# E 126 - Answers to Class Problems - Fall 2020

Concurrent Force Systems

1-1. 
$$F_R = 213 \text{ N}$$
;  $\theta = 54.8^{\circ}$  (ccw from positive *x*-axis)

1-2. 
$$\theta = 76.1^{\circ}$$
 (cw from positive x-axis)

1-3. 
$$F_R = 463 \text{ lb}$$

1-4. 
$$\theta = 21.3^{\circ}$$
;  $F_1 = 869 \text{ N}$ 

1-5. 
$$F_R = 161$$
 lb;  $\theta = 38.3^{\circ}$  (cw from positive *x*-axis)

1-6. 
$$F = 2.03 \text{ kN}$$
;  $F_R = 7.87 \text{ kN}$ 

Moment of a Force about a Point

2-1. 
$$M_O = -98.6 \text{ N-m}$$

2-2. 
$$M_{O\_FI} = +4125$$
 lb-ft;  $M_{O\_F2} = +2000$  lb-ft;  $M_{O\_FI} = +40$  lb-ft

2-3. 
$$F_3 = 1592.7 \text{ N}$$

Moment of a Force about an Axis / Moment of a Couple

2-4. 
$$M_z = 62$$
 lb-in

2-5. 
$$M_R = -53.4$$
 lb-ft

Equivalent Force and Couple Systems

2-6. 
$$F_R = 962 \text{ N}; \ \theta = 66.6^{\circ} \text{ (ccw from negative } x\text{-axis)}; \ M_{RA} = 551 \text{ N-m (cw)}$$

2-7. 
$$F_R = 991.0 \text{ N}; \ \theta = 63^{\circ} \text{ (ccw from negative } x\text{-axis)}; \ M_{RA} = 800 \text{ N-m (ccw)}$$

2-8. 
$$F_R = 420.5 \text{ N}$$
;  $\theta = 33.7^{\circ}$  (cw from positive *x*-axis);  $d = 5.07 \text{ m}$ 

Distributed Loads

2-9. 
$$a = 7.5$$
 ft;  $b = 9$  ft

2-10. 
$$F = 2700 \text{ N}$$
 @  $x = 5.56 \text{ m}$ 

# Concurrent/Particle Equilibrium

3-1. 
$$F_{AB} = 4.91 \text{ kN}$$
;  $F_{AD} = 4.25 \text{ kN}$ 

3-2. 
$$F_1 = 339 \text{ N}; F_2 = 400 \text{ N}$$

3-3. 
$$y = 1.768$$
 ft

3-4. 
$$T_A = 1507 \text{ N}$$
;  $T_B = 2970 \text{ N}$ ;  $T_C = 2832 \text{ N}$ ;  $T_D = 1416 \text{ N}$ 

3-5. 
$$T = 200 \text{ kN}$$
;  $\alpha = 6.44^{\circ}$ 

3-6. 
$$F_{BC} = 145 \text{ N}; F_{BD} = 171 \text{ N}$$

3-7. 
$$F_{BC} = 707.1 \text{ N (C)}$$
;  $F_{AB} = 500 \text{ N (T)}$ ;  $F_{AC} = 500 \text{ N (T)}$ 

# 2D Rigid Body Equilibrium

3-8. 
$$A_x = 100 \text{ N} (\rightarrow)$$
;  $A_y = 233 \text{ N} (\uparrow)$ ;  $N_B = 200 \text{ N} (\checkmark)$ 

3-9. 
$$A_x = 150 \text{ lb } (\leftarrow); A_y = 300 \text{ lb } (\uparrow); N_B = 150 \text{ lb } (\rightarrow)$$

3-10. 
$$A_x = 0$$
;  $A_y = 2$  kN (↑);  $M_R = 11$  kN-m (ccw)

3-11. 
$$R_A = 1.06 \text{ kN } (\nearrow); R_B = 1.42 \text{ kN } (\uparrow); R_C = 0.501 \text{ kN } (\rightarrow)$$

3-12. 
$$F_{AB} = 0.864 \text{ kN } (\nearrow); C_x = 2.66 \text{ kN } (\leftarrow); C_y = 6.56 \text{ kN } (\downarrow)$$

3-13. 
$$A_x = 33.4 \text{ lb } (\rightarrow); A_y = 61.3 \text{ lb } (\uparrow); T = 74.6 \text{ lb}$$

3-14. 
$$A_x = 512.8 \text{ lb } (\rightarrow)$$
;  $A_y = 638.9 \text{ lb } (\uparrow)$ ;  $B = 598 \text{ lb } (50^\circ \text{ ccw from positive } x\text{-axis})$ ;  $N_C = 199.5 \text{ lb } (\nwarrow)$ 

#### Normal and Shear Stress

4-1. 
$$\sigma_{max} = 85.7 \text{ MPa}$$

4-2. 
$$\tau_{avg} = 31.8 \text{ ksi}$$

4-3. 
$$P_{max} = 113.7 \text{ kN}; P_{max} = 54.3 \text{ kN}$$

4-4. 
$$\tau_A = 34.0 \text{ MPa}; \tau_B = 17.7 \text{ MPa}$$

4-5. 
$$\sigma_{a-a} = 500 \text{ kPa}, \ \tau_{a-a} = 0; \ \sigma_{b-b} = 375 \text{ kPa}, \ \tau_{b-b} = 217 \text{ kPa}$$

4-6. 
$$d_B = 1.18$$
 in;  $d_C = 1.11$  in

# Strain / Mechanical Properties of Materials

5-1. 
$$d_{min} = 20.6$$
 mm;  $t_{min} = 4.55$  mm

5-2. 
$$\varepsilon_{AB} = -7.93 \times 10^{-3}$$
;  $\gamma_{xy} = 0.0121 \text{ rad}$ 

5-3. 
$$\Delta L_z = 120 \ \mu\text{m}$$
-;  $\Delta L_x = -2.40 \ \mu\text{m}$ ;  $\Delta L_y = -1.20 \ \mu\text{m}$ 

5-4. 
$$E = 70.0 \text{ GPa}$$
;  $\Delta d = 0.0415 \text{ mm}$ 

#### Axial Load and Deformation

6-1. 
$$\delta_A = +0.0127$$
 in;  $\delta_{B/C} = +0.00217$  in

6-2. 
$$\delta_F = 0.225 \text{ mm } (\downarrow)$$

6-3. 
$$t_{min} = 8.73 \text{ mm}$$

6-4. 
$$\delta_{BC} = 0.102 \text{ mm}$$

6-5. 
$$F_A = 16.6 \text{ kN}$$
;  $F_B = 3.4 \text{ kN}$ 

6-6. 
$$\sigma_{Al} = 0.637 \text{ ksi}; \ \sigma_{Br} = 0.955 \text{ ksi}$$

6-7. 
$$\sigma_B = 12 \text{ ksi}$$
;  $\sigma_D = 24 \text{ ksi}$ ;  $\delta_B = 0.008 \text{ in}$ ;  $\delta_D = 0.016 \text{ in}$ 

#### Trusses

7-1. 
$$F_{AB} = 750 \text{ N (C)}$$
;  $F_{AD} = 450 \text{ N (T)}$ ;  $F_{BD} = 250 \text{ N (T)}$ ;  $F_{BC} = 600 \text{ N (C)}$ ;  $F_{CD} = 200 \text{ N (C)}$ 

7-2. 
$$F_{AB} = 5000 \text{ lb (T)}$$
;  $F_{AE} = 1000 \text{ lb (T)}$ ;  $F_{BE} = 1666.7 \text{ lb (C)}$ ;  $F_{DE} = F_{CD} = 2666.7 \text{ lb (C)}$ ;  $F_{BC} = 3333.3 \text{ lb (T)}$ ;  $F_{BD} = 2000 \text{ lb (T)}$ 

7-3. 
$$F_{CD} = 50 \text{ kN (T)}$$
;  $F_{HD} = 7.07 \text{ kN (C)}$ ;  $F_{GD} = 5 \text{ kN (T)}$ 

7-4. 
$$F_{DE} = 11.9 \text{ kN (C)}$$
;  $F_{DJ} = 2.66 \text{ kN (T)}$ ; Zero-force members: BL, BK, CK, JE, EI, IF, FH

7-5. 
$$F_{HC} = 150 \text{ kN (C)}$$
;  $F_{BC} = 40 \text{ kN (C)}$ ;  $F_{JD} = 0$ ;  $F_{KD} = 102.1 \text{ kN (C)}$ ;  $F_{LF} = 40 \text{ kN (T)}$ 

## Frames and Machines

8-1. 
$$C_x = 1230 \text{ N} (\rightarrow); C_y = 245 \text{ N} (\downarrow)$$

8-2. 
$$A_x = 70 \text{ lb } (\rightarrow)$$
;  $A_y = 8.75 \text{ lb } (\downarrow)$ ;  $F_{BE} = 61.3 \text{ lb } (\downarrow)$ ;  $C_x = 70 \text{ lb } (\leftarrow)$ ;  $C_y = 70 \text{ lb } (\uparrow)$ ;

8-3. 
$$B_x = 0$$
;  $B_y = 1050$  lb ( $\downarrow$ );  $C_x = 0$ ;  $C_y = 1000$  lb ( $\uparrow$ );  $F_E = 1500$  lb

8-4. 
$$A_x = 577 \text{ N } (\rightarrow); A_y = 1000 \text{ N } (\uparrow); C_x = 577 \text{ N } (\leftarrow); C_y = 1000 \text{ N } (\uparrow)$$

8-5. 
$$F_{DB} = 2.60 \text{ kN (C)}$$
;  $F_{FB} = 1.94 \text{ kN (C)}$ 

8-6. 
$$A_x = 300 \text{ N (} \leftarrow \text{)}; A_y = 300 \text{ N (} \uparrow \text{)}; C_x = 300 \text{ N (} \rightarrow \text{)}; C_y = 300 \text{ N (} \uparrow \text{)}$$

### Geometric Properties of Areas

9-1. 
$$x = 1.50$$
 in;  $y = 2.00$  in

9-2. 
$$x = 2.73$$
 in;  $y = 1.42$  in

9-3. 
$$I_x = 736 \text{ in}^4$$
;  $I_y = 256 \text{ in}^4$ ;  $I_{x'} = 136 \text{ in}^4$ 

9-4. 
$$I_x = 37.5 \times 10^6 \text{ mm}^4$$
;  $I_y = 53.7 \times 10^6 \text{ mm}^4$ 

Shear Force and Bending Moment Diagrams

10-1. 
$$N_C = 0$$
;  $V_C = -1$  kip;  $M_C = 56.0$  kip-ft;  $N_D = 0$ ;  $V_D = -1$  kip;  $M_D = 48.0$  kip-ft

10-2. 
$$N_B = 0$$
;  $V_B = 28.8$  kip;  $M_C = -115$  kip-ft

Flexural Loading - Bending Stresses in Beams

- 11-1.  $\sigma_{max} = -2206$  psi; Max bending stress occurs at top surface
- 11-2.  $\sigma_{max} = 281$  MPa; Max bending stress occurs at the base C, on outer surface of the pipe

11-3. 
$$\sigma_B = -11.2 \text{ MPa}; \sigma_D = 12.7 \text{ MPa}$$

11-4. 
$$\sigma_{max} = -16.2 \text{ MPa}$$

Flexural Loading - Shear Stresses in Beams

12-1. 
$$\tau_{max} = 1590 \text{ psi}; \ \tau_{web} = 1240 \text{ psi}; \ \tau_{flange} = 155 \text{ psi}$$

12-2. 
$$\tau_{glue} = 4.88 \text{ MPa}$$

12-3. 
$$(\sigma_{max})_T = 5198 \text{ psi}; (\sigma_{max})_C = 3464 \text{ psi}; \tau_F = 1082 \text{ psi}$$

Torsional Loading

13-1. 
$$\tau_{out} = 345 \text{ kPa}; \ \tau_{in} = 276 \text{ kPa}$$

13-2. 
$$\tau_A = 18.9 \text{ ksi}; \tau_B = 3.77 \text{ ksi}$$

13-3. 
$$d_{min} = 0.858$$
 in.  $\rightarrow 7/8$  in.

13-4. 
$$f = 26.6 \text{ Hz}$$

13-5. 
$$s_P = 21.2 \text{ mm}$$

13-6. 
$$\varphi = 0.085 \text{ rad}$$

Stress Transformation

14-1. 
$$\sigma_1 = 80 \text{ MPa}$$
;  $\sigma_2 = -20 \text{ MPa}$ ;  $\theta_p = 26.6^{\circ} \text{ (ccw)}$ 

14-2. 
$$\sigma_1 = 1.67$$
 ksi;  $\sigma_2 = -2.23$  ksi;  $\tau_{max} = 1.95$  ksi

14-3. 
$$\sigma_1 = 0.0723$$
 ksi;  $\sigma_2 = -0.6833$  ksi