elevated reservoir(tank) and pump components. The common property of them is that they are all

two-terminal components, therefore they can be characterized by the dynamic relationship between the pressure drop across their two corresponding endpoints and the flow through them [9].

2.1.1 Pipe networks

Pipes are the most common components of WSSs since they are used for carrying pressurized water. They serve as a connection between components. Normally, the pipe network can be split into different sub-parts, taking into account the physical characteristics and the attributes of the pipes. Therefore, water supply networks can consist of transmission mains, arterial mains, distribution mains and service lines as shown in the example below:

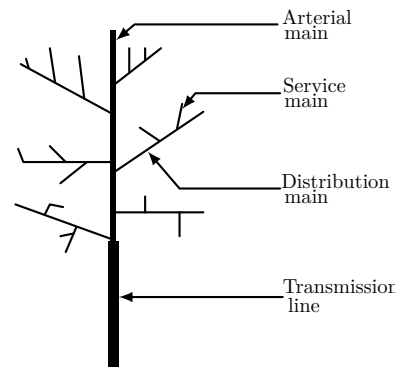


Figure 2.2: Illustration of pipe mains. Tree configuration.

Transmission mains deliver large amounts of water over long distances. Arterial and distribution mains provide intermediate steps towards delivering water to the end-users. Service lines transmit the water from the distribution mains straight to the end-users [10].

The transmission and distribution network can have a topology that is called a loop or a tree structure. *Figure 2.2* shows an example for a tree configuration. This type of configuration is most frequently used in rural areas [11]. Typically the network has only one path for the water to reach the end-users. Common problems with this configuration is that on the outer parts of the system lower pressures can be experienced due to the pressure losses from long flow paths. The flow dynamics within this kind of systems therefore consist of large flows closer to the source that turn into smaller flows on the outer parts of the system. Main disadvantage of a purely tree structure system is that due to maintenance or momentary breakdowns, the system suffers disruption of service [11].

Loop networks have a configuration as shown in *Figure 2.3*.

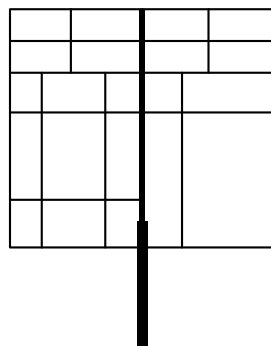


Figure 2.3: Loop configuration.

Loop networks are usually composed of smaller loops which made up of smaller distribution mains, and larger loops that are connected to arterial or transmission mains. Elevated

reservoirs are typically placed in the centre of the system due to pressure losses resulting from flows through the loop network [12]. In the presence of the larger loops, they may be used to feed an internal distribution grid or a distribution grid attached to the outer part of the loop. Loop configurations are generally associated with larger suburban and city distribution systems such as the WSS in Randers[12].

2.1.2 Elevated reservoirs

Elevated reservoirs, or tanks, are typically placed in the system to use them as buffers and level out the pressure and flow demand differences. When the demand is high, the waterworks might not be able to provide the sufficient amount of water in the network. In these cases, the elevated reservoir supplies the remaining demand. When the user consumption decreases, the system can be controlled such that the tank is being refilled to provide the required demand for the next peak time of consumption. Having such an elevated reservoir in the network, the system becomes more independent of the pump stations, as the refilled tank can itself maintain the desired pressure and flow for a limited time.

Due to the elevation of the tank, when it is filled up, the pumping stations need to provide a pressure higher than the pressure in the water tank. Therefore when the tank is being emptied, the pumping stations can reduce the amount of pressure they provide to the system, since the pressure from the elevated reservoir becomes dominant. This is due to that the dynamics of systems with large storages come primarily from the pressure of the tank [13]. This is due to the height change of the tank being very slow because of the large diameter of the tank. Due to these considerations, the effect of these elevated reservoirs has to be taken into account while modelling the system.

2.1.3 Pumps

Water pumps are used to increase pressure in hydraulic systems, thus making the water flow. Pumps are typically the main actuators of a WSS and they can be either flow or pressure controlled. Therefore, pumps can have controllers to produce a desired flow or pressure. This is done by changing the rotational speed of the pump. In this way, when the pump has a reference pressure or flow, simple control makes it possible to produce the desired flow or pressure respectively [14]. The pressure required to make the water reach some height is the sum of the pressure required to overcome the elevation and the friction losses in the pipe network.

The most common pumps in WSSs are centrifugal pumps. Normally, the characteristics of such pumps are described by two pump curves. The two curves depict the volume flow versus the pressure and the power of the pump respectively. Normally the curves describe the characteristics for one particular speed, which is denoted the nominal speed [14]. An example of these pump curves is shown in *Figure 2.4*.

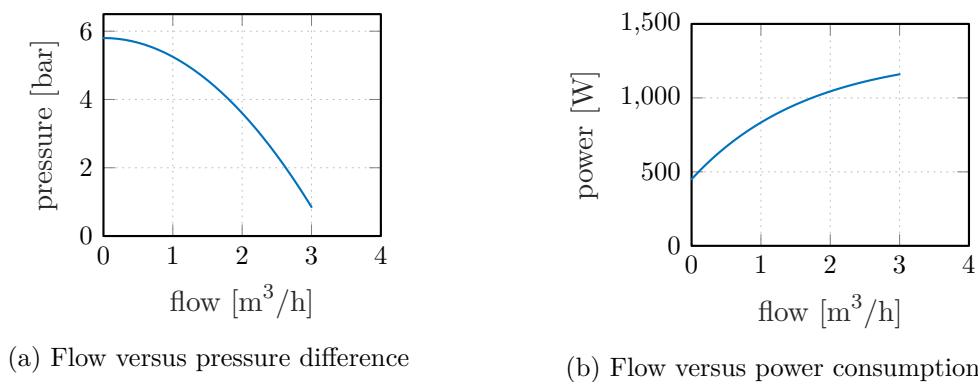


Figure 2.4: Pump curves describing the performance of a centrifugal pump at nominal speed.

As can be seen, at a given flow, the pump can deliver a pressure with a maximum limit. This pressure decreases when the flow is increasing. At a certain flow and pressure value, the pump has an optimal point where the operation is the most energy efficient. Pumps are normally designed such that the optimal point lies in the operational area for the pumping application [15].

As in almost all WSSs, the flow is varying in the system, according to the flow demand from the end-consumers. Therefore, when dealing with varying flow in the system, pumps are often placed parallel at the pump stations such that they can keep their optimum points. As the flow increases, more pumps get activated to keep the pressure constant [15].

2.1.4 Valves

Valves in the WSS can be also seen as actuators along with pump elements. Unlike the pumps, valves are passive actuators in the sense that they do not consume energy. In principal, there are many types of valves existing. They can be categorized as non-return valves, control valves and the combination of these two. Non-return valves allow waterflow only in one direction, while control valves can either adjust the flow or the pressure on their two endpoints. The former category is typically called a Flow Control Valve(FCV), while the latter is called a Pressure Reducer Valve or Pressure Regulating Valve(PRV). This project deals with all three types of valves.

Valves can be controlled such that no flow passes through. In these cases the valve is closed and thereby certain parts of the system can be isolated. Other possibility is that the valve is fully open. In such case the pressure drop between the two endpoints is experienced because of the friction loss of the valve.

2.1.5 Hydraulic head

2.2 The Randers water supply network

The Randers drinking WSS is managed by Verdo A/S and is the main supplier of drinking water and heating to the city of Randers. Verdo supplies water to approximately 46.000 customers in Randers Municipality [16]. The simulation model of the network is illustrated in *Figure 2.5*. As can be seen, the WSS in Randers is a complex, looped configuration with many different distribution areas. In overall, the system consists of around 4500 pipe elements, six tanks and more than 20 valves. The water in the network is controlled by eight pumping stations, located at different places throughout the network, thereby making it possible to deliver the desired pressure and flow to the end-users. At these pumping stations, pumps are often placed in parallel connections, due to the reason mentioned in Section 2.1.3: *Pumps*. At the outlet of the pumping stations, often PRVs or FCVs are placed.

Verdo provides the drinking water by drilling from 21 different groundwater bases and has four waterworks for water treatment [16]. The geographical position of the reservoirs at these waterworks is shown in *Figure 2.5*, encircled in red.

Missing parts:

- Which pump stations are to be controlled
- Which tanks
- Why tanks are installed at the place where they are
- which distribution areas are (mostly) dependant on which pumping stations - this would be useful to know in order to simplify the network and discard some areas in the network

To Supervisors: Since EPANET uses head as the measure of pressure, probably it is more convenient to use head in the report.

Discussion with Verdo



Figure 2.5: Reservoirs in the Randers WSS (encircled in red).

As can be seen, reservoirs are placed in the outer parts of the city, typically next to the main pumping stations. The locations of the six tanks, or tank stations, in the system is shown in (figref below)



Figure 2.6: Illustration of a WSS.

System Modelling 3

This chapter gives a mathematical description of the component modelling. Thus the different physical and mathematical measures of hydraulic systems are introduced. The similarities to electronic network modelling are shown by explaining the relevant properties of graph theory. In the end, the EPANET-based modelling approach is introduced which is used for simulation purposes within this project.

3.1 Hydraulic component modelling

In this section the mathematical relation between pressure and flow will be given for each of the different components in a WSS system to show their nonlinear behaviour. (The purpose here is not the derivation of the different models, it was done in the previous project.)

3.2 Graph-based network modelling

Graph-based network modelling has the advantage of making use of tools from circuit theory. Most of these tools are developed based on Graph Theory(GT). These methods can be used to model WSSs as directed graphs, where components of the systems, such as valves, pipes, tanks and pumps correspond to edges and each terminal of the network correspond to nodes, or equivalently, to vertices.

In case of WSSs, in order to track the pressure and flow in the desired part of the network, the equation system of the network has to be solved for the desired edges or vertices. The whole network can be described by writing up the equations for all vertices in the network, based on the mathematical modelling of the different components in the system, as shown in Section 3.1: *Hydraulic component modelling*. However, in case of complex systems as a water network for a large cities, these systems of equations are hard to handle individually and typically cannot be solved explicitly if there are loops in the system. Therefore the properties of GT are not only useful for setting up relations between flow and pressure, but to make handling of a complex system easier by exploiting the properties of matrix representations. Thereby making it convenient for implementing it in computer algorithms for iterative solving methods.

To supervisors:
Question
about
this at
the su-
pervisor
meeting.

WSSs can be described by a directed and connected graph, such that [17]:

$$\mathcal{G} = \{\mathcal{V}, \mathcal{E}\} \quad (3.1)$$

Where

\mathcal{G}	is a directed and connected graph,
\mathcal{V}	is the set of vertices, where $\mathcal{V} = \{v_1, \dots, v_n\}$,
\mathcal{E}	is the set of edges, where $\mathcal{E} = \{e_1, \dots, e_m\}$.

3.2.1 Incidence matrix

The incidence matrix, H , of a connected graph, \mathcal{G} , is a matrix where the number of rows and columns correspond to the number of vertices and edges, respectively. Therefore $H \in \mathbb{R}^{n \times m}$. In case of hydraulic networks, edges are directed in order to keep track of the direction of the flow in the system.

$$H_{i,j} = \begin{cases} 1 & \text{if the } j^{th} \text{ edge is incident out of the } i^{th} \text{ vertex.} \\ -1 & \text{if the } j^{th} \text{ edge is incident into the } i^{th} \text{ vertex.} \\ 0 & \text{if the } j^{th} \text{ edge is not connected to the } i^{th} \text{ vertex.} \end{cases} \quad (3.2)$$

3.2.2 Cycle matrix

Purely tree structure of a WSS is not common when considering distribution systems for whole cities. However, trees can be arbitrarily chosen from the underlying graph of the system. A connected tree, \mathcal{T} , of the graph is a connected sub-graph where any two vertices are connected by exactly one path [18]. Therefore a certain sub-graph which is a tree of the network can be represented as follows

$$\mathcal{T} = \{\mathcal{V}_{\mathcal{T}}, \mathcal{E}_{\mathcal{T}}\} \quad (3.3)$$

A special case of connected tree sub-graphs is the spanning tree of the network. A spanning tree contains all the vertices of \mathcal{G} and has no cycles, since it is a tree. A spanning tree of the network therefore can be represented as

$$\mathcal{T} = \{\mathcal{V}, \mathcal{E}_{\mathcal{T}}\} \quad (3.4)$$

In order to obtain a spanning tree, an edge has to be removed from each cycle. The removed edges are $\mathcal{G} - \mathcal{T}$, and called the chords of \mathcal{T} with respect to \mathcal{G} . By adding a chord to \mathcal{T} , a cycle is created which is called a fundamental cycle. A graph is conformed by as many fundamental cycles as the number of chords [18].

The set of fundamental cycles correspond to the fundamental cycle matrix, B , such that the number of rows and columns are defined by the number of chords and edges, respectively. The cycle matrix of the system is given by

$$B_{i,j} = \begin{cases} 1 & \text{if the } j^{th} \text{ edge belongs to the } i^{th} \text{ cycle and their directions agree} \\ -1 & \text{if the } j^{th} \text{ edge belongs to the } i^{th} \text{ cycle and their directions are opposite} \\ 0 & \text{if the } j^{th} \text{ edge does not belong to the } i^{th} \text{ cycle} \end{cases} \quad (3.5)$$

3.2.3 Kirchhoff's law

(in progress)

3.2.4 Partitioning the network

(in progress)

3.3 EPANET modelling

EPANET is an open source software, created by the United States Environmental Protection Agency (EPA) for simulating hydraulic networks [19]. EPANET allows to track the flow of water in each pipe, the pressure at each node and the height of water in each tank. Furthermore, it uses a node-based model approach which means that the components in the network are either treated as nodes or links. Valves, pumps, reservoirs and tanks are considered as nodes due to their fixed geographical location and geodesic level. Pipes are considered as the links between the nodes in the network. Therefore, nodes are termination points for one or more pipes. The end-user consumption flow demand is considered as an attribute of certain nodes. Such nodes are called demand nodes and they have a certain water withdrawal. Attributes of nodes in the network are the geographical and geodesic coordinates, the flow demand, the total, and the available head. [19]

In EPANET, there is a function to carry out simulations within an extended period. Time patterns can be created that make demands at the nodes vary in a periodic way over the

course of the time period. Nodal demands, reservoirs and pump schedules can all have time patterns associated with them, thus making the hydraulic simulation of the network more realistic. In order to create a schedule plan for changing reservoir levels or schedules for the pumping strategy, it is sufficient to have simulation data only at certain time steps. Therefore it is sufficient to solve the network using a set of hourly time steps (snapshots) over a period of 24 hours, and use the static, steady-state solutions for pump scheduling [19]. The main function of EPANET is this, and therefore is used within this project for extended period analysis. During the analysis, pressure and flow values, along with the demand pattern can be simulated for periodic time steps.

Since all nodes and links in the network have their unique IDs, during the project, the name of certain components will always have a reference to the original IDs in EPANET, for the better and clearer trackability.

Network simplification 4

4.1 Purpose of the model reduction

As it is described in Section 3.3: *EPANET modelling*, planning pump schedules and tank level changes in EPANET can be carried out by solving iteratively for long time steps. However, when a complex network is being used for developing different control methods, typically the aim is to find an optimisation method which determines the optimal input to the system. In order to run such an optimizing algorithm on a complex system, the network needs to be solved numerically and several online execution runs are required.

In case of complex networks with a tree structure, there is only one way from a source node to any node in the system. Therefore the network can be described by solving the equation system of the model explicitly, therefore all pressures can be directly calculated in terms of the flows. In case of loop systems however, the procedure is more difficult, since there is more path to each nodes in the network. The system of equations have to be solved numerically which can dramatically increase the simulation time of an optimization algorithm. The objective of the system reduction and simplification is therefore to reduce the number of network components and especially the loops in the network, such that the accuracy of the simplified model is as close as possible to that of the original network model.

4.2 State of the art model reduction analysis

4.2.1 Simplification of network model components

Simplification directly, by keeping the basic components but combining and replacing the individual components. (For example, series, parallel pipes. Tree substructure in the system.)

4.2.2 Black box simplification

Replacing the system partially or fully with a system which provides the same function with less complexity.

- static simplification with linearization and simplification
- neural network approach

To Supervisors: This part is in progress

Part II

System Identification

Unspecified 5

Part III

Conclusion and verification

Accepttest 6

Discussion 7

Conclusion 8

Part IV

Appendices

Assumption List A

Number	Assumptions	Section reference
1	Assumption 1.	Section ??: ??
2	Assumption 2.	Section ??: ??

Table A.1: List of assumptions

System Description

B

B.1 Components of the System

B.2 System Topology

123

B.3 Incidence Matrix

123

B.4 Cycle Matrix

Measurements



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Rettelser

Todo list

To Supervisors: Since EPANET uses head as the measure of pressure, probably it
is more convenient to use head in the report. 8

Discussion with Verdo 8

To supervisors: Question about this at the supervisor meeting. 11

To Supervisors: This part is in progress 15